

ARTICLE 14 RESPONSE

Churchtown Landfill Site



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Contents

1	ARTI	CLE 14 COMPLIANCE REQUIREMENTS	1
	1.1	Point 1	1
	1.2	Point 2	1
	1.3	Point 3	1
	1.4	Point 4	1
	1.5	Point 5	1
	1.6	Point 6	1
	1.7	Point 7	2
	1.8	Point 8	3
	1.9	Point 9	5
	1.10	Point 10	5
	1.11	Point 11	6

Tables

Table 1.1: Grid Co-ordinates for discharge locations		2
Table 1.2: Grid Co-ordinates for surface water locations		2
Table 1.3: Willow Discharge		4
Table 1.4: Metals to be analysed (mg/kg)	يو.	5
Graphs	only any other	
Graph 1.1 A series ponds	, 	3
Graph 1.2 B corios ponde		1

Graphs

Graph 1.1 A series ponds	A PUL COL	
Graph 1.2 B series ponds	otion let	
	inspirt of	
	For vie	
A		
Appendices	attor	
Appendix A Drawing	CONSE	7

Appendices

Appendix A Drawing	7
Appendix B Hydrogeological Risk Assessment	8
Appendix C Surface Water Results	9
Appendix D Discharged Effluent Results	10
Appendix E Groundwater Results	11

ARTICLE 14 COMPLIANCE REQUIREMENTS 1

The following responses have been numbered as per EPA letter dated 8th March 2019.

1.1 Point 1

Class D1 of the Waste Management Act 1996, as amended, has been identified in the application as the principal class of activity, however, the newspaper notice and site notice state that Class D4 is the principal activity. Clarify which is the principal activity and ensure that appropriate licence application fee has been submitted to the Agency in respect of this activity.

The principal activity is D4 of the Third Schedule.

1.2 Point 2

State whether the purchase of the site, which is the subject of the licence application, has been completed and Donegal County Council is now the owner of the site.

Donegal County Council is the owners of the site.

1.3 Point 3

Submit a drawing showing the site boundary. The site boundary showld be outlined as a continuous red line surrounding the entire facility. 3114

Drawing IBR1015/101A Site Boundary is included in Appendix A. section pu

1.4 Point 4

OWNETTER Explain how storm water arising from the site is managed. Include a drawing showing the storm water ofcopy discharge locations.

The existing landfill was capped with a permanent low permeability clay liner in conjunction with a willow and reed plantation and constructed wetland installed in 2014-2015. A 0.15 to 0.45 metre thick topsoil and 0.5 m clay cap with a permeability of 1x10⁻⁸ m/s was installed at the facility. The willow plantation in situated in the centre and above the capped waste (Zones 1 to 4) with a series of constructed wetlands along western and eastern side of willow plantation (Drawing IBR1015/106). This whole area is contained within a bund and all storm water arising from this area is treated in the willow/ constructed wetland before it is discharged.

All rainfall that fall on the slopes of the landfill is collected in the existing drains which run along the eastern and western boundaries of the site prior to discharge to the River Finn as shown on Drawing IBR1015/106.

1.5 Point 5

Submit the hydrological risk assessment referred to in Attachment E.4 of the application.

The Hydrogeological Risk Assessment is included in Appendix B. \

1.6 Point 6

Provide a drawing that clearly shows the flow of the pumped landfill leachate through the willow plantation and the ponds. Include the locations where the treated landfill leachate discharges to the surface water channels and locations where the treated effluent discharges to the River Finn. Include symbols and grid coordinates (6E,6N) for each discharge location.

Drawing IBR1015/104A and IBR1015/106 are provided in Appendix A. Grid coordinates (6E, 6N) for each discharge location where treated landfill leachate discharges to the surface water channels is giving in the Table 1.1 below.

Table 1.1: Grid Co-ordinates for discharge locations

Table style 1 heading	Grid Co Ordinates			
Discharge Location	Easting	Northing		
Discharge from Willow				
D1	230908.077	395942.728		
D3	231069.698	395759.633		
Discharge from Pond (ICW)				
D2	231076.621	395754.966		
D4	231172.307	395897.031		

Please refer to Drawing IBR1015/104A Churchtown Landfill Site Monitoring Locations for updated monitoring location. Treated effluent discharges to surface water channels and subsequently discharge to the River Finn at SW2 and SW5.

Station Purpose	Station Name	Description offer	Northing	Easting
		Existing		
Surface Water	SW1	Upstream of the waster body in a field drain that subsequently runs adjacent to the landfill along its north eastern boundary.	230934.07	396164.09
Surface Water	SW2	SW2 (stream) is located at the River Finn end of surface water stream that run along the easiern boundary of the site.	231177.01	395895.00
Surface Water	SW3	SW3 is midstream of the facility within the River Finn	231180.26	395840.10
Surface Water	SW4	SW4 (field drain) is located at the River Finn end drain that run along the western boundary of the site.	231026.01	395734.06
Surface Water	SW5	SW5 is also located at the end of field drain and therefore it is not representative of the river quality.	231038.03	395711.08
Surface Water	SW6	Upstream of the facility within the River Finn	230983.00	395705.11
Surface Water	SW7	Downstream of the facility within the River Finn	231248.04	395948.97

Table 1.2: Grid Co-ordinates for surface water locations

1.7 Point 7

Provide monitoring of water quality in the River Finn at locations upstream and downstream of the discharges from the facility. Include a 95% flow rate (m^3/s) in the receiving water.

Surface Water results for 2018 are provided in Appendix C. SW1 and SW2 were dry in September. SW4, SW5 and SW6 were not sampled in January, June and September as the sampling stations were destroyed during the flood in August 2017. Conditions did not allow safe plant access for reinstatement works until the bank was properly established. Annual monitoring at surface water monitoring points SW3 and SW7 was undertaken on 10th September 2018.

The 95% flow rate (m^3/s) in the receiving water (River Finn) is taken from gauging station at Dreenan Bridge. Please refer to Appropriate Assessment Section 5.4 for further details on 95% flow rate (m^3/s) in the receiving water.

1.8 Point 8

Submit monitoring results of the discharged effluent at all locations where the treated effluent leaves the facility. Include the maximum daily flow rate (m^3 /day) for each discharge location.

Monitoring results of the discharged effluent for 2019 are provided in Appendix D. Please refer to Appropriate Assessment Section 3.4.4 for further details on flow rates.

Existing monitoring of D2 and D4 (the two discharge outlets from the ICW) for ammonia for 2019 are shown in Graphs 1 and 2 below. Ammonia concentrations are below the proposed ELV in Pond 5A and 5B.



Graph 1.1 A series ponds



Graph 1.2 B series ponds

Existing monitoring of D1 and D3 (the two discharge outlets from the willow) for ammonia for 2019 is shown in Table 1.3 below. Ammonia concentrations are below the proposed ELV. Consen

Table 1.3: Willow Discharge¹

Willow Discharge			
Date	Northern Willow (D1)	Southern Willow(D3)	Limit
		Ammonia mg/l	
08/01/2019	0.013	0.358	3
18/01/2019	0.04	0.46	3
25/01/2019	0.06	0.45	3
01/02/2019	0.07	0.65	3
07/02/2019	0.06	0.82	3
13/02/2019	0.07	0.62	3

¹ Irrigation of the northern willow with leachate started in March/April. Maintenance works were undertaken as required. The Southern willow has not been irrigated with leachate this year due to a settlement problem.

Willow Discharge			
01/03/2019	0.011	0.015	3
08/03/2019	0.011	0.126	3
19/03/2019	0.26	0.18	3
29/03/2019	0.852	0.011	3
12/04/2019	0.008	0.006	3
16/04/2019	0.015	0.015	3
24/04/2019	2.48	0.015	3
24/05/2019	0.093	No sample	3
30/05/2019	0.051	0.019	3
07/06/2019	0.016	0.026	3
14/06/2019	0.1	0.015	3
20/06/2019	0.674	0.021	3
26/06/2019	<0.015	No discharge	3
05/07/2019	No discharge	No discharge	3
12/07/2019	<0.015	0.084	3

1.9 Point 9 Submit results of groundwater monitoring referred to in Attachment F.5 of the application.

Groundwater results for 2018 are provided in Appendix E.

1.10 Point 10 necromass of the wetland system.

Desludging is dependent on the cell number, cell area, and influent loading. The initial wet-land cell will require cleaning out first, however this is not expected for at least 10 years.

Sediment should be removed from the ponds as required when the pool volume has become reduced significantly or the ponds have become eutrophic.

Sediment build up in the wetland will include metals accumulated. The management of sediment will depend on the contamination concentrations and the proposed reuse or disposal. Samples will be taken from the cells for total metal concentration analyses (mg/kg).

Table 1.4: Metals to be analysed (mg/kg)

Metalsr
Boron
Cadmium
Chromium
Copper
Cyanide

Metalsr
Iron
Lead
Magnesium
Manganese
Mercury
Potassium
Sodium
Zinc

Point 11 1.11

The Non-Technical Summary states that Class D4 of the activity would include placement of sludge discards into pits, pond or lagoons. Clarify whether it is proposed to accept any sludge or any other waste from off-site sources.

No sludge or any other waste from off-site sources will be accepted on site.

1.12 Natura Impact Statement

15^{0.} Conson of copyright owner population by the source of the A Natura Impact Statement has been completed and is also included in this submission.



Appendix A

Drawing

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NOTES

2.

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4

Verifying Dimensions. The contractor shall verify dimensions against such other drawings or site conditions as pertain to this part of the work.

Existing Services.

Any information concerning the location of existing services indicated on this drawing is intended for general guidance only. It shall be the responsibility of the contractor to determine and verify the exact horizontal and vertical alignment of all cables, pipes, etc. (both underground and overhead) before work commences.

Issue of Drawings. Hard copies, dwf and pdf will form a controlled issue of the drawing. All other formats (dwg, dxf etc.) are deemed to be an uncontrolled issue and any work carried out based on these files is at the recipients own risk. RPS will not accept any responsibility for any errors arising from the use of these files, either by human error by the recipient, listing of un-dimensioned measurements, compatibility issues with the recipient's software, and any errors arising when these files are used to aid the recipients drawing production, or setting out on site.

Datum.

А	Discharge Points Added					мс	July '19
rev	amendm	ents				draw	n date
L	75	Enter Centr Letter F92 A	prise Fund Bu e, Ballyraine kenny ,Co. Do kF43 Ireland	onegal	T +3 F +3 W w E ire	853 (0) 74 853 (0) 74 ww.rpsgr eland@rps	912 1927 912 1928 coup.com/ireland sgroup.com
Clien	t						
Proje	_{ct} Churcht	owr	Licens	se Re	eview	I	
Title							
1	Monitoring Locations						
Draw	ing Status	3	Sheet Si	ze		Drawin	g Scale
Prel	Preliminary A3 1:2000					000	
Draw	Drawing Number Rev						
IB	R101	15/	104				A
Proje	Project Leader Drawn By Date				Ini	tial Review	
DD		AMB		May -2017		7 Al	McG





DESCRIPTION
D1
D2
D3
D4

Appendix B

Hydrogeological Risk Assessment

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Churchtown Landfill

WL62-1

Hydrogeological Risk Assessment



Report for: Donegal County Council

Date: 11/12/2015

Report No.: BRE14008Rp001F01

BlueRock Environmental Limited

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Where field investigations are carried out, these have been restricted to a level of detail required to meet the stated objectives of the services. The results of any measurements taken may vary spatially or with time and further confirmatory measurements should be made after any significant delay in issuing this Report.

TABLE OF CONTENTS

1	EXEC	JTIVE SUMMARY1						
2	INTRODUCTION							
	2.1	INTRODUCTION	4					
	2.2	OBJECTIVES	4					
	2.3	METHODOLOGY	4					
	2.4	SOURCES OF INFORMATION	4					
	2.5	REPORT FORMAT	5					
	2.6	REVIEW OF PREVIOUS REPORTS	5					
3	SITE [DESCRIPTION	7					
	3.1	SITE LOCATION	7					
	3.2	TOPOGRAPHY	7					
	3.3	SITE LAYOUT	7					
	3.4	SITE HISTORY	7					
	3.5	LEACHATE MANAGEMENT	8					
4	GEOL	OGY	.10					
	4.1	REGIONAL & SITE OVERBURDEN	. 10					
	4.2	REGIONAL BEDROCK GEOLOGY	. 10					
5	HYDR	OLOGY	12					
	5.1	SITE HYDROLOGY	.12					
	5.2	SURFACE WATER WFD STATUS	.13					
	5.3	DESIGNATED PROTECTED AREAS	.13					
6	HYDR	OGEOLOGY	.14					
	6.1		. 14					
	6.2	Aquifer Vulnerability	. 14					
	6.3	GROUNDWATER WFD STATUS	. 14					
	6.4	BACKGROUND GROUNDWATER QUALITY	15					
	6.5	LOCAL GROUNDWATER USAGE AND SOURCE PROTECTION AREA	15					
	6.6	RECHARGE RAINFALL	15					
	6.7	GROUNDWATER MONITORING WELLS	.16					
	6.8	GROUNDWATER LEVELS & FLOW DIRECTION	.16					
	6.9	PERMEABILITY	. 17					
7	PREL	IMINARY S-P-R	20					
8	HYDR	OCHEMISTRY	21					
	8.1	MONITORING LOCATIONS & FREQUENCY	.21					
	8.2	HUMAN HEALTH & ENVIRONMENTAL RISK ASSESSMENT FRAMEWORK	.22					
	8.3	LEACHATE QUALITY	.22					
	8.4	GROUNDWATER QUALITY	. 25					
		8.4.1 Ammoniacal Nitrogen	25					

		8.4.2	Electrical Conductivity (EC)	25
		8.4.3	Chloride	26
		8.4.4	Total Organic Carbon (TOC)	26
		8.4.5	Total Oxidised Nitrogen (TON)	26
		8.4.6	Other Parameters	27
	8.5	SURFAC	E WATER QUALITY	29
		8.5.1	Ammoniacal Nitrogen	29
		8.5.2	Electrical Conductivity (EC)	32
		8.5.3	Chloride	33
		8.5.4	Orthophosphate (ORP)	34
		8.5.5	Biochemical Oxygen Demand (BOD)	34
		8.5.6	VOCs/sVOCs/Hydrocarbons/Heavy Metals	34
	8.6	SURFAC	E WATER QUALITY SUMMARY	34
	8.7	GROUN	DWATER CONTAMINANT FLUXES / ASSIMILATIVE CAPACITY	35
9	UPDAT	ED HYD	DROGEOLOGICAL CONCEPTUAL SITE MODEL	36
	9.1	SOURCE	E AREAS	36
	9.2	PATHWA	۹۲S	36
	9.3	RECEPT	ORS	36
	9.4	UPDATE	D S-P-R - RISK SCREENING	37
	9.5	Assess	MENT OF CURRENT GROUNDWATER IMPACTS & EXTENT OF PLUMES	37
10	REMED	DIAL ST	RATEGY	38
11	COMPL	IANCE	MONITORING	39
	11.1	COMPLI	ANCE MONITORING LOCATIONS	39
	11.2	COMPLI	ANCE VALUES	39
12	SUMM	ARY & C	CONCLUSIONS & RECOMMENDATIONS	43

LIST OF TABLES

Table 4.1	Summary of Site Investigation Activities	. 10
Table 6.2	Long term mean monthly rainfall data (mm) (Met Éireann)	. 15
Table 7.1	Preliminary S-P-R	. 20
Table 8.1	Monitoring Locations	. 21
Table 8.2	Parameters and Frequency of Groundwater Monitoring	. 21
Table 8.4	Surface water sampling locations	. 29
Table 9.1	Updated S-P-R	. 37
Table 11.1	Proposed Monitoring	. 41
Table 11.2	Proposed Monitoring Parameter Thresholds	. 42

LIST OF FIGURES

Figure 1	Site Location	
Figure 2	Site Layout	
Figure 3	Monitoring Locations	
Figure 4	Groundwater Contours	
Figure 5	Conceptual Site Model of	
Figure 8.4	Ammoniacal Nitrogen Levels - Groundwater	25
Figure 8.5	Electrical Conductivity Levels - Groundwater	26
Figure 8.6	TON Levels	27
Figure 8.8	Northeastern Drain - Ammoniacal Nitrogen Levels	30
Figure 8.10	Southwestern Drain - Ammoniacal Nitrogen Levels	31
Figure 8.11	River Finn - Ammoniacal Nitrogen Levels	32
Figure 8.12	Northeastern Drain – Electrical Conductivity	33

LIST OF TABLES

Table 4.1	Summary of Site Investigation Activities	10
Table 6.2	Long term mean monthly rainfall data (mm) (Met Éireann)	15
Table 7.1	Preliminary S-P-R	20
Table 8.1	Monitoring Locations	21
Table 8.2	Parameters and Frequency of Groundwater Monitoring	21
Table 8.4	Surface water sampling locations	29
Table 9.1	Updated S-P-R	37
Table 11.1	Proposed Monitoring	41
Table 11.2	Proposed Monitoring Parameter Thresholds	. 42

LIST OF APPENDICES

- Appendix A
- GSI Geological & Hydrogeological Maps Figure A Soil Classific oction net to set Figure B Figure 5 Appendix B
 - 0
 - Bedrock Classification Aquifer Classification 0
 - o Figure C
 - Figure D GSI Well Locations 0
 - GSI mapped Groundwater Wells Figure E 0 Con
- Appendix C **Borehole Logs**
- Appendix D Hydrochemistry Data & Figures
- Appendix E Assimilative Capacity Calculation

1 EXECUTIVE SUMMARY

- A hydrogeological risk assessment of Churchtown Landfill Site was undertaken by BREL based on previous investigation reports and monitoring data between 2006 and 2015.
- It is noted that Churchtown Landfill is currently in the process of a new pilot remediation solution involving constructed wetlands and willow plantations. This programme of works is expected to significantly improve the current contaminant conditions presence at the site. Therefore the assessment undertaken within this report is based on previous and recent contaminant conditions and a reassessment of site conditions will be required following a period of 12 months post-completion of the works.
- Churchtown Landfill is a former solid waste facility where historically waste was landfilled into bunded cells which were excavated from the in-situ cohesive alluvial subsoils. The excavated soils were then used in bund construction. When landfilling ceased at Churchtown the final area of the waste body was approximately 5 hectares and waste body forms a plateau shape compared to the adjacent lands.
- The site is an unlined site historically operated on a dilute and disperses principal, whereby solid waste was tipped directly onto the underlying excavated surface with leachate allowed to percolate directly through the soils with no engineered liner installed. Landfilling began in 1987 and the site ceased operations on the 31st August 2000
- On the 19th May 2000 the Environmental Protection Agency granted the Council a Waste Licence (registration number WL62-1) for the orderly closure, capping and restoration of the landfill facility, in accordance with the Third Schedule of the Waste Management Act, 1996.
- The hydrogeological regime across the landfill comprises two groundwater bodies (*i.e.* one within the waste body and a separate groundwater body within the overburden/shallow bedrock) that are likely to be hydradically connected. A third groundwater body within the bedrock and flowing under pressurised artesian conditions may also be present based on the conditions encountered within monitoring well BH4. Shallow groundwater interacts with the waste mass and facilitates the generation of leachate.
- Groundwater level variability in the area significantly impacts on leachate levels within the waste body. The correlating increases and reductions in groundwater and leachate levels confirm this scenario with groundwater appearing to intersect the waste body. Groundwater level variations and levels upgradient of the site have a differing signature to groundwater levels closer to the River Finn. This suggests that the river is partially impacting on groundwater downgradient of the landfill.
- Following a review of the preliminary Conceptual Site Model for the site and all available water monitoring data, a revised Conceptual Site Model (CSM) was developed based on available information and monitoring data and identified a number of SPR linkages ranging from Low to Moderate risk to identified sensitive receptors *i.e.* the River Finn and the Raphoe GWB.
- The SPR linkage of concern relates to:
 - The vertical migration of leachate from the unlined waste cells to the underlying shallow groundwater aquifer which subsequently flows to the River Finn.
- The raw leachate results from the landfill are considered to represent a landfill in the methanogenic stage of decomposition of organic compounds. The leachate is considered to be relatively low strength and the levels, which are reducing over time, are expected to reduce further.
- Groundwater quality data does not indicate any upwards trends over time. This is expected to continue following completion of the current remedial measures. On-going monitoring at BH1

in conjunction with a trend analysis on receipt of sufficient monitoring data over time is recommended.

- Both groundwater and surface water contaminant fluxes from the landfill have the potential to impact on the quality of the River Finn. However, available data suggests that groundwater contaminant fluxes to the river are having a negligible effect on the river downstream of the landfill. It is noted that a more representative downgradient monitoring well is required between the landfill and the river to provide a more accurate determination of this flux. However, it also noted that site access to a suitably located downgradient monitoring may be restricted due to the proximity to the river and soft ground conditions. In relation to surface water discharges, available data suggests that surface water discharges to the river representative the predominant contaminant load to the river. The effects of this loading on the river are considered to be low with significant dilution capacity available within the river itself.
- Based on the water quality data, the landfill does not affect the current status of the River Finn and is in accordance with the WFD objectives.
- The rule of thumb of 100xGTV has not been exceeded in any groundwater monitoring well at the site. The highest Ammoniacal Nitrogen level recorded was 2.63 mg/l in BH1 (February 2009) which is approximately 15 times the GTV. In accordance with the Water Framework Directive (WFD), these levels are not likely to affect the status of the Raphoe GWB nor potentially pose a risk to the objectives of the Water Framework Directive. No groundwater contaminant plume has been identified to-date from the existing groundwater monitoring network. otheruse

The following points are noted:

- No groundwater users are located downgradient of the landfill site.
- The area of impact from the landfill leachate is considered to be minor relative to the groundwater body catchment area of the Raphoe GWB i.e. < 0.01%;
- Given the proximity to the landfill to the river, no significant plume, if any, is envisaged.
- The strength of the leachate is considered to be relatively low. Clear evidence exists that demonstrates the strength of leachate within the waste body is reducing over time.
- No groundwater monitoring well between the waste body and the River Finn exists and therefore the true contaminant groundwater flux to the river is unclear.
- The site in its present condition appears to be having a low impact on the quality of the River Finn with surface water discharges from the landfill site drains the dominant pathways for contaminant flux. No impact to the current WFD status of the river is anticipated. Additional monitoring is recommended to ascertain the impact occurring - in particular on completion of the current site restoration/remediation works.
- The site is compliant with the "prevent" or "limit" objective of the WFD and GWD. The prevention of hazardous of substances entering the groundwater system is being met based on available chemical analysis. Limiting the ingress of non-hazardous substances is also being met by the mitigation measures that have been installed to date at the site *i.e.* landfill capping and lining of surface water drains and mitigations currently being installed *i.e.* active leachate treatment by willow plantations and constructed wetlands.
- Corrective actions undertaken to-date at the site includes:
 - A permanent landfill capping across the entire waste body;
 - The development of a willow bed plantation and constructed wetlands over the waste body to treat all leachate generated on site and disposal to the River Finn. This system is currently being developed at the site, and,
 - On-going groundwater and surface water monitoring as per the licence requirements.

On-going remediation works is still in progress at the site.

• A series of additional recommendations to provide a more representative understanding of the contaminant fluxes to the River Finn have been provided in Section 10.0. It is noted that as the site is actively undergoing remediation works, it is proposed that these recommendations are considered <u>at least</u> 12 months post full completion of the works. A revised CSM will be undertaken at this stage and the proposed recommendations reassessed. In the meantime, the current monitoring programme is considered sufficient as an interim measure until completion of the remediation works.

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2 INTRODUCTION

2.1 Introduction

The following hydrogeological risk assessment is intended to satisfy the requirements of Environmental Protection Agency (EPA), relating to a waste management facility at Churchtown, Co Donegal. EPA waste license reference no. WL62-1. A site walkover was undertaken by Niall Mitchell (Hydrogeologist) and Sean Heffernan of BlueRock Environmental Ltd (BREL) on the 10th February 2015.

2.2 **Objectives**

The objectives of this assessment report include the following:

- To consolidate all available historical reports and geological, hydrogeological and hydrological • data relating to the site and its immediate environs;
- To assess and interpret all available water quality data recorded to-date;
- To develop an appropriate Conceptual Site Model (CSM) for the site;
- To assess the site's compliance with the Groundwater Regulations (S.I. No. 9 of 2010); •
- To assess the level of risk posed to sensitive receptors;
- To develop an appropriate compliance monitoring programme for the site; and, •
- Recommend suitable mitigation measures, if deemed necessary.

2.3 Methodology

For any This report was prepared in accordance with the following documentation:

- Guidance on the Authorisation of Discharges to Groundwater, EPA, 2011;
- Guidance on the Management of Contaminated Land and Groundwater at EPA Licensed Sites FOI (2013),
- Code of Practice Environmental Risk Assessment for Unregulated Waste Disposal Sites, EPA, • 2007: and
- Hydrogeological Risk Assessments for Landfills and the Derivation of Groundwater Control • and Trigger Levels, Environment Agency, 2003.

2.4 Sources of Information

The following sources of information were reviewed as part of this assessment:

- Donegal County Council Annual Environmental Reports (AERs), 2004 to 2013; •
- Replacement Wells, Drumaboden and Churchtown Landfill Sites, RPS, May 2005;
- EPA Waste Disposal License Application, Attachment C6 Hydrogeology, Donegal County • Council, 1999;
- Churchtown Landfill Site Lifford Ground Investigation 898/2293, Stratex Ltd, 23rd September • 1998:
- Geological Survey of Ireland (GSI) online databases and mapping;
- Geology of South Donegal (3) 1:100,000 Scale Bedrock Map Series, Geological Survey of Ireland:
- EPA Inspectors Report, Waste License Register Number 62-1, EPA, 30th September 1998;
- EPA online databases and mapping; •
- Irish Soil Information System Online, Teagasc; •
- Ordnance Survey of Ireland (OSI) historical mapping;
- GSI, Groundwater Protection Schemes, 1999;
- Donegal Groundwater Protection Scheme, and,

- Fitzsimons, V., Daly, D. and Deakin, J., 2003. GSI Guidelines for Assessment and Mapping of Groundwater Vulnerability to Contamination. Draft Report, Geological Survey of Ireland.
- Landfill Operational Practice's, EPA, 1997.

2.5 Report Format

This report comprises of an executive summary for chapter 1 and an introductory chapter 2 which discusses sources of information, general objectives of this hydrogeological assessment and a brief overview of historical investigative reports associated with the site.

Chapter 3 discusses the site location, layout and setting.

Chapter 4 includes detailed information on the underlying soils and bedrock.

Chapter 5 is a brief description of the local hydrology, including details of any site specific surface water bodies.

Chapter 6 discusses the hydrogeology of the site and general region, including any boreholes that have been drilled and monitoring wells in place. It discusses historic groundwater levels and flow direction.

Chapter 7 describes briefly the preliminary Source-Path-Receptor model (SPR) for the landfill.

Chapter 8 is comprehensive review of the hydrochemistry monitoring of the site in terms of groundwater, surface water and leachate quality.

Chapters 9 & 10 defines an updated conceptual site model for the landfill using site specific data coupled with the initial SPR model and provides compliance monitoring recommendations.

Chapter 11 provides recommendations for future monitoring, investigation and/or remediation and report conclusions.

2.6 Review of Previous Reports

Report Title 1: Ground Investigation Churchtown Landfill Site, Stratex Ltd, September 1998

This investigation comprised the drilling of 3 no. boreholes (*i.e.* BH1, BH3 and BH4) using Shell & Auger techniques. The site location for these wells is included in **Appendix B**. Insitu permeability testing was undertaken within each borehole.

Report Title 2: Application for Waste Disposal License (Attachment C6 - Hydrogeology), prepared by Donegal County Council, 1999.

This report provides a general overview of site conditions and background information which is incorporated into this 2015 report in the following sections.

The application identified the main risks posed by the landfill entailed the migration of leachate to both groundwater and surface waters in the vicinity of the landfill. The report confirmed that although the discharge of leachate to groundwater, which provides baseflow to the River Finn, was occurring, the discharge was not impacting on the groundwater resource or on the quality of the river itself.

Proposed mitigation measures for the landfill included:

- Enclosure of the landfill in low permeability graded clay banks constructed around the waste body;
- Increasing the compaction of the waste to reduce the volume of waste and the overall quantity
 of rainfall infiltrating the site; and,
- Intermediate and temporary capping of inactive waste areas and the phased development and restoration of the site.

Report Title 3: Replacement Wells Drumaboden & Churchtown Landfill Sites (Report No: 05-135), Glover Site Investigations & RPS, May 2005.

This report describes a site investigation detailing 2 no. additional boreholes that were drilled at Churchtown Landfill. These new boreholes were installed as replacement gas wells and labelled LG8 and LG9.

Report Title 4: Site Restoration Contract (Ground Investigation Report) Churchtown Landfill (Job Ref: 14-1170), Ground Check Ltd, February 2015.

This report describes a ground investigation for a site restoration contract at Churchtown Landfill, Lifford, County Donegal. The report details the following; investigation works undertaken

- 11 no. Shell and Auger boreholes drilled by Dando 200 drilling rig (referenced L1A, LG1A, L2, LG2A, LG2AR, LG3A, LG5A, LG6A, LG7A, LG8 and LG9);
- 1 no. Borehole (BH4) drilled using rotary drilling; and,
- Disturbed samples and water samples were taken from all investigation locations, where possible, and sent for lab analysis.

Report Title 5: Annual Environmental Reports, Churchtown Landfill, Donegal County Council, 2004 - 2015

These reports comprise the Annual Environmental Reports (AERs) prepared by Donegal County Council for the Environmental Protection Agency (EPA). These reports describe the following;

- The waste activities that have taken place on the site during the reporting period, including volumes of waste accepted and their type;
- A summary report on emissions, including details of landfill gas levels, groundwater levels and leachate levels;
- Environmental quality monitoring is also undertaken during the reporting period relative to surface waters, leachate and groundwater;
- The volume of leachate transported/discharged off site in addition to a water balance calculation for the site; and,
- Any significant site works than have taken place on the landfill site during the reporting period are also described.

3 SITE DESCRIPTION

3.1 Site Location

The site is located in County Donegal approximately 3km south west of Lifford and bordered to the northwest by the N15, which is the main Lifford to Ballybofey Road (see Figure 1). The landfill facility occupies an area footprint of approximately 9.7 hectares and it is located within the townland of Churchtown, near Lifford, Co. Donegal. The ground to the northeast and southwest of the site is the low lying and gently undulating flood plain of the River Finn, with both areas being used for grazing. The southeastern boundary is bordered by the River Finn. The River Finn delineates the boundary between the North of Ireland and the Republic of Ireland. The main access to the site is from the N15 on the northwestern site boundary. There are fourteen private residences within 500m of the landfill facility, four of which are located across the River Finn in Northern Ireland.

3.2 Topography

The landfill facility is located on the broad alluvial flood plain of the River Finn, approximately 3.18 km upstream of its confluence with the River Mourne (see Figure 1). Landfilling activities have raised the elevation of the site by approximately 5 metres above the existing low lying terrain. The landfill currently forms a raised plateau that is bounded by steep clay bunds along the southwest and northwest margins. The surrounding land appears to fall at a gentle gradient from the N15 Lifford Rd towards the River Finn. A clay dyke has been constructed along the bank of the river in order to mitigate seasonal inundation of the surrounding low lying fields. Above the road the topography rises steeply to the top of Croaghan hill at approximately 217mOD.

3.3 Site Layout

only any of Historically waste was landfilled into bunded cells which were excavated from the in-situ cohesive alluvial subsoils. The excavated soils were then used in bund construction. When landfilling ceased at Churchtown the final area of the waste body was approximately 5 hectares and waste body forms a plateau shape compared to the adjacent lands

601 A number of remediation works recently commenced on the capped waste at Churchtown as detailed below:

- The existing landfill was capped with a permanent low permeability clay liner in conjunction with a willow and reed plantation and constructed wetland;
- The willow plantation in situated in the centre and above the capped waste (Zones 1 to 4) with a series of constructed wetlands along western and eastern side of willow plantation (see Figure 2);
- As of the 9th February 2015 site walk-over undertaken by BREL the willows and reeds were planted but not yet fully grown. Pumping and treatment of leachate was expected to commence in 2015 following completion of the tender for M&E works;
- When the willow plantation is fully grown and working at capacity leachate will be pumped to the plantation before discharged to surface water. If treated leachate levels are unacceptably elevated, the leachate is treated further by circulating via the constructed wetlands before discharging to surface water.

3.4 Site History

Churchtown Landfill is an unlined site, historically operated on a dilute and disperse principal, whereby solid waste is tipped directly onto the underlying excavated surface with leachate allowed to percolate directly through the soils with no engineered liner installed.

Landfilling began in 1987 and the site ceased operations on the 31st August 2000.

Groundwater quality monitoring was originally undertaken at four locations i.e. BH1, BH2, BH3 and BH4 as listed in Table F.4.2 in the waste licence which were drilling in August 1998. However, wells

BH1 to BH3 ceased to be utilised for groundwater monitoring, as they are now located within waste. They currently serve as leachate wells (*i.e.* L1, L2 & L3).

Two additional boreholes were drilled in July 2001 *i.e.* Borehole BH1 (downstream) and BH3 (upstream). However, difficulty was encountered during the installation of a second down gradient borehole due to the proximity of the waste body to the river. No borehole logs are available at the time of compiling this report.

Groundwater monitoring is currently undertaken within BH1 and BH3 and BH4. BH3 and BH4 are representative of up gradient water quality and borehole BH1 is representative of down gradient water quality. BH4 was subsequently damaged and was replaced in 2014.

3.5 Leachate Management

The Landfill at Churchtown was originally installed on a dilute and disperse principal, whereby solid waste was deposited directly onto the exposed overburden. This originally allowed untreated leachate to migrate laterally towards the River Finn and vertically into the bedrock aquifer. Vertical migration is considered to be unlikely due to the peat overburden acting as an aquitard.

The landfill was recently capped to prevent further influx of surface water and rainwater into the waste body thereby reducing leachate generation. A permanent low permeability clay liner was installed following closure of the site. As part of the 2014-2015 willow and reed bed construction, a 0.15 to 0.45 metre thick topsoil and 0.5 m clay cap was installed at the facility. In addition, a leachate treatment system was developed and is currently being implemented at the site. A brief description of the system is outlined below and a layout of the system is provided in **Appendix**.

- Leachate shall be extracted from three pumping stations and distributed around the site via a common 90mm HDPE leachate pumping main located adjacent to an existing site access road as shown on Drawing IBR0514 /PI102. This pumping main will primarily direct leachate to the willow plantation for treatment.
- The Willow Plantation is divided into four zones, with two main irrigation feed points each located centrally between Zone 1 and 2 and Zone 3 and 4 as outlined in the drawings. The connection to willow plantations shall be via 50mm leachate pumping main via an isolating valve, a strainer and a flowmeter as shown on the drawings.
- Treated effluent discharging from Zones 1/2 and Zones 3/4 will be monitored with Ammonia Analysers. Discharge not meeting consent parameters shall activate a motorised valve which in turn shall divert flow back to either Pumping Station 1 or 2 under existing gravity pipework for re-distribution in the willow plantation until the treated effluent reaches acceptable limits. Collected runoff effluent meeting the required parameters is discharged to adjacent surface water drains as shown on the drawings.
- Discharge flow from each monitoring chamber will be recorded and monitored on the SCADA system including leachate applied to the treatment zones, treated flows to surface water drains and flows redirected back to the system for re-distribution and additional treatment.
- The primary treatment method is anticipated to be through application to the willow plantation. Where leachate is available over and above the treatment capacity of the willow plantation (either through seasonal increases in leachate generation, wet/frosty weather conditions or manual operator intervention) leachate will be diverted to the onsite Integrated Constructed Wetlands (ICWs) as a secondary alternative. The system shall also allow the site operator to intervene and permit periodic irrigation of the ICWs when sufficient leachate is available during dry weather which would ordinarily be applied to the willow plantation in order to maintain the ICWs.
- Flow of leachate to ICW's will be controlled on the pumping main with an actuated valve within a precast concrete chamber along with flow measurements via flow meter. Flow of leachate shall be recorded on the PLC /HMI within the primary control panel. Flow of leachate to ICW's shall be via weir chamber and flow split on a 60 / 40 percentage basis, with a nominal maximum limit of 20m³/day treatment capacity in the ICWs.

- Should the treatment capacity of both the willow plantation and ICWs be reached in any given 24 hour period leachate abstraction and circulation within the site will be stopped until conditions allow treatments to recommence.
- Leachate is monitored at three monitoring wells located within the waste body, designated as L1, L2 and L3. Both leachate levels and leachate quality are monitored in these wells on a regular basis and are discussed in the following sections.

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4 GEOLOGY

4.1 Regional & Site Overburden

The regional overburden in the vicinity of the site is described using the Teagasc soil associations for the greater Donegal region. It is a part of the River Alluvium association (Code 05 RIV), which consists of a further 12 sub soil series. The River Alluvium association covers an area of approximately 22.54km². The Kilgory series (0500KG) is described as a sandy river alluvium for the region. EPA soil mapping describes the overburden as river alluvium (AlluvMin) underlain by undifferentiated gravelly alluvium subsoils. The regional teagasc soils map is presented in Figure A. Appendix B.

A summary of the historical site investigations at the site is provided in Table 4.1. Site Investigations undertaken in 1998 by Stratex Ltd recorded shallow river alluvium soils consisting of a soft brownish grey, sandy, clayey, organic Silt directly overlying soft, dark brown, silty Peat.

Underlying the alluvium soils comprises fluvio-glacial layers of slightly gravelly sands with interspersed gravel horizons with occasional thin bands of greenish grey sandy silts. A summary of the borehole logs is provided in Table 4.2 and borehole logs provided in Appendix C. Boreholes BH1, BH2 and BH3 are located within the waste and describe the thickness of the waste body as ranging between 4.8m and 6.8m thick. On the basis of the ground investigation records, the general stratigraphy of the site is summarised sequentially below:

- Silty Alluvium •
- Peat
- Sands with gravel horizons and silt bands •
- Gravels / Boulders
- Bedrock (PSSAMITE)

 Silty Alluvium Peat Sands with gra Gravels / Boul Bedrock (PSS 	avel horizons and si ders AMITE)	ilt bands
Company	Date	tion purchase Boreholes Drilled
Stratex Ltd	23 rd Sept 1998	3 overburden wells (BH1, BH2 & BH3) and 1 bedrock well
RPS	May 2005 م	Gas monitoring wells LG8 & LG9
Ground Check Ltd	December 2014	1 Bedrock monitoring well (BH4 replacement), two leachate points (L1A & L2) and 9 landfill gas wells.

Table 4.1 **Summary of Site Investigation Activities**

4.2 Regional Bedrock Geology

Churchtown landfill is mapped as being underlain by three bedrock formations. (see Figure B, Appendix B).

- The Claudy Formation which consists of psammitic schists with intercalated coarse psammite and pebbly grit units, thin marble lenses and quartzite is mapped in the southwestern quadrant of the site;
- A Marble Unit; and,
- The Aghyaran & Killygordon Limestone Formation which comprises Figureitic marble. • Quartzite and psammite.

The formation is bounded to the northwest by the Pettigoe-Lough Foyle fault which trends in a northeast - southwest direction. The strata are internally complex and folded along a general southwest to northeast trend compression axis coincident with the strike of the regional (Pettigoe-Lough Foyle) fault plane.

The complex structure of the rocks and the development of an interior schistosity results from several phases of folding and refolding is associated with a number of orogenic events, the last of which took place during the Variscan Orogeny. Site investigation boreholes at the site recorded bedrock in the initial BH4 borehole was described as a psammite with Schist recorded in the replacement BH4 in 2015.

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5 HYDROLOGY

5.1 Site Hydrology

The major surface water feature at Churchtown landfill is the River Finn which borders the southeastern boundary of the site. It rises in Lough Finn and flows east through a deep mountain valley to Ballybofey and Stranorlar (on opposite sides of the river) and on to the confluence with the River Mourne at Lifford, 3.18 km to the northeast of the site. All surface water flow in the area is towards the River Finn. There are a number of natural drainage features which drain surface water from the surrounding fields into this river. No formal drainage system is provided on the site however the two land drains that run the length of the northeastern and southwestern sides of the landfill direct surface water, and any leachate emitting from the waste body, into the River Finn.

The River Finn is prone to seasonal flooding, and because of this, a clay levee has been constructed on the southeastern border of the waste body to prevent inundation during periods of high water levels.

Leachate from the landfill drains through a number of collection toe drains and into a collection chamber on the southeast corner of the waste. Leachate is currently allowed to disperse to ground until completed of the Mechanical & Electrical (M&E) works for the new treatment facility at the site. The plan is for future leachate to be treated on site and discharged directly into the River Finn via the drain at SW3. There are a total of 6 surface water sampling locations at Churchtown landfill (see **Figure 3**). SW1 and SW2 are located within the drain on the northeastern site boundary and SW4 and SW5 within a drain along the southwestern site boundary. Surface water runoff discharges from the site between SW4 and SW5 before discharging into the River Finn. SW6 is an upstream monitoring point within the River Finn. SW3 is located halfway along the landfill boundary within the river and SW7 is a downstream compliance point within the river.

Visual evidence of potential leachate impact on the surface waters in the vicinity of the landfill was observed during the site walkover as is evident in **Photo 5.1**. However, this impact is likely to have been caused prior to current works being undertaken at the site with no observed leachate breakout from the site noted post completed capping works. As mentioned above, leachate will not be contained at the site until completion of the M&E works.



Photo 5.1 Surface water Drain to southwest of waste body

Flows within the River Finn, in proximity to Churchtown landfill, were not available at this time of this report. However, EPA flow readings both upstream and downstream of the site are summarised below:

Station Number	Station Name	Easting	Northing	Distance to Site (km)	Catchment Area (m ²)	DWF (m³/sec)	95th % flow (m³/sec)
1042	DREENAN	215257	394583	15.4	353	0.33	0.42
1043	BALLYBOFEY	213511	394674	17	319	0.3	0.4

Table 5.1River Finn Flows

5.2 Surface Water WFD Status

Work completed for the Water Framework Directive has assigned 'Status' to surface waters and groundwater (www.wfdireland.ie - watermaps). Churchtown landfill is located within the River Finn Surface Water Body (IE_XB_01_1_3) and has been assigned an overall status of 'Poor', specifically with an overall ecological and macroinvertebrate status of 'Poor'. It has been designated an overall physic-chemical status of 'High'. The overall objective status for the River Finn Waterbody is 'Restore_2021', *i.e.* restore the river body to pre-pollution status. The Q-rating of the river is currently rates as Q3 *i.e.* poor quality.

5.3 Designated Protected Areas

The River Finn is a designated Special Area of Conservation (SAC), selected for the following habitats and/or species listed on Annex I / II of the E.U. Habitats Directive (* = priority; numbers in brackets are Natura 2000 codes):

- [3110] Oligotrophic Waters containing very ew minerals;
- [4010] Wet Heath;
- [7130] Blanket Bogs (Active)*
- [7140] Transition Mires
- [1106] Atlantic Salmon (Salmo salar)
- [1355] Otter (Lutra lutra)

The Finn system is one of Ireland's premier salmon waters. This SAC comprises almost the entire freshwater element of the River Finn and its tributaries the Corlacky, the Reelan sub-catchment, the Sruhamboy, Elatagh, Cummirk and Glashagh, and also includes Lough Finn, where the river rises.

6 HYDROGEOLOGY

6.1 Aquifer Classification

The site is underlain by Churchtown Groundwater Body (GWB) which is within the larger Raphoe GWB. It is likely the Churchtown GWB was delineated based on the presence of Churchtown landfill. No information is currently available on Churchtown GWB from the GSI; however it is likely to be similar to the Raphoe GWB. The vast majority (~85%) of the Raphoe GWB is underlain by a Locally Important (LI) aquifer which is moderately productive only in local zones. The remaining areas are underlain by a Poorly Productive (PI) aquifer which is generally unproductive except for local zones. The majority of the site is underlain by a locally important aquifer with the southwestern quadrant mapped as Poorly Productive (See Figure C, Appendix A).

Groundwater yields in the Raphoe GWB range from $2-330 \text{ m}^3/\text{day}$ (based on 6 wells within the GWB). Groundwater flux is expected to occur in the uppermost part of the aquifer comprising a broken and weathered zone typically less than 3m thick, a zone of interconnected fissuring around 10-15m thick, and a zone of isolated poorly connected fissuring typically less than 150m.

The underlying geology of the site, which is identified as relatively impermeable psammites and schists is expected to significantly reduce the downward movement of leachate from the landfill mass. It is therefore expected that leachate moving from the waste body is likely to migrate horizontally along the weathered boundary of the bedrock and in the direction of the nearest major water body, the River Finn.

6.2 Aquifer Vulnerability

only any other use. Groundwater vulnerability is dictated by the nature and thickness of the material overlying the uppermost groundwater. This means that vulnerability relates to the permeability and thickness of the subsoils, which will dictate the ability of surface waters percolating through to any underlying groundwater bodies. A detailed description of the groundwater vulnerability categories can be found in the Groundwater Protection Schemes document (DELG/EPA/GSI, 1999) and in the draft GSI Guidelines for Assessment and Mapping of Groundwater Vulnerability to Contamination (Fitzsimons et al, 2003). A groundwater vulnerability map can be viewed online (http://www.gsi.ie/Mapping).

The majority of the Raphoe GWB is classified as Extreme vulnerability, due to the high percentage of thin subsoil and rock outcrops. Where subsoil is thicker, such as in the valleys, the vulnerability is mainly high, with occasional small areas of Moderate that are associated with areas of deeper deposits.

Churchtown landfill is predominantly mapped y the GSI as High vulnerability with Extreme vulnerability mapped in the western region of the site where bedrock was anticipated to be close to surface. However, it is noted that depth to bedrock within BH4 in the western region of the landfill recorded bedrock at a depth of approximately 8.0 metres which represents a Moderate vulnerability classification.

6.3 Groundwater WFD Status

Work completed for the Water Framework Directive has assigned 'Status' to surface waters and groundwater (www.wfdireland.ie - watermaps). The landfill is located within the Raphoe GWB (IE_NW_G_054) that has been assigned an overall 'Good Status' (www.wfdireland.ie). It been assigned an overall objective status of 'Protect'. Overall the GWB has been given a risk status of 2b, i.e. 'Not at Risk'.

6.4 Background Groundwater Quality

There is no background groundwater quality available for the Churchtown GWB, however limited hydrochemical information is available for the larger Raphoe GWB which has similar geology to Churchtown GWB. The hydrochemical signature is that of calcareous Precambrian Marbles. Generally a CaHCO3 signature. Alkalinity (mg/l as CaCO3): range of 112-428; mean of 274 (22 data points) Total Hardness (mg/l): range of 180-436; mean of 311 (22 data points) Conductivity (µS/cm): range of 414-814; mean of 667 (22 data points).

6.5 Local Groundwater Usage and Source Protection Area

There are no source protection areas within 5km of the site, however there are three water wells within 2km of the site, as mapped by the GSI (<u>https://www.gsi.ie/Mapping</u>). A table describing these nearby water wells can be seen below in Table 6.1. Well locations are outlined on Figure E, Appendix B.

Well Code	Easting	Northing	Total Depth (m)	Depth to Rock	Yield (m³/day)	Yield Class
2039SEW016	228790	396260	3.6	1.8	21.8	Poor
2039SEW019	229530	398030	5.2	1.8	16.4	Poor
2339SWW001	231520	397460	3.1	1.2	3.1	Poor

Table 6.1	Groundwater wells within 2km of Churchtown landfill
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Jundy All local residences (within 500m) do not use private groundwater wells and are fed from the mains water supply at Lifford.

6.6 Recharge Rainfall Diffuse recharge occurs via rainfall percolating through the subsoil and rock outcrops. Due to the low permeability of some subsoil deposits and the aquifers, a high proportion of the effective rainfall will quickly discharge to the streams in the GWB. The reasonably high stream density is reflects the high proportion of surface runoff as opposed to recharge. The GSI has mapped the average groundwater recharge to be 151-200 mm/yr. Average monthly gridded rainfall data was sourced from Met Éireann COL and is presented in Table 6.2.

J	F	М	Α	М	J	J	Α	S	0	Ν	D	Annual (mm)
162.2	189.9	71.6	33.4	86.8	48.6	86.0	95.3	23.0	131.4	134.4	150.5	1213.1



The closest synoptic station to the site is at Malin Head, 105 km to the northeast, where average potential evapotranspiration (PE) is 538.38 mm/yr. This value is used as a best estimate of the site PE. Actual evapotranspiration (AE) is estimated by multiplying PE by 0.95, to allow for the reduction in evapotranspiration during periods when a soil moisture deficit is present (Water Framework Directive, 2004). Actual evapotranspiration is therefore 511.46 mm yr⁻¹ (0.95 PE). The GSI estimated recharge across the site ranging between 51 and 100 mm/year.

The Effective Rainfall (ER) for the site is determined from:
6.7 Groundwater Monitoring Wells

As mentioned previously, groundwater level monitoring is undertaken within monitoring wells BH1 and BH3 and BH4. BH3 and BH4 are considered representative of up gradient water quality and borehole BH1 is partially representative of down gradient water quality. BH4 was damaged and was subsequently replaced, in the same location, in 2014.

Borehole ID	Well Screen Horizon	Ground Level	Total Depth as per borehole log	Screen Depth	Water Strike	Depth to bedrock	Well Head Level
		mOD ¹	mbgl ²	mbgl	mbgl	mbgl	mOD
BH1	Overburden	2.74	11.0	5.0-11.0	4.0 & 8.0	N/A	3.47
BH3	Overburden	8.23	31.0	N/A	N/A	26.0	8.72
BH4	-	2.45	9.0	N/A - 9.0	4.3 & 8.2	8.2	2.89
BH4 (replaced)	Bedrock	-	12.0	9.5 – 11.0	7.0 & 9.0	7.9	-

Details of each monitoring is summarised below in **Table 6.3**.



6.8 Groundwater Levels & Flow Direction of the other section of the othe

Groundwater levels in the monitoring boreholes have been recorded on a quarterly basis since 2004. Based on the topography of the land, with a high point to the northwest and a major surface water feature of the River Finn to the southeast it is likely the groundwater flow is in a southeasterly direction with the river acting as a hydraulic boundary.

Interpreted groundwater data from the three groundwater monitoring confirms groundwater flow direction to the southeast. A number of irregularities with the groundwater levels are possibly associated with errors in recording the data. A figure providing groundwater levels is outlined below in **Figure 6.1**.

A review of groundwater levels over time indicates the following:

• Water levels recorded within BH3, located to the northwest and upgradient of the site, are consistently above BH4 and BH1 are represents upgradient groundwater levels. The monitoring well log reports an installation within the overburden; however a log note provided by Kirk McClure Morton records a total depth of 31 mbgl with bedrock encountered at 26 mbgl. No indication of the installation details of these well. The levels vary between 2.7 and 7.3 mOD. The variations over time do not appear to correlate with rainfall data. Noticeable increased in levels were recorded between December 2007 (3.0 mOD) and January 2008 (7.25 mOD) with no corresponding increase noted in the downgradient monitoring wells. The increased level remained relatively sustained until August 2008 when a sharp reduction on levels is noted. The levels appear to be broadly increasing over time since commencement of monitoring in 2006. This well should not be confused with BH3 historically drilled in the centre of the site which was installed within the overburden and was subsequently decommissioned.

¹ mOD – metres Ordnance Datum

² mbgl = metres below ground level

- Water levels within monitoring well **BH4** remained relatively consistent over time and indicate artesian conditions during particular periods. No levels have been recorded at this location since 2009. Levels recorded range between 1.6 and 2.5 mOD. The installation of the original well is unclear; however the replacement well in 2014 is installed within the bedrock. No data trends have been recorded to-date within the replaced monitoring well. However, artesian conditions were observed during the site visit in February 2015.
- Water levels within monitoring well **BH1**, located in proximity to the River Finn, and partially down gradient to the landfill, recorded levels ranging between -2.0 to 2.1 mOD. The well is installed within the overburden only. A noticeable decrease in levels was recorded between December 2007 and January 2008 with levels falling from 1.6 to -0.5 mOD. The levels continued to fall to a low of -2.0 in July 2008 before rapidly rising to 1.5 mOD in September 2008. The rational for these reductions is unclear. The sudden drop in levels corresponds with sudden increases in levels in BH3 during the same period between December 2007 and January 2008.
- Hydraulic gradients across the site based on recorded water levels range between 0.002 and 0.019 which are considered to be relatively low.

A review of leachate monitoring at Churchtown Landfill was also undertaken. The graphed data is provided in **Figure 6.2**.

- Leachate levels within well L3, located in the northern corner of the site, range between 3.6 and 6.0 mOD. A notable jump in levels was recorded between November 2007 and January 2008 ranging up to 2.1 metres. This corresponds with the sudden increase in upgradient groundwater levels in BH3. The highest levels recorded in L3 (*i.e.* 6.0 mOD) were 1.23 metres below the capping layer of the landfill. No notable downward trend is evident in leachate levels in L3.
- Leachate levels within well L1, located in the central region of the landfill, ranged between 2.5 and 5.7 mOD. A notable reduction in levels were recorded between late 2007 and mid 2008 corresponding with a fall in levels in downgradient monitoring well BH1. The highest recent level recorded in L1 (*i.e.* 5.1 mOD) remains 2.2 m below the capping layer of the landfill in this area.
- Leachate levels within well 12, located in the northeastern corner of the waste body, ranged between 1.2 and 4.5 mOD A notable fall in levels was recorded between November 2007 and April 2008 corresponding strongly with a fall in levels in downgradient groundwater monitoring well, BH1. The highest recorded level of 4.5 mOD remains 1.65 metres below the top of the landfill cap.

In summary, it would appear that groundwater level variability in the area significantly impacts on leachate levels within the waste body. The correlating increases and reductions in groundwater levels and leachate levels confirm this theory with groundwater appearing to intersect the waste body. Groundwater level variations and levels upgradient of the site have a differing signature to groundwater levels closer to the River Finn which suggests that the river is partially impacting on groundwater levels downgradient of the landfill, as expected.

6.9 Permeability

3 no. in-situ permeability tests were undertaken by Stratex Ltd in 1998 within BH1, BH3 and BH4. The coefficient of permeability recorded were 3.3×10^{-6} m/s (BH1), 4.0×10^{-6} m/s (BH3) and 2.6×10^{-3} m/s (BH4)









7 PRELIMINARY S-P-R

The hydrogeological impact assessment is guided by the source-pathway-receptor model. The S-P-R model is used to identify the sources of water and potential contaminants, the environmental assets affected by such, and the pathways by which water and contaminants reach those receptors. **Table 7.1** shows the preliminary S-P-R model for the site which can be refined as the assessment evolves and more information is acquired.

Sources	Pathways	Receptors	Risk
	Groundwater	River Finn	High
Leachate	Leachate vertical		High
Leachate	migration to groundwater	Groundwater	Low to Moderate
	Leachate horizontal migration to surface water	River Finn	High



The landfill at Churchtown was not originally developed on a containment basis *i.e.* there is no engineered liner below the landfill. The waste body has been capped since but there is likely to be strong potential for leachate generation and leakage from within the waste.

Originally there was very little mitigation measures controlling potential leachate discharge to groundwater. However, recent improvement to the landfill infrastructure will mitigate the risk of leachate migration detailed in Section 3.5 (

Give the proximity of the landfill to the River Finn, the interpreted groundwater flow direction to the river and the fact that groundwater downgradient of the landfill is not used (nor can be used) as a potable drinking supply, the Raphoe GWB is not considered to be a sensitive receptor at risk

8 HYDROCHEMISTRY

Hydrochemical data was acquired from previous reports supplied by Donegal County Council (DCC) and EPA Annual Environmental Reports (2004-2013) available online. As required under the Waste Licence for Churchtown landfill (*i.e.* WL62-1) groundwater monitoring has been and currently is undertaken at monitoring well locations as set out in the current waste licence. The schedule of the current waste licence requires the monitoring of particular parameters on a quarterly or annual basis.

8.1 Monitoring Locations & Frequency

Monitoring is undertaken within three groundwater boreholes three leachate boreholes (located within the waste) and seven surface water monitoring stations. A table of monitoring locations is presented below in **Table 8.1**. In addition, future monitoring points for the Willow Plantation and Integrated Constructed Wetland system will be included for the site.

Location	Upstream/Downstream	eam/Downstream Screened Horizon		Northing
BH1	Downgradient	Overburden/Groundwater	231,072	395,752
BH3	Upgradient	Overburden/Groundwater	230,840	396,127
BH4	Upgradient	Bedrock/Groundwater	230,818	296,041
L1	Waste	Waste/Leachate	230,999	395,925
L2	Waste	Waste/Leachate	231,169	395,887
L3	Waste	Waste/Leachate	230,931	396,142
SW1	Upstream	Surface Water	230,934	396,164
SW2	Southwest Drain	SurfaceWater	231,177	395,895
SW3	Adjacent	Surface Water	231,180	395,840
SW4	Southeast Drain	Surface Water	231,026	395,734
SW5	Adjacent	Surface Water	231,038	395,711
SW6	Upstream	Surface Water	230,983	295,705
SW7	Downstream	Surface Water	231,248	395,949



Monitoring Locations

Leachate monitoring wells, L1, L2 and L3 (formerly BH1, BH2 and BH3), were originally designated as groundwater monitoring wells. These were reassigned as leachate wells due to their installation within the actual waste body. BH1 and BH3 were subsequently re-drilled outside of the waste for groundwater monitoring purposes.

The frequencies of groundwater and leachate monitoring are presented in **Table 8.2**.

Quarterly	Annually	
Visual Inspection/Odour, Groundwater levels, Ammoniacal Nitrogen, Chloride, Dissolved Oxygen, Electrical Conductivity, pH, Temperature, Potassium, Sodium, TON, TOC, Nitrate, Nitrite, Phenols.	Boron, Cadmium, Calcium, Chromium, Copper, Cyanide, Fluoride, Iron, Lead, List I & II organic substances, Manganese, Magnesium, Mercury, Sulphate, Total Alkalinity, Total Phosphorous, Residue on evaporation, Zinc, Faecal Coliforms, Total	
	Coliforms	

Table 8.2 Parameters and Frequency of Groundwater Monitoring

The list of parameters and monitoring frequency for surface water is seen below in **Table 8.3**.

Quarterly	Annually
COD, Chloride, Ammoniacal Nitrogen, BOD, Dissolved Oxygen, Electrical Conductivity, pH, Temperature, TSS, Chlorine, Copper, Nitrate, Nitrite, Phenols, Zinc	Cadmium, Calcium, Chromium, Iron, Lead, List I & II organics, Magnesium, Manganese, Mercury, Potassium, Sulphate, Sodium, Total Alkalinity, Total Phosphorous, TON.

Table 8.3	Parameters and Frequency of Surface Water Monitoring
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8.2 Human Health & Environmental Risk Assessment Framework

Groundwater concentrations have been compared to the 2010 Groundwater Regulations Target Trigger Value (i.e. GTV) in addition to the Environmental Protection Agency Interim Guideline Values (IGV) for Groundwater as presented in EPA interim report "Towards Setting Guideline Values for the Protection of Groundwater in Ireland" 2002. The IGVs have been selected on the basis of the lowest of either the drinking water standards, historical environmental guality standards for surface water or GSI trigger values and are therefore highly conservative and protective of all groundwater receptors.

There are currently no published generic assessment criteria for groundwater derived specifically to be protective of human health via direct contact. However it can be assumed that if water is considered safe for human consumption then there are no risks from direct contact. The 2007 Drinking Water Regulations were utilised for this purpose.

All surface water levels have been compared to the 2009 Surface Water Regulations and the 1998 Salmonid Regulations. The leachate sample results were compared with licence limits as assigned by PUTPOSES the EPA.

the EPA. **8.3 Leachate Quality** Leachate quality can vary during the lifetime of landfill sites depending on the phase of decomposition. In terms of the overall suite of parameters analysed, raw leachate results from the Churchtown landfill have been compared to "Typical Leachate Composition of 30 Samples from UK/Irish Landfills accepting mainly Domestic Waste" (kandfill Operational Practices) and are within the maximum cons concentrations.

As is evident from Figures 8.1 to 8.3 leachate quality data from all leachate monitoring wells recorded a reducing trend over time. The reduction in levels are most noticeable within L3 with Ammoniacal Nitrogen, Electrical Conductivity and Chloride levels all significantly reduced since pre-capping works. Slightly increasing EC and Ammoniacal Nitrogen levels are noted in L2 since 2012.

In summary, the raw leachate results from the landfill are considered to represent a landfill in the methanogenic stage of decomposition of organic compounds. The leachate is considered to be relatively low strength and the levels, which are reducing over time, are expected to reduce further.



Figure 8.1 Ammoniacal Nitrogen Levels - Leachate



Figure 8.2 Electrical Conductivity Levels - Leachate



8.4 Groundwater Quality

Groundwater monitoring is undertaken within three monitoring boreholes as detailed in **Section 8.1.** BH4 ceased monitoring in 2007 due to inaccessibility issues and was recently re-drilled in December 2014. The following parameters are discussed in detail in relation to Churchtown landfill:

8.4.1 Ammoniacal Nitrogen

Ammoniacal Nitrogen levels within upgradient monitoring well **BH3** recorded levels ranging between 0.02 and 0.25 mg/l (see **Figure 8.4**). The levels are predominantly below the 2010 GTV with the exception no. 3 no. minor exceedances over time that are attributed to natural variations.

Ammoniacal Nitrogen levels in **BH1**, which is considered to be partially downgradient of the waste body, are also typically recorded below the 2010 GTV since May 2009 with one minor exceedance recorded in September 2013 (*i.e.* 0.3 mg/l). Since 2010 the levels detected in BH1 are consistently lower than those detected in upgradient well BH3 which suggests a low level of impact by the landfill on groundwater. It is however noted that BH1 is not truly downgradient of the waste body and may not accurately reflect the level of contaminant groundwater flux towards the River Finn.

Prior to 2007, similarly low levels of Ammoniacal Nitrogen were recorded within **BH4** with a single minor exceedance in July 2006. No samples were collected from BH4 between May 2007 and March 2015.



Figure 8.4 Ammoniacal Nitrogen Levels - Groundwater

8.4.2 Electrical Conductivity (EC)

EC levels within upgradient monitoring well **BH3** ranged between 155 and 529 μ S/cm representing background conditions. Generally reduced EC levels were recorded within **BH1** ranging between 86 and 482 μ S/cm. EC levels in **BH4** pre 2007 were recorded consistently between 335 and 385 μ S/cm and between 347 and 403 μ S/cm in 2015.

EC levels in BH1 are consistently lower than those recorded in upgradient well BH3 and do not indicate an impact to groundwater by the waste body. A notable decrease in EC in BH1 was recorded between October 2008 and March 2011 which may be attributed to surface water or river water ingress during flooding events. As highlighted in Section 8.4.1, BH1 is not truly downgradient of the waste body and may not accurately reflect the level of contaminant groundwater flux towards the River Finn.



8.4.3 Chloride

Chloride levels are recorded consistently below the 2010 GTV ranging between 16 and 60 mg/l within upgradient well BH3 and between 13 and 41 mg/l in partially downgradient well BH1. Levels within BH4 pre 2007 were recorded between 25 and 30 mg/l.

Total Organic Carbon (TOC) 8.4.4

TOC levels were generally recorded at background levels across the site with the exception of one isolated spike in BH3 (81.17mg/l). This level subsequently returned to background levels of <4 mg/l during the following sampling event. A notable increase in TOC levels were noted between June 2008 and February 2009 in downgradient well BH1 (i.e. 13.1, 16.0 and 11.9 mg/l respectively). This extended increase in TOC is unclear but may be resulting from surface water ingress to the well during this period.

Total Oxidised Nitrogen (TON) 8.4.5

Elevated and variable levels of TON were occasionally recorded in both the upgradient and downgradient monitoring wells (see Figure 8.6). High levels of TON in a water body can contribute to excessive algal growth in waterways as TON is a measure of both nitrate and nitrite in soluble compound form, readily usable by plants and algae.

There appears to be a broad decreasing trend in downgradient monitoring well BH1 and no apparent trend in the upgradient BH3. The levels recorded within BH1 are generally higher than those recorded in upgradient well BH3 which suggests an impact of the landfill on groundwater immediately upgradient of BH1.



8.4.6 **Other Parameters**

Sodium

All levels of Sodium were recorded well below the 2010 GTV of 150 mg/l. All levels were generally recorded below 20mg/l with the exception of two isolated occasions where more elevated levels were recorded in both BH1 and BH3 (*i.e.* 86 mg/l, September 2009 and 56.5 mg/l, June 2014 respectively).

Nitrate & Nitrite

No detection of Nitrate or Nitrite above the 2010 GTVs were recorded in groundwater across the site.

Sulphate

No detections of Sulphate above the GTV were recorded in groundwater across the monitoring period to 2015.

Iron

Significant uncertainties persist regarding the true results provided by the Donegal County Council laboratory in relation to units and limits of detection. Therefore no accurate assessment has been undertaken as part of the completion of this report.

ORP

Levels of ORP were recorded in BH3 ranging between 0.001 and 0.185 mg/l. The IGV for ORP is 0.03 mg/l. Notable more elevated levels were recorded in BH1 ranging between 0.001 and 0.258 mg/l over the monitoring period to date. The levels recorded within BH4 are broadly similar to those recorded in BH3 ranging between <0.01 and 0.082 mg/l. The downgradient levels in BH1 suggest the waste body was historically impacting on groundwater quality between upgradient and downgradient monitoring wells. However a downward trend is noted in BH3 since June 2008 and within Bh1 since February 2009. No detections of elevated ORP have been recorded in BH1 since May 2010.

Heavy Metals •

Heavy metals were not recorded above their respective IGV or GTV in groundwater samples during the monitoring period to-date.

Semi Volatile & Volatile Organic Compounds

No detection of VOCs or sVOCs above the laboratory limits of detection or any threshold guideline value was recorded in either upgradient or downgradient monitoring wells between 2006 and 2015.

BTEX Hydrocarbons •

No recording of BTEX (Benzene, Toluene, Ethyl-Benzene & Xylene) hydrocarbons were recorded above the limit of detection (LOD) for this suite of testing.

Phenols

Phenol analysis was occasionally undertaken in monitoring wells BH1 and BH3. The results recorded were consistently below the laboratory limit of detection.

 Trihalomethanes (THMs)
 Total-Trihalomethanes (THM) is the sum of Dichloromethane, Chloroform, Bromodichloromethane and Bromoform. Chemical analysis was occasionally undertaken in groundwater for these parameters and Consent of copyright own the results were consistently below the laboratory limit of detection.

8.5 Surface Water Quality

The primary receptor for the Churchtown landfill catchment has been identified as the River Finn (River Code; IE_XB_01_1_3). The river is located along the southeastern site boundary, flows in a northeast direction and forms the border between Donegal and Northern Ireland.

The overall status of River Finn has been described as 'Poor' by the EPA; with a General physiochemical status (PC) status of 'High', a Macroinvertebrate status (Q) of 'Poor' and an overall ecological status (ES) of 'Poor'. It has been given an overall risk status of 1a (at risk). The Q-rating for the river is current rated as Q3 – poor quality status. Surface water sampling for monitoring purposes is undertaken at 7 locations on and around Churchtown landfill (See **Table 8.4** below and **Figure 3**).

It should be noted that uncertainties surround the accuracy of the laboratory results in addition to the sampling locations within the river during sampling events.

Monitoring Point	Easting	Northing	Location
SW1	230,934	396,164	Upgradient within drain along northeastern site boundary
SW2	231,177	395,895	Downgradient within drain along northeastern site boundary
SW3	231,180	395,840	River Finn (at landfill site)
SW4	231,026	395,734	Southwestern site drain upgradient of treated leachate discharge location
SW5	231,038	395,711	Southwestern site drain downgradient of treated leachate discharge location ³
SW6	230.983	295,705	Upstream (River Finn)
SW7	231,248	395,949	Downstream (River Finn)

Table 8.4

Surface water sampling locations

8.5.1 Ammoniacal Nitrogen

Surface Water Drains

Upgradient surface water quality within the northeastern boundary landfill drain (*i.e.* **SW1**) flowing from an upgradient location before discharging into the River Finn is recorded as generally good quality with relatively low levels of Ammoniacal Nitrogen detected. The levels of Ammoniacal Nitrogen at this location range between 0.01 and 0.3 mg/l. The flow within this drain is currently unknown.

The drain discharges to the River Finn in the southeastern corner of the landfill. **SW2** is located within the drain immediately prior to its discharge to the River Finn. Water quality monitoring at SW2 over time recorded significantly elevated levels of Ammoniacal Nitrogen ranging between 0.02 and 128.2 mg/l (see **Figure 8.8**). The levels recorded indicate an impact from landfill leachate during a time when leachate was allowed to be dispersed to this drain from the flanks of uncapped landfill. The drain is currently visually impacted with heavy iron ochre, in particular towards the River Finn end of the drain (see **Figure 8.9**).

³ It is noted that SW5 is currently sampled from an adjacent site drain at the site. However, it is considered more beneficial if the sample location was collected from the River Finn within the mixing zone of the SW4 drain and the river.



Figure 8.8 Northeastern Drain - Ammoniacal Nitrogen Levels



Figure 8.9 Leachate Impact within Northeastern Boundary Drain pre-restoration

Elevated levels of Ammoniacal Nitrogen have been recorded within the southwestern boundary drain (see **Figure 8.10**) within samples **SW4** and further downgradient at **SW5**. The levels within SW4 range between 0.01 and 65.3 mg/l and within SW5 ranging between 0.01 and 151 mg/l. The levels recorded vary over time with broadly reducing levels noted between 2010 and 2014. However, increased levels have been noted in both locations since mid-2014.

The high levels of Ammoniacal Nitrogen in this drain are, similar to the northeastern drain, resultant from landfill leachate during a time when leachate was allowed to be dispersed to this drain from the flanks of uncapped landfill. These levels are expected to decrease significantly following completion of all remedial works at the site.



Figure 8.10 Southwestern Drain - Ammoniacal Nitrogen Levels

<u>River Finn</u>

Ammoniacal Nitrogen levels upgradient of the site within the River Finn at **SW6** range between 0.01 and 26.0 mg/l. The levels are generally below the 2009 Surface Water Regulations (SWR) threshold for Good Status River Water of 0.065 mg/l with elevated 'spiked' levels noted on occasion. These spiked readings were recorded on three occasions only – *i.e.* 2005, 2010 and 2014 ranging between 1.69 and 26.0 mg/l/.

Mid-gradient monitoring at **SW3**, in the immediate vicinity of the landfill, recorded slightly more elevated levels ranging between 0.01 and 1.8 mg/l. Historically the levels were recorded within or slightly above the SWR; however, increasing levels have been noted since November 2012. These increasing levels are attributed to the current uncontrolled discharge of leachate contaminated surface water drains from the landfill. Notable reductions in levels are expected at SW3 following completion of proposed leachate treatment works at the site.

Downgradient monitoring at **SW7** within the River Finn ranged between 0.01 and 1.44 mg/l. The levels are broadly similar to mid-gradient sample SW3 with levels recorded both higher and lower than levels at SW3 on various occasions. On occasions when SW7 levels are recorded above SW3, the source of this increase may be attributed to a potential downgradient agricultural source of contamination or potentially due to sampling locations/techniques within the river (*i.e.* sample collected from the river bank and/or from the centre of the river).

A comparison of levels recorded with rainfall was undertaken. A number of notable increases in downstream Ammoniacal Nitrogen levels were recorded within SW7 following periods of high rainfall.

The data suggests that the landfill is having a limited impact on the quality of the River Finn in the immediate vicinity of the landfill in its current setup. The impact at SW3 is predominantly attributed to the discharges from the site drains at the landfill site. As is evident from the site data, notable reductions are typically recorded in downstream sample SW7. In addition the assimilative capacity calculation in **Section 8.9** confirms the low impact that is occurring.



Figure 8.11 Kiver Finn - Ammoniacal Nitrogen Levels

8.5.2 Electrical Conductivity (EC)

Surface Water Drains

Upgradient EC levels within the northeastern boundary drain *i.e.* **SW1**, are consistently recorded between 200 and 300 μ S/cm whereas EC levels in downgradient sample location, **SW2**, typically recorded a notable increase in EC levels (see **Figure 8.11**) ranging between 50 and 5050 μ S/cm.



Figure 8.12 Northeastern Drain Electrical Conductivity

Elevated EC levels were recorded in both SW4 and SW5 in the southwestern site drain ranging between 50 and 3350 µS/cm. The levels recorded in both drains are broadly similar during each sampling event and confirm an impact from landfill leachate. Form

<u>River Finn</u> EC levels within the River Finn are typical of background unpolluted surface waters ranging between 40 and 232 µS/cm. Isolated 'spiked'reading were recorded at each sample location separately ranging between 561 and 1119 µS/cm. The levels recorded upgradient and downgradient of the site do not record any noticeable difference as the river flows by the landfill site.

8.5.3 Chloride

Surface Water Drains

Upgradient Chloride levels within the northeastern boundary drain *i.e.* SW1, are consistently recorded between 14 and 39 mg/l whereas Chloride levels in downgradient sample location, SW2, typically recorded a notable increase in levels ranging between 13 and 155 µS/cm.

Elevated Chloride levels were recorded in both SW4 and SW5 in the southwestern site drain ranging between 13 and 242 µS/cm. The levels recorded at both locations are broadly similar during each sampling event and confirm an impact from landfill leachate.

<u>River Finn</u>

Chloride levels within the River Finn are broadly similar between upgradient and downgradient sampling locations. Levels range between 8.3 and 36 mg/l. A single isolated high level of 172 mg/l was recorded at SW3 in September 2014.

8.5.4 Orthophosphate (ORP)

River Finn

ORP levels within the River Finn are broadly similar between upgradient and downgradient sampling locations general below the Good Status SWR. Notable isolated 'spiked' levels are recorded at midgradient sample point, SW3 ranging between 0.076 and 1.08 mg/l above the mean SWR of 0.035 mg/l. A single isolated high level of 172 mg/l was recorded at SW3 in September 2014.

Surface Water Drains

Upgradient ORP levels within the northeastern boundary drain *i.e.* SW1, are consistently recorded between 0.003 and 0.07 mg/l with an isolated high level of 3.3 mg/l recorded in February 2009. ORP levels in downgradient sample location, SW2, typically recorded a notable increase in ORP levels ranging between 0.002 and 0.28 mg/l.

Elevated ORP levels were recorded in both SW4 and SW5 in the southwestern site drain ranging between 0.002 and 0.35 mg/l. The levels recorded at both locations are broadly similar during each sampling event.

8.5.5 Biochemical Oxygen Demand (BOD)

River Finn

BOD levels within the River Finn are broadly similar between upgradient and downgradient sampling locations. Levels range between 0.04 and 7.8 mg/l. A single isolated high level of 20 mg/l was only an required for recorded at SW7 in February 2010. purposes

Surface Water Drains

Upgradient BOD within the northeastern boundary drain *i.e.* SW1, are relatively variable ranging between 0.03 and 6.12 mg/l. BOD levels in downgradient sample location, SW2, typically recorded between 0.04 and 12.2 mg/l. A single high BOD level of 46.2 in May 2011 was recorded in SW2. The BOD level at SW1 are occasional more elevated than levels detected at SW2 and vice versa.

Occasionally elevated BOD levels were recorded in both SW4 and SW5 in the southwestern site drain ranging between 0.04 and 9.0 mg/twith the levels at both locations broadly similar.

8.5.6 VOCs/sVOCs/Hydrocarbons/Heavy Metals

The remaining parameters analysed, as per Table 8.1 were recorded below laboratory limits of detection or within EQS threshold levels and are not considered further in this report.

8.6 Surface Water Quality Summary

In summary, water quality data within the northeastern and southwestern boundary drains demonstrates an impact from landfill leachate over time with elevated levels of contaminants indicative of an impact from landfill leachate. The source of the impact is from landfill leachate during a time when leachate was allowed to be dispersed to these drains from the flanks of uncapped landfill. As the remediation of the site is not completed to-date, these surface waters will continue to record an impact from leachate in the short term. However, on completion of the works, the water quality in both drains are expected to noticeable increase over time.

The quality of the River Finn, with the current discharges from the landfill drains (and to a significantly lesser extent from groundwater baseflow), does not indicate a significant impact over time as the river flows by the landfill. A slight deterioration in quality is noted within the discharge zones of the surface water drains into the River Finn, however the scale of the impact is considered to be low. On occasion where downstream contaminant levels are recorded above mid-stream levels at the landfill site, the source of this increase may be attributed to a downgradient agricultural source of contamination or potentially due to sampling locations within the river (*i.e.* sample collected from the river bank and/or from the centre of the river).

8.7 Groundwater Contaminant Fluxes / Assimilative Capacity

An estimate of the assimilative capacity of the River Finn was made by comparing the Ammoniacal Nitrogen load discharging from the landfill site via **groundwater flux** and the actual concentrations measured in the river.

Leachate discharge from the site can be described by Darcy's Law equation:

Q = KiA

- where: Q_{river} = annual mean flow of the River Finn (m³/sec) 0.4 m³/s (95th %ile) or 34,560 m³/day (see **Section 5.1**).
 - K = the hydraulic conductivity of the conducting units in this case the average of site specific readings from BH1 and BH3 (*i.e.* $3.7 \times 10^{-6} \text{ m/sec}$) see **Section 6.9**.
 - i = the hydraulic gradient utilising highest recorded gradients as a conservative measure (*i.e.* 0.019).
 - A = the area over which contaminant flow is occurring *i.e.* 100 metre length (*i.e.* and 6 metre deep vertical plane across approximately 100% of the section.

Based on the above data a daily groundwater throughput (*i.e.* Q_{gw}) of 3.6 m³/day or **3,644 litres/day** was calculated which equates to 3.5 g/day.

Therefore given the flow within the River Finn, the dilution effect in the river is estimated at approximately **9,483** times the landfill groundwater flux. Using the highest Ammoniacal Nitrogen concentration recently recorded in BH1 (*i.e.* 0.97 mg/l in September 2015), the dilution capacity within the river would reduce this level to 0.1 μ g/l approximately which represents an approximate increase of <0.01% of Ammoniacal Nitrogen levels within the river.

An assimilative capacity assessment for two monitoring periods (*i.e.* June 2013 and September 2015) was undertaken to represent most recent conditions and prior to completion of the current remediation works. This assessment is detailed in **Appendix E** and indicates the following:

- The <u>predicted</u> Ammoniacal Nitrogen concentration downstream of the landfill at SW7 in **September 2015** was recorded to be higher than the actually recorded downstream level in the river. This would imply that other factors are reducing the contaminant loading to the river *e.g.* the flow in the river at this time was greater than the 95%thile flow or lower contaminant fluxes to the river are actually occurring rather than what is being recorded in BH1. These results also suggest that the surface water discharges from the landfill are also having a low level impact on the river quality.
- The <u>predicted</u> Ammoniacal Nitrogen concentration downstream of the landfill at SW7 in June 2013 were similar to the chemical data recorded in the river for this monitoring event with no notable increase in downstream levels predicted or recorded.

Based on the above, it is evident that the current conservatively calculated groundwater contaminant flux to the river from the landfill body is having a negligible effect on the quality of the River Finn. It is also noted that the calculations ignore the further reducing effects of the peat/silty overburden and the reducing trends occurring over time which are likely to reduce impacts to the river even further. Finally, although the contributions from the surface water landfill drains are likely to have a significantly greater impact on the river in comparison to the groundwater flux, these impacts are considered to be low to negligible.

9 UPDATED HYDROGEOLOGICAL CONCEPTUAL SITE MODEL

The preliminary source-pathway-receptor approach is now revisited to facilitate a hydrogeological conceptual model of the site. A cross-sectional profile of the site is presented in **Figure 5**.

9.1 Source Areas

- The raw leachate results from the landfill are within the maximum and minimum concentrations of typical landfill leachate in Ireland and are considered to represent a landfill in the methanogenic stage of decomposition of organic compounds. The leachate is considered to be relatively low strength and is broadly reducing in strength over time.
- No Hazardous substances as per the EPA Classification of Hazardous and Non-Hazardous substances in groundwater (2010) were detected in the leachate and groundwater at the site;

Non-Hazardous Substances detected include:

✓ Ammoniacal Nitrogen.

The entire landfill waste body has been capped with an engineered cap and wetland system as agreed with the EPA. Toe drains are lined with an engineered liner. Therefore the generation of leachate is primarily from the degradation of the waste body itself and the ingress of groundwater rather than the effect of rainfall ingress.

9.2 Pathways

- The hydrogeological regime across Churchtown Landfill comprises a leachate within the waste body and a groundwater body within the overburden/shallow bedrock. Both appear to be hydraulically connected. A separate groundwater body within the deeper bedrock and flowing under pressurised artesian conditions may also be present based on the conditions encountered within monitoring well BH4. Shallow groundwater interacts with the waste mass and facilitates the generation of leachate. The migration of the leachate is likely to flow within the overburden towards the River Finn. The head of leachate is dependant on the surrounding groundwater levels and the ability of the leachate to continue to migrate from the landfill depends on the permeability and thickness of the overburden and the head of leachate within the waste body. No clear reduction in leachate levels within the waste body is evident since completion of the landfill cap. This would suggest that leachate levels are highly dependent on groundwater level variations over time.
- The relatively low permeability overburden is anticipated to encourage the horizontal migration of shallow groundwater towards the River Finn
- Groundwater levels vary between 7.3 mOD (BH3) and -0.2 mOD (BH1) metres across the site with a groundwater gradient ranging between 0.002 and 0.019 which is considered to be low.
- Historical dispersal of leachate from the waste body to the surface water drains alongside of the waste body historically occurred at the site prior to remediation works. As the remediation of the site is not completed to-date, these surface waters will continue to record an impact from leachate in the short term. This pathway is expected to be removed on completion of the site remediation works.

9.3 Receptors

The key potential environmental receptor that could be impacted by the presence of the contaminant source on the site is the River Finn. The River is an SAC and a salmonid river. Given the observed depth and size of the river, it is considered to be a hydraulic boundary for the landfill *i.e.* all groundwater discharges to it rather than under it.

There are no source protection areas mapped in the vicinity of the Landfill and no private groundwater wells are present in the vicinity of the site.

9.4 Updated S-P-R – Risk Screening

The impact assessment is guided by the source-pathway-receptor (S-P-R) model. The S-P-R model is used to identify the sources of water and potential contaminants, the environmental assets affected by such, and the pathways by which water and contaminants reach those receptors. **Table 9.1** summarises an update to the preliminary SPR linkages identified in **Table 7.1** for the landfill.

Sources	Pathways	Receptors	Risk
	Horizontal Migration of Groundwater	ontal Migration of Groundwater River Finn	
Leachate	Vertical migration to		Low to Moderate
	groundwater	Groundwater	Low
	Horizontal migration to surface water	River Finn	Moderate ¹

Note 1: This linkage is based on leachate migration for an unremediated site. This linkage is not expected to be present following completion of the current remediation programme and is not considered further in this assessment.



9.5 Assessment of Current Groundwater Impacts & Extent of Plumes

Based on average values of Ammoniacal Nitrogen levels between 2009 and 2014 the rule of thumb of 100xGTV was not exceeded in any groundwater monitoring well. The highest level recorded was 2.63 mg/l in BH1 which is approximately 15 times the GTV. It is noted that no immediate downgradient monitoring wells currently exists between the landfill body and the River Finn. It is also unclear if BH1 is truly downgradient of the waste body or being impacted due to its proximity to the waste.

In accordance with the Water Framework Directive (WFD), the groundwater contaminant levels are unlikely to affect the status of the Raphoe GWB or likely to pose a risk to the objectives of the Water Framework Directive. The prevention of hazardous of substances entering the groundwater system is being maintained. Limiting the ingress of non-hazardous substances is being met by the mitigation measures that have been installed to date at the site and will be limited even further on completion of the reed bed/constructed wetland treatment system currently being installed at the site which will provide treatment of the leachate for the first time. It is anticipated that the new leachate treatment system will be fully operational in 2016.

The following points are noted:

- No groundwater users are located downgradient of the landfill site.
- The area of impact from the landfill leachate is considered to be minor relative to the groundwater body catchment area of the Raphoe GWB *i.e.* < 0.01%;
- Given the proximity to the landfill to the river, no significant plume, if any, is envisaged.
- The strength of the leachate is considered to be relatively low. Clear evidence exists that demonstrates the strength of leachate within the waste body is reducing over time.
- No groundwater monitoring well between the waste body and the River Finn exists and therefore the true contaminant groundwater flux to the river is unclear.
- The site in its present condition appears to be having a low impact on the quality of the River Finn with surface water discharges from the landfill site drains the dominant pathways for contaminant flux. No impact to the current WFD status of the river is anticipated. Additional monitoring is recommended to ascertain the impact occurring – in particular on completion of the current site restoration/remediation works.

10 REMEDIAL STRATEGY

Based on this hydrogeological assessment and the identified potential risk posed to the River Finn, the following actions are recommended to support the existing data set for the site, to confirm the level of risk posed and to identify possible mitigation solutions, if deemed necessary.

- It is noted that Churchtown Landfill is currently in the process of a new pilot remediation solution involving constructed wetlands and willow plantations. This programme of works is expected to significantly improve the current contaminant conditions presence at the site. Therefore the assessment undertaken within this report is based on previous and recent contaminant conditions and a reassessment of site conditions will be required following a period of 12 months post-completion of the works.
- Given the uncertainty surrounding the installation and location of monitoring well BH1, two wells are recommended along the southern boundary (*i.e.* between the waste body and the River Finn). In addition, a separate bedrock well is recommended in the vicinity of BH1 to ascertain impacts to deeper groundwater from the waste body. These additional wells will provide a more accurate understanding of true shallow groundwater contaminant fluxes from the waste body. Given the soft ground conditions present between the waste body and the river it is proposed to drill shallow boreholes/piezometers by hand-held window sampling techniques. Very soft ground conditions and access restrictions in this area will not facilitate the drilling of bedrock boreholes between the waste body and the River Finn.
- Hydraulic conductivity testing should be undertaken in all monitoring wells across the site to provide accurate understanding of contaminant fluxes to the river.

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- Flow monitoring within both boundary drains should be undertaken to facilitate assimilative capacity assessments of discharges to the River Finn post remediation works.
- All iron ochre staining in all surface water drains should be appropriately remediated by excavation following completion of all current remediation works for the site to minimise the remobilisation of contaminated sediments in the drains post remediation.
- The current water monitoring programme as per the current EPA licence requirements and limited additional monitoring as detailed in **Table 11.1** are recommended.
- Laboratory Limits of Detection for Total Phenols should be reduced to <0.05 μg/l.

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- Sampling protocol should be as per present and should include the filtration of samples for metal analysis.
- Due to uncertainty relating to quality of the laboratory results provided, in particular surrounding reported units of Iron, clarification from Donegal County Council laboratory is recommended going forward in relation to required detection and reporting limits and quality control.
- It is noted that SW5 is currently sampled from an adjacent site drain at the site. However, it is considered more beneficial if the sample location was collected from the River Finn within the mixing zone of the SW4 drain and the river.
- Appropriate surface water sampling locations and methodologies within the River Finn should be determined to ensure consistent monitoring results over time and to more accurately assess the impact of the landfill on the river.

11 COMPLIANCE MONITORING

Discharge activities subject to Tier 2 or Tier 3 assessments must undertake compliance monitoring to verify predicted impact and check compliance with terms of the authorisation. Compliance monitoring dictates that receptor-based water quality standards (or threshold values) should not be exceeded at receptor locations. For this reason sampling is conducted to monitor water quality at receptors, as appropriate.

11.1 Compliance Monitoring Locations

A compliance point is the point (location, depth) at which a compliance value should be met. Generally it is represented by a borehole or monitoring well from which representative groundwater samples can be obtained. In this case, the aim is to monitor groundwater before it enters the River Finn, downgradient of the site.

It is proposed that the existing groundwater monitoring programme be continued at the site until completion of the current remediation works at the site and the recording of at least 12 months of monitoring data post works completion. A reassessment of the proposed monitoring wells in **Section 10.0** can be reassessed on completion of an updated CSM at this stage. The existing downgradient monitoring wells are considered to be partially suitably to provide appropriate downgradient compliance monitoring locations.

11.2 Compliance Values

A compliance value is the concentration of a substance and associated compliance regime that, when not exceeded at the compliance point, will prevent pollution and/or achieve water quality objectives at the receptor. In this case, the aim is to protect surface water quality in the area.

The general chemical assessment test identifies groundwater bodies where widespread deterioration in quality has, or will, compromise strategic use of groundwater for existing or planned, human consumption and/or other potential purposes. Schedule 5 of the Groundwater Regulations (SI 9 of 2010) lists Threshold Values for selected parameters that are indicative of potential pollution events when exceeded. Where significant and sustained upward trends are identified, correcting action must be taken.

Based on the recorded groundwater quality data to-date at Churchtown Landfill, there are **no** sustained upward trends in groundwater contaminant export from the site. In addition, all parameters when detected above the GTV are significantly below the 100xGTV rule of thumb and confirm that the landfill is not affecting the WFD status of the groundwater body.

Given the existing relatively good groundwater quality both upgradient and downgradient of the landfill, it is proposed to assign compliance values based on a combination of the existing 2010 GTVs, EPA IGVs and 2 x standard deviation levels of the mean values since 2010. Exceedance of these compliance levels (see **Table 11.1**) warrants further assessment. Any exceedances should also be considered in conjunction with a trend analysis of the data to ascertain increasing levels over time. Levels below these compliance values in addition to downward or stable trends confirm that the impact or risk of the landfill on groundwater and surface waters is acceptable.

It is noted that there are on-going remediation works at the site. These works will further reduce the groundwater and surface water fluxes from the site and will further reduce the risk posed to the River Finn.

Sample ID	Current Monitoring Parameter	Current Monitoring Frequency	Proposed Monitoring Parameter	Proposed Monitoring Frequency
	Groundwater levels, Ammoniacal Nitrogen, Chloride, Dissolved Oxygen, Electrical Conductivity, pH, Temperature, Potassium, Sodium, TON, TOC, Nitrate, Nitrite, Phenols. Visual Inspection/Odour.	Quarterly	EC, pH, DO & Temp (field parameters) Groundwater levels, Ammoniacal Nitrogen, Chloride, Dissolved Oxygen, Electrical Conductivity, pH, Temperature, Potassium, Sodium, TON, TOC, Nitrate, Nitrite, Phenols. Visual Inspection/Odour.	Annually and then review post remediation and CSM review
Groundwater Monitoring BH1, BH3 & BH4 and 3 no. proposed additional monitoring wells	Boron, Cadmium, Calcium, Chromium, Copper, Cyanide, Fluoride, Iron, Lead, List I & II organic substances, Manganese, Magnesium, Mercury, Sulphate, Total Alkalinity, Total Phosphorous, Residue on evaporation, Zinc, Faecal Coliforms, Total Coliforms	Annually	Heavy Metals (<i>i.e.</i> Bo, Cd, Ca, Cr, Cu, Cn, F, Pb, Mg, Mn, Hg, Zn) Residue on Evaporation ORP, Sulphate, Manganese	Annually and then review post remediation and CSM review
	TPH VOCs/sVOCs	et off once-off	No change	Review post remediation and CSM review
	Groundwater levels, Ammoniacal Nitrogen, Chloride, Dissolved Oxygen, Electrical Conductivity, pH, Temperature, Potassium, Sodium, TON, TOC, Nitrate, Nitrite, Phenols. Visual Inspection/Odour.	Quarterly	Groundwater levels, Ammoniacal Nitrogen, Chloride, Dissolved Oxygen, Electrical Conductivity, pH, Temperature, Potassium, Sodium, TON, TOC, Nitrate, Nitrite, Phenols. Visual Inspection/Odour.	Quarterly continued and reducing to biannual post remediation and CSM review
Leachate Wells (L1, L2 & L3)	Boron, Cadmium, Calcium, Chromium, Copper, Cyanide, Fluoride, Iron, Lead, List I & II organic substances, Manganese, Magnesium, Mercury, Sulphate, Total Alkalinity, Total Phosphorous, Residue on evaporation, Zinc, Faecal Coliforms, Total Coliforms	Annually	Boron, Cadmium, Calcium, Chromium, Copper, Cyanide, Fluoride, Iron, Lead, List I & II organic substances, Manganese, Magnesium, Mercury, Sulphate, Total Alkalinity, Total Phosphorous, Residue on evaporation, Zinc, Faecal Coliforms, Total Coliforms	Annually
	TPH VOCs/sVOCs	Once-off	No change	Once-off post CSM review
Surface Water River Finn SW6, SW3 & SW7, Drains SW1, SW2, SW4 & SW5	Ammoniacal Nitrogen, BOD, Dissolved Oxygen, Electrical Conductivity, pH, Temperature, TSS, Chlorine, Copper, Nitrate, Nitrite, Phenols, Zinc	Quarterly	Ammoniacal Nitrogen, BOD, Dissolved Oxygen, Electrical Conductivity, pH, Temperature, TSS, Chlorine, Copper, Nitrate, Nitrite, Phenols, Zinc, Iron, Lead, ORP	Quarterly continued and reducing to biannual post remediation and

Sample ID	Current Monitoring Parameter	Current Monitoring Frequency	Proposed Monitoring Parameter	Proposed Monitoring Frequency
				CSM review
	COD, Chloride	Quarterly	No change	Quarterly continued and reducing to biannual post remediation and CSM review
	Cadmium, Calcium, Chromium, Iron, Lead, List I & II organics, Magnesium, Manganese, Mercury, Potassium, Sulphate, Sodium, Total Alkalinity, Total Phosphorous, TON.	Annually	No change	Annually



Monitoring Well	Parameter	Compliance Value	Source
	Lead	18.5 µg/l	2010 GTV
	Ammoniacal Nitrogen	BH3 (0.175 mg/l) BH4 (0.175 mg/l) BH1 (2.08 mg/l)	2010 GTV 2010 GTV 2 times Standard Deviation of the mean from 2009
	Electrical Conductivity	1000 µS/cm	EPA IGV
	Sulphate	187.5 mg/l	2010 GTV
	Iron	200 µg/l	2007 Drinking Water Regulations
	Manganese	50 µg/l	EPA IGV
All groundwater monitoring	Chloride	BH3 (46.8 mg/l) BH4 (36.5 mg/l) BH1 (50.35 mg/l)	2 times Standard Deviation of the mean
wens	Dissolved Oxygen, pH, Temperature, Fluoride, Total Alkalinity, Orthophosphate, Total Oxidised Nitrogen, Total Organic Carbon	- office	EPA IGVs, 2010 GTVs & 2007 Drinking Water Regulations
	Metals/Non-Metals (<i>i.e.</i> B, Cd, Ca, Cr, Cu, Hg, Pb, Mg, Ni, K, Na and Zn)	For inspection purpeduite	EPA IGVs, 2010 GTVs & 2007 Drinking Water Regulations
	Hazardous Substances (<i>i.e.</i> VOCs & SVOCs Total Hydrocarbons)		EPA IGVs, 2010 GTVs & 2007 Drinking Water Regulations
All Surface Water Monitoring Locations	As per existing licence requirements	-	2009 Surface Water Regulations & 2007 Drinking Water Regulations

Table 11 2	Proposed	Monitoring	Parameter	Thresholds
	TTOPOSCU	monitoring	ranameter	Theorem

12 SUMMARY & CONCLUSIONS & RECOMMENDATIONS

- A hydrogeological risk assessment of Churchtown Landfill Site was undertaken by BREL based on previous investigation reports and monitoring data between 2006 and 2015.
- Churchtown Landfill is a former solid waste facility where historically waste was landfilled into bunded cells which were excavated from the in-situ cohesive alluvial subsoils. The excavated soils were then used in bund construction. When landfilling ceased at Churchtown the final area of the waste body was approximately 5 hectares and waste body forms a plateau shape compared to the adjacent lands.
- The site is an unlined site historically operated on a dilute and disperses principal, whereby solid waste was tipped directly onto the underlying excavated surface with leachate allowed to percolate directly through the soils with no engineered liner installed. Landfilling began in 1987 and the site ceased operations on the 31st August 2000
- On the 19th May 2000 the Environmental Protection Agency granted the Council a Waste Licence (registration number WL62-1) for the orderly closure, capping and restoration of the landfill facility, in accordance with the Third Schedule of the Waste Management Act, 1996.
- The hydrogeological regime across the landfill comprises two groundwater bodies (*i.e.* one within the waste body and a separate groundwater body within the overburden/shallow bedrock) that are likely to be hydraulically connected. A third groundwater body within the bedrock and flowing under pressurised artesian conditions may also be present based on the conditions encountered within monitoring well BH4. Shallow groundwater interacts with the waste mass and facilitates the generation of leachates
- Groundwater level variability in the area significantly impacts on leachate levels within the waste body. The correlating increases and reductions in groundwater and leachate levels confirm this scenario with groundwater appearing to intersect the waste body. Groundwater level variations and levels upgradient of the site have a differing signature to groundwater levels closer to the River Finn. This suggests that the river is partially impacting on groundwater downgradient of the landfill.
- Following a review of the preliminary Conceptual Site Model for the site and all available water monitoring data, a revised Conceptual Site Model (CSM) was developed based on available information and monitoring data and identified a number of SPR linkages ranging from Low to Moderate risk to identified sensitive receptors *i.e.* the River Finn and the Raphoe GWB.
- The SPR linkage of concern relates to:
 - The vertical migration of leachate from the unlined waste cells to the underlying shallow groundwater aquifer which subsequently flows to the River Finn.
- The raw leachate results from the landfill are considered to represent a landfill in the methanogenic stage of decomposition of organic compounds. The leachate is considered to be relatively low strength and the levels, which are reducing over time, are expected to reduce further.
- Groundwater quality data does not indicate any upwards trends over time. This is expected to continue following completion of the current remedial measures. The only upward trend was recorded within BH1 with a single elevated level of Ammoniacal Nitrogen recorded in September 2013. On-going monitoring of this detected level in conjunction with a trend analysis on receipt of sufficient monitoring data over time is recommended.
- Both groundwater and surface water contaminant fluxes from the landfill have the potential to impact on the quality of the River Finn. However, available data suggests that groundwater contaminant fluxes to the river are having a negligible effect on the river downstream of the landfill. It is noted that a more representative downgradient monitoring well is required between the landfill and the river to provide a more accurate determination of this flux.

However, it also noted that site access to a suitably located downgradient monitoring may be restricted due to the proximity to the river and soft ground conditions. In relation to surface water discharges, available data suggests that surface water discharges to the river representative the predominant contaminant load to the river. The effects of this loading on the river are considered to be low with significant dilution capacity available within the river itself.

- Based on the water quality data, the landfill does not affect the current status of the River Finn and is in accordance with the WFD objectives.
- The rule of thumb of 100xGTV has not been exceeded in any groundwater monitoring well at the site. The highest Ammoniacal Nitrogen level recorded was 2.63 mg/l in BH1 (February 2009) which is approximately 15 times the GTV. In accordance with the Water Framework Directive (WFD), these levels are not likely to affect the status of the Raphoe GWB nor potentially pose a risk to the objectives of the Water Framework Directive. No groundwater contaminant plume has been identified to-date from the existing groundwater monitoring network.

The following points are noted:

- ✓ No groundwater users are located downgradient of the landfill site.
- The area of impact from the landfill leachate is considered to be minor relative to the groundwater body catchment area of the Raphoe GWB *i.e.* < 0.01%;
- Given the proximity to the landfill to the river, no significant plume, if any, is envisaged.
- The strength of the leachate is considered to be relatively low. Clear evidence exists that demonstrates the strength of leachate within the waste body is reducing over time.
- No groundwater monitoring well between the waste body and the River Finn exists and therefore the true contaminant groundwater flux to the river is unclear.
- ✓ The site in its present condition appears to be having a low impact on the quality of the River Finn with surface water discharges from the landfill site drains the dominant pathways for contaminant flux. No impact to the current WFD status of the river is anticipated. Additional monitoring is recommended to ascertain the impact occurring – in particular on completion of the current site restoration/remediation works.
- The site is compliant with the "prevent" or "limit" objective of the WFD and GWD. The prevention of hazardous of Substances entering the groundwater system is being met based on available chemical analysis. Limiting the ingress of non-hazardous substances is also being met by the mitigation measures that have been installed to date at the site *i.e.* landfill capping and lining of surface water drains and mitigations currently being installed *i.e.* active leachate treatment by willow plantations and constructed wetlands.
- Corrective actions undertaken to-date at the site includes:
 - ✓ A permanent landfill capping across the entire waste body;
 - ✓ The development of a willow bed plantation and constructed wetlands over the waste body to treat all leachate generated on site and disposal to the River Finn. This system is currently being developed at the site, and,
 - On-going groundwater and surface water monitoring as per the licence requirements.
- In summary, based on available site data, the risk posed by Churchtown Landfill on the River Finn is considered to be low in the immediate vicinity of the e landfill. The predominant contamination linkage to the river is via surface water drain discharges from the landfill site to the river. Monitoring data indicates that these discharges are having a low impact on the quality of the River Finn. In addition, it is anticipated that on completion of the current remedial measures being implemented at the site, this impact will reduce further over time.
- A series of additional recommendations to provide a more representative understanding of the contaminant fluxes to the River Finn have been provided in Section 10.0. It is noted that as the site is actively undergoing remediation works, it is proposed that these recommendations are considered <u>after at least</u> 12 months of groundwater level and water quality monitoring post full

completion of the works. A revised CSM will be undertaken at this stage and the proposed recommendations reassessed. In the meantime, the current monitoring programme is considered sufficient as an interim measure until completion of the remediation works.

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Respectfully submitted by

NielOMtele

Niall Mitchell *Hydrogeologist / Chartered Engineer* On behalf of Donegal County Council (Waste La Cence No. WL62-1)

On behalf of Donegal County Council (Waste Locence No. WL62-1)

FIGURES












APPENDIX A





Project Churchtown Landfill Site - Provision of M&E Services Tite Dawing Status Sheet Size Drawing Status Tender A1 1:1000 Project Leader Drawn By Date Drawing Status MB Der With Council Drawing Status Drawing Status Drawing Status MB Date Unitial R	NOTES 1. Verifing Dransfore. 2. Existing Services. 2. Existing Services. 3. Faue of Dravings. 4. Data Services. 4. Data Services. 4. Data Services. 5. Detailing Out access on adjust set of all calculars of adjust set of the topological over facility. Page 500 months of topological over commerces. (addition of second adjust for the topological over commerces.). (addition of adjust for the topological over facility. Page 500 months of topological over commerces. (addition of adjust for the topological over facility. Page 500 months of topological over facility.



e no less than 4m be placed to raise levels (depth n/e 0.8m) mm Cl804 stone	+8.67 +8.67 +8.67 +8.67 +8.67 +8.67 +8.67 +8.67 +8.67 to meet existing track levels PATH PATH PATH PATH PATH PATH PATH PATH	+026 +027 +026 +027 +026 +027 +027 +027 +027 +027 +027 +027 +027	+7.85 +7.85 +8.82 +8.82 +4.83 POND Track level raised by imported fill (gradient < 1in 10) +3.45 POND Track level raised by Track level raised to 9.2m OD +8.26	Track Raising		d into supports	CD/300/12 of ction Details) to d barbed wire hown (top of					d associated E105			ovided by	b and cover 3A. 1m + 0.5m ovided by ovided by	:d. Cover slab 1514/ME103A. 1ers 1514/ME103A. 1945 1947 1945 1947 1947 1947 1947 1947	E103A
IBR0514 /ME102AProject LeaderDrawn By AMB/DRDate Mar' 15Initial Review JD	Drawing Status Sheet Size Drawing Scale Preliminary A1 1:1000 Drawing Number Rev	Project Churchtown Landfill Site - Provision of M&E Services Title	RPS Consulting Engneers Enterprise Fund Business Centre Ballyraine Letterkenny T +353 (0) 74 91 61927 +353 (0) 74 91 61928 www.rpsgroup.com/ireland Client Client E Ballyraine www.rpsgroup.com/ireland	A change to work requirements DR March 15	Existing Access Track to be raised in level	Existing Leachate Toe Drain - Collecting Leachate from beneath landfill cap and discharging to Collection Sump	Existing 63mm Ø HDPE Outfall Pipe [To Collection Sump] Existing Lined French Drain (Runoff Drainage)	Existing 150mm Ø HDPE Outfall Pipe [To Drainage Ditch] with 100mm bedding and surround	32mm Ø PEM Lateral Irrigation Pipes Existing Infrastructure	Potential Additional Works. For Information Purposes	Proposed 4 Rail Timber Fence to RCD/300/1 Proposed Wicket Gate to RCD/300/12 Proposed Ditch Crossing	(Supply) Pipes [between 90mm HDPE main and LDC1B/2B and LDC1A/2A] 150mm Ø nb uPVC pipe [between ICW Break chamber and connected to inlets to ICWs] 90mm Ø HDPE PE100 Pumping Main	and Completeed 50mm Ø MDPE Header (Supply) Pipes 50mm Ø MDPE Header	5. Key: Required Works to be priced	used to aid the recipients drawing production, or setting out on site.4. Datum: Malin Head Ordnance Datum	3. Issue of Drawings. Hard copies, dwf and pdf will form a controlled issue of the drawing. All other formats (dwg, dxf etc.) are deemed to be an uncontrolled issue and any work carried out based on these files is at the recipients own risk. RPS will not accept any responsibility for any errors arising from the use of these files, either by human error by the recipient, listing of un-dimensioned measurements, compatibility issues with the recipient's software, and any errors arising when these files are	 Existing Services. Any information concerning the location of existing services indicated on this drawing is intended for general guidance only. It shall be the responsibility of the contractor to determine and verify the exact horizontal and vertical alignment of all cables, pipes, etc. (both underground and overhead) before work commences. 	NOTES Verifying Dimensions. The contractor shall verify dimensions against such other drawings or site conditions as pertain to this part of the work.

APPENDIX B













APPENDIX C



REPLACEMENT WELLS

Drumaboden & Churchtown Landfill Sites County Donegal

Consent of conviction purposes only any other use.

Report No: 05-135

Client: Donegal County Council

Engineer: RPS Kirk McClure Morton

May 2005

Drumaboden & Churchtown Landfill Sites County Donegal Replacement Wells

CON	TENTS Page N	0.
Note	on: Methods of describing soils and rocks	
1	AUTHORITY	. 1
2	DESCRIPTION OF FIELDWORK	. 1

- Appendix 1 Borehole Logs
- Appendix 2 References
- Consent for inspection purposes only any other use. Appendix 3 Geological Map of the Sites
- Appendix 4 Site Location Plans

Methods of describing soils and rocks

Soil descriptions are based on the guidance in Section 6 of BS 5930: 1999, *The Code of Practice for Site Investigation*, with the following exceptions:

- 1. Where the strength of clay is based on field assessment without the availability of laboratory or in-situ test results the following terms are used, where applicable:
 - soft to firm: clay with undrained shear strength close to the BS5930 boundary (40kPa) between soft and firm soil.
 - firm to stiff : clay with undrained shear strength close to the BS5930 boundary (75kPa) between firm and stiff soil.
- 2. The relative density of coarse-grained soils, described in trial pit logs, is based on field observations including stability of pit sides and the ease/difficulty of excavation. The description is for indicative purposes only: as required by BS 5930, the relative density should only be determined by use of insitu tests, including standard penetration tests.



Drumaboden & Churchtown Landfill Sites County Donegal Replacement Wells

1 AUTHORITY

On the instructions of the Engineer RPS Kirk McClure Morton, Glover Site Investigations Ltd were instructed to install four replacement gas monitor wells at Drumabodan and Churchtown Landfill Sites in County Donegal on behalf of the Client Donegal County Council.

2 DESCRIPTION OF FIELDWORK

Four boreholes were drilled by means of a Competitor 130 light percussion drilling rig on the 28th and 29th of April 2005. These boreholes were LG6 and LG8 in Drumaboden and LG8 and LG9 in Churchtown. In addition, a new gas valve and flush cover were installed in LG7 at Drumaboden.

Gas monitoring standpipes were installed in each well slotted from the bottom to 1.0 metre below ground level with a gravel pack.

The top metre was backfilled with a bentonite seal and a flush lockable cover was fitted.

A stainless steel plate was installed on the flush cover to aide in identification.

The other headworks were stripped back and the installation pipe-work checked prior to new gas valves and headworks being installed in LG7 at Drumboden.

The original wells which were removed showed no visible signs of damage but were replaced as it was reported that there were no inflows of gas into the well.

The new wells were replaced to best practice standards to maximise inflows of gas into the wells, but if the gas is not present in the strata then there will be no inflows into the replacement wells.



Glo	over Sit	e Ir	ive	stigatio	ons	Ltd	Site Drumaboden Landfill Site, County Donegal	Borehole Number LG6
Boring Method Competitor 130		Casing 15	Diameter Omm cas	r ed to 6.00m	Ground	Level (mOD)	Client Donegal County Council	Job Number 05-135
	Locatio As	n Plan		Dates 2	8/04/05	Engineer RPS KMM	Sheet 1/1	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend at
Remarks Installed 38		m. Grave	pack 6.0	Water strike(1) at 3.80m. 28/04/05	have the contract of the contr	0.10 0.00 0.10 0.10 0.00	TOPSOL MADE GROUND: Soft to firm light grey brown gravelly sar CLAY (FILL) Spongy dark brown pseudofibrous PEAT Uncompact grey organic fine sandy SILT Complete at 6.00m	
Installed 38r	mm standpipe to 6.00	m. Grave	l pack 6.0	00m - 1.00m, bentonite	e 1.00m - I	0.00m. Flush l	ockable cover fitted (appr	ox) By
							1:5 Fiau) DC/CM ire No.
								05-135.LG6

ing Method Appetitor 130 Pepth (m) Sample / Tests (1 1 1 1 1 1 1 1 1 1 1 1 1	Casing Diamete 150mm ca As Plan Casing Depth (m) Vater Depth (m)	Field Records	Ground Dates 28 Level (mOD)	Level (mOD)	Client Donegal County Council Engineer RPS KMM Description TOPSOIL MADE GROUND: Soft grey brown gravelly sandy CLAY (FILL) Spongy dark brown pseudofibrous PEAT	Job Numbr 05-12 Sheet 1/1 Legend
Pepth (m) Sample / Tests (Location As Plan Casing Depth (m)	Field Records	Dates 28 (mOD)	Depth (m) (Thickness) 	Engineer RPS KMM Description TOPSOIL MADE GROUND: Soft grey brown gravelly sandy CLAY (FILL) Spongy dark brown pseudofibrous PEAT	Sheet 1/1 Legend
Depth (m) Sample / Tests (Casing Water Depth Depth (m)	Field Records	(mOD)	Depth (m) (Thickness) 	Description TOPSOIL MADE GROUND: Soft grey brown gravelly sandy CLAY (FILL) Spongy dark brown pseudofibrous PEAT	Legend
				0.10 0.70)	TOPSOIL MADE GROUND: Soft grey brown gravelly sandy CLAY (FILL) Spongy dark brown pseudofibrous PEAT	Alke shie
		Water strike(1) at 3.00m.	S INSPECTION		Uncompact grey organic fine sandy SILT Complete at 6.00m	ske ske ske ske ske ske ske ske ske ske
emarks talled 38mm standpipe to 6.00m	m. Slotted from	5.00m - 1.00m with gra	vel pack, b	pentonite seal	1.00m - 0.00m. Flush lockable cover fitted	
					Fig	ure No.

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Glo	over Sit	e ir	ive	stigatio	ons	Ltd	Site Churchtown Landfill Site, Lifford, County Donegal	Bo Nu L	rehole mber .G8
Boring Meth Competitor 1	Casing	Diamete Omm cas	ed to 6.00m	Ground	Level (mOD)	Client Donegal County Council	Jot Nu 05	5 mber 5-135a	
	Locatio As	n Plan		Dates 29	9/04/05	Engineer RPS KMM	She	eet 1/1	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Lege	and take
Remarks				Water strike(1) at 2.70m. 29/04/05 menual 29/04/05 menual	insection oprietto		TOPSOIL Firm brown slightly gravelly sandy CLAY Medium dense light brown gravelly fine to coarse SAND Soft grey (damp) sandy gravelly CLAY Soft grey (damp) sandy gravelly CLAY Complete at 6.00m		
1emarks 1stalled 38mr	m standpipe to 6.00m	n. Gravel	pack 6.00	Dm - 1.00m, bentonite	seal 1.00r	n - 0.00m. Flu	sh lockable cover fitted Scc (appl	ale Log ox) By	ged CM
							Figu	ure No.	

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Glo	ver Sit	e Ir	ive	stigatio	ons	Ltd	Site Churchtown Landfill Site, Lifford, County Donegal	Bore Num	ehole iber 39
Boring Methor Competitor 13	Casing 15	Diameter 0mm cas	r ed to 6.00m	Ground	I Level (mOD)	Client Donegal County Council	Job Num 05-	i ber 135a	
	Locatio As	n Plan		Dates 29	9/04/05	Engineer RPS KMM		e t /1	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Leger	р Water
		(m)	(m)	Water strike(1) at 2.70m. 29/04/05 onsent of c	in-section priteito	(Thickness)	TOPSOIL Medium dense light brown gravelly fine to coarse SAND Soft grey (damp) sandy gravelly CLAY Soft grey (damp) sandy gravelly CLAY Complete at 6.00m		v m ∑1
Remarks nstalled 38mm	n standpipe to 6.00m	n. Gravel	pack 6.00)m - 1.00m, bentonite	seal 1.00r	m - 0.00m with	flush cover (appro	⇒ Logg	ed
							1:50 Figur 05	DC/C e No. 5-135a.LG9	СМ)

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REFERENCES

DRUMABODEN

- 1. The Code of Practice for Site Investigation Fieldwork BS : 5930 : 1999
- 2. The Code of Practice for Site Investigation Laboratory Testing BS 1377 Parts 1 to 9 : 1990
- Ordnance Survey of Ireland Discovery Series Sheet No. 6
- 4. Geological Map of the Site Sheet No. 1 & 2

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REFERENCES

CHURCHTOWN

- 1. The Code of Practice for Site Investigation Fieldwork BS : 5930 : 1999
- 2. The Code of Practice for Site Investigation Laboratory Testing BS 1377 Parts 1 to 9 : 1990
- Ordnance Survey of Ireland Discovery Series Sheet No. 12
- 4. Geological Map of the Site Sheet No. 3&4

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SITE RESTORATION CONTRACT

CHURCHTOWN LANDFILL

LIFFORD

COUNTY DONEGAL Consent of Consent of

Issued: February 2015

CONTENTS

INTRODUCTION	. 3
Terms of Reference	.3
Method	.3
SITE DESCRIPTION	. 5
GROUND CONDITIONS	. 6
Geology	.6
Ground Conditions	.6
Groundwater	.6
	INTRODUCTION Terms of Reference Method SITE DESCRIPTION GROUND CONDITIONS Geology Ground Conditions Groundwater

FIGURES

APPENDIX A: BOREHOLE LOGS AND INSTRUMENTATION DETAILS APPENDIX B: GAS AND GROUNDWATER LEVEL MONITORING RESULTS the the APPENDIX C: GEOTECHNICAL LABORATORY TEST RESULTS ON THE APPENDIX C: GEOTECHNICAL LABORATORY TEST RESULTS For instrument of the test of test of the test of test of

1.0 INTRODUCTION

1.1 **Terms of Reference**

Ground Check Ltd was commissioned by TAL Civil Engineering Ltd, acting on behalf of Donegal County Council, to undertake a ground investigation for a site restoration contract at Churchtown Landfill, Lifford, County Donegal. The location of the site is shown by Figure 1.

1.2 Method

The ground investigation was undertaken in accordance with the guidelines set-out in BS5930:1999 + A2 2010, Code of practice for site investigations and UK Specification for Ground Investigation, 2nd edition (2011), BS EN 1997-2 (2007) and BS EN ISO 22475-1 (2006) and related standards and the scope of works comprised of the following elements.

Exploratory Holes

The locations of exploratory holes are shown by Figure 2 and logs are included in Appendix A.

- Shell and Auger Boreholes: Eleven boreholes were sunk using a Dando 2000 shell and auger drilling rig and were advanced using 200mm diameter casing and tools.
- Rotary Percussive Drilling: One borehole (BH4) was sunk using a commachio MC305 rig equipped with Symmetrix casing and tools and air flush.
- Rotary Core Drilling: Intact core specimens of rock were fecovered in one borehole (BH4) using a T2 86 owner required core barrel with double liner.

Sampling & In-situ Testing

- Disturbed samples: comprising sealed plastic bags of soil were recovered at intervals shown on the borehole logs, generally being taken at one metre depth increments and from each stratum.
- Bulk samples: comprising soil sealed in heavy gauge plastic sacks were recovered at intervals shown on the logs.
- Groundwater Samples: were recovered where possible during drilling or from borehole standpipes after purging and were contained in one litre plastic bottles.
- Standard Penetration Tests (SPT): were undertaken at intervals shown on the borehole logs and were conducted in accordance with BS1377:1990 Code of Practice: Methods of Test for Soils for Engineering Purposes - Part 9 In-Situ Tests.
- Variable Head Permeability Test: was scheduled to be undertaken in the completed standpipe installation of BH04. Due to the strong artesian flow recorded in BH04 a rising head test could not be performed.

3

Instrumentation & Monitoring

- Standpipe Installations: selected boreholes were installed with a 50mm HDPE slotted standpipe and gravel pack on completion of drilling and the depth and length of the response zone were scheduled by the Engineer. Construction details of the standpipe installations and headworks are given on the relevant borehole logs which are presented in Appendix A.
- Gas and Groundwater Monitoring: One monitoring visits were undertaken by a geotechnician and were performed in accordance with CIRIA C665 guidance using a GFM-430 gas meter fitted with an internal flow pod. Water levels were measured using an electronic dip-meter. Monitoring results are presented in Appendix C.

Geotechnical Laboratory Testing

Selected soil, groundwater and rock core samples were scheduled for the following laboratory tests which were conducted in accordance with procedures outlined in BS1377. Results are included in Appendix D.

Particle Size Distribution

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SITE DESCRIPTION 2.0

The site is situated on the southern side of the N15 road about 4kms south west of Lifford, County Donegal. The ground surface is generally flat but is elevated above the adjacent ground near its western and southern boundaries. The River Finn is located directly to the south of the landfill site. The existing layout of the site is shown by Figure 2 and Plate 1 provides an aerial overview.

Plate 1: Overview of Site



3.0 **GROUND CONDITIONS**

3.1 Geology

The geological maps of the area indicate that the site is underlain by the following strata.

- **Recent Deposits**
- **Glacial Deposits**
- Bedrock [DALRADIAN]

3.2 **Ground Conditions**

The findings of the ground investigation are listed in Table 1 and are summarised below.

- Made Ground: The site is mantled by made ground at all locations investigated except for Borehole 4. The made ground is composed generally of soft and soft to firm, brown and grey, gravelly, sandy, silty clay with low cobble and boulder content, containing domestic refuse, glass, textile, rubber and wire.
- Recent: Occurs immediately beneath the made ground at Boreholes LG8 and LG9 from respective depths of 2.30 and 2.40m and from ground level at Borehole 4. It is described generally as Loose, brown, very silty, fine to coarse sand, very soft, dark brown, slightly sandy, clayey peat, and soft, becoming soft to firm, grey, sandy, silty clay.
- Glacial Deposits: Were encountered only in Borehole 4 beneath the geologically recent deposits at 6.20m depth and are composed generally of loose, brownish grey, silty, sandy, fine to coarse gravel PUIDOS with low cobble content.
- Bedrock: Grey, highly fractured, weathered SCHET was encountered at 7.90m depth in Borehole 4 and Forthat continued to its terminal depth.. of copyrige

3.3 Groundwater

Slight flows of groundwater whtered Boreholes LA1, LG2AR, LG3A, LG7A and 4 at depths ranging between 3.80 and 7.00m below ground level. A strong sub-artesian flow entered Borehole 4 at 9.00m depth, rising to 0.50m after twenty minutes. Groundwater was not observed in the other boreholes. It should be noted, however, that such short term observations may not represent the presence or absence of a water table and that groundwater conditions can vary.

Table 1: Ground Conditions Summary

	Completion	S	Bedrock		
Reference	Depth (m)	Made Ground	Recent	Glacial	Top (m)
L1A	6.50	>6.50	-	-	-
LG1A	5.30	>5.30	-	-	-
L2	6.50	>6.50	-	-	-
LG2A	2.80	>2.80	-	-	-
LG2AR	5.00	.5.00	-	-	-
LG3A	4.50	>4.50	-	-	-
LG5A	4.30	>4.30	-	-	-
LG6A	4.20	>4.20	- net use.	-	-
LG7A	4.50	>4.50	only any other	-	-
LG8	7.20	2.30 pupose	>7.20	-	-
LG9	7.00	5 11-30-41.00 net	>7.00	-	-
4	12.00	Stool Let -	6.20	7.90	7.90
	Conser				








APPENDIX A: BOREHOLE LOGS AND INSTRUMENTATION DETAILS

	round Ch	eck	Ltd				Site Restoration Contract, Churchtown Landfill, Lifford, County Donegal			oreh umb 4	ole er
Boring Meth Rotary Percu	nod ussive	Casing 20	Diameter 0mm cas	r ed to 12.00m	Ground	Level (mOD)	Client Donegal County Council		Jo Nו 14	b 1 mb 1-11	er 70
		Locatio	n		Dates 09 10	0/12/2014- 0/12/2014	Engineer TAL Civil Engineering Ltd		Sh	1/2	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Ins	str
1.00-1.45	SPT N=2			1,0/1,0,0,1			Soft, brownish grey, sandy, silty CLAY. Sand is fine to medium. [RECENT]		▼ 2	• • • • • • •	• • • • • • • • •
1.00 2.00-2.45 2.00	D1 SPT N=3 D2			1,0/1,0,1,1							
3.00-3.45 3.00	SPT N=3 D3			1,1/1,0,1,1			Soft, becoming soft to firm, grey, sandy, silty CLAY. Sand is fine to coarse. [RECENT]	× · · · · · · · · · · · · · · · · · · ·			
4.00-4.45 4.00	SPT N=4 D4			1,1/1,1,1,1	citio	autroses ed fr	A BUY OU	×× ×× ××			
5.00-5.45 5.00	SPT N=5 D5			1,1/2,1,1,1 FO	Inspector Pyright of						
6.00	D6			Cor		6.20		× × · · · · · · · · · · · · · · · · · ·			
6.50-6.95	SPT N=7			1,1/2,2,1,2			Loose, brownish grey, silty, sandy, fine to coarse, subrounded GRAVEL with low cobble content. [GLACIAL]	×0.0×0 0.0×0 ×0.×			
7.00	D7			Slight flow(1) at 7.00m, no rise after 20 mins.		(1.70)			▼ 1		
8.00	D8					7.90	Grey, highly fractured, weathered SCHIST. [DALRADIAN]				
9.00	D9			Strong Flow(2) at 9.00m, rose to 0.50m in 20 mins.					∇ 2	2000 00000 00000000 10000000	1000 0000 1000 000 1000 000
Remarks						(4.10)		 		ي موقد موقد	d
								(approx) 1:50	Ву	LK	
								Figure N 14-11	o. 70.E	SH4	

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	Fround Ch	eck I	Ltd			Site Site Restoration Contract, Churchtown Landfill, Lifford, County Donegal		BN	orehole umber 4	
Boring Meth Rotary Percu	nod ussive	Casing 20	Diameter Omm case	ed to 12.00m	Ground	Level (mOD)	Client Donegal County Council		J N 1	ob umber 4-1170
		Locatio	n		Dates	9/12/2014- 0/12/2014	Engineer TAL Civil Engineering Ltd		S	heet 2/2
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
10.00	D10									
11.00	D11					(4.10)				
12.00	D12			consent of c	onsection opyright o	12.00	Complete at 12.00m			
Remarks Terminated a	at required depth. St	andpipe in:	stalled to	11.50m.				Scale (approx)	LB	ogged y
								1:50		LK
								rigure N 14-1	vo. 170.	BH4

G	round Ch	eck l	Ltd			Site Restoration Contract, Churchtown Landfill, Lifford, County Donegal			orehole umber L1A	
Boring Meth Cable percus	od ssive to 6.5m.	Casing 20	Diamete 0mm cas	r ed to 6.50m	Ground	Level (mOD)	Client Donegal County Council		Jo N 1	ob umber 4-1170
		Locatio 23	n 1020.6 E	395902.6 N	Dates 07 10	7/11/2014-)/11/2014	Engineer TAL Civil Engineering Ltd		S	h eet 1/1
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
1.00 1.20-1.65 2.00-2.45 3.00-3.45 4.00-4.45 5.00-5.45 6.00-6.45	B1 D1 SPT N=8 SPT N=16 SPT N=28 SPT N=13 SPT N=15 SPT N=50	(m)	(m)	2,2/1,2,2,3 1,6/3,4,2,7 07/11/2014:DRY 10/11/2014: 2,3/7,2,3,1 slight (1) at 4.80m. 3,6/8,3,2,2 FO CONSEND OF 3,2/2,6,42 10/11/2014:4.80m	unsection printen of	(1.20)	Soft to firm, light brownish grey, slightly gravelly, slightly sandy, clayey SILT. Sand is fine to coarse. Gravel is subangular to subrounded, fine to coarse. [MADE GROUND] Soft, dark brown, gravelly, slightly sandy, slity CLAY. Sand is fine to coarse. [MADE GROUND] Soft to firm, dark brown, slightly sandy, clayey SILT with plastic bags, plastic and glass bottles. Sand is fine to coarse. [MADE GROUND] Kang dure for the coarse. [MADE GROUND] Complete at 6.50m		Σ1	
Remarks Terminated a Chiselling fro	t required depth. Sta m 3.10m to 3.20m fo	andpipe in or 1.5 hou	stalled to rs. Chisel	6.5m. ling from 3.20m to 3.4	 40m for 1.	F0 hour.	-	Scale (approx) 1:50 Figure N 14-1	Lo B lo.	bigged y LK L1A

C.	round Ch	eck I	Ltd			Site Site Restoration Contract, Churchtown Landfill, Lifford, County Donegal		Borehol Number L2		ole er	
Boring Meth Cable percus	od ssive to 6.5m.	Casing 20	Diamete 0mm cas	r ed to 6.50m	Ground	Level (mOD)	Client Donegal County Council		J(N 1	ob umb 4-11	er 70
		Locatio	n		Dates 10)/11/2014	Engineer TAL Civil Engineering Ltd		S	heet 1/1	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Ins	str
1.00 1.20-1.65 2.00-2.45 3.00-3.45 4.00-4.45 5.00-5.45	B1 D1 SPT N=10 SPT N=7 SPT N=20 SPT N=21 SPT N=23			2,3/2,2,3,3 3,3/2,3,1,1 3,4/6,7,4,3 4,5/7,3,4,7 5,6/6,7,5,5 Consent of C 05/11/2014:DRY	NSPECTION Dyright of	(0.50) (0.50)	TOPSOIL. [MADE GROUND] Soft to firm, light brown, slightly gravelly, slightly sandy, slity CLAV with low cobble content. Sand is fine to coarse. Gravel is angular to subrounded, fire to coarse. [MADE GROUND] Soft to firm, dark brown, slightly sandy SILT with plastic and food packaging. Sand is fine to coarse. [MADE GROUND] MADE GROUND] Complete at 6.50m				
Remarks Terminated a	t required depth. Sta	andpipe in	stalled to	6.5m.			-	Scale (approx) 1:50 Figure N 14-1	lo.	bgge y LK .R1	d

G	round Ch	eck l	Ltd			Site Site Restoration Contract, Churchtown Landfill, Lifford, County Donegal			orehole umber .G1A	9	
Boring Meth Cable percus	nod ssive to 5.3m.	Casing 20	Diamete 0mm cas	r ed to 5.30m	Ground	Level (mOD)	Client Donegal County Council		Jo N 1	ob umber 4-1170	_
		Locatio 23	n 0864.3 E	396073.7 N	Dates 05	5/11/2014	Engineer TAL Civil Engineering Ltd		S	n eet 1/1	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr	_
0.50 1.00 1.20-1.65 2.00-2.45 2.00 2.10 2.60 3.00-3.45 4.00-4.45 5.00-5.45	D1 B1 D2 SPT N=10 SPT N=7 D3 SPT N=20 SPT N=21 SPT N=23			2,3/2,2,3,3 3,3/2,3,1,1 3,4/6,7,4,3 4,5/7,3,4,7 5,6/6,7,5,5 05/11/2014:DRY Conserved	ansection pyrinetic	(Inickless) (Inickless) (0.50) (0.90) (0.90) (0.40) (0.30) (0.30) (0.30) (0.30) (0.30) (0.30) (0.30) (0.30) (0.30) (0.40) (0.30) (0.4	TOPSOIL. [MADE GROUND] Soft to firm, brown, slightly gravelly, slightly sandy, silty CLAY with low cobble and boulder content. Sand is fine to coarse. Gravel is angular to subrounded, fine to coarse. [MADE GROUND] Soft, greyish brown, slightly gravelly, slightly sandy, silty CLAY. Sand is fine to coarse. [MADE GROUND] Soft, brown, slightly gravelly, slightly sandy, silty CLAY with plastic. Sand is fine to coarse. [MADE GROUND] Soft, dark brown, peaty silty CLAY with glass and plastic. Sand is fine to coarse. [MADE GROUND] Soft to firm, dark brown, slightly sandy, clayey SILT with plaste and wire. Sand is fine to coarse. [MADE GROUND] MADE CROUND] Complete at 5.30m				
Remarks Terminated a Chiselling fro	t required depth. Sta m 0.50m to 0.80m fo	andpipe in or 1.5 hou	stalled to rs. Chise	5.3m. lling from 4.00m to 5.3	1 30m for 1.	5 hours.	1	Scale (approx) 1:50 Figure N 14-11	Io.	Jgged y LK G1A	_

	round Ch	eck I	Ltd			Site Site Restoration Contract, Churchtown Landfill, Lif County Donegal	ford,	Borehole Number LG2A		
Boring Meth	nod ssive to 2.8m.	Casing 20	Diamete Omm cas	r ed to 2.80m	Ground	Level (mOD)	Client Donegal County Council		Job Number 14-1170	
		Locatio 23	n 0946.7 E	395975.7 N	Dates 06	5/11/2014	Engineer TAL Civil Engineering Ltd		Sheet 1/1	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description		Kater Vater	
1.00-1.45 1.00 1.00	SPT N=8 B1 D1			2,2/1,2,2,3		(1.40)	Soft to firm, light brown, slightly gravelly, slightly sa CLAY with low cobble and boulder content. Sand is coarse. Gravel is subangular to subrounded, fine to [MADE GROUND] Soft, dark brown, slightly sandy, SILT with plastic a bags, food packaging and other domestic waste. [I	and paper MADE		
2.00-2.45	SPT N=5			1,2/1,1,2,1		(1.40)	GROUNDJ			
	06/11/2014:DF			06/11/2014:DRY		2.80				
				Consent of C	Insection Synthesis		Terminated at 2.80m			
Remarks Terminated u Chiselling fro	ipon virtual refusal. E om 2.75m to 2.80m fe	Backfilled or 2.0 hou	with bento	onite. Re-setup and re	e-bored as	LG2AR		Scale (approx)	Logged By	
								1:50	LK	
								Figure No. 14-1170.LG2A		

G	round Ch	eck l	Ltd			Site Site Restoration Contract, Churchtown Landfill, Lifford, County Donegal		B N L	orehole umber G2AR	
Boring Meth Cable percus	od sive to 5.0m.	Casing 20	Diamete 0mm cas	r ed to 5.00m	Ground	Level (mOD)	Client Donegal County Council		J N 1	ob umber 4-1170
		Locatio	n 0946.7 E	395975.7 N	Dates 06	6/11/2014	Engineer TAL Civil Engineering Ltd		S	heet 1/1
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
1.00-1.45 1.00 2.00-2.45 3.00-3.45 4.00-4.45 5.00-5.45	SPT N=10 B1 D1 SPT N=7 SPT N=4 SPT N=18 SPT N=14			2,3/3,2,2,3 2,1/2,2,1,2 1,0/0,1,2,1 1,2/7,3,6,2 slight(1) at 4.50m. 06/11/2014:4.50m0 1,0/2,8,1,3 Conserved	usection Pyrietto	(1.40)	Soft to firm, light brown, slightly gravelly, slightly sandy, slity CLAY with low cobble content. Sand is fine to coarse. Gravel is subangular to subrounded fine to coarse. [MADE GROUND] Soft, dark brown, slightly sandy SILT with plastic, cloth, textiles, and rubber. Sand is fine to coarse. [MADE GROUND] Complete at 5.00m		∑1	
Remarks Terminated a Chiselling fro	t required depth. Sta m 3.50m to 4.00m fo	andpipe in or 1.0 hou	stalled to r.	5.0m.				Scale (approx) 1:50 Figure N 14-117	L B No. 70.L0	ogged y LK G2AR

C C	round Ch	eck I	Ltd			Site Site Restoration Contract, Churchtown Landfill, Lifford, County Donegal		B N L	oreh umb .G3	ole er A	
Boring Meth Cable percus	nod ssive to 4.5m.	Casing 20	Diameter Omm case	r ed to 4.50m	Ground	Level (mOD)	Client Donegal County Council		J N 1	ob umb 4-11	• er 70
		Locatio	n 0980.4 E	395939.7 N	Dates 07	7/11/2014	Engineer TAL Civil Engineering Ltd		s	heet 1/1	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Ins	str
1.00-1.45 1.00 2.00-2.45 2.00 3.00-3.45 4.00-4.45	SPT N=10 B1 D1 SPT N=8 B2 D2 SPT N=13 SPT N=15			2,2/3,2,3,2 2,3/1,3,2,2 1,0/3,7,2,1 slight(1) at 4.00m. 6,1/4,5,2,4 06/11/2014:4.00m Consent of C	inspection by the bit of	(0.50) 0.50 (1.80) (2.20) (2.20)	Dark brown TOPSOIL. [MADE GROUND] Soft to firm, light brown, slightly gravelly, slightly sandy, slity CLAY with low coble content. Sand is fine to coarse. Gravel is subangular to subrounded fine to coarse. [MADE GROUND] Soft, dark brown, slightly sandy, SILT with plastic, cloth, concrete blocks and wire. Sand is fine to coarse. [MADE GROUND] Coarse. [MADE GROUND] Complete at 4.50m		√21		
Remarks Terminated a Chiselling fro	it required depth. Sta m 2.90m to 4.00m fc	indpipe in or 1.5 hou	stalled to rs.	4.5m.				Scale (approx) 1:50 Figure M 14-11	No.	LK G3A	.d

G	round Ch	eck I	Ltd			Site Site Restoration Contract, Churchtown Landfill, Lifford, County Donegal (mOD) Client		B N L	Boreho Number LG5A		
Boring Meth Cable percus	nod ssive to 4.3m.	Casing 20	Diamete Omm cas	ed to	Ground	Level (mOD)	Client Donegal County Council		Jo N 1	ob umb 4-11	er 70
		Locatio	n 0913.4 E	396112 N	Dates 11	/11/2014	Engineer TAL Civil Engineering Ltd		S	heet 1/1	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Ins	str
1.00-1.45 1.00 2.00-2.45 3.00-3.45 4.00-4.45	SPT N=13 D1 SPT N=3 SPT N=8 SPT N=6			2,2/3,3,4,3 2,2/1,1,1,0 0,0/1,2,3,2 1,1/1,2,1,2 11/11/2014:DRY	ange cho		TOPSOIL. [MADE GROUND] Soft to firm, light brown, slightly gravelly, slightly sandy, slity CLAY with low cobble and boulder content. Sand is fine to coarse. Gravel is subangular to subrounded, fine to coarse. [MADE GROUND] Soft, dark brown, slightly sandy, clayey SILT with plastic bags, food packaging and other domestic waste. [MADE GROUND]				
Remarks Terminated a Chiselling fro	nt required depth. Sta om 3.00m to 4.20m fo	anpipe instored in the second	talled to 4 r.	.0m.	1		1	Scale (approx) 1:50 Figure N 14-11	Io. 70.L	bgge y LK .G5A	ed

G	round Ch	eck I	Ltd			Site Site Restoration Contract, Churchtown Landfill, Lifford, County Donegal		B N L	Borehol Number LG6A		
Boring Meth Cable percus	od ssive to 4.3m.	Casing 20	Diameter Omm case	ed to 4.30m	Ground	Level (mOD)	Client Donegal County Council		Jo N 1	ob umb 4-11	er 70
		Locatio	n 0984.3 E	396020.2 N	Dates 11	/11/2014	Engineer TAL Civil Engineering Ltd		s	heet 1/1	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Ins	str
1.00-1.45 1.00 2.00-2.45 3.00-3.45 4.00-4.45	SPT N=11 D1 SPT N=9 SPT N=7 SPT N=7			2,2/3,3,2,3 2,3/3,2,3,1 1,2/2,2,1,2 1,1/2,2,2,1 06/11/2014:	Insection Printletto		Dark brown TOPSOIL. [MADE GROUND] Soft to firm, light brown, slightly gravelly, slightly sandy, slity CLAY with low cobble content. Sand is fire to coarse. Gravel is subangular to subrounded fine to coarse. [MADE GROUND] Soft, dark brown, slightly sandy, SILT with plastic and waste packaging. Sand is fine to coarse. [MADE GROUND] Complete at 4.30m				
Remarks Terminated a Chiselling fro	t required depth. Sta m 3.00m to 3.80m fo	andpipe in: or 1.0 hou	stalled to	4.2m.		<u>F</u>		Scale (approx) 1:50 Figure N 14-11	Li B No. 70.L	ogge y LK G6A	d

G	round Ch	eck I	Ltd			Site Site Restoration Contract, Churchtown Landfill, Lif County Donegal	iord,	B N L	orehole umber .G7A	
Boring Meth Cable percus	nod ssive to 4.5m.	Casing 20	Diamete 0mm cas	r ed to 4.50m	Ground	Level (mOD)	Client Donegal County Council		Jo N 1	ob umber 4-1170
		Locatio 23	n 1146 E 39	95850.8 N	Dates 10	0/11/2014	Engineer TAL Civil Engineering Ltd		S	n eet 1/1
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
1.00 1.20-1.65 2.00-2.45 3.00-3.45 4.00-4.45	B1 D1 SPT N=10 SPT N=15 SPT N=6 SPT N=28			2,2/3,2,2,3 6,3/4,1,7,3 2,3/1,1,3,1 slight(1) at 3.80m. 3,7/8,6,5,9 11/11/2014:3.80m	nsection pyrietto		TOPSOIL. [MADE GROUND] Soft to firm, light brown, slightly gravelly, slightly sandy, sity CLAY with low cobble content. Sand is fine to coarse. Gravel is angular to subrounded, fine to coarse. [MADE GROUND] Soft, dark brown, slightly sandy, SILT with plastic and food packaging. Sand is fine to coarse. [MADE GROUND] Complete at 4.50m		∑1	
Remarks Terminated a Chiselling fro	nt required depth. Sta om 3.00m to 4.20m fo	ndpipe in or 1.5 hou	stalled to rs.	4.5m.	1	<u> </u>		Scale (approx) 1:50 Figure N 14-11	Le B Jo. 70.L	bgged y LK G7A

	round Ch	leck	Ltd			Site Site Restoration Contract, Churchtown Landfill, Lifford, County Donegal		Boreho Numbe		ole er 8	
Boring Meth	nod ssive to 7.2m.	Casing 20	Diamete 0mm cas	r ed to 7.20m	Ground	Level (mOD)	Client Donegal County Council		J(N 1	ob umb 4-11 ⁻	er 70
		Locatio 23	n 0912.4 E	396177.3 N	Dates 12	2/11/2014	Engineer TAL Civil Engineering Ltd		S	heet 1/1	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Ins	str
1.00-1.45 1.00 1.00 2.00-2.45 2.00 3.00-3.45 4.00-4.45 5.00-5.45 6.50-6.95	SPT N=11 D1 SPT N=11 D2 SPT N=7 SPT N=5 SPT N=4 SPT N=3			2,2/3,3,3,2 2,3/3,2,3,3 1,2/2,2,1,2 1,2/2,2,1,2 1,2/2,1,1,1 1,1/1,1,1,1 1,1/0,1,1,1 12/11/2014:DRY	a mage cito	(1.70)	Soft to firm, light brownish grey, slightly gravelly, slightly sandy, slity CLAY. Sand is fine to coarse. Gravel is subangular to subrounded, fine to coarse. [MADE GROUND] Soft to firm, light brown, slightly sandy, slity CLAY. Sand is fine to coarse. [MADE GROUND] Loose, orange brown, very slity, fine to coarse SAND. [RECENT] Soft, light greyish brown, sandy, clayey SILT. Sand is fine to coarse. [RECENT] Soft at the soft of the state of the soft				
Remarks Terminated a Chiselling fro	at required depth. Sta om 1.20m to 2.30m f	andpipe in or 1.0 hou	stalled to r.	7.0m.				Scale (approx)	B	ogge y	⊧d
							_	Figure N 14-11	 lo. 70.	LG8	

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	round Ch	o alt I	td				Site Site Restoration Contract, Churchtown Landfill, Liff	ord	B N	oreh umb	ole er
	fiouna Cn	eck I	JIA				County Donegal	oru,		LG	9
Boring Meth Cable percus	od ssive to 7.0m.	Casing 20	Diameter Omm case	r ed to 7.00m	Ground	Level (mOD)	Client Donegal County Council		Jo N 1	ob umb 4-11	er 70
		Locatio	n 0857.5 E	396123.7 N	Dates 12	/11/2014	Engineer TAL Civil Engineering Ltd		S	heet 1/1	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Ins	str
							Loose, brownish grey, silty, very gravelly, fine to coarse SAND. Gravel is subangular to subrounded fine to coarse. [MADE GROUND]	,		è	••••
0.50	D1										
1.00	D2					<u>-</u>					30 42 05 80 00 05 90 00 05
1.20-1.65	SPT N=10			2,2/3,2,2,3							
1.50 2.00-2.45	B1 SPT N=15			6,3/4,1,7,3		(4.00)					42,55 02,050 42,55 55,55 55 55 55 55 50,55 55 55 55 55 55 50,55 55 55 55 55 55 55 55 55 55 55 55 55
2.00	D3										a ori oribo di alto ori oribo 43 aoris di cinto di 23 aoris 24 aoris di cinto di 23 aoris di 24 aoris di 20 aoris di 24 aoris
3.00-3.45 3.00 3.00	SPT N=6 B2 D4			2,3/1,1,3,1			wet 1000				42.5% 0.2 0.5% 42.5% 80.0% 2.5% 75.0% 80.0% 2.5% 75.0% 80.0% 2.5% 75.0%
							· NOT				
4.00-4.45 4.00	SPT N=28 D5			3,7/8,6,5,9		4.00 c	Very soft, dark brown, slightly sandy, clayey PEAT. [RECENT]				2014 of 05 04 05 04 05 04 05 05 05 05 05 05 05 05 05 05 05 05 05
4.50	B3				citor	4.60	Loose light brown very silty fine to coarse SAND	X			
5.00	D6			FO	inspector ovinght or		[RECENT]	× × ×			5 02 05 05 05 0 0 05 05 05 05 0 0 05 05 05 05 0 0 05 05 05 05 05 05 05 05 05 05 05 05 05
				Consent of c	D. x	(2.40)					300 42.05 002 00300 42.06 6 40 005 02 005 40 005 6 00 005 00 005 40 005 9 00 005 00 005 40 005
6.00 6.00	B4 D7										and and a set of a se
7.00	D8			12/11/2014:0.00m		7.00		××××			
							Complete at 7.00m				
Remarks Terminated a	t required depth. Sta	andpipe in	stalled to	4.5m.		F		Scale (approx)	L	ogge y	∌d
								1:50		LK	
								Figure N	lo.		
								14-11	70.I	_G9	

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									5	Site						I	Borehole
	C	3rc	oui	nd Cl	heck	Ltd				Site Resto County Do	ration Co onegal	ontract, C	hurchtow	n Landfill	, Lifford,		4
Installat Standp	tior ipe	і Тур)e		Dimension Interna	ons al Diameter of Tube [A] = 50 r	nm		C	Client						ì	Job Number
					Diame	ter of Filter Zone = 150 mm				Donegal C	County Co	ouncil					14-1170
					Location		Ground	Level (m	OD) E	Ingineer						\$	Sheet
										TAL Civil E	Ingineeri	ing Ltd					1/1
Legend	Vater	Ins (A	tr .)	Level (mOD)	Depth (m)	Description				G	roundwa	ater Strik	es Durin	g Drilling	1		
× <u> </u>	>			· · /	0.25	Concrete			Depth	Casing				Read	ings		Depth
× ×	2	÷	<u></u> .				Date	Time	Struck (m)	Depth (m)	Inflo	w Rate	5 min	10 min	15 min	20 min	Sealed (m)
× × ×									7.00		Slight f Strong	low Flow				7.00	
××									0.00		ouong					0.00	
× ×																	
× ×																	
××																	
×										Gr	oundwa	ter Obsei	rvations	During D	rilling		
×										Start of S	hift			E	End of SI	nift	
×							Date	T :	Depth	Casing	Water	Water	Time	Depth	Casing	Water	Water
× <u>· · · · · · · · · · · · · · · · · · ·</u>								Time	(m)	Uepth (m)	(m)	(mOD)	Time	(m)	(m)	(m)	(mOD)
×									•								
× ×		-								net 0	è.						
× ×						Bentonite Seal		14.	Nothe								
××								کم	5 on for	80							
× · · · · · · · · · · · · · · · · · · ·								Out Pot	JICC .								
× ×		-					ونن	onvertee	,	Instru	ument G	roundwa	ter Obse	rvations			
· · · · · · · · · · · · · · · · · · ·							(III)	5 [A] Type	• Stand	nine							
0.0×.0						~	for the										
ו•••	71					at C	Š	Ins	trument	[A]				Dem			
0×0 0•0×	z _ 1					Conser	Date	Time	Depth	Level				Rema	arks		
ו•••						· ·			(m)	(mod)							
0.0.																	
	Z 2																
					9 50												
	0.0				5.50												
	2 - 11 D O					Slotted Standpipe											
	× n o o = u																
	2.0				11.50												
Domort	'e																
Remark	3																

	C		nd C	hoalz	T +d			:	Site Resto	vration Co	ontract C	'hurchtow	n I andfill	l lifford		Borehole Number
	UT I	.ou		Dimonsi					County Do	onegal		luionon		I, LIIIOIG,		L1A
Standpip	be	уре		Interna Diame	al Diameter of Tube [A] = 50 m ter of Filter Zone = 200 mm	ım			Client Donegal C	County Co	ouncil					Job Number 14-1170
				Location	l	Ground	Level (m	OD) I	Engineer							Sheet
	-			23102	0.6 E 395902.6 N				TAL Civil E	Engineeri	ng Ltd					1/1
Legend		nstr (A)	Level (mOD)	Depth (m)	Description				G	roundwa	ater Strik	es Durin	g Drilling	9		-1
	••••			-0.50	Concrete	Date	Time	Depth Struck	Casing Depth (m)	Inflo	w Rate	5 min	Read	lings	20 mir	Depth Sealed
				0.25				4.80	(,	slight					20	. (,
					Bentonite Seal											
									Gr	oundwat	ter Obsei	rvations	Durina D	Drillina	<u> </u>	
									Start of S	hift			-	End of SI	hift	
						Time	Depth	Casing	Water	Water	Time	Depth	Casing	Water	Water	
	000	1		2.00		07/11/14		(m)	(m)	(m)	(mOD)	pm	(m) 3.00	(m)	dry	. (mOD)
						10/11/14				e. P		pin	0.50		4.00	
		ించాడి? సి. కిల్లాం లైంట్ రాజ్ కార్యాల్ కార్యాల్ కార్యాల్						4.	Vother							
								sonly.	30.3							
							Purpot	mel								
						Pectil	ON DELL		Instru	ument G	roundwa	ter Obse	rvations			
					Ŕ	or in prist.	[A] Type	: Stand	pipe							
					A A	COA	Ins	trument	: [A]				_			
					Conser	Date	Time	Depth (m)	Level (mOD)				Rema	arks		
		6.000 0.000 1.0000 1.00000 1.00000 1.00000000			Slotted Standpipe											
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×																
		2000 000000000000000000000000000000000														
				6 50												
Remarke	0.000	3622		0.00												
i contai no																
Remarks		ము లో జిల్లా రామా రామా రామా రామా రామా రామా రామా రా		6.50												

	Ģr	011	nd C	hock	Itd			:	Site Site Resto	oration Co	ontract. C	hurchtow	vn Landfill	. Lifford.		Borehole Number
Installatio		ne		Dimensi					County Do	onegal				,,		L2
Standpipe	e	he		Interna Diame	al Diameter of Tube [A] = 50 m ter of Filter Zone = 200 mm	nm			Client Donegal (County Co	ouncil					Job Number 14-1170
				Location 23102	0.6 E 395902.6 N	Ground	Level (m	IOD)	Engineer TAL Civil I	Engineeri	ng Ltd					Sheet 1/1
Vater Vater	In (/	str A)	Level (mOD)	Depth (m)	Description				G	roundwa	iter Strik	es Durin	g Drilling	9		
	•			-0.50	Concrete	Date	Time	Depth Struck	Casing Depth	Inflov	v Rate		Read	lings		Depth Sealed
	·	0.00		0.25				(m)	(m)			5 min	10 min	15 min	20 mir	n (m)
					Bentonite Seal											
		12000		1.00												
									Gr	oundwat	er Obse	rvations	During D	rilling		
						Data			Start of S	hift				End of S	hift	
						Date	Time	Depti Hole (m)	n Casing Depth (m)	Water Depth (m)	Water Level (mOD)	Time	Depth Hole (m)	Casing Depth (m)	Wate Depti (m)	r Water h Level (mOD)
						05/11/14						pm	6.50		dry	
									neri	Op						
								27.	Myoth							
							-0 ⁴	e of tot	Ĩ							
							on pure	ALL.	Instru	ument Gi	roundwa	ter Obse	ervations		<u> </u>	
						inspect	owne	01								
					Ŷ	OF INST.	[A] Iype	: Stand								
					Slotted Standpipe	Date		strumen					Rem	arks		
					Colle		Time	Depth (m)	n Level (mOD)							
				6.50												
Remarks	يجهوا	1588		<u> </u>		1				<u> </u>						

								:	Site							Borehole Number
	Gr	ou	nd C	heck	Ltd				Site Resto County Do	oration Co onegal	ontract, C	hurchtow	vn Landfil	, Lifford,		LG1A
Installatio Standpipe	on Ty e	ре		Dimensi Interna Diame	ons al Diameter of Tube [A] = 50 n eter of Filter Zone = 200 mm	nm		(Client Donegal C	County Co	ouncil					Job Number 14-1170
				Location 23086	4.3 E 396073.7 N	Ground	Level (m	IOD) I	Engineer TAL Civil E	Engineeri	ng Ltd					Sheet 1/1
Legend >	In	str A)	Level (mOD)	Depth (m)	Description				G	roundwa	ater Strik	es Durin	g Drilling	J		
	· · ·		(-0.50	Eoncrete			Depth	Casing		-		Read	lings		Depth.
	• • •			0.25		Date	Time	(m)	(m)	Inflo	w Rate	5 min	10 min	15 min	20 mir	n (m)
					Bentonite Seal											
				1 00												
				1.00					Gr	oundwa	ter Obsei	rvations	During D	rilling		
		00000							Start of S	hift				End of SI	hift	
						Date	Time	Depth Hole	Casing	Water Depth	Water Level	Time	Depth Hole	Casing Depth	Wate	r Water h Level
						05/11/14		(11)	(111)	(11)		pm	5.30	(11)	dry	(IIIOD)
										e.						
									Vother							
							, and the second se	s only	311.							
							Diffo	an ^{ol}								
						- oecily	on er te		Instru	ument G	roundwa	ter Obse	ervations			
					4	or institut	[A] Type	: Stand	pipe							
					Slotted Standpipe	CON.	Ins	trument	t [A]							
					Conserve	Date	Time	Depth (m)	Level (mOD)				Rem	arks		
		200 000 200 000 200 000 200 000														
		bo 425 00 80000 00 80000 00														
				5.30												
Remarks		1														

								5	Site							Borehole
	Gr	ou	nd C	heck	Ltd				Site Resto County Do	oration Co onegal	ontract, C	hurchtow	n Landfill	l, Lifford,		LG2AR
Installat Standpi	i on Ty pe	/pe		Dimensi Interna	ons al Diameter of Tube [A] = 50 m	nm		C	Client	Sound 4 Co	u noil					Job Number
				Diame		1			Donegar		Juncii					14-1170
				Location 23094	6.7 E 395975.7 N	Ground	Level (m	OD) E	Engineer TAL Civil E	Engineeri	ng Ltd					Sheet 1/1
Lonord	ul ater	str	Level	Depth	Description				G	roundwa	ter Strik	es Durin	a Drillina	1		
	≥ (A)	(mod)	-0.50	Concrete			Depth	Casing				Read	lings		Depth
		• • • •		0.25		Date	Time	Struck (m)	Depth (m)	Inflov	v Rate	5 min	10 min	15 min	20 mir	Sealed n (m)
								4.50		slight						
					Bentonite Seal											
				1.00												
									Gr	oundwat	er Obsei	rvations	During D	rilling		
									Start of S	hift			E	End of SI	hift	
						Date	Time	Depth Hole (m)	Casing Depth (m)	Water Depth (m)	Water Level (mOD)	Time	Depth Hole (m)	Casing Depth (m)	Wate Depti (m)	r Water Level (mOD)
						06/11/14						pm	5.00		4.50	
									. et S	e. P						
								23.	onyothe							
							-0 ²⁴	ed for								
						×	on pure	⁶ 17	Instru	ument Gi	oundwa	ter Obse	rvations			
						(Inspect	Al Type	• Standi	nine							
						OF AS.	Ins	trument	- [A]							
					Slotted Standpipe	Date							Rema	arks		
					Cor		Time	Depth (m)	Level (mOD)							
Z	1															
				5.00												
Remark	5															

	a			1.	T . 1			;	Site					. 1.155 - and		Borehole Number
	Ġr	ou	nd C	heck	Ltd				Site Resto County Do	oration Co onegal	ontract, U	hurchtow	n Landfiii	l, Littord,		LG3A
Installatio Standpip	on Ty e	/pe		Dimensio Interna Diame	ons al Diameter of Tube [A] = 50 m ter of Filter Zone = 200 mm	ım			Client Donegal (County Co	ouncil					Job Number 14-1170
				Location 23098	0.4 E 395939.7 N	Ground	Level (m	OD) I	Engineer TAL Civil E	Engineeri	ng Ltd					Sheet 1/1
/ater	In	str A)	Level	Depth	Description				G	roundwa	ater Strik	es Durin	g Drilling	3		
	, (~ /	(IIIOD)	-0.50	Concrete			Depth	Casing				Read	lings		Depth
	• • •			0.25		Date	Time	Struck (m)	(m)	Inflo	w Rate	5 min	10 min	15 min	20 mir	n (m)
								4.00		slight						
					Bentonite Seal											
		196.09		1.00				1	Gr	oundwat	ter Obsei	rvations	During D	Drilling		
	Date									hift			E	End of SI	nift	
						Time	Depth Hole (m)	n Casing Depth (m)	Water Depth (m)	Water Level (mOD)	Time	Depth Hole (m)	Casing Depth (m)	Wate Depti (m)	r Water h Level (mOD)	
											pm	4.50		4.00		
									nerv	e.						
								mily.	2019 Ott							
							1005	s of for								
						rectif	on pureo	ç,	Instru	ument G	roundwa	ter Obse	rvations			
					4	of install	A] Type	: Stand	pipe							
					ু ঠ	COB3	Ins	trument	t [A]							
					Slotted Standpipe	Date	Time	Depth (m)	Level (mOD)				Rema	arks		
				4.50												
Remarks																

					T . 1			;	Site							Borehole Number
	Gr	oui	nd C	heck	Ltd				Site Resto County Do	negal	ontract, C	hurchtow	n Landfill	I, Lifford,		LG5A
Installation Standpip	on Tyj e	pe		Dimensi Interna Diame	ons al Diameter of Tube [A] = 50 n eter of Filter Zone = 200 mm	nm			Client Donegal C	county Co	ouncil					Job Number 14-1170
				Location	1	Ground	Level (m	OD)	Engineer							Sheet
				23091	3.4 E 396112 N				TAL Civil E	ingineeri	ng Ltd					1/1
Legend S	Ins (A	str A)	Level (mOD)	Depth (m)	Description		I	1	G	roundwa	iter Strik	es Durin	g Drilling	3		
	•••			-0.70	Concrete	Date	Time	Depth Struck	Casing Depth	Inflo	w Rate	5 min	Read	lings	20 mir	Depth Sealed
	• • • • • •			0.25				(111)	(11)			5 mm			20 1111	. (,
					Bentonite Seal											
	6000	19000		0.80												
									Gr	oundwat	ter Obsei	rvations	During D	rilling		
						Date		Donth	Start of S	hift	Watar	<u> </u>	E Donth	End of SI	hift Wata	r Watar
							Time	Hole (m)	Depth (m)	Depth (m)	Level (mOD)	Time	Hole (m)	Depth (m)	Depti (m)	n Level (mOD)
						11/11/14						pm	4.30		dry	
									net	è.						
								ald.	2017 OIL							
							ي الم	ed for								
						, tit	on Pure	⁶ 11	Instru	ument Gr	roundwa	iter Obse	rvations	<u> </u>	1	
						(inspect	AT Type	· Stand	Inine							
					Slotted Standpipe	OF THE	Ine	trumon								
					sento	Date			· [~]				Rem	arks		
					Cons		Time	Depth (m)	n Level (mOD)							
				4.00												
Remarks		I			1											

	2			1	*			\$	Site							Borehole Number
	G	rou	nd C	heck	Ltd				Site Resto County Do	oration Co onegal	ontract, C	hurchtow	/n Landfii	l, Littord,		LG6A
Installat Standpi	t ion T ipe	уре		Dimensi Interna Diame	ons al Diameter of Tube [A] = 50 n eter of Filter Zone = 200 mm	nm		(Client Donegal C	County Co	ouncil					Job Number 14-1170
				Location	1 24 3 E 306020 2 N	Ground	Level (m	OD) E	Engineer							Sheet
	I ter	nstr	Level	Depth	P					Ingineen						
Legend	A Na	(A)	(mOD)	-0.50	Concrete				G	rounawa	ater Strik	es Durin	g Drilling Reac	linas		
	•••			0.05	Concrete	Date	Time	Depth Struck (m)	Depth (m)	Inflo	w Rate	5 min	10 min	15 min	20 min	Depth Sealed (m)
	<u>.</u>	· · · · ·		0.25												
					Bentonite Seal											
															<u> </u>	
	200	1000		1.00					Gr	oundwa	ter Obse	rvations	During L			
						Date	Timo	Depth	Start of S	Water	Water	Timo	Depth	End of SI	Water	Water
	0,00,00,0000					06/11/14	Time	(m)	(m)	(m)	(mOD)	pm	4.30	(m)	(m)	(mOD)
	0,00,00,00,00,00,00,00,00,00,00,00,00,0									e.						
		<u>98800 085</u> 00 00 <u>100000000000000000000000000000000</u>						14.	Nother							
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						كدن	on Purp-		Instru	ıment G	roundwa	ter Obse	ervations		<u> </u>	
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				Location 23085	n i7.5 E 396123.7 N	Ground I	Level (m	OD) E	E ngineer TAL Civil E	Engineeri	ng Ltd					Sheet 1/1
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APPENDIX B: GAS AND GROUNDWATER MONITORING RESULTS

<u>Consent of constrainty of the property of the</u>

Churchtown Landfill 14-1170 12/01/2015



11111000000011	Monitoring Point Reference	Atmospheric Pressure (Pa)	Flow Range (I/hr)	Time passed (s)	Methane % v/v	Methane % LEL	Carbon Dioxide %v/v	Oxygen % v/v	Water Level (mBGL)	
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(63)(74)(7				60	2.9	66.0	3.9	20.7		
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Image: constraint of the section of				150	64.2	>>>	18.4	17.3		
InthInthInthInthInthInthInth100110 <td></td> <td></td> <td></td> <td>180</td> <td>64.2</td> <td>>>></td> <td>18.4</td> <td>17.3</td> <td></td>				180	64.2	>>>	18.4	17.3		
IndexIndexIndexIndexIndex100031.60.1*9058.60>>>17.1018.50100011.058.60>>>17.1018.60110012058.60>>>17.1018.601100180058.70>>>17.1018.601100180058.60>>>17.1018.601100180058.60>>>17.1018.601100180058.60>>>17.1018.601100180018.60>>>17.1018.601100180018.6036.6036.6020.001100180018.6036.6036.6020.60110011012.011.6020.70110011.012.011.0020.80110011.012.0011.0020.80110011.012.0011.0020.80110011.012.0011.0020.80110011.012.0011.0020.80110011.012.0011.0020.80110011.0011.0011.0020.80110011.0011.0011.0020.80110011.0011.0011.0020.80110011.0011.0011.0020.80110011.0011.0011.0020.80110011.0011.0011.0020.80110011.0011.00 <t< td=""><td></td><td></td><td></td><td>15</td><td>58.3</td><td>>>></td><td>16.7</td><td>18.4</td><td></td></t<>				15	58.3	>>>	16.7	18.4		
IndexHereIndexIndexIndexIndex100031.60.1*10058.6>>>17.118.6100010058.6>>>17.118.6100010058.6>>>17.118.6100010058.6>>>17.118.6100010058.6>>>17.118.6100010058.6>>>17.118.6100010058.6>>>17.118.6100018.062.03.62.0416.6100018.030.71.620.4100011.610.120.61.8100011.610.120.61.8100011.610.120.71.6100011.611.710.020.4100011.222.711.020.4100011.223.411.320.7100011.223.411.320.7100011.110.011.220.8101011.120.811.220.8101011.110.011.220.8101011.110.011.220.8101011.110.011.617.7101010.110.311.617.8101010.110.311.617.8101010.110.511.617.7101010.110.511.617.7 <td></td> <td></td> <td></td> <td>30</td> <td>58.5</td> <td>>>></td> <td>17.0</td> <td>18.5</td> <td></td>				30	58.5	>>>	17.0	18.5		
IAM IAM <thiam< th=""> <thiam< th=""> <thiam< th=""></thiam<></thiam<></thiam<>	114	1000	21.6.0.1*	60	58.6	>>>	17.0	18.5	2 0 2	
110 110 120 <td>LIA</td> <td>1000</td> <td>51.0-0.1</td> <td>120</td> <td>58.0</td> <td>>>></td> <td>17.1</td> <td>18.6</td> <td>5.82</td>	LIA	1000	51.0-0.1	120	58.0	>>>	17.1	18.6	5.82	
Init Init <th< td=""><td></td><td></td><td></td><td>120</td><td>58.7</td><td>>>></td><td>17.1</td><td>18.0</td><td></td></th<>				120	58.7	>>>	17.1	18.0		
Information Information <thinformation< th=""> <thinformation< th=""></thinformation<></thinformation<>				180	58.6	>>>	17.1	18.6	1	
Initial				15	4.2	84.1	4.2	20.0		
IC689990.5-0.1*602.344.32.820.5901.835.22.120.61201.530.71.620.710020.710020.81501.222.71.020.81501.122.01.020.81501.122.01.020.81501.122.01.020.41601.528.51.720.71601.528.51.720.71701501.121.01.21501.121.01.220.81501.121.01.220.81501.120.01.220.81501.120.01.220.81501.120.01.220.81501.120.81.520.81501.120.81.520.81501.120.81.520.81501.120.81.520.815064.033.916.317.815064.033.916.317.815064.133.816.517.715064.133.916.517.6				30	3.4	62.0	3.6	20.4		
LG8990.5-0.1*901.835.22.120.63.851201.530.71.620.71501.222.71.020.71801.122.01.020.81801.122.01.020.81801.122.01.020.81801.122.01.020.41801.122.01.020.61801.528.51.720.71901.223.41.320.71501.121.01.220.81501.121.01.220.81501.121.01.220.81501.120.81.220.81501.120.81.220.81501.120.81.220.81501.120.81.220.81501.120.81.220.816063.93.51.6.21.712064.03.516.317.89990.19064.03.516.417.715064.03.516.417.718064.03.516.517.7				60	2.3	44.3	2.8	20.5		
101101.530.71.620.71501.222.71.020.71801.122.01.020.81552.754.03.020.41601.528.51.720.6601.528.51.720.71001.223.41.320.71101.223.41.320.71101.223.41.320.71101.223.41.320.71101.121.01.220.81101.121.01.220.81101.120.81.220.81101.120.81.220.811120.81.220.811120.81.220.811120.81.220.811262.5>>16.21131.71.81141.120.81.211562.5>>>16.311563.9>>>16.31151101.21.31121.61.711064.0>>16.4115116117.6	LG8	999	0.5-0.1*	90	1.8	35.2	2.1	20.6	3.85	
150 1.2 22.7 1.0 20.7 180 1.1 22.0 1.0 20.7 180 1.1 22.0 1.0 20.8 199 15 2.7 54.0 3.0 20.4 169 999 0.1 15 2.7 54.0 3.0 20.4 160 1.5 28.5 1.7 20.6 60 1.5 28.5 1.7 20.7 120 1.2 23.4 1.3 20.7 3.70 150 1.1 21.0 1.2 20.8 1.7 20.7 150 1.1 21.0 1.2 20.8 1.7 20.8 150 1.1 20.8 1.2 20.8 1.6 1.7.9 160 63.9 >>> 16.2 17.9 1.0 1.0 1.6 1.7.8 999 0.1 90 64.0 >>> 16.4 17.7 4.02 120 <td< td=""><td></td><td></td><td></td><td>120</td><td>1.5</td><td>30.7</td><td>1.6</td><td>20.7</td><td></td></td<>				120	1.5	30.7	1.6	20.7		
IG9 180 1.1 22.0 1.0 20.8 169 1.1 22.0 1.0 20.8 15 2.7 54.0 3.0 20.4 30 2.0 39.0 2.2 20.6 60 1.5 28.5 1.7 20.7 99 0.1 90 1.2 23.4 1.3 20.7 100 1.2 22.2 1.2 20.8 1.1 20.8 110 1.1 21.0 1.2 20.8 1.1 20.8 110 1.1 20.8 1.2 20.8 1.1 20.8 110 1.1 20.8 1.2 20.8 1.1 20.8 1.1 110 1.1 20.8 1.2 20.8 1.1 20.8 1.1 20.8 1.1 20.8 1.1 20.8 1.1 20.8 1.1 20.8 1.1 20.8 1.1 20.8 1.1 20.8 1.1 <				150	1.2	22.7	1.0	20.7	1	
1.5 2.7 3+0 5.0 20.4 1.69 999 0.1 30 2.0 39.0 2.2 20.6 1.60 1.5 28.5 1.7 20.7 1.00 1.2 23.4 1.3 20.7 120 1.2 22.2 1.2 20.8 150 1.1 21.0 1.2 20.8 150 1.1 21.0 1.2 20.8 180 1.1 20.8 1.2 20.8 199 1.1 20.8 1.2 20.8 180 1.1 20.8 1.2 20.8 199 15 62.5 >>> 16.2 17.9 30 63.8 >>> 16.3 17.8 60 63.9 >>> 16.3 17.8 999 0.1 90 64.0 >>> 16.4 17.7 120 64.0 >>> 16.5 17.6 17.6 <td></td> <td></td> <td></td> <td>15</td> <td>1.1</td> <td>22.0</td> <td>1.0</td> <td>20.8</td> <td></td>				15	1.1	22.0	1.0	20.8		
LG9 999 0.1 1.0 1.0 1.1 20.0 1.1 20.0 1.1 20.0 1.1 20.0 1.1 20.0 1.1 20.0 1.1 20.0 1.1 20.0 1.1 20.0 1.1 20.0 1.1 20.0 1.1 20.7 1.1 20.7 1.1 1.1 20.0 1.1 </td <td></td> <td></td> <td></td> <td>30</td> <td>2./</td> <td>39.0</td> <td>2.0</td> <td>20.4</td> <td>1</td>				30	2./	39.0	2.0	20.4	1	
LG9 999 0.1 90 1.2 23.4 1.3 20.7 3.70 120 1.2 22.2 1.2 20.8 1.1 20.9 1.1 20.8 1.1 20.8 1.1 20.8 1.1 20.8 1.1 20.8 <td></td> <td></td> <td></td> <td>60</td> <td>1.5</td> <td>28.5</td> <td>1.7</td> <td>20.7</td> <td>1</td>				60	1.5	28.5	1.7	20.7	1	
120 1.2 22.2 1.2 20.8 150 1.1 21.0 1.2 20.8 180 1.1 20.8 1.2 20.8 180 1.1 20.8 1.2 20.8 180 1.1 20.8 1.2 20.8 1999 0.1 62.5 >>> 16.2 17.9 30 63.8 >>> 16.3 17.8 60 63.9 >>> 16.3 17.8 999 0.1 90 64.0 >>> 16.4 17.7 120 64.0 >>> 16.5 17.7 150 64.1 >>> 16.5 17.6	LG9	999	0.1	90	1.2	23.4	1.3	20.7	3.70	
150 1.1 21.0 1.2 20.8 180 1.1 20.8 1.2 20.8 180 1.1 20.8 1.2 20.8 180 1.1 20.8 1.2 20.8 15 62.5 >>> 16.2 17.9 30 63.8 >>> 16.3 17.8 60 63.9 >>> 16.3 17.8 999 0.1 90 64.0 >>> 16.4 17.7 120 64.0 >>> 16.5 17.7 180 64.1 >>> 16.5 17.6				120	1.2	22.2	1.2	20.8		
R1 100 1.1 20.8 1.2 20.8 999 0.1 15 62.5 >>> 16.2 17.9 30 63.8 >>> 16.3 17.8 60 63.9 >>> 16.3 17.8 999 0.1 90 64.0 >>> 16.4 17.7 120 64.0 >>> 16.5 17.6				150	1.1	21.0	1.2	20.8		
R1 999 15 62.5 >>> 16.2 17.9 30 63.8 >>> 16.3 17.8 60 63.9 >>> 16.3 17.8 999 0.1 90 64.0 >>> 16.4 17.7 120 64.0 >>> 16.4 17.7 150 64.1 >>> 16.5 17.6				180	1.1	20.8	1.2	20.8		
R1 999 0.1 30 63.8 >>> 16.3 17.8 999 0.1 60 63.9 >>> 16.3 17.8 90 64.0 >>> 16.4 17.7 4.02 120 64.0 >>> 16.4 17.7 150 64.1 >>> 16.5 17.6				15	62.5	>>>	16.2	17.9		
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				30	17.8	1				
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	R1	999	0.1	90	64.0	>>> 、、、	16.3	17.8	4.02	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	R1	999	0.1	120	64.0	>>>	16.4	17.7	4.02	
180 64.1 >>> 16.5 17.6				150	64.1	>>>	16.5	17.7	4.02	
				180	64.1	>>>	16.5	17.6	1	

APPENDIX C: GEOTECHNICAL LABORATORY TEST RESULTS

	froun	d Cl	heck <mark>I</mark>	Ltd									I	Laborato	ory	Tes	st	Res	ults		
Site : S	Site Restora	ation Co	ntract, Chur	rchtown I	_andfill	, Lifford, C	ounty Dor	negal											J	Job N	umber
						,, -		- 0 -												14	-1170
Client : L	Jonegal Co	unty Co	uncii																S	Sheet	
Engineer: 7	TAL Civil Er	ngineerir	ng Ltd																	1	/5
				DE	TER	MINATI	ON OF	PA	RTICL	E SIZ	E DIS	STRI	BU'	TION					I		
Borehole / Trial Pit	Depth (m)	Samp	ble						I	_aborat	tory Des	scriptic	on								
L1A	1.00	B1																			
		<u> </u>																	Siev Parti Siz	/e / icle ze	% Passing
100													Π			Π		7	20 m	m	100.0
90																		-	14 mi	m	94.5
80										**									10 m	m	89.9
								×	*			_ي.							6.3 m	im	86.5
70											ther	\$°				T			3.35 1	mm	81.5
60										23 5	30				-	+		-	2 mm	1	77.4
50						A				T of								_	1.18 ו	mm	72.7
									DIL OUIT										600 µ	ım	66.8
40								otion	Net										425 µ	ım	64.2
30			*				192	11 02					++			++		-	300 µ	ım	60.4
20	*						K0 8	ø										-	212 µ	im	56.3
10							ð												75 un	n	40.1
10						OUG													63 µn	n	38.2
0	0.002	0.006	0.02	0.06	5 5	0.2	0.6		2	6	20)	60	200)	6	00		20 µn	n	30.3
	Fine	Me	edium Co	harse	Fine	Medi	um Coa	arse	Fine	Me	dium	Coarse	<u>م</u>					Г	6 µm		26.3
	AY SIL	.T		Julioo	SAN	D			GRAV	/EL		Couro	- (COBBLES I	BOUL	_DEI	RS		2 µm		23.4
		F						_													
			C	Grading	Analys	sis			F	Particle	Propor	tions									
			D85			5.4 mm			Cobbles	+ Bou	Iders	-									
		Ļ	D60			291.2 µr	m		Gravel			22.	6%								
		ŀ	D10			<2.0 µm		-	Sand			39.	6%								
		+	11				_	+	Silt			14.	5% 1%								
			Uniformit	y Coeffi	cient	-			Clay			23.	+ 70								
Method of	Preparatio	n:BS	1377:PART	1:1990:7	.3 Initi	al preparat	ion 1990:	:7.4.5	Particle s	ize test	s										
Method of	Test	: BS 1	1377:PART	2:1990:9	Deter	mination of	f particle s	size di	stribution												
Domortes																					
Remarks		÷																			
										Produc	red by th	e GEOt	echni	ical DAtabase	SYst	om (GE		V) (C) all r	riahts	reserved

G	roun	d Cl	heck <mark>Lt</mark>	d					Laborator	ry Test Res	ults	
Site : Si	ite Restora	ation Co	ontract. Church	town Landfill	Lifford, County Doneg	al					Job N	lumber
				Lown Landin	, Elliora, County Donog	A1					1	4-1170
Client : D	onegal Co	ounty Co	ouncil								Shee	t
Engineer: T/	AL Civil Er	ngineerir	ng Ltd									2/5
				DETER	MINATION OF P	ARTICLE	SIZE DIS	TRIBU	JTION			
Borehole / Trial Pit	Depth (m)	Sam	ple			Lab	oratory Des	cription				
LG1A	1.00	B1	1									
		<u> </u>									Sieve / Particle Size	% Passing
100								***			50 mm	100.0
90										$\left \left \left$	37.5 mm	97.6
						**					28 mm	96.8
80								.01			20 mm	96.1
70							net of	50			14 mm	94.4
60							Off.				6.3 mm	92.1
50						es of the	\$ O				5 mm	88.9
30						NIPONITEC					3.35 mm	86.7
40	*	*-			لأنثير المحاص	51 et recr					2 mm	84.0
30						3 ⁷⁴¹					1.18 mm	80.0
20					NO NO						600 µm	74.7
20											425 μm	72.3
10					OTEN						212 µm	64.9
0	0 002	0 006	0.02	0.06		2 6	20	6	0 200	600	150 μm	59.2
	5.002										75 μm	50.7
	Y SIL	_T	edium Coar	se Fine SANI	D Medium Coarse	GRAVEL	Medium	Coarse	COBBLES BO		63 µm	49.1
											20 µm	42.8
											6 µm	39.1
		Γ									2 µm	35.3
			Gra	ading Analys	sis	Part	icle Proport	tions				
			D85		2.5 mm	Cobbles + F	Boulders	-				
		-	D60		158.8 µm	Gravel		16.0%				
		F	D10		<2.0 µm	Sand		35.1%)			
						Silt		13.5%	>			
			Uniformity C	Coefficient	-	Clay		35.3%				
		L										
Method of P	Preparatio	n:BS	1377:PART 1:1	1990:7.3 Initia	al preparation 1990:7.4	.5 Particle size	tests					
Mathematica	4	. 50			mination of a set of the st	diatrik						
wethod of T	est	: 88	13/7:PART 2:1	1990:9 Deter	mination of particle size	USUIDUTION						
Remarks		:										
								0504	nicol DAtata 0	Notom (0500 - 2	V) (0) all similar	*000 *******

	Groun	d Ch	eck Ltd		Laboratory Test Results											
Site : S	Site Restora	ation Cont	ract, Churchtown Landfil	l, Lifford, Cou	nty Donegal								Job	Number		
Client : [Donegal Co	unty Cour	ncil											14-1170		
	Donegai Oo												She	et		
Engineer:	TAL Civil Er	ngineering	Ltd											3/5		
			DETER	MINATIO	n of Pai	RTICLES	SIZE DIS	TRIBL	JTION							
Borehole / Trial Pit	Depth (m)	Sample	•			Lab	oratory Des	cription								
LG2A	1.00	B1														
													Sieve / Particle Size	% Passing		
100									řIII			\square	63 mm	100.0		
90							***	**					50 mm	96.3		
						* * *							37.5 mm	94.6		
80								2.3					28 mm	93.0		
70							S S					++-	20 mm	91.5		
60							OTT			_			14 mm	90.3		
						CAN .	5 20 -						6 3 mm	88.2		
50						11P Jike							5 mm	87.1		
40					tion	or en							3.35 mm	85.2		
30			*			\$				_			2 mm	82.0		
	*	*-			of Ninght								1.18 mm	78.0		
20				8	- OK								600 µm	72.9		
10				N ^e nt									425 µm	70.7		
0				Cor									300 µm	67.7		
	0.002	0.006	0.02 0.06	0.2	0.6	2 6	20	6	60	200	60	00	150 um	58.5		
CL/	AY Fine	Med	ium Coarse Fine	Medium	n Coarse	Fine	Medium	Coarse	COBBLE	ES BOI	JLDEI	RS	75 μm	47.4		
	SIL	-1	SAN	0		GRAVEL							63 µm	39.5		
													20 µm	31.4		
													6 µm	26.3		
			Grading Analys	sis		Part	icle Proport	ions					2 µm	23.1		
					┥ ┝											
			085	3.3 mm	┥ ┝	Cobbles + E	Boulders	0.8%								
		H	10 0	<2.0 µm	┥ ┝	Gravel		42.9%	,							
			510	pini		Silt		16.1%								
			Jniformity Coefficient	-		Clay		23.1%))							
								1								
Method of	Proparatio	n · RC 10	77·PART 1·1000·7 2 Initi	al nrenaration	1000.7 / F	Particle size	tests									
Method of	Trat			mination	ortiolo											
Method of	rest	: BS 13	11:PART 2:1990:9 Deter	mination of pa	articie size dis	stridution										
Remarks		:														

	Groun	d Ch	eck Ltd						Labora	itory	Tes	st Re	sults		
Site :	Site Restora	ation Cont	ract, Churchtown Landfill	, Lifford, Cour	nty Donegal	I							J	Job N	umber
				, ,	., <u>.</u>									14	-1170
Client :	Donegal Co	ounty Cour	ncil										5	Sheet	
Engineer:	TAL Civil Er	ngineering	Ltd											4	/5
			DETER	MINATIO	N OF PA	ARTICLE S	IZE DIS	STRIBL	JTION				1		
Borehole / Trial Pit	Depth (m)	Sample				Labo	ratory Des	scription							
LG3A	2.00	B2													
													Siev Part Siz	ve / icle ze	% Passing
100							×*	*				\square	28 mi	m	100.0
90										+ +			20 m	m	99.1
80													14 m	m	97.9
80								.0.					10 m	m	95.0
70							net	50					6.3 m	nm	92.0
60							0 ¹¹						3 35	mm	87.1
50						es of so	- ²⁸						2 mm	1	82.3
50						NIP NITE							1.18	mm	75.1
40					, ijo	Not the second							600 µ	ım	66.5
30			*			3 ³ 2							425 µ	ım	63.0
20	Î			4	of their								300 µ	ım	58.9
20				. 8	COK								212 µ	ım	54.6
10				N ^{SON}									150 µ	ım	49.9
o 📖													75 µn	n	41.9
	0.002	0.006	0.02 0.06	0.2 (0.6	2 6	20) 6	50 2	200	60	00	20 un	n	33.4
CL	AY Fine	Med	ium Coarse Fine	Medium	Coarse	Fine N	Medium	Coarse	COBBLE	BOU	LDEF	รร¦	6 µm		29.6
	SIL	_1	SAN)		GRAVEL							2 μm		27.2
			Grading Analys	iis		Partic	cle Proport	tions							
		-	785	2.8 mm			ouldoro	-							
		H	D60	334.5 µm		Gravel	GUIGELS	17.7%	, b						
			D10	- <2.0 μm		Sand		42.9%	6						
						Silt		12.2%	, b						
			Jniformity Coefficient	-		Clay		27.2%	, b						
					J										
Method of	Preparatio	on: BS 13	77:PART 1:1990:7.3 Initia	al preparation	1990:7.4.5	5 Particle size to	ests								
Method of	Test	: BS 13	77:PART 2:1990:9 Deter	mination of pa	article size o	distribution									
Remarks		:													

	froun	d Cł	neck L	td													La	bora	tory	/ Te	est	Re	sults		
Site : S	Site Restora	ation Cor	ntract. Churc	chtown La	ndfill. L	ifford. (Count	v Do	negal															Job N	lumber
				20			Journ.	, 20	gu															1.	4-1170
Client : [Donegal Co	ounty Cou	uncil																					Shee	t
Engineer: 7	TAL Civil Er	ngineerin	g Ltd																						5/5
				DET	ERM	INAT	ION	OI	F PA	RTI	CLE	ΞS	IZE	DI	STR	lB	UTI	ON						1	
Borehole / Trial Pit	Depth (m)	Samp	le								L	abo	rator	y De	scrip	tion									
LG9	2.00	D3																							
	1	<u> </u>																					S Pa	ieve / article Size	% Passing
100																90	mm	100.0							
90										_				~	~×	**	1					_	75	mm	97.8
												**		~									63	mm	95.5
80											×				.01								50	mm	93.9
														37.	5 mm	92.2									
60								×1		_	++		100	3112								-	20	mm	88.9
50											es d	30	104										14	mm	87.7
50						$\left \right\rangle$				11P	37. C												10	mm	85.9
40									jó	2 St 4												-	6.3	mm	83.3
30										2.ar												_	5 n	nm	81.9
					X		RÓ		Str														3.3	5 mm	79.2
20							S C	ØK.															2 n	nm	74.9
10		-*	*			تعيي ا	*			+	++											-	1.1	8 mm	69.8
0	*					Con																	60) µm	63.0
-	0.002	0.006	0.02	0.06		0.2	0.	6		2		6		20	0		60	2	200	(600		42	5 µm	59.2
	AY Fine	Ме	dium Coa	arse Fi	ne	Med	lium	Co	arse	Fir	ne	Ν	/lediu	ım	Coa	rse	со	BBLES	во	JLDE	ERS	- -	21		52.0 45.4
	SIL	_T		Ş	SAND					G	3RAVI	EL										_	150) um	33.2
																							75	μm	19.2
		Г							г									-					63	μm	16.2
			C.	rading Ar	nalveie						Р	artic	le Pi	ronoi	rtione								20	μm	10.0
																-							6 µ	m	6.4
		Ļ	D85			8.7 mm	۱ 		-	Cob	bles	+ B(oulde	ers	4	.8%		-					2 µ	m	4.6
		-	D60			462.6 µ	Im		ŀ	Gra	vel				2	20.3%	6	-							
		-	D10			∠u.0 µr	n		ŀ	San	d					1 20	/o /	-							
		-	liniferenti	Coofficient	t	00 4	-		-	Silt						1.3%	Û	-					-		
			Uniformity	COEILICIE	;iii	23.1					/				-										
Method of	Preparatio	n : BS 1	377:PART 1	:1990:7.3	Initial	orepara	ition	1990):7.4.5	5 Parti	icle si	ze te	ests												<u> </u>
Method of	Test	: BS 1	377:PART 2	2:1990:9 D	etermi	nation o	of part	ticle	size c	distribu	ution														
Bornerise																									
Remarks		:																							
												Bro	ducas			040.01	hulaal	DAtab		otom			SV) (C) 4	ll rights	recorved



T: +44 (0) 28 94488084 E: admin@ground-check.com


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eotecentcal

REPORT



41a Tullyard Road, Drumbo, LISBURN Co. Antrim, N Ireland BT27 5JN Telephone: Belfast (01232) 826734 Fax: Belfast (01232) 826096

Geolechnical Report



<u>CONTENTS</u>



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CLIENT: DONEGAL COUNTY COUNCIL

CONSULTING ENGINEERS: KIRK McCLURE MORTON

CHURCHTOWN LANDFILL SITE LIFFORD GROUND INVESTIGATION

The following factual report describes an investigation carried out at the above site in late August and early September 1998 on instructions from Kirk McClure Morton.

The investigation was carried out to facilitate monitoring installations and to characterise the geology and hydrogeology of the site. The contract provides for the formation of four cable tool percussion boreholes with associated sampling, testing and laboratory testing.

Petion purpose of

SITE AND GEOLOGY

The existing landfill site is located of Churchtown, Lifford, Co Donegal.

Geologically the site is underlain by rocks of Dalradian origin which are probably overlain by Glacial deposits and, possibly, Recent peaty materials.

FIELDWORK

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Four boreholes were put down at the locations instructed as indicated approximately in figure 1. One of our Dando 150 Investigator shell and auger rigs was used, boring to produce a nominally 200 mm diameter hole. Borehole completion depths ranged from 9.00 to 14.60 metres below ground level and considerable obstruction time was incurred in Boreholes 1, 3 and 4, four attempts being made to drill Borehole 1 and three attempts to drill Borehole 3.

Three unsuccessful attempts were made at undisturbed 105 mm sampling within cohesive soil. The remaining soils were predominantly granular and Standard Penetration Tests (SPT) were performed generally at abut 1.50 metre intervals as well as within rock. Where full 0.3 metre penetration was not achieved, the number of blows for the quoted penetration is aiven. Disturbed samples were taken at intervals or at change of stratum for classification purposes and sealed in polythene containers. Water samples were taken in amber glass containers and sent to our testing laboratary.

Standpipes were installed in all boreholes to the depths shown on the logs for future groundwater sampling and monitoring. These were constructed in accardance with your standard detail and were capped at surface with raised steel covers with padlocks.

In-situ variable head permeability tests were performed in the standpipe installations in each borehole and test results are given in tabular form in Appendix 1.

LABORATORY TESTING

stony any other use. The water samples were despatched to our feeting laboratory for analysis in accordance with the Engineer's specified suite of tests and results are included in Appendix 2.

STRATEX LIMITED

Consentate

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		Bore	e h	ole	Lo	g	Borehole Sheet 1	No. of	1 3
ST	RATEX	Client	DON	EGAL C	OUNTY CO	DUNCIL			Scale
		Engineer	KIR	(MCCL	URE MOR				Ground
	Telephone: Belfast (01232) 826734	Site	CHUR	ACHTOM ATY DO	N LANDF: NEGAL	ILL SIT	E, LIFFORD,		Level
Date and				Depth		Reduced	s	ampies/Tes	ts
Water Level	Description			(m)	Legena	Level	Depth (m)	Sample Type	Test
25/08/98	TOPSOIL (0.20)			0.20			0.20	D	
	Domestic refuse, metal, timber, plastic, matrix of brown clayey, gravelly, sandy	in a silt (FILL)	1-1-1-						
			L. t. t. 1			र प्र			
							1.00	b	
26/08/98	-								
			1-1-1-			e e			
			_				2.00	D	
							3.00	D	
				15 ^{0.}					
		A. 5	Sollie				3.60	м	
FRUCK water t 4.00m		ses official		1			4.00	D	
IGHT FLOW		DUTPOSITIEC	1						
	ection and the section of the sectio	neite		1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1					
PM 4.00m	Lance arcentic area slightly covering	cilty		5.00			5.00	D	
27/08/98 AM 4.00m	fine, medium and coarse grained SAND con bands_of_grey, very sandy silt (4.00)	taining			* 		5.15-5.450 5.30	W	SN4
	[[FLUVIO GLACIAL]		1,1		×		5.50	D	
	Cor				× ×		6.00	D	
					×				
					× ×		6.60	W	
					××××		7,00	D	
					× ×				
					×		7.50	D	ar
TRUCK water			-		x x		7.65-7.950 8.00	D	S N5
LIGHT FLOW					×	}	0.00		
					×				
			•	9 00	×		9.00		
	Loose, greenist grey, very silty, fine ar grained SAND (2.00)	nd medium	-		× ×		9.15-9.450	Ų	S N4
	[FEUVIO GLACIAL]		1.		×				
			1.5.4	;	× ×		10.00	<u>,</u>	
	<u></u>			:	e		10.00	U lah N-	
iemarks Four_a	attempts were made to drill borehole		s	Stand Test	samı ard Penetr 	ation D	vy Disturbed Sample	να αυτ).
irst Second Third	attempt met refusal at 1.00m depth 1 attempt met refusal at 1.20m depth attempt met refusal at 1.50m depth		c r	Vane Core Rock	Test Recovery (Quality	%) 8 W P	Bulk Sample Water Sample Piston	229	3
,			s	Desig PT: Whe	nation (RQ are full 0.3r	n ponetra	Tube tion has not	Figure	
Juilling Math		to: 11.00m		for t for t	he quoted N-value),	, uie num penetratio	oar or brows on is given	2	

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	<u>; x ;</u>	······································	Bor	eh	ole	Lo	g	Borehole	No.	1 3
	RATI		Client	DON	EGAL C	OUNTY CO	- DUNCIL			Scale
			Engineer	KIR	K MCCL	URE MORT	ON			1:50
	ZZ. Teleph	one: Beilast (01232) 826734	Site	CHU COU	RCHTOW NTY DO	N LANDFI NEGAL	LL SIT	E, LIFFORD,		Ground Level
Date and		.			Depth		Reduced	S	amples/Tes	ts
Water Level		Description			(m)	Legend	Level	Depth (m)	Sample Type	Tes
	END OF BOREHOLE				11.00			10.50 10.65-10.9 11.00	D 150 D	SN
			mpurposes only.		JUSE.					
nnarks Four a First Second Third	ttempts were made to attempt met refusal attempt met refusal attempt met refusal	drill borehole at 1.00m depth at 1.20m depth at 1.50m depth		S VOI	Stand Test Vane Core I Rock Desig	Samp ard Penetra Test Recovery (? Quality nation (RQI	te/Test K ation D (8 %) B t 9 %) V V P f D %) U T	ey Disturbed Sample Bulk Sample Vater Sample Piston Ube	Job No 2293	3
				s	PT: Whe	n achievod,	n penetrat the numb	ion has not ber of blows	rigure	
	Shall & August	oco Disertar Carad	ter 11 00 .		tor t	ne quoted p N-value),	senetratio	n is given	2	

		Bor	eh	ole	Lo	g	Borehole Sheet	e No. 3 of	<mark>1</mark> 3
STI	RATEX	Client	DON	EGAL C	OUNTY CO	DUNCIL	1		Scale
		Engineer	KIR	K MCCL	URE MORT	FON			Ground
-	Telephone: Bellast (01232) 826734	SITE	CHUI	KCHTCWI NTY DOI	N LANDF1 NEGAL	ILL SIT	E, LIFFORD		Lovel
Date and	Description			Depth	Legend	Reduce	t 5	amples/Tes	ts
Water Level				(m)	-	Level	Depth (m)	Туре	Test
	i. Obstruction time from 1.00m to 1 3.00 hours. i. Distall standpipe to 5.00m. Install standpipe to 11.00m. GROUNDWATER OBSERVATIONS Date Time Depth Depth 26/08/98 AM 5.00 5.00 4 All depths are given in metres	.50m for .90m for ater evel .00 .00		thee.					
Remarks					Samp	le/Test K	cy	Job No).
Four a First Second	ttempts were made to drill borehole attempt met refusal at 1.00m depth attempt met refusal at 1.20m depth			Stand Test Vane Core f	ard Penetri Test Recoverv (*	ation D B %) W	Disturbed Sample Bulk Sample Water Samole	2297	3
Third	attempt met refusal at 1.50m depth		ľ	Rock (Design PT: Who	Quality nation (RQ ire full 0.2~	D %) U	Piston Tube	Figure	
Drilling Moth	od. Shell & Auger: 200mm Diameter, Cased	to: 11.00m		for t for t	he quoted N-value).	, tho num penetratio	ber of blows on is given	2	

		Bor	ehole	e Lo	g	Borehole Sheet 1	No. of	2 3
STI	RATEX	Client	DONEGAL (COUNTY CO	UNCIL			Scale
~		Engineer	KIRK MCCI	URE MORT	ON	······································		1:50
-	Telephone: Beilast (01232) 826734	Site	CHURCHTON COUNTY DO	N LANDFI	LL SIT	E, LIFFORD,		Level
Date and			Depth	Locard	Reduced	Sa	mples/Tes	ts
Water Level	Description		(m)	Legena	Level	Depth (m)	Sample Type	Test
28/08/98	TOPSOIL (0.50)				الا شرارية بالم	0.50	D	
	glass, in a matrix of brown, clayer, gra sandy silt [FILL] [MADE GROUND] (6.00)	avelly,				1.00	D	
						2,00	D	
	,					3.00	D	
			ther use			3.50	D	
		23.	ATTY OF			3.50	W U	
at 3.80m SLIGHT TO MODERATE FLOW	For inspect	on pupposes of for						
	Soft, dark brown, clayey, peaty SILT (1. [RECENT]	50)	6.50			5.50 6.50-6.95	DU	(NO REC)
			-	x x x x x x x x		7.50	D	
			-	× × × × × × × × ×		7.65-7.950		SN4
	lesse der bes a clichtly convolly of	iltu fico		× × ×		8.00	D	
	medium and coarse grained SAND (0.50)	nicy, nine,	-	× ×		8,20	D	
	Locse, dark brown, slightly silty, very grained GRAVEL 1.00)	sandy, fine		x x		8,50	D	
	TEDATO GENORAL			×		9.00	D	
				×		9.15-9.450	-	S N5
PM 6.20m 31/08/98 AM 4.90m	Leese, dark brown, slightly gravelly, fi and coarse grained SAND (3.00) FillWID GLACIAL	ine, medium		xx		9.50	υ	
·						10.00	D	
Remarks			S Star Test V Van	Sam Idaro Peneti e Test	ple/Test⊁ ∙ation D B	Cey Disturbed Sample Bulk Sample	Job No	D.
2			C Core T Roc Desi	Accovery (k Quality Ignation (BC	(%) Ŵ P 20%) Ŭ	Water Sample Piston Tube	229	3
			SPT: WI	here full 0,3 en achieved r the guoted of N-value)	m penetra 4, the num penetrati	tion has not ber of blows on is given	Figure 3	
Drilling Meth	Od: Shell & Auger: 200mm Diameter, Cased	1 to: 12.50m	(1)				where 18-0	7-2010-02-50

		Bore	e h	ole	Lo	g	Borehole	No.	2 3
		Client	DON	EGAL C	OUNTY CO	UNCIL		01	Scale
		Engineer	XIS	K MCCI	URE MORT	ON			1:50
	Telephone: Belfast (01232) 826734	Site	CHU COU	RCHTOW NTY DO	N LANDFI NEGAL	LL SIT	E, LIFFORD,		Ground Level
Date and		la		Depth		Reduced	Sa	mples/Test	s
Water Level	Description			(m)	Legend	Level	Depth (m)	Sample Type	Test
					X		10.50 10.65-10.95 11.00	50 50	SN4
	ారుది దెలె సెదినల్లెటిద్దారా			12.50			12.00 12.15-12.45	D	S N5
	END OF BOXEHULE		Revenue and a second	her use.					
	For inspect	on puposes of for							· · ·
	Courser								
			<u></u>						
									•
Remarks				S Stand Test V Vane	Sam dard Peneti Test	ple/Test } ration D	(ey Disturbed Sample Bulk Sample	Job No).
2				Core ř Rock Desig	Recovery i Quality Ination (RC	(**) W P 2D %) U	vrater Sample Piston Tube	Figure	
Drillina Meth	od: Shell & Auger: 200mm Diameter, Cased	l to: 12.50m		SPT: Wh bee for (no	ere full 0.3 en achieved the quoted t N-value).	m penetra 1, the num penetrati	ition has not iber of blows on is given	3	

,

		Bor	ehole	Lo	g	Boreho Sheet	le No. 3 of	2 3
STI	RATEX	Client	DONEGAL C	OUNTY C	OUNCIL			Scale
		Engineer	KIRK MCCL	URE MOR	TON			1:50
	Telephone: Belfast (01232) 826734	Site	CHURCHTOW COUNTY DO	N LANDF NEGAL	ILL SITE	E, LIFFOR),	Ground Level
Date and	Description		Depth		Reduced		Samples/Tes	ts
Water Level	Description		(m)	Legend	Level	Depth (m	Sample) Type	Test
	i. Install standpipe to 6.50m. Install standpipe to 12.50m. GROUNDWATER OBSERVATIONS Borehole Casing W 28/08/98 PM 9.50 9.50 4 All depths are given in metres	ater evel .20 .90	and the second sec					
marks			S Shada	Sample d Reported	/TestKey	tubed	Job No.	
ž			V Vane Te C Core Re	est stoverv (%)	ion D Dis Sai B Bul) W Wa	nuroed mple Ik Sample iter Samole	2293	
			r Rock Qu	uality	P Pis	ton	4630	•
			Designa	tion (nGD	70) U 100	Je	Ela	1

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Ę.¥.⇒	E	Boreh	ole	Lo	g	Borehole Sheet ²	No. of	З 3
STRAT	EX Clier	it DOM	NEGAL C	OUNTY CO	UNCIL	L		Scale
	Engi	neer KI	RK MCCL	URE MORT	ON			I:50
I teiop	Site (01232) 826734	CHU COU	JRCHTOW JNTY DO	N LANDFI NEGAL	LL SITE	E, LIFFORD,		Level
Date and			Depth		Reduced	Sa	mples/Test	S
Water Level	Description		(m)	Legend	Level	Depth (m)	Sample Type	Test
Dense, grey, sand large boulders (2 [FLUVIO GLACIAL] END OF BOREHOLE	y GRAVEL containing cobbles 2:00)	and	12.60 14.60			10.50 10.65-10.95 11.00 12.00 12.15-12.45 12.60-12.50 13.00 13.50-13.68 14.00		S N6 S N4 S 50 S 50
Remarks Three attempts were mad First attempt met refus	te to drill borehole		S Star Tes V Van	Sar Sar ndaro Pene t te Test	nplo/Test tration C	Key Disturbed Sample Bulk Sample	Job N	0.
Second attempt met refu	isal at 4.50m depth		C Cor I Roc Des	e Recovery k Quality lignation (R	(%) P (QD%) U	Piston Tube	Figure	e

DONEGAL COUNTY COUNCIL KIRK MCCLURE MORTON CHURCHTOWN LANDFILL SITE, LIFFORD, COUNTY DONEGAL Depth Legend Reduced Samp Level Depth (m) S County Done (m) S County Depth (m) S County Depht (m) S County Depth (m) S County Depth (m) S Coun	Scale 1:50 Ground Level Sample Type Test
KIRK MCCLURE MORTON CHURCHTOWN LANDFILL SITE, LIFFORD, COUNTY DONEGAL Depth Legend Reduced Samp Level Depth (m) S 	1:50 Ground Level Dies/Tests Sample Type Test
CHURCHTOWN LANDFILL SITE, LIFFORD, COUNTY DONEGAL Depth Reduced Samp (m) Legend Level Depth (m) S	Ground Level Sample Type Test
Depth (m) Legend Reduced Samp Level Depth (m) S	oles/Tests Sample Type Test
(m) Legend Level Depth (m) S	Sampie Type Test
AN and the use	

	2.X	Bore	hole	Lo	g	Borehole I Sheet 1	No. of	4 2
STI	ratex ^r	Client (DONEGAL C	OUNTY CO	UNCIL			Scale
	F	Engineer 🕴	KIRK MCCL	URE MORT	ON			1:50
-	Image: Selfast (01232) 826734	Site (CHURCHTOW	N LANDFI NEGAL	ILL SIT	E, LIFFORD,		Level
Date and			Depth	[Reduced	d Sar	nples/Tes	ts
Water Level	Description		(m)	Legend	Level	Depth (m)	Sample Type	Test
03/09/98	1TOPSOIL (0.10)		0.10			0.10	G	
	Soft, brownish grey, sandy, clayey, orgar (0.60)	nic SILT	-	× × × //// × × × × ×	-			
	Soft. dark brown, silty PEAT (1.30)		0.70	× N/4 ×		0.70 0.80	אמ	
	[RECENT]			Niz Niz × Niz ×		1.00	U	
				Alle X	4	1.50	D	(110.05)
				XIZ ANZ	4	1.50-1.95	U	(NO REC
	Soft dark brown silty sandy PEAT (0.30	})	2.00	<i>الد</i> يم الد. مالاد ب	4	2.00	D	
	[RECENT]	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	2.30	N/. ×		2.30	D	
	Loose, grey, silty SAND containing bands greenish grey, sandy silt (4.70)	OT		x x	.] •]			
				××××		3.00	Ð	
	``		The.	× ×		3.15-3.450		S N8
		م در در	otte	××××				
		sontor an	-	× × ×		3.80	D	
		ITPOSICIED ,		×		4.00	U	
TRUCK water	itor	Per real	-	×		4.50	D	
LIGHT FLOW	Sector Sector	AL.		×		4,65-4.950		SN4
	Fortige			* * *	1	5.00	D	
	, A COT			× ,				
	OBSER		-	× ×				
				×. ×		6.00	D	
				×		5.15-6.450		SN4
			-	× ×				
]	×				
	Medium dense, brown, şilty, sandy GRAVEL	containing	7.00	×	1	5.90 7.00	D D	
	cobbles and boulders (1.20) [FLUVIO GLACIAL]		-	$\sum O$		7.50	Ð	
				$\left \begin{array}{c} \bigcirc \\ \frown \end{array} \right $		7.65-7.950	-	S N13
			_	≥ 0		8.00	D	
STRUCK water	Light brown, highly to moderately weather	red	8.20					
STRONG FLOW	PSXMMITE (0.80) [MOINIAN]							
] - 0 ∩∩			9,00-9 280		S 50
	END OF BOREHOLS					9.00	D	
			-					
					ļ			<u> </u>
Remarks			S Star	San ndard Pene	nple/Test tration D	Key Disturbed	Job N	0.
2			V Van C Cor	t o Test e Recovorv		Sample 3 Bulk Sample 4 Water Sample	22	93
			r Roc Des	k Quality ignation (R	QD %)	Piston J Tube	Figure	9
			SPT; W bo fo	here full 0. een achiove r the quote	3m peneta ad, the nu d penetra	ration has not mber of blows tion is given	5	
Dritting Met	hod: Shell & Auger: 200mm Diameter, Cased	to: 9,40m	(n	iot N-value)	•	EPA E	<u>xport 18-0</u>)7-2019:03: (

		Воі	reho	ble	Lo	g	Borehole Sheet	e No. 2 of	4 2
STE	RATEX	Client	DONE	GAL C	OUNTY CO	UNCIL			Scale
		Engineer	KIRK	MCCL	JRE MORT	ON			1:50
	Telephone: Belfast (01232) 826734	Site	CHUR COUN	CHTOWI TY DOI	N LANDFI NEGAL	LL SITE	, LIFFORD,	•	Ground Level
Date and	Description		(Depth	Legend	Reduced	S	amples/Tes	ts
Water Level				(m)	2030/12	Level	Depth (m)	Sample Type	Test
	i. Obstruction time from 7.10m to 7 1.00 hour. Obstruction time from 8.40m to 8 1.50 hours. ii. Install standpipe to 9.00m.	40m for 90m for	den banden dan den den den den den den den den den de	hertis					,
emarks			s v c r	Stand Test Vane Core I Rock Desig	Sam ; ard Penetr Test Recovery (Quality nation (RQ	Die/Test K ation D %) W D %) U	ey Disturped Sampie Bulk Sampie Water Sampie Piston Fube	Job No 229). 3
			SF	PT: Whe bee	ere full 0.3r n achieved	n penetra: , the num	ion has not ber of blows	Figure	
	Sholl & Augory 200mm Diameter Case	1 to: 9 46m	÷	for t (not	he quoted N-value).	ponetratio	in is given	5	

APPENDIX 1

PERMEABILITY PERSON RESULTS

2

CHURCHTOWN LANDFILL SITE, LIFFORD

GROUND INVESTIGATION

APPENDIX 1

RESULTS OF RISING HEAD PERMEABILITY TEST

BOREHOLE NO 1

DATE: 27.8.98

	DEFITTED WATER SURFACE FROM GROUND LEVEL
<u> </u>	<u> </u>
	5./2
2	5,4/
3	5.27
4	5.14
5	5.03
6	4.93
7	۰ <u>4.86</u>
8	4.79
9	4.72
10	4.87% 2011
11	\$4,82
12	5 ¹¹² 013.57
13	100 ret 5.51
14	4.48
15	4.45
16	j. J. 4.43
17	4.39
18	4.36
19	4.33
20	4.31
21	4.28
22	4.26
23	4.24
24	4.22
25	4.20
26	4.19
27	4.18
28	417
29	416
30	4 1 5
31	414
<u></u>	4 13
V 4	4.10

DATE: 27.08.98

BOREHOLE NO 1 (continued)

TIME (mins)	DEPTH TO WATER SURFACE FROM GROUND LEVEL (m)
35	4.10
36	4.09
37	4.08
38	4.07
39	4.06
40	4.05
41	4.04
42	4.03
43	4.02
44	4.01
45	4.00
46	3.99
47	3.99
48	3.98
49	3.98
50	• 3.97
51	3.97
52	3.96. 001
53	3.8 & 2 ¹¹
54	<u>\$</u> 3,96
55	0 ¹¹¹ 10 ³ .95
56	
57	
58	<u>601 112 3.93</u>
59	<u> </u>
60	<u>8400 3.93</u>

COEFF. OF PERMEABILITY (k) = 3.3 X 10 -6 m/s

BOREHOLE NO 3

DATE: 2.09.98

TIME (mins)	DEPTH TO WATER SURFACE FROM GROUND LEVEL (m)
0	6.00
1	5.80
2	5.80
3	5.80
4	5.77
5-60	5.77

WATER FLOW TOO STRONG TO BALE OUT ANY DEEPER

COEFF. OF PERMEABILITY (k) = $4.0 \times 10^{-6} \text{ m/s}$

BOREHOLE NO 4

r

Υ

3

والمتبتية المتكالية وتشعرهم المعتين

DATE: 4.09.98

TIME (mins)	DEPTH TO WATER SURFACE FROM GROUND LEVEL (m
0	1.52
1	1.20
2	1.11
3	1.02
4	0.98
5	0.94
6	0.89
7	0.84
8	0.73
9	0.70
10	0.67
11	0.64
12	0.61
13	0.59
14	0.58
15	· 0.50
16	0.56
17	0.55
18	0.54.61
19	80.5 ¹
20	N.50
21	ction 0.49
22	115 10 0.48
23	For 5100 0.47
24	0.46
25	0,45
26	0.44
27	0.43
28	0.42
29	0.42
30	0.41
31	0.41
32	0.41
33	0.40
34	0.40
35	0.40
36	0.40
37	0.39
38	0.39
39	0.39
40	0.39
41	0.39
42	0.39

DATE: 4.09.98

BOREHOLE NO 4 (continued)

TIME (mins)	DEPTH TO WATER SURFACE FROM GROUND LEVEL (m)
43	0.38
44	0.38
45	0.58
46	0.38
47	0.38
48	0.37
49	0.37
50	0.37
51	0.37
52	0.37
53	0.37
54	0.36
55	0.36
56	, 0.36 es.
57	0.36
58	0.364. 2
59	0,38,61
60	0.36
COE	EFF. OF PERMEABILITY (R) = 2.6 X 10 -3 m/s

APPENDIX 2

CONSERVATION TEAMONT RESULTS



100mm

NOTES LG GRAS MONITORING POINTS L ELACHATE SAMPLING POINTS BH GROUNDWATER MONITORING POINTS BH GROUNDWATER MONITORING POINTS SW SUBFACE WATER MONITORING POINTS SW SUBFACE WATER MONITORING POINTS SW DUST MONITORING POINTS N NOISE MONITORING POINTS NO NORE ANG ON A GAR PROFILE LOCATION DAREM BY PINAM GREEK ANG ON A GAR PROFILE STEE AJS CIEUT DONEGAL COUNTY COUNCIL NONE COUNTORING ENGINEERS ELSCOBRETS NONE ON SECONSULTING ENGINEERS ELSCOBRETS ELMONCONCEST REDUCTING ELSCOBRETS ELMONCONCEST ELSCOBRETS ELMONCONCEST ELSCOBRETS ELMONCONCEST ELSCOBRETS ELMONCONCEST ELSCOBRETS					nt to wàsek Tideo _v iew	-	SWIT				.6	-			
	RPS Kirk McClure Morton consulting engineers ELMWOOD HOUSE 74 BOUCHER ROAD BELFAST BT12 RRZ TEL: 028 9066 7914 FAX: 028 9066 8286 www.kikmeduremoton.com ARCHITECT DRAMING No. 5234.63/107 DRELIM. FAX: 025 006 8286 DRAWING No. 5234.63/107 DRELIM. CONST. OWG. FAX: 025 006 8286	MONITORING POINTS	PROJECT CHURCHTOWN LANDFILL SITE	DONEGAL COUNTY COUNCIL	DRAWN BY PMCM CHECK BY AMCG APPROVED DJD DATE AUG 04 DATE AUG 04 DATE OCT 04 PLOT SCALE SCHEDULES SHEET SIZE A3	REV DESCRIPTION BY CHECK		LG NEW BOREHOLE LOCATION	n 🗢 noise monitoring points	DG 🗢 DUST MONITORING POINTS	SW 🗢 SURFACE WATER MONITORING	BH O GROUNDWATER MONITORING POINT	l 💛 leachate sampling points	LG GAS MONITORING POINTS	NOTES



A3C-REV 1 EPA Export 18-07-2019:03:58:53

APPENDIX D
































































APPENDIX E



Assimilative Capacity Assessment

Q (flow)=KiA	Darcy's Law		
I (hydraulic gradient)	0.019		conservative gradient between BH1 and BH3
k (hydraulic permeability)	3.70E-06	m/s	average between BH1 and BH3
A (cross section area of flux)	600	m^2	100m x 6m
Qflux (groundwater flux under landfill towards river)	4.22E-05	m^3/s	
Qflux	3.64E+00	m^3/day	
Qflux	3.64E+03	litres/day	
Qriver (River flow)	4.00E-01	m^3/sec	
Qriver	3.46E+04	m^3/day	
Qriver	3.46E+07	litres/day	
Groundwater Contaminatn Flux to River (Ammonical			
Nitrogen)	QxCd		
Cup(Sept 13) - Conc Ammoniacal nitrogen upstream in			
River	0.2	mg/l	
Custream (June 15) - Conc Ammoniacal nitrogen upstream			
in River	0.069	mg/l	
Cd (sept 13) - Ammoniacal Nitrogen @ BH1	0.3	mg/l	
Cd (June 15) - Ammoniacal Nitrogen @ BH1	0.97	mg/l	at the.
		A. A.	the second s
(Sept 13)	Cdown	S 110.2	
Predicted Ammoniacal Nitrogen Conc in river downstream	003	ited	
(june 15)	Cdown putted	1.04	
Actual Ammoniacal Nitrogen Conc in river downstream	Spectic winer		
(Sept 13)	Cdown (Sept 13)	0.2	
Actual Ammoniacal Nitrogen Conc in river downstream	FORMER		
(June 15)	Coown (June 15)	0.58	
River/Gwflux	9483.17	dilution ra	te
Predicted Reduction in Gflux (sept 15)	0.0001	mg/l	
% of reduced level relative to actual flux (sept 15)	0.01%	-	
Reduction in Gflux (June 13)	0.0000	mg/l	
% of reduced level relative to actual flux (June 13)	0.02%		

Appendix C

Surface Water Results

Consent of copyright owner required for any other use.

Location		Churchtown, Lifford, Co Donegal											
Sample Type							Surface	e Water					
Site No							SI	<i>N</i> 1					
Date of Sample		Jan-18	Feb-18	Mar-18	Apr-18	May-18	Jun-18	.lul-18	Aug-18	Sep-18	Oct-18	Nov-18	Dec-18
	1	oun ro	10010	mai ro	7.01.10	May 10	our ro	001 10	nug io	000 10	00010	1101 10	00010
nH		72					6 54					7.37	
Temp	C	13.8					14.5					9.6	
Electrical Conductivity	uS/cm	273					1329					222	
Ammonical Nitrogen	mg/l	0 409					20 252					0.099	
COD	mg/l	11					86					12	
BOD	mg/l	11					29					11	
Dissolved Oxygen	mg/l	11.03					1.46					10.87	
SS	mg/l	6					138					< 5	
Besidue on Evaporator	mg/l	0					100					~ 0	
Calcium	ug/l												
Cadmium													
Chromium								.Ø)*					
Chloride	mg/l	49.63					120	070				20	
Chlorine	mg/l	40.00					120					20	
Copper	ug/l	< 0.003					0.006					0 108	
Cvanide	mg/l	< 0.000				23						0.100	
Total Iron						0.00	5-0- 5-						
Lead						Ser of							
Magnesium						N N							
Magnesium					d d	CON CONT							
Manganese					100 0	<u> </u>							
Nickel	mg/l				el str								
Potassium	mg/l				5° 0 0								
Sodium	mg/l				1.00								
Sulphate	mg/l			- Xor	<u>~</u>								
Zinc	ug/l	1127		COV.			0.032					0.003	
Total Alkalinity as CaCO3	mg/l	112.1		, ð			0.002					0.000	
Total Organic Carbon	mg/l			off									
Total Oxidised Nitrogen	mg/l												
	mg/l												
Barium	mg/l												
Boron	ug/l												
Fluoride	ma/l												
Total Phenols	mg/l	< 0.1										< 0.001	
Phosphorous	mg/l												
Selenium	ma/l												
Silver	ma/l												
Mircrotox	oxic Unit	s											
Microtox	oxic Unit	s											
Nitrite	mg/l	< 0.005					< 0.005					< 0.013	
Nitrate	ma/l	5.9					1					6.78	
Phosphate - ORTHO	ma/l												
Phosphate - TOTAL	mg/l												
Total Coliforms													
Facel Coliforms													
Depth	m												

Location		Churchtown, Lifford, Co Donegal											
Sample Type							Surfac	e Water					
Site No							SI	N2					
Date of Sample		Jan-18	Feb-18	Mar-18	Apr-18	May-18	Jun-18	Jul-18	Aug-18	Sep-18	Oct-18	Nov-18	Dec-18
Lab No	I												
pH		7.69					7.06					7.1	
Temp	C	10.3					15.7					9.5	
Electrical Conductivity	uS/cm	96.1					1409					381	
Ammonical Nitrogen	ma/l	0.041					42.576					4.788	
COD	ma/l	28					73					20	
BOD	mg/l	1					8.8					1.5	
Dissolved Oxygen	mg/l	11.48					2.12					7.28	
SS	ma/l	6					28					5	
Residue on Evaporator	mg/l												
Calcium	ua/l												
Cadmium	ua/l												
Chromium	ug/l							<u>e</u> .					
Chloride	ma/l	29.78					55 🖌	07				26	
Chlorine	ma/l						these						
Copper	ua/l	< 0.003					0.013					0.022	
Cvanide	mg/l					23	ST.						
Total Iron	ua/l					50.80	5- 5-						
Lead	ua/l					Serge							
Magnesium	ua/l					Rit							
Manganese	ua/l				<u> </u>	reor							
Mercury	ua/l				il Ore								
Nickel	ma/l				e Ar								
Potassium	mg/l				FY XO								
Sodium	mg/l			a di F	28								
Sulphate	mg/l			¥.ó	5								
Zinc	ug/l	102		ిలి			0.002					0.002	
Total Alkalinity as CaCO3	mg/l			^v O ^v									
Total Organic Carbon	mg/l			en									
Total Oxidised Nitrogen	mg/l		-05	2									
Arsenic	mg/l		0										
Barium	mg/l												
Boron	ug/l												
Fluoride	mg/l												
Total Phenols	mg/l	< 0.1										1.1	
Phosphorous	mg/l												
Selenium	mg/l												
Silver	mg/l												
Mircrotox	oxic Unit	S											
Microtox	oxic Unit	s											
Nitrite	mg/l	< 0.005					0.095					0.061	
Nitrate	mg/l	0.52					1					5.55	
Phosphate - ORTHO	mg/l												
Phosphate - TOTAL	mg/l												
Total Coliforms													
Facel Coliforms													
Depth	m												

Location		Churchtown, Lifford, Co Donegal											
Sample Type							Surfac	e Water					
Site No							S	W3					
Date of Sample		Jan-18	Feb-18	Mar-18	Apr-18	May-18	Jun-18	Jul-18	Aug-18	Sep-18	Oct-18	Nov-18	Dec-18
Lab No	I												
pH		7.91					7 41			8.8		7 36	
Temp	C	99					17.6			14.1		9.5	
Electrical Conductivity	uS/cm	62.2					107.7			95.9		146.4	
Ammonical Nitrogen	ma/l	0.019					0.263			0.073		0.042	
COD	ma/l	21					72			30		29	
BOD	mg/l	1					1.1			< 1		1.7	
Dissolved Oxygen	ma/l	12.13					6.79			8.9		10.43	
SS	ma/l	6.5					44			15		< 5	
Residue on Evaporator	mg/l												
Calcium	ua/l									8			
Cadmium	ua/l									0			
Chromium	ug/l							<u>e</u> .		< 0.001			
Chloride	ma/l	19.85					21 💰	07		12		14	
Chlorine	ma/l						ines						
Copper	ua/l	< 0.003					0.014			12		0.016	
Cvanide	mg/l					23	ST.						
Total Iron	ua/l					50.80	\$ <u>`</u>			1.6			
Lead	ua/l					Serge				0.0005			
Magnesium	ua/l					C II				2			
Manganese	ua/l				<u> </u>	100				0.2			
Mercurv	ua/l				il ^{Or} e					0.00079			
Nickel	ma/l				e Ar								
Potassium	ma/l				EX.					1.1			
Sodium	ma/l			. Ar	.~~					9.6			
Sulphate	mg/l			× 5	5					6.4			
Zinc	ug/l	91.9		^د ی ،			0.02			5.6		0.003	
Total Alkalinity as CaCO3	mg/l			⁰						18			
Total Organic Carbon	mg/l			er									
Total Oxidised Nitrogen	mg/l		. 5	2						0.175			
Arsenic	mg/l		0										
Barium	mg/l												
Boron	ug/l												
Fluoride	mg/l												
Total Phenols	mg/l	< 0.1										< 0.001	
Phosphorous	mg/l												
Selenium	mg/l												
Silver	mg/l												
Mircrotox	oxic Unit	S											
Microtox	oxic Unit	S											
Nitrite	mg/l	< 0.005					< 0.005					< 0.013	
Nitrate	mg/l	0.16					1					0.67	
Phosphate - ORTHO	mg/l									< 0.009			
Phosphate - TOTAL	mg/l												
Total Coliforms													
Facel Coliforms													
Depth	m												

Location						Church	htown, Lif	ford, Co D	onegal				
Sample Type							Surfac	e Water					
Site No							SI	W4					
Date of Sample		Jan-18	Feb-18	Mar-18	Apr-18	May-18	Jun-18	Jul-18	Aug-18	Sep-18	Oct-18	Nov-18	Dec-18
Lab No	I												
Hq												7.15	
Temp	С											9.7	
Electrical Conductivity	uS/cm											236	
Ammonical Nitrogen	ma/l											0.728	
COD	ma/l											33	
BOD	ma/l											1.7	
Dissolved Oxygen	ma/l											9.95	
SS	ma/l											21	
Residue on Evaporator	ma/l												
Calcium	ug/l												
Cadmium	ua/l												
Chromium	ug/l							<u>ی</u> .					
Chloride	ma/l						4	0 ⁵				23	
Chlorine	ma/l						the						
Copper	ug/l						100					0.008	
Cvanide	ma/l					23	all.						
Total Iron	ua/l						5						
Lead	ug/l					Se d							
Magnesium	ua/l					N ST							
Manganese	ug/l				. Š	1000							
Mercury	ug/l				102 0								
Nickel	ma/l				-e Ar								
Potassium	ma/l				5 X OT								
Sodium	ma/l			A.	:00								
Sulphate	ma/l			80	5								
Zinc	ua/l			^{بر} ي .								0.011	
Total Alkalinity as CaCO3	ma/l			Ű, Őř									
Total Organic Carbon	ma/l			off									
Total Oxidised Nitrogen	ma/l		. 5	P									
Arsenic	ma/l		0										
Barium	ma/l												
Boron	ua/l												
Fluoride	ma/l												
Total Phenols	ma/l											< 0.001	
Phosphorous	ma/l												
Selenium	ma/l												
Silver	ma/l												
Mircrotox	oxic Unit	s											
Microtox	oxic Unit	s											
Nitrite	mg/l											0.013	
Nitrate	ma/l											0.68	
Phosphate - ORTHO	ma/l	1		1			1		1	1			1
Phosphate - TOTAL	ma/l												
Total Coliforms													
Facel Coliforms													
Depth	m												
2.300													

Location			Churchtown, Lifford, Co Donegal										
Sample Type							Surface	e Water					
Site No							SI	N5					
Date of Sample		Jan-18	Feb-18	Mar-18	Apr-18	May-18	Jun-18	Jul-18	Aug-18	Sep-18	Oct-18	Nov-18	Dec-18
Lab No	Ι												
DH												7.23	
Temp	C											9.4	
Electrical Conductivity	uS/cm											147	
Ammonical Nitrogon	mall											0.031	
	mg/l											27	
BOD	mg/l											1 2	
Disselved Ovygen	mg/l											1.3	
Dissolved Oxygen	mg/i											10.0	
33	mg/i											1	
Residue on Evaporator	mg/i												
	ug/i												
Cadmium	ug/i												
Chromium	ug/l							. <u></u>					
Chloride	mg/l						- A	~				14	
Chlorine	mg/l						lle						
Copper	ug/l						E.					0.007	
Cyanide	mg/l					- III	- Str.						
Total Iron	ug/l						*						
Lead	ug/l					of eq							
Magnesium	ug/l					P JII							
Manganese	ug/l				~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	100							
Mercury	ug/l				ilor e								
Nickel	mg/l				e wh								
Potassium	mg/l				France Contraction								
Sodium	mg/l			. A .	.~??								
Sulphate	mg/l			5 5	5								
Zinc	ua/l			*ى ،								0.003	
Total Alkalinity as CaCO3	ma/l			, Ôr									
Total Organic Carbon	mg/l			off									
Total Oxidised Nitrogen	ma/l			2									
Arsenic	ma/l		0										
Barium	mg/l												
Boron	ua/l												
Fluoride	mg/l												
Total Phenois	mg/l											< 0.001	
Phosphorous	mg/l											\$ 0.001	
Selenium	mg/l												
Silvor	mg/l												
Mircrotov	Oxic Unit	s											
Microtox	Toxic Unit												
Nitrito	ma/l	3										< 0.012	
Nitrato	mg/l											0.02	
Phoenbate OPTUO	mg/l											0.93	
Phosphate - UK THU	mg/l												
Phosphate - TOTAL	mg/l												
Facel Collforms													
Depth	m												

Location		Churchtown, Lifford, Co Donegal											
Sample Type							Surfac	e Water					
Site No							SI	N6					
Date of Sample		Jan-18	Feb-18	Mar-18	Apr-18	May-18	Jun-18	Jul-18	Aug-18	Sep-18	Oct-18	Nov-18	Dec-18
Lab No	1					,			Ŭ				
pH												7.18	
Temp	С											9.4	
Electrical Conductivity	uS/cm											142.1	
Ammonical Nitrogen	mg/l											0.061	
COD	mg/l											326	
BOD	mg/l											1.2	
Dissolved Oxygen	ma/l											10.47	
SS	mg/l											400	
Residue on Evaporator	mg/l												
Calcium	ug/l												
Cadmium	ug/l												
Chromium	ug/l							<u>ی</u> و.					
Chloride	mg/l						Å	0				14	
Chlorine	mg/l						illo.						
Copper	ug/l						100					0.047	
Cvanide	ma/l					27	Sr.						
Total Iron	ua/l						×						
Lead	ug/l					50.00							
Magnesium	ug/l					Rat							
Manganese	ug/l				<u> </u>	, eos							
Mercury	ug/l				ior e								
Nickel	ma/l				e Ar								
Potassium	ma/l				R.X.								
Sodium	ma/l				S.								
Sulphate	ma/l			Y of									
Zinc	ug/l			ک ې د ک								0.203	
Total Alkalinity as CaCO3	ma/l			¹ 0 ¹									
Total Organic Carbon	ma/l			est.									
Total Oxidised Nitrogen	ma/l		, all	ľ									
Arsenic	ma/l		C,										
Barium	mg/l												
Boron	ug/l												
Fluoride	mg/l												
Total Phenols	mg/l											< 0.001	
Phosphorous	mg/l												
Selenium	mg/l												
Silver	mg/l												
Mircrotox	oxic Unit	s											
Microtox	oxic Unit	s											
Nitrite	mg/l											0.019	
Nitrate	mg/l											0.69	
Phosphate - ORTHO	mg/l												
Phosphate - TOTAL	mg/l												
Total Coliforms													
Facel Coliforms													
Depth	m												

Location		Churchtown, Lifford, Co Donegal											
Sample Type							Surfac	e Water					
Site No							SI	N7					
Date of Sample		Jan-18	Feb-18	Mar-18	Apr-18	May-18	Jun-18	Jul-18	Aug-18	Sep-18	Oct-18	Nov-18	Dec-18
Lab No	I												
pH		8.37					7.3			7.61		7.17	
Temp	C	9.7					17.7			14		9.5	
Electrical Conductivity	uS/cm	62.8					107.5			94.8		147.3	
Ammonical Nitrogen	ma/l	0.016					0.305			0.071		0.04	
COD	ma/l	21					101			27		28	
BOD	mg/l	1					1.7			< 1		1.5	
Dissolved Oxygen	mg/l	12.14					6.9			9.11		10.82	
SS	mg/l	6					96			< 5		12	
Residue on Evaporator	mg/l												
Calcium	ua/l									8.4			
Cadmium	ua/l									< 0.0001			
Chromium	ug/l							<u>e</u> .		< 0.001			
Chloride	ma/l	22.83					16 🧹	07		14		14	
Chlorine	ma/l						these						
Copper	ua/l	< 0.003					0.017			11		0.004	
Cvanide	mg/l					23	ST.						
Total Iron	ua/l					50.80	5- 5-			1.3			
Lead	ua/l					Serge				0.0004			
Magnesium	ua/l					Rit				2			
Manganese	ua/l				<u> </u>	reor				0.1			
Mercury	ua/l				i Or e					0.00027			
Nickel	ma/l				e Ar								
Potassium	ma/l				FY XO					1			
Sodium	ma/l			. Ar	28					9.1			
Sulphate	mg/l			× 5	5					7.1			
Zinc	ug/l	6.1		^د ی ،			0.03			3.4		0.006	
Total Alkalinity as CaCO3	mg/l			⁰						20			
Total Organic Carbon	mg/l			er									
Total Oxidised Nitrogen	mg/l		. 5	2						< 0.138			
Arsenic	mg/l		0										
Barium	mg/l												
Boron	ug/l												
Fluoride	mg/l												
Total Phenols	mg/l	< 0.1										< 0.001	
Phosphorous	mg/l												
Selenium	mg/l												
Silver	mg/l												
Mircrotox	oxic Unit	S											
Microtox	oxic Unit	s											
Nitrite	mg/l	< 0.005					< 0.005					< 0.013	
Nitrate	mg/l	< 0.15					3.2					0.82	
Phosphate - ORTHO	mg/l									< 0.009			
Phosphate - TOTAL	mg/l												
Total Coliforms													
Facel Coliforms													
Depth	m												

Appendix D

Discharged Effluent Results

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Churchtown ICW and Willows Monitoring

Sample	Station	Sample Date	mg/l
192500033	Pond 1 outfall	08 January 2019	45.2
192500034	Pond 1A outfall	08 January 2019	16.8
192500035	Pond 2A Outfall	08 January 2019	1.47
192500036	Pond 3 A Outfall	08 January 2019	1.03
192500037	Pond 4 A Outfall	08 January 2019	0.031
192500038	Pond 5 A outfall	08 January 2019	0.008
192500039	Pond 1B Outfall	08 January 2019	32.7
192500040	Pond 2B Outfall	08 January 2019	0.368
192500041	Pond 3B Outfall	08 January 2019	0.043
192500042	Pond 4B Outfall	08 January 2019	0.003
192500043	Pond 5B Discharge Monitoring Point.	08 January 2019	0.008
192500044	Northern Willow Discharge Monitoring Point.	08 January 2019	0.013
192500046	Southern Willow Discharge Monitoring Point	08 January 2019	0.358
192500177	Pond 1 outfall	18 January 2019	52.5
192500178	Pond 1A outfall	18 January 2019	34.9
192500179	Pond 2A Outfall	18 January 2019	3.21
192500180	Pond 3 A Outfall	18 January 2019	0.546
192500181	Pond 4 A Outfall	18 January 2019	0.31
192500182	Pond 5 A outfall	🕺 January 2019	0.042
192500183	Pond 1B Outfall	18 January 2019	41
192500184	Pond 2B Outfall	18 January 2019	3.34
192500185	Pond 3B Outfall	18 January 2019	0.079
192500186	Pond 4B Outfall	18 January 2019	0.007
192500187	Pond 5B Discharge Monitoring Point	18 January 2019	0.004
	inst to		
192500188	Northern Willow Discharge Monitoring Point.	18 January 2019	0.04
192500189	Southern Willow Discharge Monitoring Point	18 January 2019	0.46
	ent		
192500367	Pond 1 outfall	25 January 2019	12.1
192500368	Pond 1A outfall	25 January 2019	19.5
192500369	Pond 2A Outfall	25 January 2019	11.7
192500370	Pond 3 A Outfall	25 January 2019	1.89
192500371	Pond 4 A Outfall	25 January 2019	0.8
192500372	Pond 5 A outfall	25 January 2019	0.6
192500373	Pond 1B Outfall	25 January 2019	12.3
192500374	Pond 2B Outfall	25 January 2019	11.8
192500375	Pond 3B Outfall	25 January 2019	1.08
192500376	Pond 4B Outfall	25 January 2019	0.122
192500377	Pond 5B Discharge Monitoring Point.	25 January 2019	0.009
192500378	Northern Willow Discharge Monitoring Point.	25 January 2019	0.06
192500379	Southern Willow Discharge Monitoring Point	25 January 2019	0.45
192500422	Pond 1 outfall	01 February 2019	18.7
192500423	Pond 1A outfall	01 February 2019	4.87
192500424	Pond 2A Outfall	01 February 2019	0.085
192500425	Pond 3 A Outfall	01 February 2019	0.014
192500426	Pond 4 A Outfall	01 February 2019	0.035
192500427	Pond 5 A outfall	01 February 2019	0.123
192500428	Pond 1B Outfall	01 February 2019	20.8

192500429	Pond 2B Outfall	01 February 2019	3.98
192500430	Pond 3B Outfall	01 February 2019	2.47
192500431	Pond 4B Outfall	01 February 2019	0.453
192500432	Pond 5B Discharge Monitoring Point.	01 February 2019	0.031
192500433	Northern Willow Discharge Monitoring Point.	01 February 2019	0.07
192500434	Southern Willow Discharge Monitoring Point	01 February 2019	0.65
192500600	Pond 1 outfall	07 February 2019	12.3
192500601	Pond 1A outfall	07 February 2019	6.23
192500602	Pond 2A Outfall	07 February 2019	0.492
192500603	Pond 3 A Outfall	07 February 2019	0.015
192500604	Pond 4 A Outfall	07 February 2019	0.019
192500605	Pond 5 A outfall	07 February 2019	0.018
192500606	Pond 1B Outfall	07 February 2019	13.4
192500607	Pond 2B Outfall	07 February 2019	0.312
192500608	Pond 3B Outfall	07 February 2019	0.369
192500609	Pond 4B Outfall	07 February 2019	0.15
192500610	Pond 5B Discharge Monitoring Point.	07 February 2019	0.012
192500611	Northern Willow Discharge Monitoring Point.	07 February 2019	0.06
192500612	Southern Willow Discharge Monitoring Point	07 February 2019	0.82
		15 ⁰ .	
192500680	Pond 1 outfall	A February 2019	7.24
192500681	Pond 1A outfall	13 February 2019	0.1
192500682	Pond 2A Outfall	13 February 2019	0.023
192500683	Pond 3 A Outfall	13 February 2019	0.015
192500684	Pond 4 A Outfall	13 February 2019	0.021
192500685	Pond 5 A outfall	13 February 2019	0.023
192500686	Pond 1B Outfall	13 February 2019	4.13
192500687	Pond 2B Outfall	13 February 2019	0.302
192500688	Pond 3B Outfall	13 February 2019	0.299
192500689	Pond 4B Outfall	13 February 2019	0.021
192500690	Pond 5B Discharge Monitoring Point.	13 February 2019	0.012
192500691	Northern Willow Discharge Monitoring Point.	13 February 2019	0.07
192500692	Southern Willow Discharge Monitoring Point	13 February 2019	0.62
192500828	Pond 1 outfall	01 March 2019	64.4
192500829	Pond 1A outfall	01 March 2019	34.7
192500830	Pond 2A Outfall	01 March 2019	8.47
192500831	Pond 3 A Outfall	01 March 2019	0.303
192500832	Pond 4 A Outfall	01 March 2019	0.104
192500833	Pond 5 A outfall	01 March 2019	0.042
192500834	Pond 1B Outfall	01 March 2019	35.5
192500835	Pond 2B Outfall	01 March 2019	0.436
192500836	Pond 3B Outfall	01 March 2019	0.079
192500837	Pond 4B Outfall	01 March 2019	0.071
192500838	Pond 5B Discharge Monitoring Point.	01 March 2019	0.02
192500839	Northern Willow Discharge Monitoring Point.	01 March 2019	0.011
192500840	Southern Willow Discharge Monitoring Point	01 March 2019	0.015
192501056	Pond 1 outfall	08 March 2019	26.4
192501057	Pond 1A outfall	08 March 2019	31.5

192501058	Pond 2A Outfall	08 March 2019	9.8
192501059	Pond 3 A Outfall	08 March 2019	1.902
192501060	Pond 4 A Outfall	08 March 2019	0.045
192501061	Pond 5 A outfall	08 March 2019	0.02
192501062	Pond 1B Outfall	08 March 2019	18.1
192501063	Pond 2B Outfall	08 March 2019	8.62
192501064	Pond 3B Outfall	08 March 2019	0.069
192501065	Pond 4B Outfall	08 March 2019	0.022
192501066	Pond 5B Discharge Monitoring Point.	08 March 2019	0.011
192501067	Northern Willow Discharge Monitoring Point.	08 March 2019	0.011
192501068	Southern Willow Discharge Monitoring Point	08 March 2019	0.126
192501171	Pond 1 outfall	19 March 2019	7.42
192501172	Pond 1A outfall	19 March 2019	0.022
192501173	Pond 2A Outfall	19 March 2019	0.027
192501174	Pond 3 A Outfall	19 March 2019	0.027
192501175	Pond 4 A Outfall	19 March 2019	0.026
192501176	Pond 5 A outfall	19 March 2019	0.025
192501177	Pond 1B Outfall	19 March 2019	6.44
192501178	Pond 2B Outfall	19 March 2019	0.294
192501179	Pond 3B Outfall	19 March 2019	0.081
192501180	Pond 4B Outfall	19 March 2019	0.04
192501181	Pond 5B Discharge Monitoring Point.	💉 19 March 2019	0.069
	4. d	б	
192501182	Northern Willow Discharge Monitoring Point	19 March 2019	0.26
192501183	Southern Willow Discharge Monitoring Point	19 March 2019	0.18
	ALL STREET		
	at for		
192501318	Pond 1 outfall	29 March 2019	54.6
192501318 192501319	Pond 1 outfall	29 March 2019 29 March 2019	54.6 13.1
192501318 192501319 192501320	Pond 1 outfall point for the section of the section	29 March 2019 29 March 2019 29 March 2019	54.6 13.1 0.128
192501318 192501319 192501320 192501321	Pond 1 outfall Clowner Pond 1A outfall insertion Pond 2A Outfall Yourfall Pond 3 A Outfall Yourfall	29 March 2019 29 March 2019 29 March 2019 29 March 2019	54.6 13.1 0.128 0.036
192501318 192501319 192501320 192501321 192501322	Pond 1 outfall potential Pond 1A outfall Instruction for the second sec	29 March 2019 29 March 2019 29 March 2019 29 March 2019 29 March 2019	54.6 13.1 0.128 0.036 0.035
192501318 192501319 192501320 192501321 192501322 192501323	Pond 1 outfall construction for the construction of the	29 March 2019 29 March 2019 29 March 2019 29 March 2019 29 March 2019 29 March 2019	54.6 13.1 0.128 0.036 0.035 0.028
192501318 192501319 192501320 192501321 192501322 192501323 192501324	Pond 1 outfall ction for the company of the company	29 March 2019 29 March 2019 29 March 2019 29 March 2019 29 March 2019 29 March 2019 29 March 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7
192501318 192501320 192501320 192501321 192501322 192501323 192501324 192501325	Pond 1 outfall protection for the constraint Pond 1A outfall instruction for the constraint Pond 2A Outfall For protection for the constraint Pond 3 A Outfall for protection for the constraint Pond 4 A Outfall constraint Pond 5 A outfall constraint Pond 1B Outfall Pond 2B Outfall	29 March 2019 29 March 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146
192501318 192501319 192501320 192501321 192501322 192501323 192501324 192501325 192501326	Pond 1 outfall construction Pond 1A outfall instruction Pond 2A Outfall Fortune Pond 3 A Outfall construction Pond 4 A Outfall construction Pond 5 A outfall construction Pond 1B Outfall Pond 2B Outfall Pond 3B Outfall Pond 3B Outfall	29 March 2019 29 March 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025
192501318 192501320 192501320 192501322 192501323 192501323 192501324 192501325 192501326 192501327	Pond 1 outfall Construction Pond 1A outfall Instruction Pond 2A Outfall Construction Pond 3 A Outfall Construction Pond 4 A Outfall Construction Pond 5 A outfall Construction Pond 1B Outfall Pond 1B Outfall Pond 3B Outfall Pond 3B Outfall	29 March 2019 29 March 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024
192501318 192501320 192501320 192501321 192501323 192501323 192501325 192501325 192501326 192501327 192501328	Pond 1 outfall Pond 1A outfall Pond 1A outfall Pond 1A outfall Pond 2A Outfall Pond 1A outfall Pond 3 A Outfall Pond 1A outfall Pond 4 A Outfall Pond 1A outfall Pond 5 A outfall Pond 1B Outfall Pond 1B Outfall Pond 2B Outfall Pond 3B Outfall Pond 3B Outfall Pond 4B Outfall Pond 5B Discharge Monitoring Point.	29 March 2019 29 March 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02
192501318 192501320 192501321 192501322 192501323 192501324 192501325 192501326 192501327 192501328	Pond 1 outfall perturnet Pond 1A outfall perturnet Pond 2A Outfall Formation Pond 3 A Outfall formation Pond 3 A Outfall formation Pond 4 A Outfall formation Pond 5 A outfall control Pond 1B Outfall control Pond 2B Outfall Pond 3B Outfall Pond 3B Outfall Pond 4B Outfall Pond 5B Discharge Monitoring Point. Point.	29 March 2019 29 March 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02
192501318 192501320 192501320 192501322 192501323 192501324 192501325 192501326 192501327 192501328 192501328	Pond 1 outfall point for the construction of the constructio	29 March 2019 29 March 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.024 0.02
192501318 192501320 192501321 192501322 192501323 192501323 192501325 192501326 192501327 192501328 192501328	Pond 1 outfall Pond 1A outfall Pond 1A outfall Pond 1A outfall Pond 2A Outfall Pond 1A outfall Pond 3 A Outfall Pond 1A outfall Pond 4 A Outfall Pond 1A outfall Pond 5 A outfall Pond 1B Outfall Pond 1B Outfall Pond 1B Outfall Pond 3B Outfall Pond 3B Outfall Pond 3B Outfall Pond 3B Outfall Pond 5B Discharge Monitoring Point. Pond 5B Outfall	29 March 2019 29 March 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.852 0.011
192501318 192501320 192501321 192501322 192501323 192501324 192501325 192501326 192501327 192501328 192501328 192501329 192501330	Pond 1 outfall Pond 1A outfall Pond 1A outfall Pond 1A outfall Pond 2A Outfall Pond 1A outfall Pond 3 A Outfall Pond 1A outfall Pond 4 A Outfall Pond 1A outfall Pond 5 A outfall Pond 1B Outfall Pond 1B Outfall Pond 1B Outfall Pond 3B Outfall Pond 3B Outfall Pond 4B Outfall Pond 5B Discharge Monitoring Point. Southern Willow Discharge Monitoring Point Southern Willow Discharge Monitoring Point	29 March 2019 29 March 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.024 0.02 0.024 0.02
192501318 192501329 192501320 192501322 192501323 192501323 192501326 192501326 192501327 192501328 192501328 192501329 192501329 192501330	Pond 1 outfall point for the construction of the constructio	29 March 2019 29 March 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.852 0.011 62
192501318 192501319 192501320 192501321 192501323 192501323 192501324 192501326 192501326 192501328 192501329 192501329 192501330 192501442 192501443	Pond 1 outfall point for the constraint of the constrating the constraint of the constraint of the constraint	29 March 2019 29 March 2019 12 April 2019 12 April 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.852 0.011 62 43.7
192501318 192501320 192501321 192501322 192501323 192501323 192501325 192501326 192501327 192501328 192501329 192501329 192501329 192501442 192501443	Pond 1 outfall Pond 1A outfall Pond 1A outfall Forther production of the product of the pro	29 March 2019 29 March 2019 12 April 2019 12 April 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.852 0.011 62 43.7 24.3
192501318 192501320 192501320 192501321 192501323 192501323 192501324 192501326 192501327 192501327 192501328 192501329 192501330 192501442 192501444 192501444	Pond 1 outfall Pond 1A outfall Pond 1A outfall Pond 1A outfall Pond 2A Outfall Pond 1A outfall Pond 3 A Outfall Pond 1A outfall Pond 3 A Outfall Pond 1A outfall Pond 4 A Outfall Pond 1A outfall Pond 5 A outfall Pond 1B Outfall Pond 1B Outfall Pond 2B Outfall Pond 3B Outfall Pond 3B Outfall Pond 4B Outfall Pond 5B Discharge Monitoring Point. Northern Willow Discharge Monitoring Point. Southern Willow Discharge Monitoring Point. Pond 1 outfall Pond 2A Outfall Pond 2A Outfall Pond 3A Outfall	29 March 2019 29 March 2019 12 April 2019 12 April 2019 12 April 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.024 0.025 0.024 0.02 0.852 0.011 62 43.7 24.3 11.6
192501318 192501320 192501320 192501322 192501323 192501323 192501326 192501326 192501326 192501328 192501328 192501329 192501330 192501442 192501443 192501445 192501445	Pond 1 outfall Pond 1A outfall Pond 1A outfall Pond 1A outfall Pond 2A Outfall Pond 1A outfall Pond 3 A Outfall Pond 1A outfall Pond 4 A Outfall Pond 1A outfall Pond 5 A outfall Pond 1B Outfall Pond 1B Outfall Pond 1B Outfall Pond 3B Outfall Pond 3B Outfall Pond 3B Outfall Pond 3B Outfall Pond 5B Discharge Monitoring Point. Pond 1B Outfall Pond 1 outfall Pond 1 outfall Pond 1 outfall Pond 1 outfall Pond 1 outfall Pond 1 outfall Pond 3 A Outfall Pond 1 outfall Pond 4 A Outfall Pond 4 A Outfall	29 March 2019 29 March 2019 12 April 2019 12 April 2019 12 April 2019 12 April 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.852 0.011 62 43.7 24.3 11.6 4.8
192501318 192501320 192501321 192501322 192501323 192501323 192501324 192501326 192501327 192501328 192501329 192501329 192501329 192501442 192501443 192501444 192501444 192501444	Pond 1 outfall Pond 1A outfall Pond 1A outfall Forestard Pond 2A Outfall Forestard Pond 3 A Outfall Forestard Pond 3 A Outfall Forestard Pond 4 A Outfall Forestard Pond 5 A outfall Forestard Pond 1B Outfall Forestard Pond 1B Outfall Forestard Pond 2B Outfall Forestard Pond 3B Outfall Forestard Pond 4B Outfall Forestard Pond 5B Discharge Monitoring Point. Forestard Northern Willow Discharge Monitoring Point. Forestard Southern Willow Discharge Monitoring Point Forestard Pond 1 outfall Forestard Pond 1 outfall Forestard Pond 1 outfall Forestard Pond 1 A outfall Forestard Pond 3 A Outfall Forestard Pond 4 A Outfall Forestard Pond 5 A outfall Forestard Pond 5 A outfall Forestard	29 March 2019 29 March 2019 12 April 2019 12 April 2019 12 April 2019 12 April 2019 12 April 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.02 0.011 62 43.7 24.3 11.6 4.8 0.07
192501318 192501320 192501320 192501321 192501323 192501323 192501324 192501326 192501327 192501327 192501328 192501329 192501330 192501443 192501443 192501444 192501445 192501447 192501447	Pond 1 outfall Pond 1A outfall Pond 1A outfall Forther Pond 2A Outfall Pond 2A Outfall Forther Pond 2A Outfall Pond 3 A Outfall Forther Pond 2A Outfall Pond 4 A Outfall Forther Pond 5 A outfall Pond 1B Outfall Forther Pond 2B Outfall Pond 2B Outfall Forther Pond 3B Outfall Pond 3B Outfall Pond 3B Outfall Pond 4B Outfall Pond 5B Discharge Monitoring Point. Northern Willow Discharge Monitoring Point. Southern Willow Discharge Monitoring Point Pond 1 outfall Pond 1 outfall Pond 2A Outfall Pond 1 outfall Pond 3 A Outfall Pond 1 outfall Pond 1 outfall Pond 3 A Outfall Pond 3 A Outfall Pond 3 A Outfall Pond 5 A outfall Pond 5 A outfall Pond 5 A outfall Pond 5 A outfall Pond 1B Outfall Pond 5 A outfall	29 March 2019 29 March 2019 12 April 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.852 0.011 62 43.7 24.3 11.6 4.8 0.07 62.2
192501318 192501320 192501321 192501323 192501323 192501324 192501325 192501326 192501327 192501328 192501329 192501329 192501320 192501328 192501329 192501320 192501320 192501320 192501442 192501444 1925014445 1925014446 1925014448 1925014448 1925014448 1925014448	Pond 1 outfall Pond 1A outfall Pond 1A outfall Fortunation Pond 2A Outfall Fortunation Pond 3 A Outfall Fortunation Pond 3 A Outfall Fortunation Pond 4 A Outfall Fortunation Pond 5 A outfall Fortunation Pond 5 A outfall Control Pond 1B Outfall Fortunation Pond 2B Outfall Pond 2B Outfall Pond 3B Outfall Pond 3B Outfall Pond 5B Discharge Monitoring Point. Pond 5B Discharge Monitoring Point. Southern Willow Discharge Monitoring Point Pond 1 outfall Pond 1 outfall Pond 2A Outfall Pond 1 outfall Pond 3 A Outfall Pond 3 A Outfall Pond 3 A Outfall Pond 3 A Outfall Pond 3 A Outfall Pond 5 A outfall Pond 2A Outfall Pond 5 A outfall Pond 2B Outfall Pond 1B Outfall Pond 2B Outfall	29 March 2019 29 March 2019 12 April 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.024 0.025 0.011 62 43.7 24.3 11.6 4.8 0.07 62.2 12.9
192501318 192501319 192501320 192501321 192501323 192501324 192501325 192501326 192501327 192501328 192501329 192501329 192501329 192501329 192501329 192501442 192501443 192501444 192501444 192501444 192501444 192501444 192501444 192501445 192501445 192501445 192501445 192501445 192501445 192501445 192501445	Pond 1 outfall Pond 1A outfall Pond 1A outfall Forther production of the product of the pro	29 March 2019 29 March 2019 12 April 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.024 0.02 0.011 62 43.7 24.3 11.6 4.8 0.07 62.2 12.9 0.035
192501318 192501320 192501321 192501323 192501323 192501323 192501324 192501325 192501326 192501327 192501328 192501329 192501329 192501329 192501329 192501442 192501443 192501443 192501444 192501445 192501445 192501445 192501445 192501445 192501445	Pond 1 outfall Pond 1A outfall Pond 1A outfall Forther for the former of th	29 March 2019 29 March 2019 12 April 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.024 0.02 0.852 0.011 62 43.7 24.3 11.6 4.8 0.07 62.2 12.9 0.035 0.042
192501318 192501320 192501321 192501323 192501323 192501324 192501325 192501326 192501327 192501328 192501327 192501328 192501329 192501329 192501329 192501329 192501329 192501329 192501442 192501443 192501444 192501445 192501447 192501448 192501448 192501445 192501445 192501445 192501445 192501445 192501445 192501445	Pond 1 outfall Pont the formation of the form	29 March 2019 29 March 2019 12 April 2019	54.6 13.1 0.128 0.036 0.035 0.028 31.7 0.146 0.025 0.024 0.02 0.024 0.02 0.024 0.02 0.024 0.02 0.024 0.02 0.024 0.02 0.024 0.02 0.025 0.011 62 43.7 24.3 11.6 4.8 0.07 62.2 12.9 0.035 0.042 0.015

192501453	Northern Willow Discharge Monitoring Point.	12 April 2019	0.008
192501454	Southern Willow Discharge Monitoring Point	12 April 2019	0.006
192501613	Pond 1 outfall	16 April 2019	41.5
192501614	Pond 1A outfall	16 April 2019	29.3
192501615	Pond 2A Outfall	16 April 2019	27
192501616	Pond 3 A Outfall	16 April 2019	12.6
192501617	Pond 4 A Outfall	16 April 2019	2.9
192501618	Pond 5 A outfall	16 April 2019	0.029
192501619	Pond 1B Outfall	16 April 2019	59.6
192501620	Pond 2B Outfall	16 April 2019	14.1
192501621	Pond 3B Outfall	16 April 2019	0.031
192501622	Pond 4B Outfall	16 April 2019	0.015
192501623	Pond 5B Discharge Monitoring Point.	16 April 2019	0.015
192501624	Northern Willow Discharge Monitoring Point.	16 April 2019	0.015
192501625	Southern Willow Discharge Monitoring Point	16 April 2019	0.015
192501636	Pond 1 outfall	24 April 2019	74
192501637	Pond 1A outfall	24 April 2019	38.4
192501638	Pond 2A Outfall	24 April 2019	16.1
192501639	Pond 3 A Outfall	24 April 2019	3.09
192501640	Pond 4 A Outfall	24 April 2019	0.545
192501641	Pond 5 A outfall	24 April 2019	0.034
192501642	Pond 1B Outfall	24 April 2019	63.6
192501643	Pond 2B Outfall	24 April 2019	15.7
192501644	Pond 3B Outfall	24 April 2019	0.03
192501645	Pond 4B Outfall	24 April 2019	0.02
192501646	Pond 5B Discharge Monitoring Point	24 April 2019	0.018
	in the		
192501627	Northern Willow Discharge Monitoring Point.	24 April 2019	2.48
192501628	Southern Willow Discharge Monitoring Point	24 April 2019	0.015
	a la still attention		
	Pond 1 outfall		
	Pond 1A outfall		
	Pond 2A Outfall		
	Pond 3 A Outfall		
	Pond 4 A Outrall	24 May 2010	No comulo oveilable
	Pond 5 A outrall	24 May 2019	No sample available
	Pond 3B Outfall		
102501070	Pond 58 Discharge Monitoring Point	24 May 2010	0.02
192301970		24 May 2019	0.02
102501071	Northern Willow Discharge Monitoring Point	24 May 2019	0.093
152501571	Southern Willow Discharge Monitoring Point	24 May 2013	No sample available
192502077	Pond 1 outfall	30 May 2019	74.8
192502077	Pond 1A outfall	30 May 2019	12 3
192502073	Pond 2A Outfall	30 May 2019	3 04
192502080	Pond 3 A Outfall	30 May 2019	0.043
192502081	Pond 4 A Outfall	30 May 2019	0.033
192502082	Pond 5 A outfall	30 May 2019	0.024
192502083	Pond 1B Outfall	30 May 2019	53

192502084	Pond 2B Outfall	30 May 2019	1.92		
192502085	Pond 3B Outfall	30 May 2019	0.02		
192502086	Pond 4B Outfall	30 May 2019	0.016		
192502087	Pond 5B Discharge Monitoring Point.	30 May 2019	0.02		
192502088	Northern Willow Discharge Monitoring Point.	30 May 2019	0.051		
192502089	Southern Willow Discharge Monitoring Point	30 May 2019	0.019		
		-			
192502274	Pond 1 outfall	07 May 2019	30.4		
192502275	Pond 1A outfall	07 May 2019	18.1		
192502276	Pond 2A Outfall	07 May 2019	6.46		
192502277	Pond 3 A Outfall	07 May 2019	0.501		
192502278	Pond 4 A Outfall	07 May 2019	0.158		
192502279	Pond 5 A outfall	07 May 2019	0.033		
192502280	Pond 1B Outfall	07 May 2019	24		
192502281	Pond 2B Outfall	07 May 2019	14.4		
192502282	Pond 3B Outfall	07 May 2019	0.34		
192502283	Pond 4B Outfall	07 May 2019	0.023		
192502284	Pond 5B Discharge Monitoring Point.	07 May 2019	0.016		
192502285	Northern Willow Discharge Monitoring Point.	07 May 2019	0.016		
192502286	Southern Willow Discharge Monitoring Point	07 May 2019	0.026		
		150.			
192502385	Pond 1 outfall	💉 14 June 2019	46.8		
192502386	Pond 1A outfall	14 June 2019	8.77		
192502387	Pond 2A Outfall	14 June 2019	2.04		
192502388	Pond 3 A Outfall	14 June 2019	0.038		
192502389	Pond 4 A Outfall	14 June 2019	0.133		
192502390	Pond 5 A outfall	14 June 2019	0.026		
192502391	Pond 1B Outfall	14 June 2019	38.4		
192502392	Pond 2B Outfall	14 June 2019	3.55		
192502393	Pond 3B Outfall	14 June 2019	0.032		
192502394	Pond 4B Outfall	14 June 2019	0.049		
192502395	Pond 5B Discharge Monitoring Point.	14 June 2019	0.024		
192502396	Northern Willow Discharge Monitoring Point.	14 June 2019	0.1		
192502397	Southern Willow Discharge Monitoring Point	14 June 2019	0.015		
192502447	Pond 1 outfall	20 June 2019	86.6		
192502448	Pond 1A outfall	20 June 2019	10.1		
192502449	Pond 2A Outfall	20 June 2019	0.874		
192502450	Pond 3 A Outfall	20 June 2019	0.033		
192502451	Pond 4 A Outfall	20 June 2019	0.033		
192502452	Pond 5 A outfall	20 June 2019	0.023		
192502453	Pond 1B Outfall	20 June 2019	55		
192502454	Pond 2B Outfall	20 June 2019	0.677		
192502455	Pond 3B Outfall	20 June 2019	0.047		
192502456	Pond 4B Outfall	20 June 2019	0.025		
192502457	Pond 5B Discharge Monitoring Point.	20 June 2019	0.015		
192502458	Northern Willow Discharge Monitoring Point.	20 June 2019	0.674		
192502459	Southern Willow Discharge Monitoring Point	20 June 2019	0.021		
	Pond 1 outfall				
	Pond 1A outfall				

	Pond 2A Outfall		
	Pond 3 A Outfall		
	Pond 4 A Outfall		
	Pond 5 A outfall	26 June 2019	0.038
	Pond 1B Outfall		
	Pond 2B Outfall		
	Pond 3B Outfall		
	Pond 4B Outfall		
No sample	Pond 5B Discharge Monitoring Point.	26 June 2019	No discharge
	Northern Willow Discharge Monitoring Point.	26 June 2019	<0.015
No sample	Southern Willow Discharge Monitoring Point	26 June 2019	No discharge
192502788	Pond 1 outfall	05 July 2019	72.4
192502789	Pond 1A outfall	05 July 2019	1.52
No sample	Pond 2A Outfall	05 July 2019	No discharge
192502790	Pond 3 A Outfall	05 July 2019	0.164
192502791	Pond 4 A Outfall	05 July 2019	0.051
No sample	Pond 5 A outfall	05 July 2019	No discharge
192502792	Pond 1B Outfall	05 July 2019	0.373
192502793	Pond 2B Outfall	05 July 2019	0.614
192502794	Pond 3B Outfall	05 July 2019	0.33
192502795	Pond 4B Outfall	Q S July 2019	0.135
No sample	Pond 5B Discharge Monitoring Point.	05 July 2019	No discharge
	at a	5 5	
No sample	Northern Willow Discharge Monitoring Point	05 July 2019	No discharge
No sample	Southern Willow Discharge Monitoring Point	05 July 2019	No discharge
	a fur real		
192502882	Pond 1 outfall	12 July 2019	71.2
192502883	Pond 1A outfall	12 July 2019	4.64
192502884	Pond 2A Outfall	12 July 2019	0.527
192502885	Pond 3 A Outfall	12 July 2019	0.181
192502886	Pond 4 A Outfall	12 July 2019	0.123
192502887	Pond 5 A outfall	12 July 2019	0.068
192502888	Pond 1B Outfall	12 July 2019	21.1
192502889	Pond 2B Outfall	12 July 2019	0.798
192502890	Pond 3B Outfall	12 July 2019	0.056
192502891	Pond 4B Outfall	12 July 2019	0.092
192502892	Pond 5B Discharge Monitoring Point.	12 July 2019	0.048
192502893	Northern Willow Discharge Monitoring Point.	12 July 2019	<0.015
192502894	Southern Willow Discharge Monitoring Point	12 July 2019	0.084

Appendix E

Groundwater Results

Consent of conviction purposes only any other use.

Location		Churchtown, Lifford, Co Donegal											
Sample Type		Ground Water											
Site No		BH1											
Date of Sample		Jan-18	Feb-18	Mar-18	Apr-18	May-18	Jun-18	Jul-18	Aug-18	Sep-18	Oct-18	Nov-18	Dec-18
Lab No													
Ha		7.85					7.25			7.18		7.11	
Temp	С	9.8					15.3			13.7		10.2	
Electrical Conductivity	uS/cm	95.1					137			149		152	
Ammonical Nitrogen	ma/l	0.012					0.053			0.011		< 0.005	
COD	ma/l												
BOD	ma/l												
Dissolved Oxvgen	ma/l	6.5					6.29			2.12		6.24	
SS	mg/l												
Residue on Evaporator	mg/l												
Calcium	ug/l									18			
Cadmium	ug/l									<0.1			
Chromium	ug/l						c.	ی .		<1.0			
Chloride	mg/l	19.85					14 🔊			14		13	
Chlorine	mg/l						the						
Copper	ug/l						1. 40			0.009			
Cyanide	mg/l					, S	2. 22.			<10			
Total Iron	ug/l					3	KO1			93			
Lead	ug/l					-0 ⁵ ec				<0.3			
Magnesium	ug/l					allall				3.6			
Manganese	ug/l					2 root				78			
Mercury	ug/l				, in the second s	lot et				0.22			
Nickel	mg/l				200	- AR							
Potassium	mg/l	3			: All the	P.,	2.7			3.4		3	
Sodium	mg/l	24.6			A A A		9.5			10.2		11.6	
Sulphate	mg/l				Y ST					7.4			
Zinc	ug/l									4.2			
Total Alkalinity as CaCO3	mg/l				× Or					53			
Total Organic Carbon	mg/l	8.51			en		9			8.81		7.82	
Total Oxidised Nitrogen	mg/l	0.26			0		1.1			0.569		0.92	
Arsenic	mg/l			U									
Barium	mg/l												
Boron	ug/l									< 0.02			
Fluoride	mg/l									<0.10			
Total Phenols	mg/l	< 0.1								<4.0		< 0.001	
Phosphorous	mg/l												
Selenium	mg/l												
Silver	mg/l												
Mircrotox	Toxic Units												
Microtox	Toxic Units	0.005					0.005			0.010		0.005	
Nitrite	mg/l	< 0.005					< 0.005			< 0.013		< 0.005	
Nitrate	mg/l	0.26					1.1			0.57		0.92	
Phosphate - ORTHO	mg/l									<0.009			
Phosphate - TOTAL	mg/l									400			
Total Coliforms										488			
Facel Coliforms		10								40		10	
Depth	m	1.9					1.4			1.5		1.2	

Location		Churchtown, Lifford, Co Donegal											
Sample Type		Surface Water											
Site No		BH3											
Date of Sample		Jan-18	Feb-18	Mar-18	Apr-18	Mav-18	Jun-18	Jul-18	Aug-18	Sep-18	Oct-18	Nov-18	Dec-18
Lab No													
H		7.93					7.4			6.91		7.14	
Temp	С	10.2					13.1			12.5		10.3	
Electrical Conductivity	uS/cm	494					540			536		520	
Ammonical Nitrogen	ma/l	0.009					0.032			0.013		0.026	
COD	ma/l												
BOD	mg/l												
Dissolved Oxygen	ma/l	7.55					6.04			3.82		3.52	
SS	ma/l												
Residue on Evaporator	ma/l												
Calcium	ua/l									92			
Cadmium	ug/l									<0.1			
Chromium	ug/l						d	ی .		<1.0			
Chloride	ma/l	37.72					19 🗸 🏷			13		24	
Chlorine	mg/l						ther						
Copper	ua/l						A A A			0.005			
Cyanide	mg/l					Ś.	2 212			<10			
Total Iron	ua/l					°,	(O)			<20			
Lead	ug/l					Se. 6				<0.3			
Magnesium	ug/l					all all				12.2			
Manganese	ug/l					2 . eot				4.6			
Mercury	ug/l					Of et				0.06			
Nickel	ma/l				ec.	What							
Potassium	mg/l	1.9			. 154	0	1.8			1.6		1.7	
Sodium	mg/l	15.4			A		15.3			14.7		15.5	
Sulphate	mg/l				4.5					6.5			
Zinc	ug/l				. د ^{ور}					3.8			
Total Alkalinity as CaCO3	mg/l				¹ O ²					200			
Total Organic Carbon	mg/l	1.17			ell		3.25			1.34		0.99	
Total Oxidised Nitrogen	mg/l	0.23		. 1	P		0.84			<0.138		0.18	
Arsenic	mg/l			C.									
Barium	mg/l												
Boron	ug/l									<0.02			
Fluoride	mg/l									<0.10			
Total Phenois	mg/l	< 0.1								<1.0			
Phosphorous	mg/l												
Selenium	mg/l												
Silver	mg/l												
Mircrotox	Toxic Units												
Microtox	Toxic Units												
Nitrite	mg/l	< 0.005					< 0.005			< 0.013		< 0.005	
Nitrate	mg/l	0.23					0.86			<0.12		0.18	
Phosphate - ORTHO	mg/l									< 0.009			
Phosphate - TOTAL	mg/l												
Total Coliforms										2420			
Facel Coliforms										10			
Depth	m	4					5.2			5.9		4.6	

Location		Churchtown, Lifford, Co Donegal											
Sample Type		Ground Water											
Site No		BH4											
Date of Sample		Jan-18	Feb-18	Mar-18	Apr-18	May-18	Jun-18	Jul-18	Aug-18	Sep-18	Oct-18	Nov-18	Dec-18
Lab No						,			Ū				
Hq		6.86					7.49			6.54		6.69	
Temp	С	10.3					12.9			14		10.3	
Electrical Conductivity	uS/cm	412					547			406		562	
Ammonical Nitrogen	ma/l	0.007					0.07			0.12		0.035	
COD	ma/l												
BOD	ma/l												
Dissolved Oxygen	mg/l	6.48					6.35			2.66		5.76	
SS	ma/l												
Residue on Evaporator	ma/l												
Calcium	ua/l									64			
Cadmium	ua/l									<0.1			
Chromium	ua/l									<1.0			
Chloride	mg/l	74.44					23			28		79	
Chlorine	mg/l							150°				-	
Copper	ua/l							é.		< 0.003			
Cyanide	mg/l						000	-		<10			
Total Iron	ua/l						13.02			<20			
Lead	ug/l					6	0,01			<0.3			
Magnesium	ug/l						200			6.3			
Manganese	ua/l					all all	2			46			
Mercurv	ua/l					N 100				0.05			
Nickel	mg/l				Å	lot not							
Potassium	mg/l	2.5			(Del	OTH	1.8			3.4		3.6	
Sodium	mg/l	8.7			in off	-	15.7			16.9		33.4	
Sulphate	mg/l	-			FORME		-			40			
Zinc	ug/l				COL					5.4			
Total Alkalinity as CaCO3	mg/l				Ŏ.					104			
Total Organic Carbon	mg/l	1.24		es	P		0.6			3.08		1.47	
Total Oxidised Nitrogen	mg/l	8.8		COL			0.89			10.528		7.9	
Arsenic	mg/l			\sim									
Barium	mg/l												
Boron	ug/l									<0.02			
Fluoride	mg/l									<0.10			
Total Phenols	mg/l	< 0.1								<1.0		< 0.001	
Phosphorous	mg/l												
Selenium	mg/l												
Silver	mg/l												
Mircrotox	Toxic Units												
Microtox	Toxic Units												
Nitrite	mg/l	< 0.005					< 0.005			<0.013		< 0.005	
Nitrate	mg/l	8.8					0.91			10.52		7.9	
Phosphate - ORTHO	mg/l									<0.009			
Phosphate - TOTAL	mg/l												
Total Coliforms										1986			
Facel Coliforms										10			
Depth	m	0					0.2			0.9		0.1	