

TRAFFIC AND TRANSPORTATION 13

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INTRODUCTION

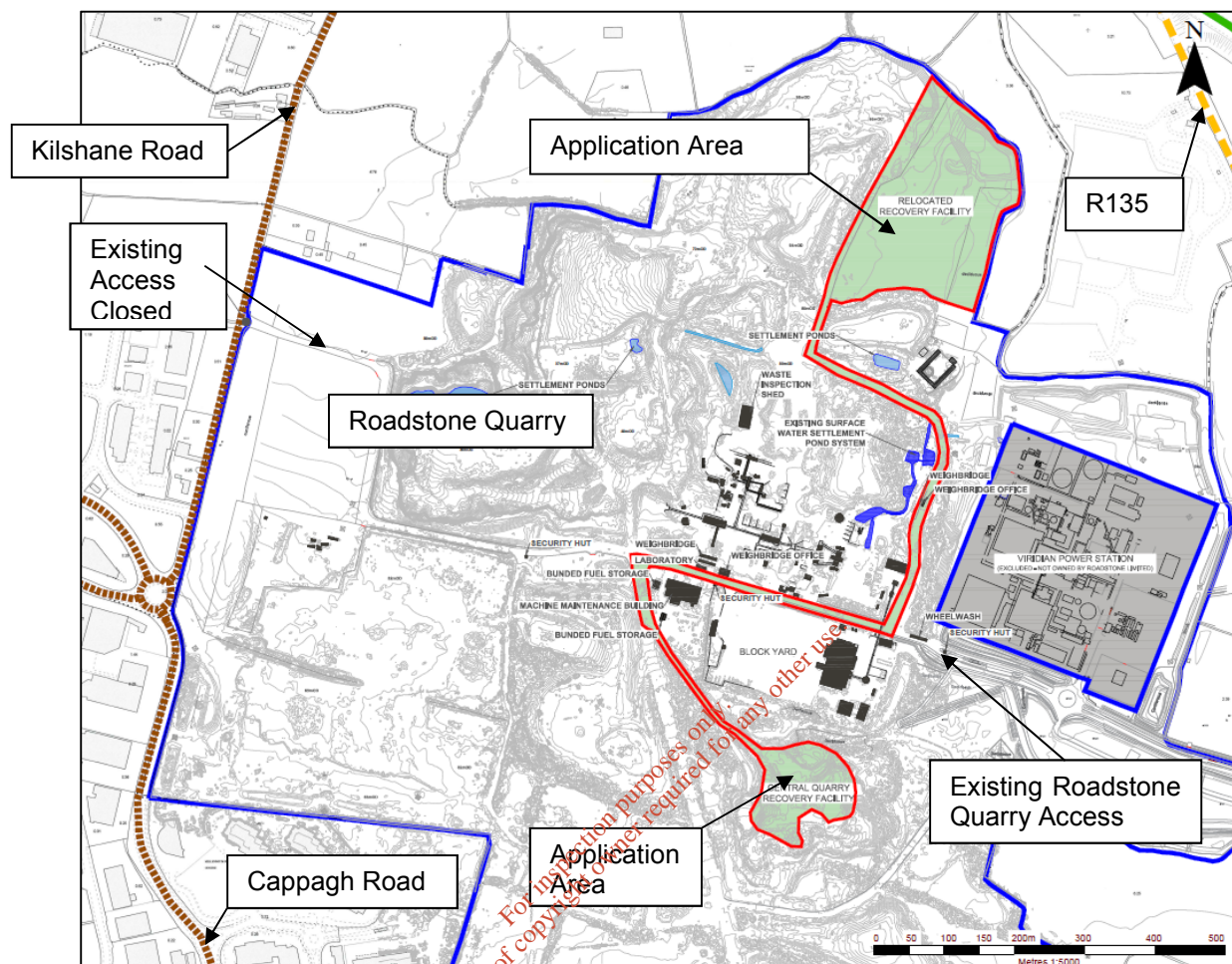
Background

- 13.1 Roadplan Consulting was appointed by SLR Consulting Ireland to identify and assess the traffic and transportation impacts of a planning application to increase the permitted intake rate of construction and demolition (C&D) waste to its C&D waste recovery facility at the Huntstown Quarry Complex at North Road, Finglas, Dublin 11, from a maximum of 24,950 tonnes per annum at the present time to 95,000 tonnes per annum in future years.
- 13.2 No further C&D waste will be imported to the existing waste recovery facility, located on a 1.9 hectare site in the Central Quarry. The planning application provides for processing and off-site dispatch of C&D waste stockpiled at the existing facility in the near-term (2-3 years), following publication of End of Waste criteria for recycled aggregate. It also provides for
- (i) relocation of C&D waste recovery activities to a dedicated new long-term recovery facility on a 5.2 hectare site in the north-eastern corner of the Huntstown Quarry Complex and
 - (ii) construction of a hardstanding area, waste processing shed, surface water management infrastructure and upgraded internal access road at the new waste recovery facility.
- 13.3 This chapter predicts the trips expected to be generated by the proposed development, and the impacts of those trips on the operational performance of the local road network and junctions, in particular the following:
- R135 / N2 Slip Road - Priority Junction
 - R135 / Elm Road - Signalised Junction
 - R135 / L3125 - Signalised Junction
 - R135 / N2 - Roundabout Junction

Existing Development

- 13.4 Existing development at Huntstown Quarry is primarily tied to ongoing rock extraction and processing. In addition to established quarrying activities, several associated value-added activities are undertaken at the quarry, including production of concrete blocks and readymix concrete. A large soil recovery facility which is also operating within the quarry complex has a permitted soil waste intake of 1,500,000 tonnes per annum.
- 13.5 The quarry and application site are located within the townlands of Huntstown and Kilshane, Co. Dublin, approximately 2.5km north-west of the Dublin suburb of Finglas and 2km north-west of the interchange between the N2 Dual Carriageway and the M50 Motorway. Positioned north of the M50 motorway, the quarry has two access points: the primary entrance located on the eastern boundary of the Roadstone property holding, located off the R135 Regional Road, also known as the North Road, and a secondary access (which has been closed in recent years and is no longer in use), located on the western boundary of the property, along the Kilshane Road. The location of the application site, and both existing accesses, are shown on Figure 13-1.
- 13.6 The existing site infrastructure at the quarry complex includes internal haul roads, offices and staff welfare facilities, plant storage and maintenance sheds, refuelling facilities and crushing, grading and processing plant used to process blasted rock.

**Figure 13-1
Site Location Map**



Proposed Development

13.7 This application relates to a proposal to increase the permitted rate of C&D waste intake/ recovery at the existing C&D recovery facility at Huntstown Quarry, from a maximum of 24,950 tonnes per annum at present to a maximum of 95,000 tonnes per annum in future years. It is also proposed to transfer the C&D waste recovery activities from its current location at the Central Quarry to a replacement site in the north-eastern corner of the quarry complex.

Existing Site Access

13.8 Huntstown Quarry is currently accessed via the North Road (R135). The access road leading from North Road to the quarry complex is shared by quarry traffic and traffic going to and from the soil recovery facility and Huntstown Power Station. The access road is approximately 7.3m wide at the site entrance and divides as it runs towards the principal quarries and waste recovery facility. The widths of the inbound and outbound lanes are approximately 3.7m.

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Proposed Site Access

13.9 This application is tied to a previous grant of planning permission and an established C&D waste recovery facility which has operated under a Local Authority waste permit for many years. All of the proposed increase in C&D waste intake and export of recycled aggregate will be carried via the existing entrance and access road leading off the R135 North Road, located on the eastern side of the Huntstown Quarry complex. The former access road from the Kilshane Road to the west of the quarry complex will not be used.

Information Reviewed

13.10 In preparing this assessment, Roadplan Consulting has made reference to:

- the *Fingal Development Plan 2011 – 2017*,
- The Institute of Highways and Transportation *Guidelines on the Preparation of Traffic Impact Assessments*,
- the *TII Transport Assessment Guidelines*,
- the *TII National Traffic Model*.

Existing Road Network

13.11 The existing road network within the vicinity of the application site is illustrated in Figure 13-1 and is described further below.

13.12 The existing road network around the recovery facility and application site is defined by:

- The R135 regional road to the east, which previously served as the N2 National Primary Road (up to May 2006). This road is known locally as the North Road. It intersects with the N2 Dual carriageway at the Cherryhound Interchange to the north and forms a cul-de-sac to the south (severed by the re-aligned N2);
- a local road, known as the Kilshane Road (or Cappagh Road) to the west and north of the Huntstown quarry complex; and
- The M50 Motorway which lies south of the existing quarry.

13.13 The N2 Dual Carriageway between the M50 Motorway and Cherryhound Interchange runs immediately east of the R135 Regional Road. It continues northwards from the Cherryhound Interchange as the M2 Motorway to the north of Ashbourne Co. Meath. From there, it becomes the N2 National Primary Road and continues northwards as a single carriageway road through the counties of Meath, Louth and Monaghan to the border with Northern Ireland.

13.14 In relation to the local road network, the application site and the Huntstown Quarry complex in general, is located to the north of the M50 motorway, west of the R135 Regional Road (North Road) and the N2 Dual Carriageway and east and south of the Kilshane Road.

13.15 Much of the road network around the application site has been upgraded in recent years. The N2 dual carriageway / M2 motorway opened in May 2006 and led to a large and immediate reduction in traffic levels along the former N2 National Primary Road (now the R135 Regional Road) immediately east of Huntstown Quarry. Upgrading of the M50 to provide three lanes of traffic in both directions was also completed in 2010, as was the upgrading of its interchange with the N2 dual carriageway at Finglas to provide for a free-flow interchange.

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- 13.16 The existing R135 (North Road) comprises of a single carriageway road generally of about 7.5m width with hard shoulders of varying width. The alignment essentially runs straight from the existing quarry entrance northwards up to the N2 / M2 motorway at the Cherryhound interchange and southwards to the point at which it is severed by the M50 motorway at Finglas.
- 13.17 A speed limit of 50kph applies along the existing R135 regional road. This speed limit applies to traffic which travels between the R135 / N2 roundabout junction and the existing entrance to the quarry complex at Huntstown.

Methodology

- 13.18 HGV traffic travelling to the recovery facility via Dublin City and the N2 National Primary Road and/or M50 motorway to the south will access it via the R135 / N2 Slip Road priority junction at Coldwinters. HGV traffic travelling to the facility from the N2 or N3 to the north and north-west will travel via the R135 / N2 Roundabout junction at the Cherryhound Interchange.
- 13.19 HGV traffic departing the recovery facility with recycled aggregates and travelling toward the city and M50 motorway will initially travel via the R135 / Elm Road signalised junction and turn back onto the southbound carriageway of the N2 dual carriageway. HGV traffic departing the facility and travelling towards the N2 Dual Carriageway northbound or the N3 will travel via the R135 / N2 Roundabout junction.
- 13.20 Having regard to the established / future pattern of traffic movements, the methodology adopted for this assessment is summarised as follows:
- A 12-hour Manual Classified Traffic Counts was undertaken by Tracsis Traffic and Data Services on the 22nd of June 2016. Count information was obtained at the following junctions (shown on Figure 13-1):
 1. R135 / N2 Slip Road Priority Junction at Coldwinters
 2. R135 / Elm Road Signalised Junction at Newtown
 3. R135 / L3125 Kilshane Cross Signalised Junction
 4. R135 / N2 Roundabout Junction (link to Cherryhound Interchange)
 - Existing Traffic Assessment – A spreadsheet model was created which contains the base year do-nothing traffic count data described above. The traffic count data was used to develop a PICADY model of the R135 / N2 Slip Road priority junction, an ARCADY model of the R135 / N2 roundabout junction and an OSCADY PRO model of the R135 / Elm Road signalised junction and the R135 / L3125 signalised junction.
 - Future Year Assessment – The estimated future year traffic volumes on the study area road network, as a result of the increase in background traffic and additional development related traffic was used to assess the future operational performance of the junctions at the year of opening (2017), five years after (2022) and fifteen years after opening (2032).

THE EXISTING ENVIRONMENT

Existing Traffic Flows

- 13.21 The 12-hour traffic flows for each junction are provided in Appendix 13-A – Traffic Count Data. Traffic flows along the R135 were recorded for the four junctions identified previously. The daily profile of traffic flow at each junction along the R135 is shown in the figures below, as is the percentage of each vehicle class. The traffic flows during the busiest AM and PM peak hours were also abstracted from the surveyed data and are shown in the tables below:

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Figure 13-2
R135 / N2 Slip Road Priority Junction Traffic Profile

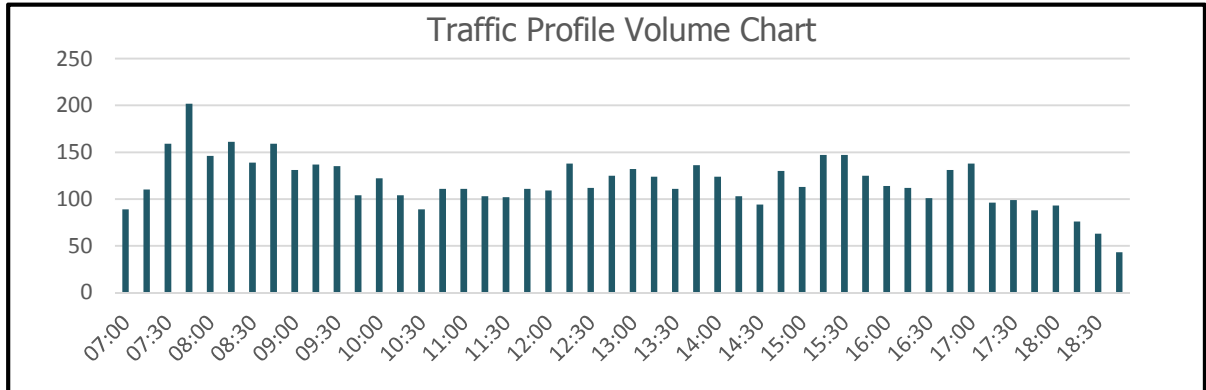


Figure 13-3
R135 / N2 Slip Road Priority Junction Vehicle Percentages

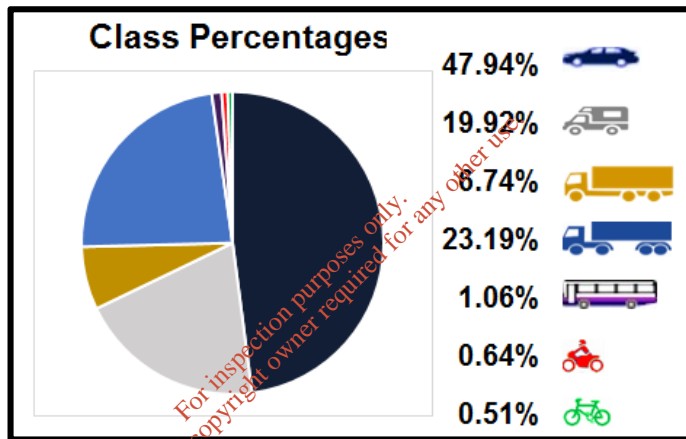


Table 13-1
2016 AM Peak Existing (07:30 – 08:30)

From / To	R135 (North)	N2 Slip Rd	R135 (South)	Totals
R135 (North)		0	58	58
N2 Slip Rd	396		128	524
R135 (South)	86	0		86
Totals	482	0	186	668

Table 13-2
2016 PM Peak Existing (16:45 – 17:45) –

From / To	R135 (North)	N2 Slip Rd	R135 (South)	Totals
R135 (North)		0	20	20
N2 Slip Rd	308		20	328
R135 (South)	116	0		116
Totals	424	0	40	464

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Principal features of the existing traffic flows at the R135 / N2 Slip Road priority junction are as follows:

- Overall flows are higher in the am peak compared to the pm peak, with the main traffic flows travelling along the N2 Slip Road;
- The N2 slip is one-way only with turning from the R135 Regional Road prohibited;
- The percentage of HGV movement is high with HGVs comprising approximately 30% of traffic travelling through the junction.

13.22 The daily profile of traffic flow at for the R135 / Elm Road signalised junction is shown in the figures below, as is the percentage of each vehicle class. The traffic flows during the busiest AM and PM peak hours were abstracted from the surveyed data and are shown in the tables below:

Figure 13-4
R135 / Elm Road Signalised Junction

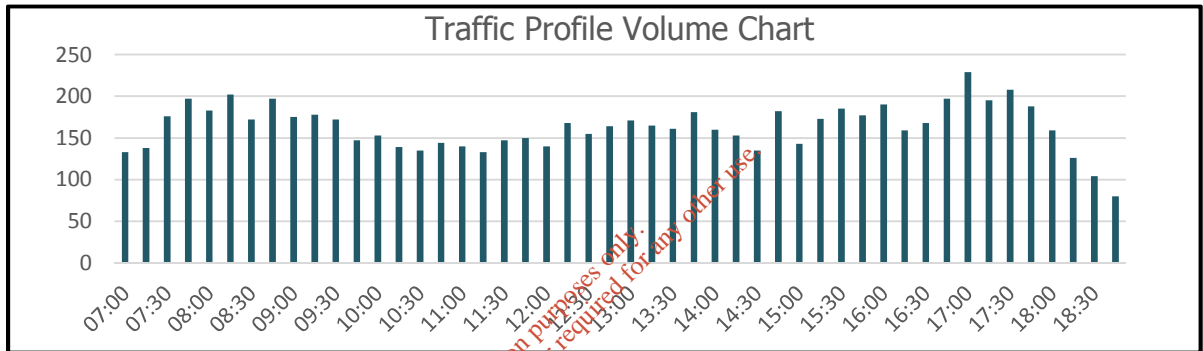
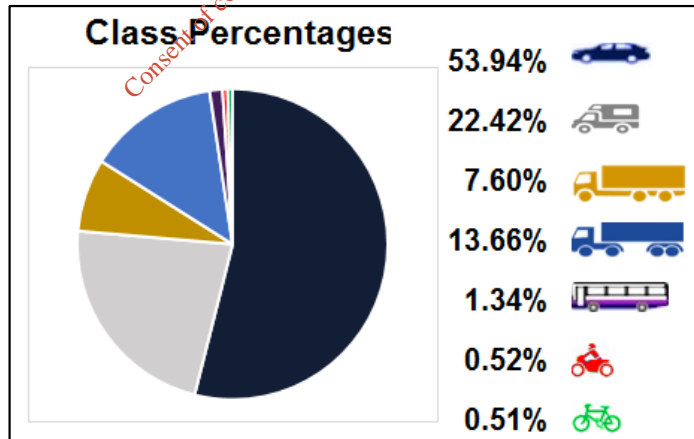


Figure 13-5
R135 / Elm Road Signalised Junction Vehicle Percentages



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Table 13-3
2016 AM Peak Existing (07:30 – 08:30)

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)		230	63	293
Elm Road	21		3	24
R135 (South)	319	122		441
Totals	340	352	66	758

Table 13-4
2016 PM Peak Existing (16:45 – 17:45)

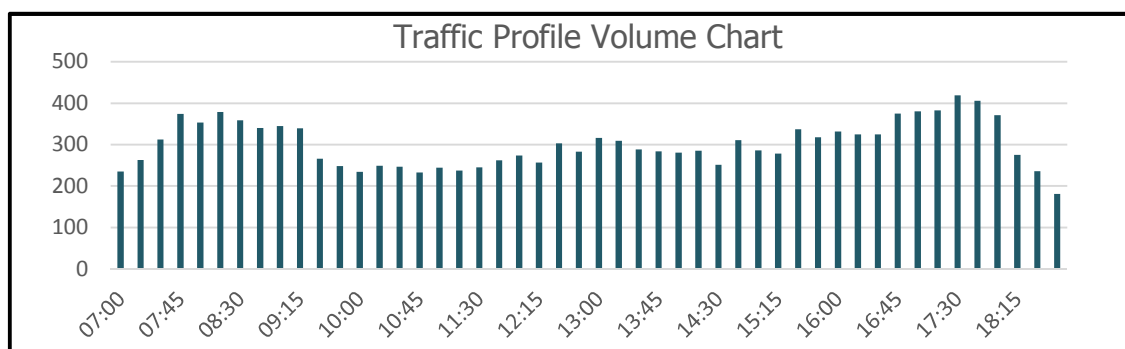
From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)		313	21	334
Elm Road	29		2	31
R135 (South)	320	144		464
Totals	349	457	23	829

Principal features of the existing traffic flows at the R135 / Elm Road Signalised junction are as follows:

- Overall flows are slightly higher in the pm peak compared to the am peak;
- Traffic volumes turning from Elm Road are low in both the am and pm peak;
- The R135 / Elm Rd junctions provides access to the N2 for vehicles travelling southbound only;
- The percentage of HGV movement is high with HGVs comprising approximately 21% of traffic travelling through the junction.

13.23 The daily profile of traffic flow at for the R135 / L3125 Kilshane crossroads signalised junction is shown in the figures below, as is the percentage of each vehicle class. The traffic flows during the busiest AM and PM peak hours were abstracted from the surveyed data and are shown in the tables below:

Figure 13-6
R135 / L3125 Signalised Junction Traffic Profile



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Figure 13-7
R135 / L3125 Signalised Junction Vehicle Percentages

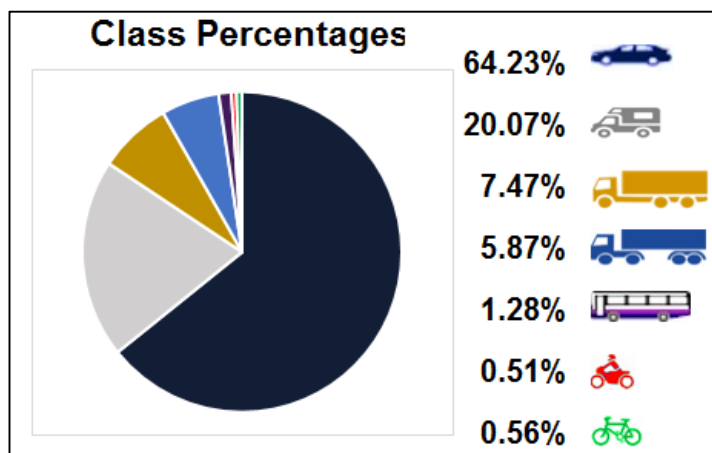


Table 13-5
2016 AM Peak Existing (07:30 – 08:30)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)		219	127	29	365
L3125 (East)	82		90	320	492
R135 (South)	59	73		205	337
L3125 (West)	9	134	81		224
Totals	150	426	288	554	1418

Table 13-6
2016 PM Peak Existing (16:45 – 17:45)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)		168	86	16	270
L3125 (East)	204		85	199	488
R135 (South)	111	120		110	341
L3125 (West)	22	273	163		458
Totals	337	561	334	325	1557

Principal features of the existing traffic flows at the existing R135 / L3125 Signalised junction are as follows;

- Overall flows are slightly higher in the pm peak compared to the am peak;
- The traffic flow travelling along the L3125 local road is higher than that along the R135 Regional Road;
- The L3125 provides access to Ballycoolin Industrial estate to the west and Dublin airport to the east.

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13.24 The daily profile of traffic flow at for the R135 / N2 roundabout junction is shown in the figures below, as is the percentage of each vehicle class. The traffic flows during the busiest AM and PM peak hours were abstracted from the surveyed data and are shown in the tables below:

Figure 13-8
R135 / N2 Roundabout Junction Traffic Profile

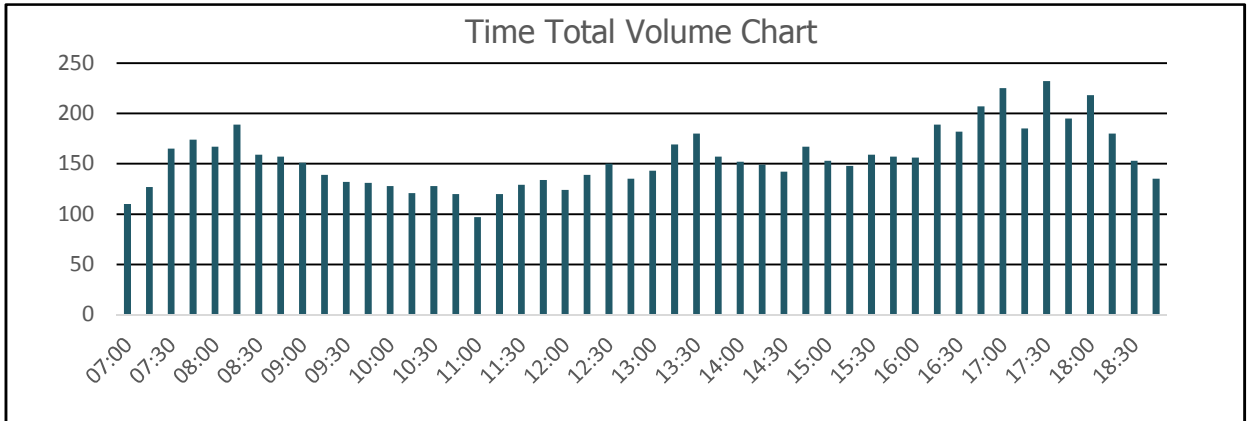


Figure 13-9
R135 / L3125 Signalised Junction Vehicle Percentages

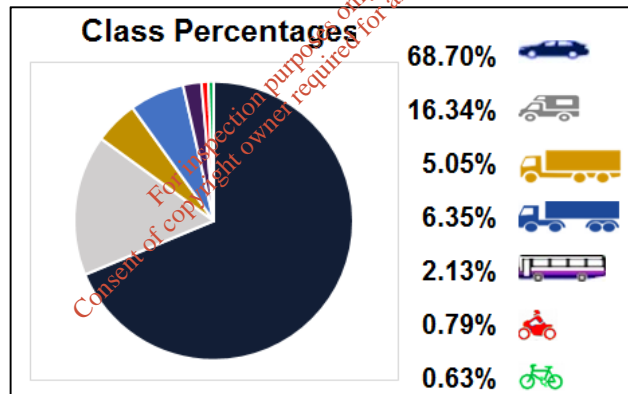


Table 13-7
2016 AM Peak Existing (07:30 – 08:30)

From / To	R135 (North)	R135 (South)	N2 Slip Rd	Totals
R135 (North)		170	139	309
R135 (South)	48		88	136
N2 Slip Rd	63	186		249
Totals	111	356	229	694

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Table 13-8
2016 PM Peak Existing (16:45 – 17:45)

From / To	R135 (North)	R135 (South)	N2 Slip Rd	Totals
R135 (North)		111	86	197
R135 (South)	185		175	360
N2 Slip Rd	127	160		278
Totals	312	271	261	844

Principal features of the existing traffic flows at the R135 / N2 Roundabout junction are as follows:

- Overall flows are slightly higher in the pm peak compared to the am peak;
- The flows along the N2 Slip Road are low.

Non-Car Access / Transport

- 13.25 To ascertain opportunities for sustainability, consideration has been given to non-car accessibility for the application site.
- 13.26 Although operation of the facility will be very dependent on road based haulage, staff will require transport means to get to and from work. Non-car accessibility will help reduce traffic impact and also promote social inclusion by providing means of transport to the application site to those who do not have access to a private car.
- 13.27 Access to the application site on foot is by way of the existing pedestrian footway situated to the eastern side of North Road. At approximately 1.5m in width, this footway extends along North Road northwards until reaching the northbound N2 Slip Road / North Road priority junction where it terminates. Pedestrians can then utilise the footpath situated to the western side of the R135 which extends for a short distance in the direction of Cherryhound interchange before terminating at a bus stop. For employees wishing to access this bus stop on foot, it is an approximate distance of 1.8 kilometres away or a 25 minute walk (based upon an average walking distance of 1.2m per second.)
- 13.28 A large number of residential dwellings can be reached within a 5 kilometre cycle from the recovery facility which, assuming an average speed of 15kmh, equates to a 20 minute cycle ride. This includes the northern suburbs of Santry, Finglas, Charlestown, Dunsink and Castlenock and settlements to the north of the M50 Blanchardstown, Mulhuddart and Tyrellstown.
- 13.29 Adequate provision has already been made for pedestrians and cyclists within the visitor area with internal pedestrian walkways segregated from vehicles and cycle parking provided for those members of staff (or visitors) wishing to travel via bicycle. Whilst the Applicant is limited by what can realistically be undertaken to further improve non-car accessibility within the quarry complex, should the development proposal give rise to an increase in members of staff / visitors wishing to travel via bicycle then additional bicycle parking can be provided.

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- 13.30 The closest bus stop to the application site is situated on the North Road (R135), to the north of the N2 off-slip. The bus stop is a simple flag-post stop which is served by the Number 103 and 107 services operated by Bus Eireann. Both of the routes that service the bus stop run from Dublin to surrounding towns and villages.
- 13.31 The Number 103 service runs between Dublin City and Ashbourne / Ratoath. The service operates 7 days a week and begins at 06:30 in the morning and runs at approximately 20 minute intervals throughout the day until approximately midnight.
- 13.32 The Number 107 service runs between Dublin City and Navan, Nobber and Kells. The service operates 7 days a week and begins at 09:00 in the morning and runs at least four services in either direction throughout the day.
- 13.33 As identified above the local bus service is limited; however there are regular 20 minute services available from central Dublin throughout the day, seven days per week. Individual travel by bus to the application site is therefore a realistic alternative to the car.

Site Access Sightlines

- 13.34 The requirements for sightlines are stated in the TII Design Manual for Urban Roads and Streets. Sightlines are measured along the mainline from a driver's viewing position on the access road, 3.0m back from the mainline edge.
- 13.35 Access to the development is from the existing Roadstone Quarry Access onto the R135. The posted speed limit of the R135 at the existing access to the quarry is 50kph. The existing access provides a sightline in excess of 200m in each direction when assessed in accordance with the DMURS.

Accident History

- 13.36 Accident data has been obtained from the Road Safety Authority website www.rsa.ie. The website provides an interactive online mapping tool which has been navigated to the area of interest in order to determine the local accident history.
- 13.37 Accident data covering a nine year period 2005 to 2013 is provided. The information is rudimentary and only provides the number of collisions and the seriousness of the collisions, along with basic details of each incident. The definitions of the severities are given below:
- Fatal (a crash resulting in death)
 - Serious (detention in hospital: includes paralysis, fractures and severe lacerations)
 - Minor (includes whiplash, strains and minor lacerations)
- 13.38 No accidents were recorded at the existing access to the quarry. Two minor accidents were recorded south of the existing access and one minor accident was recorded north of the existing access. At the existing R135 / Elm Road signalised junction one minor accident was recorded.
- 13.39 At the existing R135 / L3125 signalised junction (Kilshane Cross), one fatal accident, one serious accident and eight minor accidents were recorded. A summary of the collision history broken down into vehicle type is provided in Table 13.9 below:

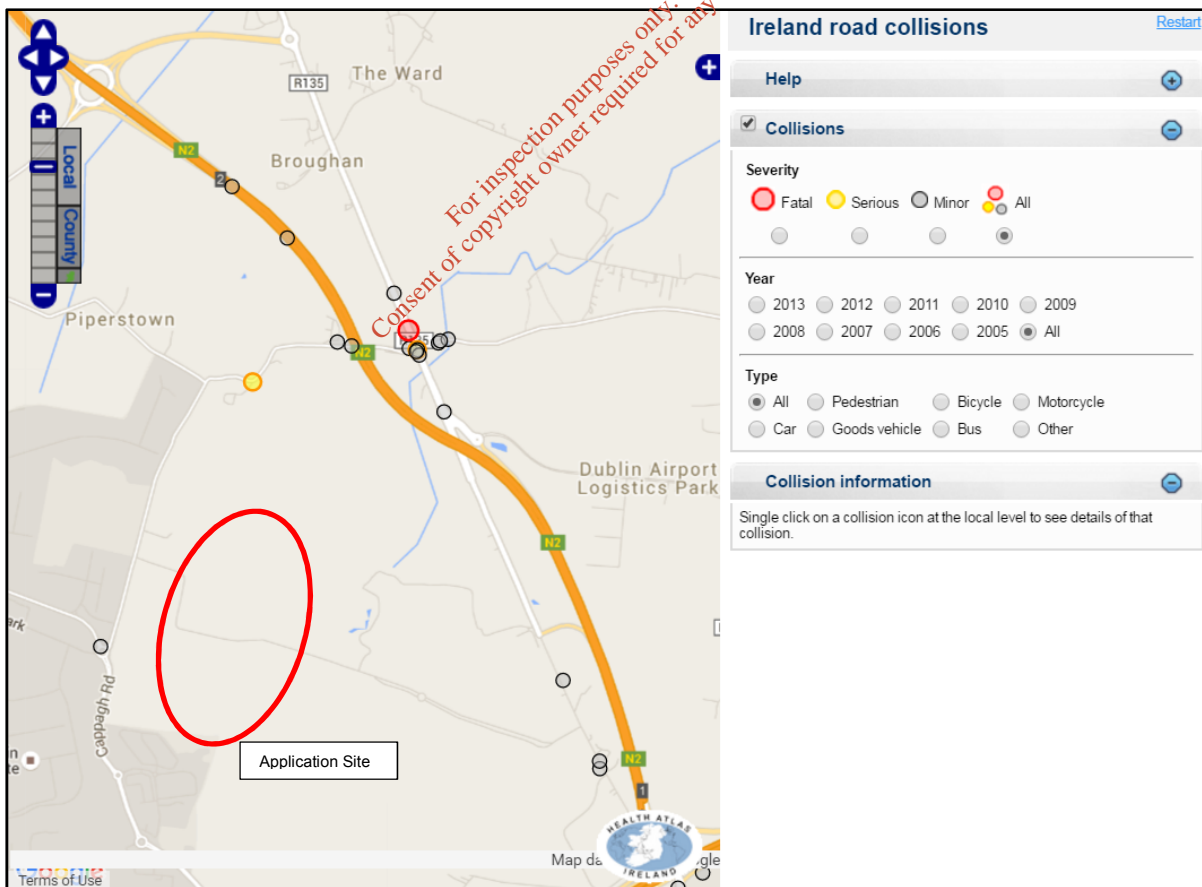
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**Table 13-9
Road Collision History**

Vehicle Type	Fatal Collision	Serious Collision	Minor Collision	All Collision
Bus	0	0	0	0
Goods Vehicle	0	1	2	3
Car	0	0	6	6
Motorcycle	1	0	4	5
Bicycle	0	0	0	0
Pedestrian	0	0	0	0
Other	0	0	0	0
Total	1	1	12	14

13.40 The location of each collision is shown in Figure 13.10 below

**Figure 13-10
Road Collisions**



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Proposed Road Network Improvements

- 13.41 Fingal County Council has future proposals to provide a western link from the R135 / N2 roundabout to Dublin Airport. This road scheme is set as a development objective in the Airport Local Area Plan.
- 13.42 It is envisaged that existing traffic flows at the R135 / L3125 Kilshane Crossroad signalised junction will significantly reduce pending the construction of the airport link road scheme due to re-distribution of traffic flows.

IMPACT OF THE PROPOSED DEVELOPMENT

Future Trip Generation

- 13.43 The proposed development will generate an increase in the rate of C&D waste recovery intake from 24,950 tonnes per annum to 95,000 tonnes per annum (an increase of 70,050 tonnes per annum).
- 13.44 The existing quarry and recovery facility operate between 08:00 to 19:00 on Monday to Friday and between 08.00 hours and 13.00 hours on Saturdays. It will operate / open on approximately 300 days per annum (based on 6 days a week for 50 weeks).
- 13.45 It is anticipated that the waste material will arrive in 18 tonne consignments. Table 13.10 below provides a forecast of the average weekday traffic that is anticipated to be generated by the proposed increase of waste material intake.
- 13.46 In addition, it is expected that the recovery facility will produce up to 90,000 tonnes of recycled (secondary) aggregate per annum to be exported off-site from the C&D waste recovery facility.
- 13.47 It is anticipated that the secondary aggregate will be exported off-site in 18 tonne consignments. Table 13.10 below provides a forecast of the average weekday traffic that is anticipated to be generated by the proposed export of secondary aggregate from the C&D waste recovery facility.

Table 13-10
Future Trip Generation

	Volume (Tonnes)	Annual Loads (HGVs)	Average Daily Loads (HGVs)	Average Hourly Loads (HGVs)
<u>OPERATION</u> Waste Recovery Material Intake (18t)	70,050	3,900	13	1
<u>OPERATION</u> Export of Recycled Aggregate (18t)	95,000	5,300	18	2

Distribution and Assignment

- 13.48 Table 13-10 above indicates that the average increase in daily trip generation of HGV traffic as a result of the increase in waste recovery material intake and the export of secondary aggregate is 31 HGV loads per day. This equates to a total of 62 HGV movements per day (two-way trips).

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- 13.49 The proposed development will be operational for approximately 11 hours per day with a total of 31 additional trips per day. Therefore for a 1 hour period there will be approximately 3 trip per hour (31 trips / 11 hours) arriving and departing from the proposed development. The table below shows the expected AM and PM peak flows to and from the development.

Table 13-11
Peak hour Flows

	Trips to Development	Trips from Development
AM Peak	3	3
PM Peak	3	3

- 13.50 It is envisaged that all HGV's importing C&D waste material and exporting recycled (secondary) aggregate to/from the recovery facility at the Huntstown quarry complex will approach it along the M50 Motorway and / or the N2 Dual Carriageway and enter the site using the existing North Road entrance.
- 13.51 As such, all the proposed development traffic within the assessment have been distributed in line with the existing traffic patterns as surveyed.

Other Planned Developments

- 13.52 Planning permission was granted in November 2016 for an increase in the rate of soil and stone importation to the adjoining soil waste recovery facility at Huntstown from 750,000 tonnes per annum to 1,500,000 tonnes per annum (Fingal County Council Ref. No. FW16A/0120). This activity will generate approximately 130 **additional** trips per day (based on working 6 days a week for 50 weeks, arriving and departing in 20 tonne consignments). If it operates at maximum capacity, the permitted soil recovery and restoration activities at the North Quarry and West Quarry would be completed in around 6 years.
- 13.53 Planning has previously been granted to Roadstone for a total aggregate and concrete output of up to 2 million tonnes per annum from the established quarry operations over a 20 year period from 2014 (Fingal County Council Ref. No. FW12A-0022 and An Bord Pleanála Ref. No. 06F.241693). The proposed development will generate approximately 330 trips per day (based on working 6 days a week for 50 weeks, arriving and departing in 20 tonne consignments).
- 13.54 Planning permission has also been granted for a proposed Anaerobic Digestion (AD) Facility at Huntstown Quarry. (Planning Ref. FW13A/0089). The proposed development will generate approximately 60 trips per day. Access to the proposed AD facility will be via the existing R135 / Quarry Access priority junction.
- 13.55 Sensitivity testing of the above developments has been carried out to show the impact these development will have on the existing road network when the C&D waste recovery facility development is operational.

Traffic Increase

- 13.56 The TII issues a range of traffic growth factors to be applied to existing traffic flows which are broken down into three groups; low growth, medium growth and high growth. Due to the close proximity of the M50 motorway and the N2 dual carriageway it is assumed that medium growth is most likely for the R135 regional road.

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13.57 The zone in which the site is located is numbered 821 in the TII National Traffic Model. The medium growth factors for each operational phase are as follows:

**Table 13-12
Future Year Traffic Growth**

Road	2016 Existing	2017 Development Operational	2022 Development Operational	2032 Development Operational
All Roads	1.00	+ 0.71%	+ 4.36%	+ 8.59%

13.58 These percentages have been used to predict the increase in background traffic along the R135 that will occur in future years. It should be noted that these growth factors do not apply to predicted trips generated by the development. Full summary tables and predicted future traffic flows for 2017, 2022 and 2032 future years are included in Appendix 13B – Traffic Flow Sheets.

Junction Capacity Assessment

13.59 Capacity assessments using the computer programme PICADY for the existing R135 / N2 Slip Road priority junction and ARCADY for the existing R135 / N2 roundabout junction has been carried out. Full details and results of capacity assessments are contained in Appendix 13C – PICADY Results and in Appendix 13D – ARCADY Results

13.60 The parameters shown in the tables are defined as follows:

- **Ratio of Flow to Capacity (RFC)** is a factor indicating the flow on a junction arm relative to its capacity. An RFC of 1.0 means the junction has reached its ultimate capacity and an RFC of 0.85 means that the junction has reached its reserve capacity.
- **Avg. Queue** is the average number of vehicles queued over the time period on the junction approach.
- **Queue delay** is the average number of seconds delay to each vehicle in the time period.
- **Total Delay** is the total number of vehicle hours of delay to all vehicles at the junction over the time period.

R135 / N2 Slip Road Priority Junction

13.61 The following tables show the predicted RFC values (Ratio of Flow to Capacity), average queue lengths, average vehicle delay and total delays for the existing R135 / N2 Slip Road priority junction at Coldwinters.

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Table 13-13
R135 / N2 Slip Road Priority Junction : AM Peak

AM Peak		2016	2017 No Dev	2017 With Dev	2022 No Dev	2022 With Dev	2022 Sens Flows	2032 No Dev	2032 With Dev	2032 Sens Flows	
R135 / N2 Slip Road Priority Junction	R135 North	RFC value	-	-	-	-	-	-	-	-	
		Average Queue (Vehicles)	-	-	-	-	-	-	-	-	
		Average delay (sec / veh)	-	-	-	-	-	-	-	-	
		Total Delay (veh / min)	-	-	-	-	-	-	-	-	
	N2 Slip Road	RFC value	0.87	0.87	0.88	0.90	0.91	0.96	0.95	0.95	0.98
		Average Queue (Vehicles)	6	6	6	7	7	11	10	10	14
		Average delay (sec / veh)	28	28	29	32	33	45	40	41	60
		Total Delay (veh / min)	5.12	5.29	5.39	6.36	6.51	9.60	8.53	8.82	11.89
	R135 South	RFC value	-	-	-	-	-	-	-	-	-
		Average Queue (Vehicles)	-	-	-	-	-	-	-	-	-
		Average delay (sec / veh)	-	-	-	-	-	-	-	-	-
		Total Delay (veh / min)	-	-	-	-	-	-	-	-	-

Table 13-14
R135 / N2 Slip Road Priority Junction : PM Peak

AM Peak		2016	2017 No Dev	2017 With Dev	2022 No Dev	2022 With Dev	2022 Sen Flows	2032 No Dev	2032 With Dev	2032 Sen Flows	
R135 / N2 Slip Road Priority Junction	R135 North	RFC value	-	-	-	-	-	-	-	-	
		Average Queue (Vehicles)	-	-	-	-	-	-	-	-	
		Average delay (sec / veh)	-	-	-	-	-	-	-	-	
		Total Delay (veh / min)	-	-	-	-	-	-	-	-	
	N2 Slip Road	RFC value	0.68	0.69	0.69	0.72	0.72	0.74	0.75	0.75	0.76
		Average Queue (Vehicles)	2	2	2	2	2	2	3	3	3
		Average delay (sec / veh)	17	18	18	19	19	19	20	20	21
		Total Delay (veh / min)	2.13	2.16	2.18	2.38	2.39	2.51	2.65	2.67	2.93
	R135 South	RFC value	-	-	-	-	-	-	-	-	-
		Average Queue (Vehicles)	-	-	-	-	-	-	-	-	-
		Average delay (sec / veh)	-	-	-	-	-	-	-	-	-
		Total Delay (veh / min)	-	-	-	-	-	-	-	-	-

13.62 Tables 13-13 and 13-14 above indicate that at present the R135 / N2 Slip Road priority junction operates at capacity with queues and delays during the AM peak hour. During the PM peak hour, the junction will operate within capacity with small queues and delays.

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- 13.63 Queues were noted at the junction and a maximum queue of 8 vehicles were noted at the junction during the counts which correlates with the junction assessment.
- 13.64 With the proposed development in place in 2017, 2022 and 2032, the junction will operate at capacity with queues and delays during the AM peak hour. During the PM peak hour the junction will operate within capacity with small queues and delays.
- 13.65 In 2032 during the AM peak hour the junction has a maximum queue of 10 vehicles. The slip road from the N2 is approximately 200m in length and it is unlikely that vehicles will queue back to the N2 dual carriageway.
- 13.66 Sensitivity testing on other committed and other planned development was carried out with the proposed development in place in 2017, 2022 and 2032. The junction will operate at capacity with queues and delays during the AM peak hour. During the PM peak hour the junction will operate within capacity with small queues and delays.

R135 / N2 Link Road Roundabout Junction

13.67 The following tables show the predicted RFC values (Ratio of Flow to Capacity), average queue lengths, average vehicle delay and total delays for the existing R135 / N2 Link Road priority junction.

Table 13-15
R135 / N2 Link Road Roundabout Junction : AM Peak

AM Peak		2016	2017 No Dev	2017 With Dev	2022 No Dev	2022 With Dev	2022 Sen Flows	2032 No Dev	2032 With Dev	2032 Sen Flows	
R135 / N2 Link Road Roundabout Junction	R135 North	RFC value	0.22	0.22	0.23	0.23	0.23	0.25	0.24	0.24	0.26
		Average Queue (Vehicles)	0	0	0	0	0	0	0	0	0
		Average delay (sec / veh)	3	3	3	3	3	3	3	3	3
		Total Delay (veh / min)	0.33	0.34	0.35	0.36	0.36	0.39	0.37	0.38	0.40
	R135 South	RFC value	0.09	0.10	0.10	0.10	0.10	0.12	0.10	0.10	0.12
		Average Queue (Vehicles)	0	0	0	0	0	0	0	0	0
		Average delay (sec / veh)	2	2	2	2	2	2	2	2	2
		Total Delay (veh / min)	0.12	0.13	0.13	0.13	0.13	0.16	0.14	0.14	0.16
	N2 Link Road	RFC value	0.18	0.18	0.19	0.19	0.19	0.19	0.19	0.20	0.20
		Average Queue (Vehicles)	0	0	0	0	0	0	0	0	0
		Average delay (sec / veh)	3	3	3	3	3	3	3	3	3
		Total Delay (veh / min)	0.26	0.26	0.27	0.27	0.27	0.27	0.29	0.29	0.29

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Table 13-16
R135 / N2 Link Road Roundabout Junction : PM Peak

PM Peak		2016	2017 No Dev	2017 With Dev	2022 No Dev	2022 With Dev	2022 Sen Flows	2032 No Dev	2032 With Dev	2032 Sen Flows	
R135 / N2 Link Road Roundabout Junction	R135 North	RFC value	0.13	0.14	0.14	0.14	0.14	0.16	0.15	0.15	0.16
		Average Queue (Vehicles)	0	0	0	0	0	0	0	0	0
		Average delay (sec / veh)	2	2	2	2	2	2	2	2	2
		Total Delay (veh / min)	0.18	0.19	0.19	0.20	0.20	0.22	0.21	0.21	0.22
	R135 South	RFC value	0.22	0.22	0.22	0.23	0.23	0.25	0.24	0.24	0.25
		Average Queue (Vehicles)	0	0	0	0	0	0	0	0	0
		Average delay (sec / veh)	2	2	2	2	2	2	2	2	2
		Total Delay (veh / min)	0.33	0.34	0.34	0.35	0.35	0.38	0.38	0.38	0.40
	N2 Link Road	RFC value	0.22	0.22	0.22	0.23	0.23	0.23	0.24	0.24	0.24
		Average Queue (Vehicles)	0	0	0	0	0	0	0	0	0
		Average delay (sec / veh)	3	3	3	3	3	3	3	3	3
		Total Delay (veh / min)	0.33	0.34	0.34	0.35	0.35	0.36	0.38	0.38	0.38

13.68 Tables 13-15 and 13-16 above indicate that at present the R135 / N2 Link Road roundabout junction will operate within capacity, with no queues and minimal delays during the AM and PM peak hours.

13.69 With the proposed development in place in 2017, 2022 and 2032 the roundabout junction will operate within capacity with no queues and minimal delays during the AM and PM peak hours.

13.70 Sensitivity testing was undertaken to model the impact of other permitted development with the proposed development in place in 2022 and 2032. These analyses indicated that the roundabout junction will operate within capacity, with no queues and minimal delays during AM and PM peak hours.

Signalised Junctions

13.71 Capacity assessments using the computer programme OSCADY PRO have also been carried out on the following junctions:

- R135 / Elm Road Signalised Junction
- R135 / L3125 Signalised Junction

13.72 Full details of the capacity assessments are contained in Appendix 13E – OSCADY PRO Results.

13.73 The parameters shown in the tables are defined as follows:

- **Max Degree of Saturation (%)** is a ratio of demand to capacity on each approach to the junction, with a value of 100% meaning that demand and capacity are equal and no further traffic is able to progress through the junction. Values over 90% are typically regarded as suffering from traffic congestion, with queues of vehicles beginning to form.

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- **Queue at end of Red** is the number of vehicles queued on the approach arm at the end of red.
- **Average Delay** is the average number of seconds delay to each vehicle in the time period.
- **Practical Reserve Capacity** is the capacity available relative to a capacity of 90%. A positive PRC indicates that a junction has spare capacity and may be able to accept more traffic. A negative PRC indicates that the junction is over capacity and is suffering from traffic congestion.

R135 / Elm Road Signalised Junction

13.74 The following tables show the predicted degree of saturation, average queue lengths, average vehicle delay, total junction delays and practical reserve capacity for the existing R135 / Elm Road signalised junction.

Table 13-17
R135 / Elm Road Signalised Junction : AM Peak

AM Peak		2016	2017 No Dev	2017 With Dev	2022 No Dev	2022 With Dev	2022 With Sens Flows	2032 No Dev	2032 With Dev	2032 With Sens Flows	
R135 / Elm Road Signalised Junction	R135 North	Max DoS %	28%	29%	29%	30%	30%	31%	31%	31%	32%
		Q red (pcu's)	5	5	5	5	5	6	6	6	6
		Average delay (s)	24	25	25	25	25	28	25	25	27
		PRC %	219.81	213.13	212.13	202.50	201.57	190.85	195.14	189.97	180.55
	N2 Slip Road	Max DoS %	18%	18%	18%	19%	19%	19%	20%	20%	20%
		Q red (pcu's)	1	1	1	1	1	1	1	1	1
		Average delay (s)	56	56	57	57	57	57	57	57	57
		PRC %	396.98	396.98	396.98	377.11	377.11	377.11	358.75	358.75	358.75
	R135 South	Max DoS %	29%	29%	29%	30%	30%	31%	32%	32%	32%
		Q red (pcu's)	4	4	5	5	5	5	5	5	5
		Average delay (s)	29	28	28	28	28	28	29	29	29
		PRC %	218.59	218.11	216.46	206.92	205.38	191.82	188.42	193.64	181.15

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Table 13-18
R135 / Elm Road Signalised Junction : PM Peak

PM Peak		2016	2017 No Dev	2017 With Dev	2022 No Dev	2022 With Dev	2022 With Sens Flows	2032 No Dev	2032 With Dev	2032 With Sens Flows	
R135 / Elm Road Signalised Junction	R135 North	Max DoS %	31%	31%	31%	33%	32%	34%	34%	34%	35%
		Q red (pcu's)	5	5	5	6	6	6	6	6	6
		Average delay (s)	20	20	21	21	20	23	21	21	22
		PRC %	193.95	192.36	187.66	174.14	180.97	165.20	166.81	166.14	157.61
	N2 Slip Road	Max DoS %	25%	25%	25%	26%	26%	26%	27%	27%	27%
		Q red (pcu's)	1	1	1	1	1	1	1	1	1
		Average delay (s)	59	59	59	59	59	59	60	60	60
		PRC %	261.44	261.44	261.44	250.81	250.81	250.81	231.32	231.32	231.32
	R135 South	Max DoS %	31%	32%	32%	33%	33%	34%	34%	34%	35%
		Q red (pcu's)	4	4	4	4	5	5	5	5	5
		Average delay (s)	33	33	33	33	34	32	34	34	32
		PRC %	189.15	185.94	191.94	173.79	175.23	164.92	166.68	165.30	162.20

13.75 Tables 13-17 and 13-18 above indicate that at present the R135 / Elm Road signalised junction operates within capacity with minimal queues and delays during the AM and PM peak hour

13.76 With the development in place in 2017, 2022 and 2032 the junction will continue to operate within capacity with minimal queues and delays during the AM and PM peak hour.

13.77 Sensitivity testing was undertaken to model the impact of other permitted development with the proposed development in place in 2022 and 2032. These analyses indicated that the signalised junction will operate within capacity, with no queues and minimal delays during the AM and PM peak hours.

13.78

R135 / L3125 Kilshane Crossroads Signalised Junction

13.79 The following tables show the predicted degree of saturation, average queue lengths, average vehicle delay, total junction delays and practical reserve capacity for the existing R135 / L3125 signalised junction at Kilshane Cross.

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Table 13-19
R135 / L3125 Signalised Junction : AM Peak

AM Peak		2016	2017 No Dev	2017 With Dev	2022 No Dev	2022 With Dev	2022 With Sens Flows	2022 With Link Rd	2032 No Dev	2032 With Dev	2032 With Sens Flows	2032 With Link Rd	
R135 / L3125 Signalised junction	R135 North	Max DoS %	47%	47%	49%	49%	51%	50%	32%	53%	53%	54%	34%
		Q red (pcu's)	12	12	12	12	12	13	8	13	13	14	9
		Average delay (s)	42	42	44	43	44	43	39	44	44	44	40
		PRC %	92.75	91.34	83.93	84.99	77.84	79.79	178.98	70.97	70.58	68.97	167.01
	L3125 East	Max DoS %	84%	85%	85%	90%	89%	90%	86%	92%	92%	94%	90%
		Q red (pcu's)	17	17	17	19	19	19	17	21	20	23	19
		Average delay (s)	51	51	54	59	59	59	55	61	61	79	65
		PRC %	6.11	5.38	5.38	-0.38	1.86	-0.38	4.56	-2.21	-2.21	-4.36	0.33
	R135 South	Max DoS %	84%	85%	88%	88%	90%	91%	84%	94%	94%	95%	87%
		Q red (pcu's)	12	12	13	13	14	15	12	15	15	18	14
		Average delay (s)	95	97	103	100	110	97	72	88	89	109	79
		PRC %	6.78	5.83	2.90	2.47	-0.39	-0.94	7.12	-4.02	-4.29	-5.43	2.93
	L3125 West	Max DoS %	88%	89%	86%	89%	89%	93%	85%	93%	92%	93%	89%
		Q red (pcu's)	12	13	12	13	13	14	12	14	14	15	14
		Average delay (s)	82	83	81	81	89	89	39	88	89	102	87
		PRC %	1.30	0.66	4.94	1.09	1.40	-2.74	5.54	-2.77	-2.77	-2.77	1.19

- 13.80 Table 13.19 above indicates that at present the R135 / L3125 signalised junction at Kilshane Cross is operating at capacity with queues and delays during the AM peak hour.
- 13.81 With the development in place in 2017 the signalised junction will continue to operate at capacity with queues and delays during the AM peak hour.
- 13.82 With the development in place in 2022 and 2032 the junction will reach its maximum capacity with queues and delays forming during the AM peak hour.
- 13.83 It should be noted that without the proposed development in place in 2022 and 2032, the junction will have reached its maximum capacity with queues and delays forming during the AM peak hour.
- 13.84 Sensitivity testing of granted planning applications within the area indicates that the signalised junction will have reached its maximum capacity in 2022 and 2032, with queues and delays forming during the AM peak hour.
- 13.85 With the opening of the Western (Airport) Link Road from the R135 / N2 roundabout to Dublin Airport, the impact on the R135 / L3125 signalised junction will be reduced. For the purposes of this assessment it was assumed that 50% of the traffic flows travelling from the R135 North direction to the L3125 East and from the L3125 East direction to the R135 North would now travel via the Western Link Road. As a result, the signalised junction will operate within capacity in 2022 and 2032, with queues and delays during the AM peak hour.

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**Table 13-20
R135 / L3125 Signalised Junction : PM Peak**

PM Peak		2016	2017 No Dev	2017 With Dev	2022 No Dev	2022 With Dev	2022 With Sens Flows	2022 With Link Rd	2032 No Dev	2032 With Dev	2032 With Sens Flows	2032 With Link Rd	
R135 / L3125 Signalised junction	R135 North	Max DoS %	44%	44%	43%	46%	45%	46%	29%	46%	46%	49%	29%
		Q red (pcu's)	9	9	9	10	10	10	6	9	10	10	6
		Average delay (s)	47	47	46	53	53	54	42	46	46	47	42
		PRC %	102.77	101.43	110.10	92.53	103.41	96.21	211.73	95.33	97.14	85.67	213.29
	L3125 East	Max DoS %	91%	92%	92%	96%	96%	98%	87%	99%	99%	99%	91%
		Q red (pcu's)	19	20	20	23	23	25	16	30	26	26	18
		Average delay (s)	63	64	64	72	72	79	65	106	81	81	69
		PRC %	-1.40	-2.41	-2.41	-6.26	-6.29	-8.46	3.22	-9.38	-9.38	-9.38	-1.35
	R135 South	Max DoS %	93%	93%	92%	98%	96%	105%	90%	108%	99%	102%	90%
		Q red (pcu's)	13	13	13	15	14	11	9	16	10	18	13
		Average delay (s)	119	121	121	131	133	165	96	265	142	145	113
		PRC %	-3.47	-4.14	-4.93	-8.25	-6.90	-14.13	-0.05	-17.33	-9.19	-12.11	-0.55
	L3125 West	Max DoS %	89%	90%	93%	94%	97%	94%	88%	97%	100%	101%	91%
		Q red (pcu's)	15	15	17	18	19	18	15	21	22	22	17
		Average delay (s)	73	74	82	82	92	82	42	113	100	100	73
		PRC %	0.37	-0.36	-3.63	-4.52	-7.65	-4.52	1.74	-7.52	-10.56	-10.56	-1.46

13.86 Table 13-20 above indicates that at present the R135 / L3125 signalised junction at Kilshane Cross operates outside capacity with queues and delays during the PM peak hour.

13.87 In 2017, 2022 and 2032 without the proposed development in place, the R135 / L3125 signalised junction operates outside capacity with queues and delays during the PM peak hour.

13.88 With the development in place in 2017, 2022 and 2032, the junction will continue to operate outside capacity, with queues and delays during the PM peak hour.

13.89 Sensitivity testing of granted planning applications within the area indicates that the signalised junction will have reached its maximum capacity in 2022 and 2032, with queues and delays forming during the AM peak hour.

13.90 With the opening of the Western (Airport) Link Road from the R135 / N2 roundabout to Dublin Airport, the impact on the R135 / L3125 signalised junction will be reduced. For the purposes of this assessment it was assumed that 50% of the traffic flows travelling from the R135 North direction to the L3125 East and from the L3125 East direction to the R135 North would now travel via the Western Link Road. As a result the signalised junction will operate within capacity in 2022 and 2032, with queues and delays during the PM peak hour.

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Road Capacity Assessment

- 13.91 A capacity assessment of the R135 has been undertaken to determine the impact the proposed development flows will have on the R135 Regional Road. The AM and PM peak hour traffic counts have been converted to AADT (Annual Average Daily Traffic) using the methodology in TII Project Appraisal Guidelines 'Unit 16.2 Expansion Factors for Short Period Traffic Counts'.
- 13.92 The vehicle flows (Annual Average Daily Traffic) given in Table 6/1 of TD 9/12 of the Design Manual for Roads and Bridges represent the approximate two-way flows which correspond to Level of Service D in reasonably level terrain. This is the level of service at which passing becomes extremely difficult and begins to affect the overall flow of the road.
- 13.93 Table 6/1 of the TII TD 9/12 indicates that the R135 would be considered as a Type 2 Single Carriageway, with a capacity of 8,600 AADT for a Level of Service D.
- 13.94 The following tables calculate the existing AADT for the R135 and the future AADT for the R135 when the development is operational in the years 2017, 2022 and 2032.

**Table 13-21
2016 Existing Annual Average Daily Traffic**

STEP 1: Conversion of short period traffic counts to average daily traffic				
	Flows	PAG* factor	Daily flow (Flows/PAG*factor)	Average Daily traffic for 22 nd June 2016
AM peak (07:30 – 08:30)	633	0.07	9,042	8,315
PM peak (16:45 – 17:45)	683	0.09	7,588	
* Project Appraisal Guidelines				
STEP 2: Conversion of average daily traffic to weekly average daily traffic				
Average Daily traffic for 22 nd June 2016		PAG* factor	WADT Weekly Average Daily Traffic (Avg. Daily traffic * PAG factor)	
8,315		0.97	8,065	
STEP 3: Conversion of weekly average daily traffic to annual average daily traffic)				
WADT		PAG* factor	AADT Annually Average Daily Traffic (WADT * PAG factor)	
8,065		0.96	7,742	

- 13.95 From Table 13-21 above, the existing AADT for the R135 is 7,742 AADT which is below the recommended 8,600 AADT for a Level of Service D for a Type 2 Single Carriageway.

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Table 13-22
2017 Proposed Annual Average Daily Traffic with Development Flows

STEP 1: Conversion of short period traffic counts to average daily traffic				
	Flows	PAG* factor	Daily flow (Flows/PAG*factor)	Average Daily traffic for 22 nd June 2017
AM peak (07:30 – 08:30)	639	0.07	9,128	8,402
PM peak (16:45 – 17:45)	691	0.09	7,677	
<i>* Project Appraisal Guidelines</i>				
STEP 2: Conversion of average daily traffic to weekly average daily traffic				
Average Daily traffic for 22 nd June 2017		PAG* factor	WADT <i>Weekly Average Daily Traffic</i> (Avg. Daily traffic * PAG factor)	
8,402		0.97	8,150	
STEP 3: Conversion of weekly average daily traffic to annual average daily traffic)				
WADT		PAG* factor	AADT <i>Annually Average Daily Traffic</i> (WADT * PAG factor)	
8,150		0.96	7,824	

- 13.96 From Table 13-22 above, the AADT for the R135 in 2017 with the development fully operational is 7,824 AADT which is below the recommended 8,600 AADT for a Level of Service D for a Type 2 Single Carriageway.

Table 13-23
2022 Proposed Annual Average Daily Traffic with Development Flows

STEP 1: Conversion of short period traffic counts to average daily traffic				
	Flows	PAG* factor	Daily flow (Flows/PAG*factor)	Average Daily traffic for 22 nd June 2032
AM peak (07:30 – 08:30)	659	0.07	9,414	8,674
PM peak (16:45 – 17:45)	714	0.09	7,933	
<i>* Project Appraisal Guidelines</i>				
STEP 2: Conversion of average daily traffic to weekly average daily traffic				
Average Daily traffic for 22 nd June 2016		PAG* factor	WADT <i>Weekly Average Daily Traffic</i> (Avg. Daily traffic * PAG factor)	
8,674		0.97	8,414	
STEP 3: Conversion of weekly average daily traffic to annual average daily traffic)				
WADT		PAG* factor	AADT <i>Annually Average Daily Traffic</i> (WADT * PAG factor)	
8,414		0.98	8,246	

- 13.97 From Table 13-23 above, the AADT for the R135 in 2022 with the development fully operational is 8,246 AADT which is below the recommended 8,600 AADT for a Level of Service D for a Type 2 Single Carriageway.

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Table 13-24
2032 Proposed Annual Average Daily Traffic with Development Flows

STEP 1: Conversion of short period traffic counts to average daily traffic				
	Flows	PAG* factor	Daily flow (Flows/PAG*factor)	Average Daily traffic for 22 nd June 2032
AM peak (07:30 – 08:30)	688	0.07	9,828	9,036
PM peak (16:45 – 17:45)	742	0.09	8,244	
<i>* Project Appraisal Guidelines</i>				
STEP 2: Conversion of average daily traffic to weekly average daily traffic				
Average Daily traffic for 22 nd June 2016		PAG* factor	WADT <i>Weekly Average Daily Traffic</i> (Avg. Daily traffic * PAG factor)	
9,036		0.97	8,765	
STEP 3: Conversion of weekly average daily traffic to annual average daily traffic)				
WADT		PAG* factor	AADT <i>Annually Average Daily Traffic</i> (WADT * PAG factor)	
8,765		0.98	8,590	

13.98 From the table above the AADT for the R135 in 2032 with the development fully operational is 8,590 AADT which is below the recommended 8,600 AADT for a Level of Service D for a Type 2 Single Carriageway.

PROPOSED MITIGATION MEASURES

13.99 It has been demonstrated in this chapter that the proposed development, involving increased intake and output to / from the existing C&D waste recovery facility at Huntstown, would generate an increase in HGV movements on the surrounding local network when compared to existing levels. HGV traffic can be of particular concern to both local residents and highway users, and the mitigation measures outlined below are designed to alleviate any adverse impacts:

- Roadstone Ltd would adhere to a routing policy to ensure all movements are made via the strategic road network to avoid HGV's passing through residential areas as far as is practical.
- Roadstone Ltd would employ a policy of safety and environmental awareness for all HGV drivers accessing the site.
- It may be possible to reduce the number of additional traffic movements generated by the increased activity at the C&D waste recovery facility by encouraging "backloading", whereby trucks delivering aggregates / blocks from the adjoining facility will return with pre-sorted construction and demolition waste for the recovery facility. Recycled secondary aggregates may also be dispatched off-site in place of virgin aggregates from the adjoining quarry.
- Efforts should be made to promote backloading practices as aside from reducing traffic impact, it can offer opportunities to significantly reduce haulage costs, enhance the viability of the recovery operation and contribute to a reduction in traffic related emissions generated by the construction and development sector.

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CONCLUSIONS

- 13.100 This chapter assesses the traffic and transport implications of the proposed increase in the current rate of waste intake at its licensed construction & demolition recovery facility at Huntstown Quarry from a maximum of 24,950 tonnes per annum at present to a maximum of 95,000 tonnes per annum in future years
- 13.101 The principal objective of this assessment was to provide a detailed consideration of the proposed development in terms of highways and transportation planning. This includes details of all the traffic and movement activity associated with the proposed development and any resulting traffic and transport related impacts.
- 13.102 In accordance with the scale of development and location, realistic consideration has been given to the non-car accessibility of the site. This demonstrated that non-car accessibility is adequate for the type of development located in this area.
- 13.103 The application site is well located in terms of access to the strategic highway network and all HGV traffic would be routed on roads considered suitable to accommodate frequent HGV movement.
- 13.104 Junction capacity assessment was carried out to determine the impact the additional development trips would have on the existing junctions within the vicinity of the proposed development. The analysis showed that the existing R135 / Elm Road signalised junction and the R135 / N2 Slip Road roundabout junction will operate within capacity when the development is operational in 2017, 2022 and 2032.
- 13.105 The R135 / N2 Slip Road priority junction and the R135 / L3125 signalised junction at Kilshane Cross are currently operating at capacity. With the development operational in 2017, 2022 and 2032 both junctions will continue to operate at capacity with queues and delays during the AM and PM peak hours. It should be noted the development flows will have an insignificant impact on the operational performance of both junctions as the junctions are currently operating at capacity and will continue to do so with no development in place in 2017, 2022 and 2032 during the AM and PM peak hours.
- 13.106 Sensitivity testing was undertaken for other committed and other planned development with the proposed development in place in 2022 and 2032. These analyses showed that the existing R135 / Elm Road signalised junction and the R135 / N2 Slip Road roundabout junction will operate within capacity when the developments are operational 2022 and 2032. The R135 / N2 Slip Road priority junction and the R135 / L3125 signalised junction are currently operating at capacity. With all developments operational in 2022 and 2032 both junctions will continue to operate at capacity with queues and delays during the AM and PM peak hours.
- 13.107 With the future opening of the Western Link Road, traffic flows through the R135 / L3125 signalised junction at Kilshane Cross travelling to and from Dublin Airport will re-distribute onto the Western Link Road. As a result, the R135 / L3125 signalised junction will operate within capacity in 2022 and 2032 during the AM and PM peak hours with the proposed development in place.
- 13.108 A road capacity assessment of the R135 (North Road) was carried out to determine the impact the additional development flows would have on the R135 Regional Road. The AM and PM peak hour flows were converted to

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AADT using the methodology in TII Project Appraisal Guidelines. Table 6/1 of the TII TD 9/12 indicated that the R135 would be considered as a Type 2 Single Carriageway with a capacity of 8,600 AADT for a level of Service D.

- 13.109 The assessment showed that in 2016 the R135 operates within capacity for a level of service D with an existing AADT level 7,742 vehicles.
- 13.110 In 2017, 2022 and 2032 with the additional development trips and an increase in the background flows, the R135 will have a proposed AADT of 7,824 in 2017, a proposed AADT of 8,246 in 2022 and a proposed AADT of 8,590 in 2032 which is below the recommended AADT capacity for a Level of Service D for a Type 2 Single Carriageway.
- 13.111 A review of accident records on the surrounding highway network covering the period from 2006 to 2013 showed that no fatal or serious incidents were recorded at the North Road access to the Huntstown Quarry Complex. The one fatal incident and one serious incident recorded both occurred at Kilshane Cross.
- 13.112 Three minor incidents occurred in close proximity to the North Road quarry access. However it appears these took place prior to the N2 road realignment and upgrading. None are therefore specifically relevant to the development proposal, both in terms of location and incident detail. It is considered that the proposed development would not have a significant impact on road safety.
- 13.113 Overall it is considered that the development proposal would have a minimal impact in terms of highway and transportation. For the above reasons the proposed development of the site accords with the national, regional and county planning policies and is considered to be acceptable in traffic and transport terms.

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APPENDICES

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APPENDIX 13-A TRAFFIC COUNT DATA

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Entry: Arm A - R135 (N)										Destination: Arm B - R135 (S)										Destination: Arm C - N2 On/Off Slip Road										Arm Totals
Destination: Arm A - R135 (N)										Destination: Arm B - R135 (S)										Destination: Arm C - N2 On/Off Slip Road										Arm Totals
CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	Arm Totals						
07:00	0	0	0	0	0	0	0	15	5	1	0	0	0	1	22	18	8	2	1	0	0	0	29	51						
07:15	0	0	0	0	0	0	0	17	6	3	3	2	0	0	31	15	6	4	1	1	0	0	27	58						
07:30	0	0	0	0	0	0	0	24	10	0	1	2	1	0	38	17	6	6	1	1	0	0	31	69						
07:45	0	0	0	0	0	0	0	34	6	1	1	3	0	2	47	22	3	1	1	0	0	1	29	76						
1 Hr	0	0	0	0	0	0	0	90	27	5	5	7	1	3	138	73	23	13	4	2	1	0	116	254						
08:00	0	0	0	0	0	0	0	37	8	0	0	2	0	1	48	22	7	2	3	0	0	0	34	82						
08:15	0	0	0	0	0	0	0	31	4	1	0	0	1	0	37	28	8	3	1	3	2	0	45	82						
08:30	0	0	0	0	0	0	0	19	4	1	0	1	0	0	25	20	5	3	1	0	0	0	29	54						
08:45	0	0	0	0	0	0	0	19	3	1	1	2	0	0	26	23	5	1	1	2	1	0	33	59						
1 Hr	0	0	0	0	0	0	0	106	19	3	1	5	1	1	136	93	25	9	6	5	3	0	141	277						
09:00	0	0	0	0	0	0	0	20	3	1	1	1	0	1	27	20	3	1	0	0	0	0	24	51						
09:15	0	0	0	0	0	0	0	25	4	1	0	2	0	0	32	15	2	0	3	0	0	0	20	52						
09:30	0	0	0	0	0	0	0	28	6	0	0	0	0	0	34	14	4	2	1	0	0	0	21	55						
09:45	0	0	0	0	0	0	0	30	5	0	0	0	0	0	36	11	8	2	3	0	0	0	24	60						
1 Hr	0	0	0	0	0	0	0	103	18	2	1	4	0	1	129	60	17	5	7	0	0	0	89	218						
10:00	0	0	0	0	0	0	0	11	4	1	1	2	0	1	20	13	10	3	3	0	0	0	29	49						
10:15	0	0	0	0	0	0	0	13	4	1	3	0	0	0	21	13	5	1	0	0	0	0	19	40						
10:30	0	0	0	0	0	0	0	22	5	1	0	1	0	0	29	9	1	1	0	0	2	0	13	42						
10:45	0	0	0	0	0	0	0	9	2	1	2	1	1	0	16	15	3	1	1	0	0	0	22	38						
1 Hr	0	0	0	0	0	0	0	55	15	4	6	4	1	1	86	50	19	7	5	0	2	0	83	169						
11:00	0	0	0	0	0	0	0	16	7	0	1	2	0	1	27	3	1	3	0	0	0	0	7	34						
11:15	0	0	0	0	0	0	0	17	3	2	1	0	0	0	23	11	1	1	0	0	0	0	13	36						
11:30	0	0	0	0	0	0	0	22	2	4	1	1	0	0	30	7	3	3	2	0	0	0	15	45						
11:45	0	0	0	0	0	0	0	23	5	0	0	2	0	0	31	10	2	1	1	0	0	0	14	45						
1 Hr	0	0	0	0	0	0	0	78	17	6	4	5	0	1	111	31	7	8	3	0	0	0	49	160						
12:00	0	0	0	0	0	0	0	12	5	1	1	0	0	0	19	10	0	3	1	0	0	0	14	33						
12:15	0	0	0	0	0	0	0	15	5	2	0	1	1	1	25	15	3	0	1	0	1	0	20	45						
12:30	0	1	1	0	0	0	2	19	5	0	1	0	0	0	25	13	6	2	0	0	1	0	22	49						
12:45	1	1	0	0	0	0	1	16	4	2	0	1	0	0	23	9	9	1	1	0	0	0	11	35						
1 Hr	0	2	1	0	0	0	3	62	19	5	2	2	1	1	92	47	10	6	2	0	2	0	67	162						
13:00	0	0	0	0	0	0	0	14	6	0	0	2	0	0	22	17	4	1	1	0	0	0	23	45						
13:15	1	0	0	0	0	0	3	29	7	2	1	0	0	0	39	10	1	1	0	0	0	0	12	54						
13:30	0	0	0	0	0	0	0	27	5	1	1	1	0	1	37	20	3	0	1	0	0	0	24	61						
13:45	0	0	0	0	0	0	0	19	3	0	2	1	1	0	26	22	2	2	0	0	0	0	22	52						
1 Hr	1	0	0	0	0	0	3	89	21	3	4	4	2	1	124	69	10	4	2	0	0	0	85	212						
14:00	1	0	0	0	0	0	1	23	3	1	0	1	0	0	28	19	4	2	1	0	0	0	26	55						
14:15	0	0	0	0	0	0	0	19	9	1	0	1	0	0	30	9	4	4	1	0	0	0	18	48						
14:30	0	0	0	0	0	0	0	26	7	0	0	1	0	0	34	11	5	1	1	0	0	0	18	52						
14:45	0	0	0	0	0	0	0	28	7	0	0	1	1	2	37	16	6	2	0	0	0	0	24	61						
1 Hr	1	0	0	0	0	0	1	94	26	2	0	4	1	2	129	55	19	9	3	0	0	0	86	216						
15:00	0	0	0	0	0	0	0	16	3	0	0	2	0	0	21	23	2	2	0	0	0	0	27	48						
15:15	0	0	0	0	0	0	0	20	1	0	0	0	0	0	21	18	3	3	0	0	0	0	24	45						
15:30	0	0	0	0	0	0	0	19	6	0	1	0	0	0	26	12	5	1	0	0	0	0	18	46						
15:45	0	0	0	0	0	0	0	18	3	1	1	1	0	1	25	11	3	2	1	0	1	0	18	43						
1 Hr	0	0	0	0	0	0	0	73	13	1	3	4	0	1	95	64	13	8	1	0	1	0	87	182						
16:00	0	0	0	0	0	0	0	17	4	3	0	2	0	0	26	12	5	0	0	0	0	0	17	43						
16:15	0	0	0	0	0	0	0	18	2	0	0	0	4	0	24	19	5	0	0	1	1	0	26	50						
16:30	0	0	0	0	0	0	0	15	1	0	0	1	0	0	18	16	1	0	1	1	0	0	21	39						
16:45	0	0	0	0	0	0	0	28	3	1	0	1	0	0	33	16	7	0	0	0	0	0	23	56						
1 Hr	0	0	0	0	0	0	0	79	10	4	0	4	4	0	101	63	21	1	1	1	1	0	87	188						
17:00	0	0	0	0	0	0	0	23	4	0	1	1	0	3	32	19	6	1	0	0	0	0	26	58						
17:15	0	0	0	0	0	0	0	13	6	0	1	2	0	1	23	15	1	1	2	0	0	0	18	41						
17:30	0	0	0	0	0	0	0	14	6	0	1	0	0	0	23	15	1	1	0	0	0	0	18	42						
17:45	0	0	0	0	0	0	0	18	1	0	0	0	1	0	20	13	2	0	0	0	0	0	16	36						
1 Hr	0	0	0	0	0	0	0	68	17	1	3	3	2	4	98	62	16	2	2	3	0	0	79	177						
18:00	0	1	0	0	0	0	1	17	1	1	1	3	0	1	24	20	3	0	0	0	0	0	24	49						
18:15	0	0	0	0	0	0	0	19	1	0	0	0	0	0	20	14	3	2	0	0	0	0	19	39						
18:30	0	0	0	0	0	0	0	17	1	0	0	0	0	0	18	14	3	2	0	0	0	0	19	39						
18:45	0	0	0	0	0	0	0	17	1	0	0	3	0	0	21	15	3	1	1	0	0	0	18	37						
1 Hr	0	1	0	0	0	0	1	70	4	1	1	6	0	1	93	60	11	3	1	0	0	0	75	159						
12 Hrs	2	3	1	0	0	2	8	967	206	37	30	52	13	17	1322	727	187	75	37	8	10	0	1044	2374						
Check							8	967	206	37	30	52	13	17	1322	727	187	75	37	8	10	0	1044	2374						

Entry: Arm B - R135 (S)										Destination: Arm A - R135 (N)										Destination: Arm C - N2 On/Off Slip Road										Arm Totals
Destination: Arm B - R135 (S)										Destination: Arm A - R135 (N)										Destination: Arm C - N2 On/Off Slip Road										Arm Totals
CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	Arm Totals						
07:00	9	2	0	0	0	0	11	0	0	0	0	0	0	0	0	3	0	2	1	0	0	0	6	17						
07:15	6	6	0	1	0	0	13	0	0	0	0	0	0	0	0	8	1	2	3	0	0	0	14	27						
07:30	1	3	0	0	3	0	7	0	0	0	0	0	0	0	0	12	4	0	3	1	0	0	20	27						
07:45	16	4	4	1	0	0	15	0	0	0	0	0	0	0	0	11	1	0	0	0	0	0	21	36						
1 Hr	26	15	0	2	3	0	46	0	0	0	0	0	0	0	0	34	8	4	14	1	0	0	61	107						
08:00	7	2	0	1	1	0	12	0	0	0	0	0	0	0	0	12	5	0	3	0	0	0	20	32						
08:15	5	5	0	0	1	2	14	0	0	0	0	0	0	0	0	14	6	3	4	0	0	0	27	41						
08:30	6	1	3	0	1	0	11	0	0	0	0	0																		

Entry: Arm C - N2 On/Off Slip Road

	Destination: Arm A - R135 (N)								Destination: Arm B - R135 (S)								Destination: Arm C - N2 On/Off Slip Road								Arm Totals		
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total			
07:00	3	2	3	2	0	0	0	10	23	6	2	1	0	0	0	32	0	0	0	0	0	0	0	0	42		
07:15	2	4	1	0	0	0	0	7	25	5	0	2	1	0	1	34	0	1	0	0	0	0	0	0	42		
07:30	8	4	0	0	0	0	0	12	39	9	1	5	2	1	0	57	0	0	0	0	0	0	0	0	69		
07:45	57	6	5	0	0	0	0	62	30	13	0	2	0	0	0	45	0	0	0	0	0	0	0	0	62		
1 Hr	22	16	5	2	0	0	0	45	117	33	3	10	3	1	1	168	0	2	0	0	0	0	0	0	215		
08:00	10	4	0	0	0	0	0	14	27	8	2	2	0	0	0	39	0	0	0	0	0	0	0	0	53		
08:15	16	4	0	0	0	0	1	21	34	6	1	4	0	0	0	45	0	0	0	0	0	0	0	0	66		
08:30	19	6	0	1	0	0	0	26	37	7	1	3	0	0	0	48	0	0	0	0	0	0	0	0	74		
08:45	13	4	4	1	1	0	0	19	34	6	2	3	0	0	0	45	0	0	0	0	0	0	0	0	64		
1 Hr	58	18	1	2	0	1	0	80	132	27	6	12	0	0	0	177	0	0	0	0	0	0	0	0	257		
09:00	11	4	3	1	0	0	0	19	25	8	2	3	3	0	0	41	0	0	0	0	0	0	0	0	60		
09:15	13	3	1	1	0	0	0	18	19	5	1	6	0	0	0	31	0	0	0	0	0	0	0	0	49		
09:30	16	3	2	2	0	0	0	23	11	4	0	6	1	0	0	22	0	0	0	0	0	0	0	0	45		
09:45	16	5	0	0	0	0	0	21	15	3	0	5	1	0	0	24	0	0	0	0	0	0	0	0	42		
1 Hr	56	16	6	4	0	0	0	82	70	20	3	15	5	0	0	113	0	0	1	0	0	0	0	0	196		
10:00	11	6	3	1	0	0	0	22	10	4	1	4	0	0	0	19	1	2	0	0	0	0	0	0	44		
10:15	9	3	2	1	0	0	0	15	17	1	2	1	0	0	0	21	1	0	0	0	0	0	0	0	37		
10:30	9	2	0	0	0	0	0	11	12	4	1	3	0	0	0	21	1	0	0	0	0	0	0	0	33		
10:45	16	3	4	0	0	0	0	17	14	2	0	5	1	0	0	22	0	0	0	0	0	0	0	0	39		
1 Hr	39	14	9	2	0	0	0	65	53	11	4	14	1	0	0	83	3	2	0	0	0	0	0	0	153		
11:00	8	2	1	0	0	0	0	11	10	4	2	2	0	0	0	18	0	0	0	0	0	0	0	0	29		
11:15	12	5	2	2	0	0	0	23	14	3	1	0	0	0	0	18	1	0	0	0	0	0	0	0	42		
11:30	14	3	1	0	0	0	0	19	11	5	1	2	1	0	0	20	1	0	0	1	0	0	0	0	41		
11:45	10	2	2	1	0	0	0	15	11	5	0	4	0	0	0	20	0	0	0	0	0	0	0	0	41		
1 Hr	50	12	6	3	0	3	0	74	46	17	4	8	1	0	0	76	2	0	0	1	0	0	0	0	153		
12:00	12	3	1	1	0	0	0	18	12	3	0	1	0	0	0	16	3	0	0	0	0	0	0	0	37		
12:15	9	2	1	1	0	0	0	13	18	3	1	2	1	0	0	25	0	0	0	0	0	0	0	0	38		
12:30	17	4	4	2	0	1	0	28	19	3	1	0	1	0	0	24	0	0	0	0	0	0	0	0	52		
12:45	17	5	1	1	0	0	0	25	14	2	0	5	0	0	0	25	0	0	0	0	0	0	0	0	48		
1 Hr	52	14	10	4	0	2	0	82	63	11	2	9	5	0	0	90	3	0	0	0	0	0	0	0	3	175	
13:00	15	3	3	0	0	0	0	21	17	3	1	2	0	0	0	23	0	0	0	0	0	0	0	0	44		
13:15	16	4	3	0	0	0	0	23	19	3	2	1	0	0	0	25	0	0	0	0	0	0	0	0	48		
13:30	27	5	2	0	0	0	0	34	25	3	1	1	0	0	0	30	2	0	0	0	0	0	0	0	66		
13:45	17	5	1	1	0	0	0	24	18	4	4	2	0	0	0	31	0	0	0	0	0	0	0	0	48		
1 Hr	75	17	9	1	0	0	0	102	79	13	4	6	0	0	0	102	2	0	0	0	0	0	0	0	0	206	
14:00	15	5	0	1	0	0	0	22	10	2	2	4	1	0	1	20	0	0	0	0	0	0	0	0	42		
14:15	16	5	2	2	0	0	0	25	15	1	2	5	1	0	0	24	0	0	0	0	0	0	0	0	49		
14:30	11	4	2	0	0	0	0	17	18	6	3	2	1	0	0	30	0	0	0	0	0	0	0	0	47		
14:45	11	2	3	0	0	0	0	16	12	4	2	5	0	0	0	23	0	0	0	0	0	0	0	0	39		
1 Hr	53	16	7	3	1	0	0	80	55	13	9	16	3	0	1	97	0	0	0	0	0	0	0	0	0	177	
15:00	20	4	2	0	1	0	0	27	13	3	0	4	1	0	0	21	0	1	0	0	0	0	0	0	49		
15:15	21	4	2	0	0	0	0	27	15	5	1	3	0	0	0	24	0	0	0	0	0	0	0	0	51		
15:30	11	1	2	2	0	0	0	16	22	3	2	3	0	0	0	30	0	0	0	0	0	0	0	0	46		
15:45	13	6	0	0	0	0	0	22	13	3	0	5	0	0	0	24	0	0	0	0	0	0	0	0	46		
1 Hr	65	15	6	5	1	0	0	92	63	14	6	15	1	0	0	99	0	1	0	0	0	0	0	0	1	192	
16:00	17	9	3	1	0	0	0	31	12	2	0	1	0	0	0	15	1	0	0	0	0	0	0	0	47		
16:15	19	7	2	3	0	0	0	31	10	3	2	7	1	0	0	23	0	0	0	0	0	0	0	0	54		
16:30	30	7	0	1	1	0	0	39	29	7	1	1	0	0	0	38	1	1	0	0	0	0	0	0	68		
16:45	18	7	2	2	0	0	0	29	23	3	1	3	0	0	0	31	0	0	0	0	0	0	0	0	58		
1 Hr	84	30	7	7	1	0	0	130	74	15	4	12	2	0	0	107	2	0	0	0	0	0	0	0	0	239	
17:00	26	5	3	0	0	1	0	35	30	5	0	2	0	0	0	37	0	0	1	0	0	0	0	0	73		
17:15	18	8	2	0	0	0	0	28	35	4	2	1	0	0	0	42	0	0	0	0	0	0	0	0	70		
17:30	27	5	1	0	1	0	0	35	44	0	1	5	0	0	0	50	1	1	0	0	0	0	0	0	86		
17:45	26	3	2	1	1	0	0	35	22	3	2	2	0	0	0	29	1	0	0	0	0	0	0	0	83		
1 Hr	97	21	8	2	1	2	0	131	131	12	5	10	0	0	0	158	2	0	1	0	0	0	0	0	3	292	
18:00	26	3	3	1	0	1	0	34	20	4	2	4	0	1	0	31	0	0	0	0	0	0	0	0	65		
18:15	22	4	1	3	0	1	0	31	11	3	0	2	0	1	1	18	0	0	0	0	0	0	0	0	49		
18:30	25	2	2	1	0	0	0	30	17	2	0	4	1	0	0	24	0	0	0	0	0	0	0	0	54		
18:45	22	3	1	1	0	0	0	27	9	0	0	2	1	0	0	12	0	0	0	0	0	0	0	0	39		
1 Hr	95	12	6	5	0	4	0	122	57	9	2	12	2	2	1	85	0	0	0	0	0	0	0	0	0	207	
12 Hrs	746	201	80	40	4	12	2	1085	940	195	52	139	23	3	3	1355	14	5	2	1	0	0	0	0	22	2462	
Check								1085								1355										2462	
Total	746	201	80	40	4	12	2	1085	940	195	52	139	23	3	3	1355	14	5	2	1	0	0	0	0	0	2462	
Check								1085								1355											2462

ORIGIN SUMMARY

	Origin: Arm A - R135 (N)								Origin: Arm B - R135 (S)								Origin: Arm C - N2 On/Off Slip Road								Origin Totals
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	
07:00	33	13	3	1	0	0	0	51	12	2	2	1	0	0	0	17	26	8	5	3	0	0	0	0	42
07:15	32	12	7	4	3	0	0	58	14	7	2	4	0	0	0	27	27	10	1	2	1	0	1	4	42
07:30	41	16	6	2	3	1	0	69	13	7	0	3	4	0	0	27	47	13	1	5	2	1	0	0	69
07:45	57	9	2	2	3	1	2	75	21	7	0	8	0	0	0	36	27	7	0	1	0	0	0	0	63
1 Hr	163	50	18	9	2	1	2	254	60	23	4	1													

DESTINATION SUMMARY

	Destination: Arm A - R135 (N)							Total	Destination: Arm B - R135 (S)							Total	Destination: Arm C - N2 On/Off Slip Road							Total	Dest Totals
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL		CAR	LGV	OGV1	OGV2	PSV	MCL	PCL		CAR	LGV	OGV1	OGV2	PSV	MCL	PCL		
07:00	12	4	3	2	0	0	0	21	38	11	3	1	0	0	1	54	21	8	4	2	0	0	0	35	110
07:15	8	10	1	1	0	0	0	20	42	11	3	5	3	0	1	65	23	8	6	4	1	0	0	42	127
07:30	9	7	0	0	3	0	0	19	63	19	1	6	4	2	0	95	29	10	6	4	2	0	0	51	165
07:45	19	10	1	1	0	0	0	31	54	19	1	3	3	0	2	92	34	7	1	8	0	1	0	51	174
1 Hr	48	31	5	4	3	0	0	91	207	60	8	15	10	2	4	306	107	33	17	18	3	1	0	179	576
08:00	17	6	0	1	1	0	1	26	64	16	2	2	2	0	1	87	34	12	2	6	0	0	0	54	167
08:15	21	9	0	0	1	3	1	35	65	10	2	4	0	1	0	82	42	14	6	5	3	2	0	72	189
08:30	25	7	3	1	1	0	0	37	56	11	2	3	1	0	0	73	32	7	4	5	0	1	0	49	159
08:45	20	7	1	2	1	0	0	31	53	10	3	4	2	0	0	72	37	8	2	4	2	1	0	54	157
1 Hr	83	29	4	4	4	3	2	129	238	47	9	13	5	1	1	314	145	41	14	20	5	4	0	229	672
09:00	18	6	5	2	2	0	0	33	45	11	3	4	4	0	1	68	35	5	4	6	0	0	0	50	151
09:15	19	8	2	4	1	0	0	34	44	9	2	6	2	0	0	63	25	3	2	12	0	0	0	42	139
09:30	29	4	4	3	0	0	0	40	39	10	0	6	1	0	0	56	23	7	3	3	0	0	0	36	132
09:45	21	12	1	1	2	0	0	37	45	9	0	0	2	0	0	55	17	11	5	6	0	0	0	39	131
1 Hr	87	30	12	10	5	0	0	144	173	38	5	16	9	0	1	242	100	26	14	27	0	0	0	167	553
10:00	22	12	3	1	1	0	1	40	21	8	2	5	2	0	1	39	24	14	4	6	1	0	0	49	128
10:15	25	6	5	2	0	0	0	38	30	5	3	4	0	0	0	42	23	11	1	5	1	0	0	41	121
10:30	26	10	1	2	1	0	0	40	35	9	2	3	2	0	0	51	21	5	3	5	1	2	0	37	128
10:45	25	5	5	0	1	0	0	36	23	4	1	8	1	1	0	38	32	6	3	4	1	0	0	46	133
1 Hr	98	33	14	5	3	0	1	154	109	26	8	20	5	1	1	170	100	36	11	20	4	2	0	173	497
11:00	25	4	2	2	1	0	0	34	26	11	2	3	2	0	1	45	8	3	4	2	1	0	0	18	97
11:15	29	9	4	4	2	2	2	52	31	6	3	1	0	0	0	41	19	3	1	3	1	0	0	27	120
11:30	27	9	3	1	0	1	0	41	33	7	5	3	2	0	0	50	17	6	7	7	1	0	0	38	129
11:45	37	7	3	2	2	0	0	51	34	10	0	5	2	0	0	51	16	4	4	5	1	1	1	32	134
1 Hr	118	29	12	9	5	3	2	178	124	34	10	12	6	0	1	187	60	16	16	17	4	1	1	115	480
12:00	37	13	3	2	1	1	0	57	24	8	1	2	0	0	0	35	20	2	4	6	0	0	0	32	124
12:15	33	6	1	1	0	1	0	42	33	8	3	2	2	1	1	50	32	7	2	4	1	1	0	47	139
12:30	34	10	6	2	2	1	0	55	38	9	2	1	1	0	0	51	25	8	4	4	1	1	1	44	150
12:45	34	15	6	2	2	0	0	59	30	6	2	6	4	0	0	48	20	2	4	2	0	0	0	28	135
1 Hr	138	44	16	7	5	3	0	213	125	31	8	11	7	1	1	184	97	19	14	16	2	2	1	151	548
13:00	36	6	4	0	1	0	0	47	34	9	1	2	2	0	0	48	34	8	2	3	1	0	0	48	143
13:15	50	8	3	0	1	2	0	64	48	10	4	3	0	0	0	65	25	7	3	4	1	0	0	40	169
13:30	46	6	4	0	0	0	0	56	52	8	2	2	1	1	1	67	45	6	3	3	0	0	0	57	180
13:45	39	10	3	2	1	0	0	55	37	8	0	4	1	1	0	51	33	5	5	8	0	0	0	51	157
1 Hr	171	30	14	2	3	2	0	222	171	35	7	11	4	2	1	231	137	26	13	18	2	0	0	196	649
14:00	34	7	0	2	3	0	0	46	33	5	3	4	2	0	1	48	38	10	4	6	0	0	0	58	152
14:15	35	12	3	2	1	1	0	54	34	10	3	5	2	0	0	54	23	6	6	6	0	0	0	41	149
14:30	33	7	2	1	0	0	0	43	44	13	3	2	2	0	0	64	21	6	3	5	0	0	0	35	142
14:45	33	7	4	2	2	1	0	49	38	11	2	5	1	1	1	60	35	14	4	4	1	1	1	58	167
1 Hr	135	33	9	7	6	2	0	192	149	39	11	16	7	1	3	226	117	36	17	21	1	0	0	192	610
15:00	41	11	2	2	2	0	0	58	29	6	0	5	3	0	0	43	33	8	6	5	0	0	0	52	153
15:15	42	10	2	0	1	0	0	55	35	6	1	3	0	0	0	45	38	5	3	2	0	0	0	48	148
15:30	46	1	4	3	0	0	0	54	42	10	2	5	1	0	0	60	27	8	1	7	2	0	0	45	159
15:45	34	11	1	4	2	0	0	56	31	6	4	6	1	0	1	49	28	10	3	8	2	2	1	52	157
1 Hr	163	33	9	9	5	0	4	223	137	28	7	19	5	0	1	197	126	31	13	22	4	1	0	197	617
16:00	39	12	6	3	1	1	1	63	29	6	3	1	2	0	0	41	32	14	1	5	0	0	0	52	156
16:15	48	16	4	4	1	0	0	73	28	5	2	7	1	4	0	47	46	12	2	7	1	1	0	69	189
16:30	51	11	1	4	1	1	0	69	45	8	1	2	1	0	0	57	35	12	4	4	1	0	0	56	182
16:45	47	15	3	4	1	0	1	71	51	6	2	4	2	0	0	65	52	15	1	3	0	0	0	71	207
1 Hr	185	54	14	15	4	2	2	276	153	25	8	14	6	4	0	210	165	53	8	19	2	1	0	248	734
17:00	55	15	7	0	2	3	2	84	53	10	0	3	1	0	3	70	49	14	4	3	0	0	1	71	225
17:15	52	13	3	3	0	0	1	72	48	10	2	3	2	0	1	66	32	6	3	5	0	1	0	47	185
17:30	66	12	2	1	1	1	2	85	58	6	2	6	0	1	0	73	59	13	1	1	0	0	0	74	232
17:45	53	6	4	2	3	1	4	73	40	6	4	2	2	0	1	49	65	7	0	1	0	0	0	73	195
1 Hr	226	46	16	6	6	5	9	314	199	30	6	14	3	2	4	258	205	40	8	10	0	1	1	265	837
18:00	66	10	6	1	3	2	2	90	37	5	3	5	3	1	1	55	61	7	1	3	0	0	1	73	218
18:15	52	5	1	3	2	4	0	67	30	5	0	2	0	1	1	39	57	12	3	2	0	0	0	74	180
18:30	44	7	1	0	0	2	0	54	35	4	0	4	1	0	0	44	47	6	1	1	0	0	0	55	153
18:45	40	9	1	1	2	2	1	56	26	1	0	2	4	0	0	33	33	6	3	3	1	0	0	45	135
1 Hr	202	31	9	5	7	10	3	267	128	15	3	13	8	2	2	171	198	31	8	8	1	0	1	248	686
12 Hrs	1654	423	134	83	56	30	23	2403	1913	408	90	174	75	16	20	2696	1557	388	158	217	28	13	4	2360	7459
Overall	1654	423	134	83	56	30	23	2403	1913	408	90	174	75	16	20	2696	1557	388	158	217	28	13	4	2360	7459

For inspection purposes only
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Entry: Arm A - R135 (N)

	Destination: Arm A - R135 (N)							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
07:00	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0
10:15	0	0	0	0	0	0	0	0
10:30	0	0	0	0	0	0	0	0
10:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
11:00	0	0	0	0	0	0	0	0
11:15	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0
13:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
14:00	0	0	0	0	0	0	0	0
14:15	0	0	0	0	0	0	0	0
14:30	0	0	0	0	0	0	0	0
14:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0
18:15	0	0	0	0	0	0	0	0
18:30	0	0	0	0	0	0	0	0
18:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
12 Hrs	0	0	0	0	0	0	0	0
Check	0	0	0	0	0	0	0	0

	Destination: Arm B - Elm Road							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
07:00	31	6	5	6	0	2	0	50
07:15	29	10	6	4	2	0	1	52
07:30	25	12	10	7	2	1	1	58
07:45	26	11	6	2	3	0	0	53
1.Hr	120	39	27	19	7	3	2	217
08:00	26	17	4	7	1	0	0	55
08:15	34	13	10	1	0	1	1	60
08:30	18	15	9	3	1	0	0	46
08:45	25	12	6	5	1	0	0	49
1.Hr	103	57	29	16	3	1	1	210
09:00	24	19	8	1	1	0	0	53
09:15	35	15	6	5	1	0	1	63
09:30	29	20	8	2	1	0	0	60
09:45	21	15	10	6	0	0	0	53
1.Hr	109	69	32	14	3	0	1	228
10:00	19	16	4	3	1	0	0	43
10:15	27	19	2	8	1	0	0	57
10:30	22	22	3	3	1	0	0	51
10:45	20	18	6	5	1	0	0	51
1.Hr	88	75	15	20	4	0	0	202
11:00	26	12	5	4	0	0	0	47
11:15	20	12	4	2	1	0	0	39
11:30	31	20	9	4	1	1	0	66
11:45	24	18	8	4	0	0	0	54
1.Hr	101	62	23	16	2	1	0	205
12:00	24	16	7	8	1	0	0	56
12:15	19	14	7	4	1	0	1	46
12:30	30	11	7	3	0	0	0	51
12:45	26	19	3	5	1	0	0	55
1.Hr	109	60	24	20	3	0	1	217
13:00	28	10	3	2	1	0	0	44
13:15	31	12	1	6	1	1	0	52
13:30	29	18	4	4	1	1	0	57
13:45	31	15	6	5	0	0	1	59
1.Hr	119	56	14	17	3	2	1	212
14:00	30	14	7	4	1	0	0	56
14:15	33	13	3	2	2	1	0	54
14:30	30	18	6	5	1	0	0	60
14:45	26	17	8	4	0	0	0	55
1.Hr	119	62	24	15	4	1	0	225
15:00	29	9	3	3	2	1	0	47
15:15	28	8	7	3	0	0	0	46
15:30	40	13	1	6	1	0	0	61
15:45	39	15	5	3	1	0	0	63
1.Hr	136	45	16	18	4	1	0	220
16:00	43	19	4	3	2	0	0	71
16:15	39	10	5	4	0	0	0	58
16:30	47	18	1	1	1	3	0	71
16:45	48	12	3	3	0	0	0	66
1.Hr	177	59	13	11	3	3	0	268
17:00	58	13	1	3	1	0	0	76
17:15	63	12	1	3	1	1	0	81
17:30	70	9	4	3	1	2	1	90
17:45	78	10	2	4	0	0	0	94
1.Hr	269	44	8	13	3	3	1	341
18:00	64	6	1	1	1	1	0	74
18:15	34	7	1	0	1	0	0	43
18:30	39	7	0	1	0	1	0	48
18:45	27	4	0	0	2	0	0	33
1.Hr	164	24	2	2	4	2	0	199
12 Hrs	1614	652	227	181	43	17	7	2741
Check								416
Total	1614	652	227	181	43	17	7	2741
Check								412

	Destination: Arm C - R135 (S)							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
07:00	5	2	0	1	0	0	2	10
07:15	2	1	0	2	1	0	0	6
07:30	16	2	0	3	0	0	2	23
07:45	11	7	0	1	0	0	0	19
1.Hr	34	12	0	7	1	0	4	58
08:00	7	3	0	0	0	0	3	13
08:15	3	1	0	4	0	0	0	8
08:30	2	0	0	2	0	0	1	5
08:45	2	2	0	4	0	0	0	8
1.Hr	14	6	0	10	0	0	4	34
09:00	1	2	1	3	0	0	0	7
09:15	4	2	0	3	0	0	1	10
09:30	6	1	0	7	0	0	0	14
09:45	6	3	0	2	0	0	0	11
1.Hr	17	8	1	15	0	0	1	42
10:00	2	4	1	2	0	0	1	10
10:15	3	4	0	2	0	0	0	9
10:30	3	0	0	1	0	0	0	4
10:45	5	1	0	7	0	0	0	13
1.Hr	13	9	1	12	0	0	1	36
11:00	5	2	0	1	0	0	1	9
11:15	3	2	0	3	0	0	0	8
11:30	2	3	0	0	0	0	0	5
11:45	4	4	0	2	0	0	0	11
1.Hr	14	7	0	8	0	0	1	30
12:00	7	0	0	3	0	0	0	10
12:15	9	2	0	0	0	0	0	11
12:30	4	2	0	1	0	0	0	7
12:45	6	3	0	6	0	0	0	15
1.Hr	29	6	1	4	0	0	0	40
13:00	9	2	1	6	0	0	0	18
13:15	6	2	0	5	0	0	0	13
13:30	2	2	0	5	0	0	0	9
13:45	7	1	0	7	0	0	0	15
1.Hr	24	7	1	17	0	0	0	49
14:00	5	1	0	4	0	0	0	10
14:15	7	1	1	1	0	0	0	10
14:30	2	1	0	0	0	0	0	3
14:45	6	3	0	6	0	0	0	15
1.Hr	20	6	1	11	0	0	0	38
15:00	1	0	0	4	0	0	0	5
15:15	7	0	0	5	0	0	0	12
15:30	2	3	0	1	0	0	0	6
15:45	2	2	1	2	0	0	0	7
1.Hr	12	3	1	12	0	0	0	28
16:00	0	2	1	4	0	0	0	7
16:15	2	0	1	2	0	0	0	5
16:30	0	0	0	2	0	0	0	2
16:45	4	4	0	2	0	0	0	10
1.Hr	6	2	2	11	0	0	0	21
17:00	2	1	0	1	0	0	1	5
17:15	2	0	1	2	0	0	1	5
17:30	1	1	0	4	0	0	4	9
17:45	6	3	0	2	0	0	3	12
1.Hr	6	3	0	6	0	0	2	17
18:00	0	0	0	5	1	0	1	9
18:15	1	0	0	2	3	0</		

Entry: Arm C - R135 (S)

	Destination: Arm A - R135 (N)								Total	Destination: Arm B - Elm Road								Total	Destination: Arm C - R135 (S)								Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL			CAR	LGV	OGV1	OGV2	PSV	MCL	PCL			CAR	LGV	OGV1	OGV2	PSV	MCL	PCL		
07:00	29	13	6	2	0	0	0	50	8	11	0	3	0	0	0	22	0	0	0	0	0	0	0	0	72		
07:15	28	9	5	4	1	0	0	47	8	12	1	7	0	0	0	28	0	0	0	0	0	0	0	0	75		
07:30	31	13	7	10	3	0	0	64	9	8	0	9	0	0	0	26	0	0	0	0	0	0	0	0	90		
07:45	52	26	15	10	2	1	0	101	9	8	1	1	0	0	0	23	0	0	0	0	0	0	0	0	115		
1 Hr	140	61	20	26	5	0	0	252	34	37	2	27	0	0	100	0	0	0	0	0	0	0	0	0	352		
08:00	46	14	5	6	0	0	1	72	10	5	0	21	0	0	0	36	0	0	0	0	0	0	0	0	108		
08:15	62	20	2	5	1	2	0	92	21	3	1	11	0	0	0	36	0	0	0	0	0	0	0	0	128		
08:30	50	22	3	2	0	1	0	78	27	4	0	7	0	0	0	38	0	0	0	0	0	0	0	0	116		
08:45	51	15	10	2	1	1	0	80	20	1	1	15	0	0	0	33	0	0	0	0	0	0	0	0	133		
1 Hr	215	71	19	25	3	4	1	338	78	13	2	54	0	0	147	0	0	0	0	0	0	0	0	0	485		
09:00	47	15	8	12	1	0	0	83	20	1	1	5	0	0	0	27	0	0	0	0	0	0	0	0	110		
09:15	44	14	3	8	2	1	0	72	11	4	1	11	0	0	0	27	0	0	0	0	0	0	0	0	99		
09:30	33	18	5	9	3	0	0	68	8	5	0	15	0	0	0	28	0	0	0	0	0	0	0	0	96		
09:45	23	17	5	7	1	0	0	57	7	4	1	14	0	0	0	25	0	0	0	0	0	0	0	0	79		
1 Hr	147	70	22	34	6	1	1	280	46	14	3	45	0	0	108	0	0	0	0	0	0	0	0	0	388		
10:00	30	17	9	7	1	1	0	65	8	2	3	18	0	0	0	31	1	0	0	0	0	0	0	1	97		
10:15	25	12	6	6	2	0	0	51	8	2	1	10	0	0	0	21	0	0	0	0	0	0	0	0	72		
10:30	26	22	3	4	2	0	0	57	8	5	0	8	0	0	0	21	0	0	0	0	0	0	0	0	78		
10:45	22	17	5	7	1	0	0	52	11	3	0	12	0	0	0	26	0	0	0	0	0	0	0	0	78		
1 Hr	103	66	23	24	6	1	0	225	35	12	4	48	0	0	99	1	0	0	0	0	0	0	0	1	325		
11:00	27	14	8	4	1	1	0	55	9	3	2	14	0	0	0	28	0	0	0	0	0	0	0	0	83		
11:15	17	22	7	10	0	0	0	56	9	4	1	14	0	0	0	28	0	0	0	0	0	0	0	0	84		
11:30	17	21	6	9	0	0	0	53	6	1	2	12	0	0	0	21	0	0	0	0	0	0	0	0	74		
11:45	21	17	10	6	1	1	0	58	8	2	1	17	0	0	0	28	0	0	0	0	0	0	0	0	86		
1 Hr	88	74	28	28	3	1	0	222	32	10	6	57	0	0	108	0	0	0	0	0	0	0	0	0	327		
12:00	26	15	9	5	1	0	0	56	4	2	0	10	0	0	0	16	0	0	0	0	0	0	0	0	72		
12:15	40	23	3	10	2	0	0	78	15	4	1	10	0	0	0	30	0	0	0	0	0	0	0	0	108		
12:30	42	17	8	5	2	0	0	74	4	4	0	10	0	0	0	18	0	0	0	0	0	0	0	0	92		
12:45	34	10	10	6	0	1	0	63	10	2	2	9	0	0	0	25	0	0	0	0	0	0	0	0	83		
1 Hr	142	65	30	26	5	0	0	288	33	12	3	39	0	0	107	0	0	0	0	0	0	0	0	0	355		
13:00	34	19	10	7	1	0	0	71	15	5	0	11	0	0	0	31	0	0	0	0	0	0	0	0	102		
13:15	39	16	4	4	1	0	0	64	15	4	0	11	1	0	0	31	0	0	0	0	0	0	0	0	95		
13:30	37	11	8	6	0	0	0	62	8	5	1	12	0	0	0	26	0	0	0	0	0	0	0	0	88		
13:45	44	21	10	6	1	1	0	67	16	2	3	8	0	0	0	28	0	0	0	0	0	0	0	0	98		
1 Hr	154	67	29	27	4	0	0	281	54	16	4	42	1	0	117	0	0	0	0	0	0	0	0	0	398		
14:00	25	15	5	9	1	1	0	56	16	3	0	14	0	0	0	33	0	0	0	0	0	0	0	0	89		
14:15	27	20	8	4	1	0	0	60	8	4	1	10	0	0	0	23	0	0	0	0	0	0	0	0	83		
14:30	27	9	4	10	1	0	0	51	8	2	0	8	0	0	0	18	0	0	0	0	0	0	0	0	69		
14:45	38	22	11	8	2	1	0	62	14	11	1	12	0	0	0	28	0	0	0	0	0	0	0	0	110		
1 Hr	117	66	28	31	4	2	0	248	48	10	3	42	0	0	103	0	0	0	0	0	0	0	0	0	351		
15:00	27	11	9	7	1	0	0	55	16	4	2	12	0	1	0	35	0	0	0	0	0	0	0	0	90		
15:15	38	18	8	12	2	0	0	78	22	3	1	9	0	0	0	35	0	0	0	0	0	0	0	0	113		
15:30	37	26	8	9	2	0	0	82	19	3	1	10	0	0	0	33	0	0	0	0	0	0	0	0	115		
15:45	29	19	9	6	0	1	0	67	14	11	1	12	0	0	0	38	0	0	0	0	0	0	0	0	105		
1 Hr	131	74	34	34	6	1	2	282	71	21	5	43	0	1	141	0	0	0	0	0	0	0	0	0	423		
16:00	21	24	7	12	1	0	0	65	16	13	2	8	0	0	0	39	0	0	0	0	0	0	0	0	104		
16:15	36	12	5	8	1	0	0	62	11	12	5	4	0	0	0	32	0	0	0	0	0	0	0	0	94		
16:30	32	13	2	14	0	2	1	64	16	6	0	4	0	0	0	26	0	0	0	0	0	0	0	0	90		
16:45	49	14	7	10	2	0	0	82	21	4	4	0	0	1	0	38	0	0	0	0	0	0	0	0	120		
1 Hr	138	63	21	44	4	2	1	273	64	39	11	20	0	1	138	0	0	0	0	0	0	0	0	0	408		
17:00	56	17	8	7	1	1	1	91	35	7	4	1	0	2	0	49	0	0	0	0	0	0	0	0	140		
17:15	47	12	2	7	2	2	0	72	23	6	2	1	0	0	0	32	0	0	0	0	0	0	0	0	104		
17:30	49	11	7	4	1	0	3	75	13	8	1	3	0	0	0	25	0	0	0	0	0	0	0	0	100		
17:45	42	8	7	4	1	1	0	63	11	9	1	2	0	0	0	23	0	0	0	0	0	0	0	0	85		
1 Hr	194	46	24	22	6	4	5	301	82	30	8	7	0	2	129	0	0	0	0	0	0	0	0	0	430		
18:00	33	4	4	2	2	1	2	48	15	7	1	0	0	0	0	23	0	0	0	0	0	0	0	0	71		
18:15	34	6	3	5	2	1	0	51	12	5	0	4	0	0	0	21	0	0	0	0	0	0	0	0	72		
18:30	28	6	3	3	1	1	1	43	7	2	1	0	0	0	0	10	0	0	0	0	0	0	0	0	53		
18:45	15	5	3	0	1	1	1	27	11	5	0	1	0	0	0	17	0	0	0	0	0	0	0	0	44		
1 Hr	111	21	13	10	6	4	4	169	45	19	2	5	0	0	71	0	0	0	0	0	0	0	0	0	240		
12 Hrs	1680	746	291	331	58	20	13	3139	622	233	53	429	1	4	0	1342	1	0	0	0	0	0	0	1	4482		
Check	1680	746	291	331	58	20	13	3139	622	233	53	429	1	4	0	1342	1	0	0	0	0	0	1	4482			
Total	1680	746	291	331	58	20	13	3139	622	233	53	429	1	4	0	1342	1	0	0	0	0	0	1	4482			

ORIGIN SUMMARY

	Origin: Arm A - R135 (N)								Total	Origin: Arm B - Elm Road								Total	Origin: Arm C - R135 (S)								Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL			CAR	LGV	OGV1	OGV2	PSV	MCL	PCL			CAR	LGV	OGV1	OGV2	PSV	MCL	PCL		
07:00	36	8	5	7	0	2	2	60	0	1	0	0	0	0	0	7	37	24	6	5	0	0	0	72	133		
07:15	31	11	6	6	3	0	1	58	2	0	3	0	0	0	0	5	36	21	6	11	1	0	0	75	138		
07:30	41	14	10	10	2	1	3	81	2	3	0	0	0	0	0	7	40	21	7	19	3	0	0	90	176		
07:45	46	18	6	3	3	0	0	76	4	2	0	0	0	0	0	7	51	32	3	18	1	0	0	115	197		
1 Hr	154	51																									

DESTINATION SUMMARY

	Destination: Arm A - R135 (N)							Total	Destination: Arm B - Elm Road							Total	Destination: Arm C - R135 (S)							Total	Dest Totals
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL		CAR	LGV	OGV1	OGV2	PSV	MCL	PCL		CAR	LGV	OGV1	OGV2	PSV	MCL	PCL		
07:00	29	14	6	2	0	0	0	51	39	17	5	9	0	2	0	72	5	2	0	1	0	0	2	10	133
07:15	30	9	8	4	1	0	0	52	37	22	7	11	2	0	1	80	2	1	0	2	1	0	0	6	138
07:30	32	15	7	10	3	0	0	67	34	20	10	16	2	1	1	84	17	3	0	3	0	0	2	25	176
07:45	56	26	9	10	1	0	0	97	44	17	7	10	3	0	0	81	11	7	0	1	0	0	0	19	197
1 Hr	147	66	23	26	5	0	0	267	154	76	29	46	7	3	2	317	35	13	0	7	1	0	4	60	644
08:00	48	19	5	6	0	0	1	79	36	22	4	28	1	0	0	91	7	3	0	0	0	0	3	13	183
08:15	65	21	2	6	1	2	0	97	55	16	11	12	0	1	1	96	3	1	1	4	0	0	0	9	202
08:30	52	23	4	3	0	1	0	83	45	19	9	10	1	0	0	84	2	0	0	2	0	0	1	5	172
08:45	69	17	9	12	2	1	0	103	45	13	7	20	1	0	0	86	2	2	0	4	0	0	0	8	197
1 Hr	227	80	20	27	3	4	1	362	181	70	31	70	3	1	1	357	14	6	1	10	0	0	4	35	754
09:00	49	17	9	12	1	0	0	88	44	20	9	6	1	0	0	80	1	2	1	3	0	0	0	7	175
09:15	45	17	4	9	2	1	0	78	46	19	7	16	1	0	1	90	4	2	0	3	0	0	1	10	178
09:30	34	18	6	9	3	0	0	70	37	25	8	17	1	0	0	88	6	1	0	7	0	0	0	14	172
09:45	23	24	6	5	0	0	0	58	28	19	11	20	0	0	0	78	6	3	0	2	0	0	0	11	147
1 Hr	151	76	25	35	6	1	0	294	155	83	35	59	3	0	1	338	17	8	1	15	0	0	1	42	672
10:00	30	19	9	7	1	1	0	67	27	18	7	21	1	0	0	74	4	4	1	2	0	0	1	12	153
10:15	25	12	7	6	2	0	0	52	35	21	3	18	1	0	0	78	3	4	0	2	0	0	0	9	139
10:30	27	23	3	4	2	0	0	59	30	27	3	11	1	0	0	72	3	0	0	1	0	0	0	4	135
10:45	22	19	5	7	1	0	0	54	31	21	6	18	1	0	0	77	5	1	0	7	0	0	0	13	144
1 Hr	104	73	24	24	6	1	0	232	123	67	19	68	4	0	0	301	15	9	1	12	0	0	1	38	571
11:00	27	14	8	4	1	1	0	55	35	15	7	18	0	0	0	75	6	2	0	1	0	0	1	10	140
11:15	17	23	7	10	0	0	1	58	29	16	5	16	1	0	0	67	3	2	0	3	0	0	0	8	133
11:30	17	23	6	9	0	0	0	55	37	21	11	16	1	1	0	87	2	3	0	0	0	0	0	5	147
11:45	29	17	7	5	2	0	0	60	33	20	6	23	0	0	0	82	4	0	0	4	0	0	0	8	150
1 Hr	90	77	28	28	3	1	1	228	134	72	29	73	2	1	0	311	15	7	0	8	0	0	1	31	570
12:00	27	16	9	5	1	0	0	58	28	18	7	18	1	0	0	72	7	0	0	3	0	0	0	10	140
12:15	42	24	3	10	2	0	0	81	34	18	8	14	1	0	1	76	9	2	0	0	0	0	0	11	168
12:30	45	19	8	5	2	0	0	79	34	15	7	13	0	0	0	69	4	2	0	1	0	0	0	7	155
12:45	37	11	10	6	0	0	0	64	45	21	5	14	1	0	0	87	9	3	1	0	0	0	0	13	164
1 Hr	151	70	30	26	5	0	0	282	142	72	27	59	3	0	1	304	29	7	1	4	0	0	0	41	627
13:00	37	19	11	7	1	0	0	75	44	15	3	13	1	0	0	76	11	2	1	6	0	0	0	20	171
13:15	41	18	4	4	2	0	0	69	46	16	1	17	2	1	0	83	6	2	0	5	0	0	0	13	165
13:30	43	11	9	6	0	0	0	69	37	23	5	16	1	1	0	83	2	2	0	5	0	0	0	9	161
13:45	44	21	7	10	2	0	0	84	47	18	9	13	0	0	1	86	7	1	0	1	0	0	0	9	181
1 Hr	165	69	31	27	5	0	0	297	174	72	18	59	4	2	1	330	26	7	1	17	0	0	0	51	678
14:00	28	16	5	9	1	1	0	60	46	17	7	18	1	0	0	89	5	2	0	4	0	0	0	11	160
14:15	33	20	8	4	1	0	0	66	41	17	4	12	2	1	0	77	7	1	1	1	0	0	0	10	153
14:30	29	10	4	10	1	0	0	54	38	20	6	13	1	0	0	78	2	1	0	0	0	0	0	3	135
14:45	39	22	11	9	1	1	0	83	42	18	10	14	0	0	0	84	6	3	0	6	0	0	0	15	182
1 Hr	129	68	28	32	4	2	0	263	167	72	27	57	4	1	0	328	20	7	1	11	0	0	0	39	630
15:00	27	11	9	8	1	0	0	56	45	13	5	15	2	2	0	82	1	0	0	4	0	0	0	5	143
15:15	39	18	8	13	2	0	0	80	50	11	8	12	0	0	0	81	7	0	0	5	0	0	0	12	173
15:30	40	26	8	9	2	0	0	85	59	16	2	16	1	0	0	94	2	3	0	1	0	0	0	6	185
15:45	29	20	9	6	1	1	2	68	53	25	6	18	1	0	0	104	2	0	1	2	0	0	0	5	177
1 Hr	135	75	34	36	6	1	2	288	207	66	21	61	4	2	0	361	12	3	1	12	0	0	0	28	678
16:00	24	27	8	12	1	0	1	73	59	32	6	11	2	0	0	110	0	2	1	4	0	0	0	7	190
16:15	37	13	5	8	1	0	0	64	50	22	10	8	0	0	0	90	2	0	1	2	0	0	0	5	159
16:30	35	14	3	14	0	2	1	69	63	24	1	5	1	3	0	97	0	0	0	2	0	0	0	2	168
16:45	52	15	7	10	2	0	0	86	59	20	7	7	0	1	0	104	4	0	0	3	0	0	0	7	197
1 Hr	148	69	23	44	4	2	2	292	241	98	24	31	3	4	0	401	6	2	2	11	0	0	0	21	714
17:00	61	19	9	7	1	1	1	99	93	20	5	4	1	2	0	125	2	1	0	1	0	0	1	5	229
17:15	50	12	2	7	2	2	1	76	86	18	3	4	1	1	0	113	3	0	0	2	0	0	1	6	195
17:30	62	11	7	4	1	0	3	88	83	17	5	6	1	2	1	115	2	2	0	1	0	0	0	5	208
17:45	46	6	7	4	2	1	1	67	99	19	3	6	0	0	0	117	2	0	0	2	0	0	0	4	188
1 Hr	219	48	25	22	6	4	6	330	351	74	16	20	3	5	1	470	9	3	0	6	0	0	2	20	820
18:00	35	5	4	2	2	1	4	53	79	13	2	1	1	1	0	97	2	0	0	5	1	0	1	9	159
18:15	35	8	3	6	2	1	0	55	46	12	1	4	1	0	0	64	1	0	2	3	0	0	1	7	126
18:30	28	7	3	3	1	1	1	44	46	9	1	1	0	1	0	58	0	0	0	0	0	0	0	2	104
18:45	17	6	3	0	1	1	1	29	38	9	0	1	2	0	0	50	0	1	0	0	0	0	0	1	80
1 Hr	115	26	13	11	6	4	6	181	209	43	4	7	4	2	0	269	5	1	2	8	1	0	2	19	469
12 Hrs	1781	797	304	338	59	20	18	3317	2238	885	280	610	44	21	7	4085	203	73	1	121	2	0	15	425	7827
Check	1781	797	304	338	59	20	18	3317	2238	885	280	610	44	21	7	4085	203	73	1	121	2	0	15	425	7827

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Entry: Arm A - R135 (N)

	Destination: Arm A - R135 (N)							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
07:00	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0
09:45	1	0	0	0	0	0	0	1
1.Hr	1	0	0	0	0	0	0	1
10:00	0	0	0	0	0	0	0	0
10:15	0	0	0	0	0	0	0	0
10:30	0	0	0	0	0	0	0	0
10:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
11:00	0	0	0	0	0	0	1	1
11:15	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	1	1
12:00	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0
13:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
14:00	0	0	0	0	0	0	0	0
14:15	1	0	0	0	0	0	0	1
14:30	0	0	0	0	0	0	0	0
14:45	0	0	0	0	0	0	0	0
1.Hr	1	0	0	0	0	0	0	1
15:00	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0
18:15	0	0	0	0	0	0	0	0
18:30	0	0	0	0	0	0	0	0
18:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
12 Hrs	2	0	0	0	0	0	1	3
Check	2	0	0	0	0	0	1	3

	Destination: Arm B - N2 Offslip							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
07:00	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0
10:15	0	0	0	0	0	0	0	0
10:30	0	0	0	0	0	0	0	0
10:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
11:00	0	0	0	0	0	0	0	0
11:15	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0
13:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
14:00	0	0	0	0	0	0	0	0
14:15	0	0	0	0	0	0	0	0
14:30	0	0	0	0	0	0	0	0
14:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0
18:15	0	0	0	0	0	0	0	0
18:30	0	0	0	0	0	0	0	0
18:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0
12 Hrs	0	0	0	0	0	0	0	0
Check	0	0	0	0	0	0	0	0

	Destination: Arm C - R135 (S)							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
07:00	2	1	0	1	0	0	2	6
07:15	5	1	0	2	1	0	0	9
07:30	15	3	1	2	0	0	2	23
07:45	9	7	0	5	0	0	0	16
1.Hr	31	12	1	5	1	0	4	54
08:00	4	3	0	1	0	0	3	11
08:15	2	2	0	4	0	0	0	8
08:30	2	0	0	2	0	0	1	5
08:45	1	1	1	4	0	1	0	8
1.Hr	9	6	1	11	0	1	4	32
09:00	0	1	1	3	0	0	0	5
09:15	2	2	0	2	0	0	1	7
09:30	5	1	0	8	0	0	0	14
09:45	2	2	0	2	0	0	0	5
1.Hr	9	5	1	15	0	0	1	31
10:00	2	3	0	2	0	0	0	7
10:15	3	3	0	2	0	0	1	9
10:30	3	0	0	0	0	0	0	3
10:45	2	0	0	7	0	0	0	9
1.Hr	10	6	0	11	0	0	1	28
11:00	5	3	0	1	0	0	0	9
11:15	2	2	0	3	0	0	0	7
11:30	2	1	0	0	0	0	0	3
11:45	6	2	0	2	0	0	0	6
1.Hr	12	7	0	6	0	0	0	25
12:00	4	0	0	5	0	0	0	9
12:15	5	2	0	0	0	0	0	7
12:30	3	2	0	1	0	0	0	6
12:45	5	2	0	1	0	0	0	8
1.Hr	17	7	1	6	0	0	0	31
13:00	7	1	1	5	0	0	0	14
13:15	5	1	0	5	0	0	0	11
13:30	3	3	0	6	0	0	0	12
13:45	4	1	1	2	0	0	0	8
1.Hr	19	6	1	17	0	0	0	43
14:00	3	2	0	4	0	0	0	9
14:15	3	1	1	1	0	0	0	6
14:30	2	0	0	0	0	0	0	2
14:45	2	0	0	5	0	1	0	10
1.Hr	10	5	1	10	0	1	0	27
15:00	2	0	0	5	0	0	0	7
15:15	3	1	0	5	0	0	0	9
15:30	1	2	0	1	0	0	0	4
15:45	0	0	1	2	0	0	0	3
1.Hr	6	3	1	13	0	0	0	23
16:00	0	2	1	4	0	0	0	7
16:15	1	0	1	2	0	0	0	4
16:30	1	0	0	2	0	0	0	3
16:45	4	2	2	1	0	0	0	9
1.Hr	4	2	2	11	0	0	0	19
17:00	1	1	0	1	0	0	1	4
17:15	3	0	0	2	0	0	1	6
17:30	2	2	0	1	0	0	0	5
17:45	1	2	0	2	0	0	0	3
1.Hr	7	3	0	6	0	0	2	18
18:00	0	0	0	5	1	0	1	9
18:15	1	0	0	2	3	0	1	7
18:30	0	0	0	0	0	0	0	1
18:45	4	0	0	0	0	0	0	4
1.Hr	4	0	0	8	1	0	2	17
12 Hrs	138	62	11	119	2	2	14	348
Check	138	62	11	119	2	2	14	348

Entry: Arm C - R135 (S)

	Destination: Arm A - R135 (N)								Destination: Arm B - N2 Offslip								Destination: Arm C - R135 (S)								Arm Totals	
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total		
07:00	1	1	0	0	0	0	0	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6	
07:15	1	0	0	4	0	0	0	12	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	12	
07:30	1	1	1	11	1	0	0	15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	15	
07:45	2	3	0	14	0	0	0	21	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	21	
1 Hr	5	5	0	40	0	0	0	54	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	54	
08:00	2	2	1	26	0	0	0	31	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	32	
08:15	4	1	1	10	0	2	0	18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	18	
08:30	7	3	0	7	0	0	0	17	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	17	
08:45	1	1	1	21	0	0	0	26	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	28	
1 Hr	18	7	3	64	0	2	1	95	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	95	
09:00	3	1	0	11	0	0	0	15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	15	
09:15	3	3	2	13	0	0	0	21	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	21	
09:30	3	4	0	22	0	0	0	29	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	29	
09:45	12	7	2	19	0	0	0	36	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	39	
1 Hr	14	16	3	61	0	0	0	94	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	94	
10:00	8	5	1	15	0	0	0	29	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	29	
10:15	4	2	1	11	0	0	0	18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	18	
10:30	7	4	0	10	0	0	0	21	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	21	
10:45	15	2	0	15	0	0	0	32	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	32	
1 Hr	24	13	2	51	0	0	0	90	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	90	
11:00	9	1	2	17	0	0	0	29	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	29	
11:15	6	4	3	14	0	0	0	27	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	27	
11:30	3	2	1	14	0	0	0	20	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	20	
11:45	7	3	1	19	0	0	0	30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	30	
1 Hr	25	10	7	64	0	0	0	106	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	106	
12:00	12	7	1	13	0	0	0	33	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	33	
12:15	15	0	1	21	0	0	0	37	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	37	
12:30	8	6	0	5	0	0	0	19	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	19	
12:45	12	2	2	13	0	0	0	31	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	31	
1 Hr	53	15	4	48	0	0	0	120	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	120	
13:00	19	3	1	11	0	0	1	35	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	35	
13:15	20	6	1	14	0	0	0	41	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	41	
13:30	10	0	2	11	0	0	0	23	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	23	
13:45	12	2	0	16	0	0	0	31	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	31	
1 Hr	61	11	5	52	0	0	1	130	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	130	
14:00	12	1	1	17	0	0	0	31	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	31	
14:15	6	4	1	10	0	1	0	22	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	22	
14:30	7	0	0	14	0	0	0	21	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	21	
14:45	11	3	2	13	0	0	0	29	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	29	
1 Hr	36	8	4	54	0	1	0	103	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	103	
15:00	15	0	2	14	0	0	0	31	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	31	
15:15	24	1	1	16	0	0	0	42	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	42	
15:30	21	2	0	15	0	0	0	38	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	38	
15:45	12	7	7	12	0	0	0	36	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	36	
1 Hr	72	10	5	58	0	0	2	147	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	147	
16:00	13	8	1	17	0	0	0	39	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	39	
16:15	15	6	3	10	0	0	0	34	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	34	
16:30	15	3	0	9	0	0	1	28	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	28	
16:45	24	8	1	4	0	1	0	38	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	38	
1 Hr	67	25	5	40	0	1	1	138	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	138	
17:00	26	5	2	4	0	2	1	40	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	40	
17:15	8	5	0	4	0	0	2	19	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	19	
17:30	13	3	0	2	0	0	0	17	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	17	
17:45	14	2	0	1	0	0	0	19	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	19	
1 Hr	61	15	2	11	0	2	4	95	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	96	
18:00	21	2	0	1	1	0	2	27	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	27	
18:15	19	3	0	2	0	0	0	24	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	24	
18:30	15	2	0	1	0	0	1	19	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	19	
18:45	7	1	0	0	0	0	0	9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9	
1 Hr	62	8	0	4	1	0	4	79	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	79	
12 Hrs	498	143	41	547	2	8	13	1252	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1253	
Check								255																	255	
Total	498	143	41	547	2	8	13	1252	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1253	
Check								155																		155

ORIGIN SUMMARY

	Origin: Arm A - R135 (N)								Origin: Arm B - N2 Offslip								Origin: Arm C - R135 (S)								Origin Totals
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	
07:00	2	1	0	1	0	0	0	6	43	22	6	6	0	0	0	0	0	1	0	0	4	0	0	0	89
07:15	5	1	0	2	1	0	0	9	48	25	7	8	1	0	0	0	0	1	0	0	11	0	0	0	110
07:30	15	3	1	2	0	0	2	23	62	30	7	20	2	0	0	0	0	1	1	1	11	0	0	0	159
07:45	9	7	0	0	0	0	0	16	111	38	2	11	1	2	0	0	0	2	3	0	14	0	0	2	216
1 Hr	31	12	1	5	1	0	4	54	284	115	22	45	4	2	0	0	0	5	5	1	40	1	2	0	560
08:00	4	3	0	1	0	0	3	11	63	19	6	14	0	0	0	0	0	2	2	1	10	0	0	1	

Entry: Arm A - R135 (N)										Destination: Arm B - Kilsbane Way (E)										Destination: Arm C - R135 (S)										Destination: Arm D - Kilsbane Way (W)										Arm Totals
Destination: Arm A - R135 (N)										Destination: Arm B - Kilsbane Way (E)										Destination: Arm C - R135 (S)										Destination: Arm D - Kilsbane Way (W)										
CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total									
07:00	0	0	0	0	0	0	0	20	8	2	1	1	0	0	32	12	3	1	1	0	0	0	1	18	0	0	0	0	0	0	50									
07:15	0	0	0	0	0	0	0	22	7	2	1	1	0	0	34	5	4	3	2	2	0	0	0	16	3	2	1	1	0	0	7									
07:30	0	0	0	0	0	0	0	45	12	0	2	3	1	0	63	19	5	4	3	1	0	0	2	34	2	1	0	0	0	0	100									
07:45	0	0	0	0	0	0	0	30	15	1	3	0	0	0	49	19	8	0	1	3	0	0	1	32	5	1	0	0	0	0	87									
1 Hr	0	0	0	0	0	0	0	117	42	5	7	5	1	1	178	55	20	8	7	6	0	4	100	10	4	1	1	0	0	0	294									
08:00	0	0	0	0	0	0	0	36	4	0	2	1	0	0	43	14	9	0	2	1	0	4	30	4	6	0	0	0	0	10										
08:15	0	0	0	0	0	0	0	53	8	2	1	0	0	0	64	11	3	1	4	4	0	1	1	21	7	2	1	0	0	0	95									
08:30	0	0	0	0	0	0	0	49	9	1	0	0	0	0	59	7	8	0	1	1	0	1	18	4	1	0	0	0	0	5										
08:45	0	0	0	0	0	0	0	31	7	1	3	0	0	0	42	12	2	0	3	2	0	0	0	19	9	1	1	0	0	0	12									
1 Hr	0	0	0	0	0	0	0	169	28	4	6	1	0	0	208	44	22	1	10	4	1	6	88	24	10	2	0	0	1	0	37									
09:00	0	0	0	0	0	0	0	27	6	0	1	3	0	0	37	9	5	3	1	0	0	1	19	8	2	0	0	0	0	11										
09:15	0	0	0	0	0	0	0	17	3	1	2	1	0	0	24	19	2	1	3	3	1	0	0	27	5	3	0	0	0	0	8									
09:30	0	0	0	0	0	0	0	19	3	1	1	1	0	0	25	14	7	3	6	1	0	0	0	31	3	1	0	0	0	0	4									
09:45	0	0	0	0	0	0	0	24	7	0	0	1	0	0	32	13	7	1	2	2	0	0	0	23	2	0	0	0	0	0	2									
1 Hr	0	0	0	0	0	0	0	95	20	3	3	5	0	0	126	55	21	8	12	2	0	2	100	18	6	0	0	1	0	0	25									
10:00	0	0	0	0	0	0	0	19	3	2	1	1	0	0	24	9	5	0	1	1	0	0	16	0	1	0	1	0	0	0	4									
10:15	0	0	0	0	0	0	0	23	3	3	1	1	0	0	31	7	4	0	1	1	0	0	13	4	1	0	1	0	0	0	2									
10:30	0	0	0	0	0	0	0	5	2	2	1	0	1	0	11	4	5	0	6	1	0	0	16	3	0	0	0	0	0	0	5									
10:45	0	0	0	0	0	0	0	64	11	8	4	2	1	0	90	25	19	0	10	4	0	1	59	8	4	2	2	0	0	0	16									
11:00	0	0	0	0	0	0	0	19	6	3	1	1	0	0	29	13	4	0	1	1	0	0	17	2	2	2	0	0	0	0	4									
11:15	0	0	0	0	0	0	0	19	4	5	0	0	0	0	28	6	7	0	3	1	0	0	17	1	1	1	0	0	0	0	4									
11:30	0	0	0	0	0	0	0	19	2	1	1	1	0	0	24	8	2	3	0	1	0	0	14	4	3	0	0	0	0	0	7									
11:45	0	0	0	0	0	0	0	20	5	0	0	0	0	0	26	12	3	2	4	0	0	0	21	4	4	2	2	0	0	0	9									
1 Hr	0	0	0	0	0	0	0	77	18	8	2	2	0	0	107	37	16	5	8	2	0	1	69	11	8	3	0	0	0	0	198									
12:00	0	0	0	0	0	0	0	18	0	0	1	1	0	0	20	8	3	1	2	1	0	0	15	4	2	0	0	0	0	0	6									
12:15	0	0	0	0	0	0	0	20	5	1	2	1	0	0	29	10	4	0	1	1	0	1	17	3	2	0	0	0	1	0	5									
12:30	0	0	0	0	0	0	0	31	5	2	0	1	0	0	39	12	4	4	0	0	0	0	20	3	2	0	0	0	0	0	5									
12:45	0	0	0	0	0	0	0	22	4	0	4	0	0	0	30	10	10	4	0	0	0	0	4	5	3	1	2	0	0	0	4									
1 Hr	0	0	0	0	0	0	0	91	14	3	4	4	0	0	116	40	15	5	3	3	0	1	67	13	7	2	2	0	0	0	23									
13:00	0	0	0	0	0	0	0	21	3	2	0	2	0	0	28	16	5	0	1	7	1	0	0	30	4	0	1	0	0	0	0	5								
13:15	0	0	0	0	0	0	0	35	4	2	0	0	0	0	41	17	6	0	5	1	0	0	30	11	1	0	0	0	0	0	12									
13:30	0	0	0	0	0	0	0	20	3	2	0	0	0	0	26	13	9	0	5	1	1	0	29	6	3	0	0	0	0	0	9									
13:45	0	0	0	0	0	0	0	28	2	0	0	0	0	0	31	12	7	4	0	3	0	0	18	5	3	1	2	0	0	0	5									
1 Hr	0	0	0	0	0	0	0	104	13	6	0	2	0	0	126	58	24	2	20	3	1	0	108	23	4	1	0	0	0	0	28									
14:00	0	0	0	0	0	0	0	21	3	3	2	2	0	0	32	16	5	0	2	1	0	0	24	7	2	0	0	0	0	0	9									
14:15	0	0	0	0	0	0	0	30	8	1	4	1	0	0	44	15	3	2	1	1	1	0	27	3	0	0	0	0	0	0	3									
14:30	0	0	0	0	0	0	0	28	9	2	0	1	0	0	39	14	14	3	1	0	0	0	23	1	1	1	0	0	0	0	6									
14:45	0	0	0	0	0	0	0	18	7	2	3	0	0	0	29	12	3	0	5	0	1	0	21	4	1	0	0	0	0	0	5									
1 Hr	0	0	0	0	0	0	0	97	26	8	9	4	0	0	147	57	20	3	10	3	2	0	95	15	4	0	0	0	0	0	19									
15:00	0	0	0	0	0	0	0	31	8	0	1	1	0	0	41	7	4	0	2	2	0	0	15	6	2	2	0	0	0	0	10									
15:15	0	0	0	0	0	0	0	21	1	1	0	0	0	0	24	16	2	0	4	0	0	0	22	2	0	1	0	0	0	0	3									
15:30	0	0	0	0	0	0	0	28	5	0	4	0	0	0	37	19	7	1	0	0	0	0	28	19	1	2	0	0	0	0	7									
15:45	0	0	0	0	0	0	0	19	5	1	1	0	1	1	28	11	3	2	1	1	0	0	18	1	2	0	0	0	0	0	3									
1 Hr	0	0	0	0	0	0	0	99	19	2	7	1	1	1	130	53	16	2	8	4	0	0	83	14	5	5	0	0	0	0	24									
16:00	0	0	0	0	0	0	0	18	3	1	1	1	0	0	24	11	6	1	4	1	0	0	23	3	1	0	0	0	0	0	4									
16:15	0	0	0	0	0	0	0	21	5	3	2	0	1	0	33	7	5	0	2	1	3	0	18	2	2	1	0	0	0	0	5									
16:30	0	0	0	0	0	0	0	31	6	0	0	0	0	0	37	10	7	0	2	0	0	0	18	2	2	1	0	0	0	0	6									
16:45	0	0	0	0	0	0	0	37	3	0	0	1	0	0	41	19	3	1	3	0	0	1	27	3	0	0	1	0	0	0	4									
1 Hr	0	0	0	0	0	0	0	107	17	4	3	3	1	0	135	47	15	3	11	2	0	1	82	10	5	2	2	0	0	0	19									
17:00	0	0	0	0	0	0	0	28	5	0	3	1	0	0	38	9	5	0	1	0	0	1	16	4	0	0	0	0	0	0	4									
17:15	0	0	0	0	0	0	0	41	3	0	1	0	0	0	45	14	14	5	2	1	0	0	21	7	2	1	0	0	0	0	7									
17:30	0	0	0	0	0	0	0	37	4	0	3	0	0	0	44	10	4	2	1	0	0	1	20	4	0	0	1	0	0	0	5									
17:45	0	0	0	0	0	0	0	34	2	0	1	1	0	0	38	19	2	2	0	3	0	0	1	27	4	0	0	1	0	0	0	6								
1 Hr	0	0	0	0	0	0	0	140	14	1	7	2	0	1	165	52	17	4	6	2	1	4	86	11	1	0	2	0	0	0	14									
18:00	0	0	0	0	0	0	0	25	1	0	1	1	1	1	31	15	2	1	2	2	0	0	17	1	0	0	0	0	0	0	1									
18:15	0	0	0	0	0	0	0	25	3	0	0	0	0	0	31	8	4	2	3	0	0	0	17	1	0	0	0	0	0	0	1									
18:30	0	0	0	0	0	0	0	27	4	0	2	1	0	0	34	15	0	0	0	0	0	0	17	4	0	0	0	0	0	0	4									
18:45																																								

Entry: Arm C - R135 (S)

Table with columns: Destination: Arm A - R135 (N), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Destination: Arm B - Kishane Way (E), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Destination: Arm C - R135 (S), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Destination: Arm D - Kishane Way (W), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Arm Totals. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Summary table for Arm C - R135 (S) with columns: 12 Hrs, 506, 193, 52, 155, 49, 10, 18, 983.

Summary table for Arm B - Kishane Way (E) with columns: 497, 181, 76, 102, 3, 7, 0, 866.

Summary table for Arm C - R135 (S) with columns: 0, 0, 0, 0, 0, 0, 0, 0.

Summary table for Arm D - Kishane Way (W) with columns: 754, 458, 180, 83, 7, 4, 3, 1489.

Summary table for Arm Totals with columns: 3338, 3338, 3338, 3338, 3338, 3338, 3338, 3338.

Entry: Arm D - Kishane Way (W)

Table with columns: Destination: Arm A - R135 (N), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Destination: Arm B - Kishane Way (E), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Destination: Arm C - R135 (S), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Destination: Arm D - Kishane Way (W), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Arm Totals. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Summary table for Arm A - R135 (N) with columns: 130, 65, 20, 5, 0, 0, 0, 220.

Summary table for Arm B - Kishane Way (E) with columns: 1330, 369, 155, 38, 9, 11, 10, 1922.

Summary table for Arm C - R135 (S) with columns: 705, 349, 164, 72, 3, 4, 3, 1300.

Summary table for Arm D - Kishane Way (W) with columns: 0, 0, 0, 0, 0, 0, 0, 0.

Summary table for Arm Totals with columns: 3442, 3442, 3442, 3442, 3442, 3442, 3442, 3442.

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ORIGIN SUMMARY

	Origin: Arm A - R135 (N)							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
07:00	32	11	3	2	1	0	1	50
07:15	30	13	6	4	3	0	1	57
07:30	66	18	4	5	4	1	2	100
07:45	54	24	1	4	3	0	1	87
1 Hr	182	66	14	15	11	1	5	294
08:00	54	19	0	4	2	0	4	83
08:15	71	13	4	5	0	1	1	95
08:30	60	15	1	1	0	0	1	77
08:45	52	10	2	6	2	1	0	73
1 Hr	237	60	7	16	5	2	6	333
09:00	44	13	3	2	4	0	1	67
09:15	49	9	2	4	1	0	1	67
09:30	36	11	4	7	2	0	0	60
09:45	39	14	1	2	1	0	0	57
1 Hr	168	47	11	15	8	0	2	251
10:00	23	10	3	4	2	0	1	43
10:15	28	9	2	2	1	0	0	42
10:30	34	8	3	3	2	0	0	50
10:45	12	7	2	7	1	1	0	30
1 Hr	97	34	10	16	6	1	1	165
11:00	32	12	2	2	1	0	1	50
11:15	28	12	6	3	1	0	0	48
11:30	31	7	4	1	2	0	0	45
11:45	36	11	4	4	0	0	0	55
1 Hr	125	42	16	10	4	0	1	198
12:00	30	5	3	2	0	0	0	41
12:15	33	11	1	3	2	1	0	52
12:30	46	11	6	0	1	0	0	64
12:45	35	9	2	1	2	0	0	49
1 Hr	144	36	10	7	3	1	1	208
13:00	41	4	7	3	0	0	0	55
13:15	63	11	3	5	1	0	0	83
13:30	39	15	2	5	1	1	1	64
13:45	42	7	0	3	0	0	0	52
1 Hr	185	41	9	20	5	1	1	263
14:00	44	10	3	4	3	0	1	65
14:15	48	11	3	5	2	1	0	70
14:30	43	18	3	2	2	0	0	68
14:45	34	34	2	2	1	0	2	59
1 Hr	169	50	11	19	7	2	3	262
15:00	44	14	2	3	3	0	0	66
15:15	39	3	2	5	0	0	0	49
15:30	52	13	2	5	1	0	0	73
15:45	31	21	2	2	1	1	1	49
1 Hr	166	40	9	15	5	1	1	237
16:00	32	10	2	5	2	0	0	51
16:15	33	8	5	5	1	1	0	53
16:30	40	13	1	2	1	3	0	60
16:45	58	6	4	1	0	1	0	70
1 Hr	164	37	9	16	5	4	1	236
17:00	41	10	0	4	1	0	2	58
17:15	57	10	1	1	1	0	1	70
17:30	51	7	2	5	1	1	1	69
17:45	54	4	2	5	1	0	1	67
1 Hr	203	32	5	15	4	1	5	265
18:00	39	2	1	6	3	1	1	53
18:15	34	7	2	0	1	0	1	47
18:30	46	6	0	0	0	0	0	52
18:45	29	4	0	1	3	0	0	37
1 Hr	148	19	3	11	7	2	2	192

	Origin: Arm B - Kilshane Way (E)							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
07:00	75	8	3	4	0	2	0	92
07:15	93	19	2	3	0	0	0	117
07:30	91	11	3	8	1	1	0	114
07:45	92	26	3	4	0	0	1	126
1 Hr	351	64	11	17	1	3	2	479
08:00	104	15	5	3	0	0	2	129
08:15	103	12	6	2	0	0	0	123
08:30	97	12	4	0	0	1	0	114
08:45	90	19	6	4	0	1	0	120
1 Hr	394	58	25	13	0	2	2	494
09:00	101	20	12	6	1	0	2	142
09:15	85	27	6	2	1	0	0	119
09:30	48	17	7	2	1	0	0	75
09:45	46	13	8	5	0	1	0	73
1 Hr	280	77	33	21	3	1	3	418
10:00	45	13	10	4	1	0	0	73
10:15	60	10	7	8	1	0	0	86
10:30	46	22	6	7	1	0	0	82
10:45	59	17	5	8	1	0	0	90
1 Hr	210	62	28	27	4	0	0	331
11:00	44	17	7	7	4	0	1	80
11:15	89	8	14	4	1	1	0	108
11:30	53	30	8	3	1	0	0	95
11:45	46	9	10	8	1	1	1	76
1 Hr	182	64	39	22	7	2	2	318
12:00	50	16	10	5	1	0	0	82
12:15	44	13	7	5	1	1	0	71
12:30	44	11	9	6	2	0	1	73
12:45	57	16	10	5	0	0	0	88
1 Hr	195	56	36	21	4	1	1	314
13:00	79	16	10	3	2	0	0	110
13:15	49	15	9	3	0	2	0	78
13:30	63	22	7	3	0	0	0	95
13:45	60	15	6	5	0	0	0	87
1 Hr	251	69	32	14	2	2	0	370
14:00	54	15	6	7	0	0	0	82
14:15	61	12	6	2	1	1	0	83
14:30	49	12	3	3	0	0	0	68
14:45	60	23	12	7	1	0	0	123
1 Hr	244	62	27	19	2	2	0	370
15:00	58	20	8	6	0	0	0	92
15:15	59	16	6	2	0	1	0	84
15:30	72	18	6	6	3	0	0	105
15:45	68	17	10	4	1	0	0	99
1 Hr	247	71	29	20	3	2	2	374
16:00	66	24	8	4	0	0	0	102
16:15	67	21	9	6	0	0	0	103
16:30	62	16	4	4	1	0	0	87
16:45	69	19	10	4	1	0	0	104
1 Hr	284	80	31	18	2	1	0	416
17:00	75	15	4	6	0	0	0	101
17:15	110	18	3	4	0	0	0	135
17:30	105	15	1	1	1	0	0	128
17:45	117	12	3	3	2	0	0	137
1 Hr	406	60	16	14	3	0	0	499
18:00	125	15	4	4	0	1	4	153
18:15	88	13	2	1	5	3	0	112
18:30	67	17	2	4	0	0	0	90
18:45	48	10	2	2	4	1	0	67
1 Hr	317	55	10	11	9	6	4	412

	Origin: Arm C - R135 (S)							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
07:00	28	16	6	2	0	0	0	52
07:15	30	9	7	4	1	0	0	51
07:30	26	15	6	14	2	0	0	63
07:45	60	34	2	10	0	0	1	107
1 Hr	144	75	21	25	4	0	1	270
08:00	43	19	4	7	1	0	1	75
08:15	65	18	3	6	1	2	0	95
08:30	55	4	4	3	0	1	0	63
08:45	60	16	7	12	2	1	0	98
1 Hr	223	83	18	28	4	4	1	361
09:00	49	17	9	7	1	0	0	83
09:15	41	17	6	14	2	0	0	81
09:30	36	23	5	8	1	0	0	73
09:45	24	22	6	6	2	0	0	60
1 Hr	150	79	26	35	6	1	0	297
10:00	31	21	8	5	1	1	0	67
10:15	28	12	9	7	2	0	0	58
10:30	25	23	4	4	2	0	0	58
10:45	26	15	5	6	1	0	0	53
1 Hr	110	71	26	22	6	1	0	236
11:00	23	16	8	4	4	1	1	53
11:15	23	19	6	10	0	0	2	60
11:30	13	20	6	6	0	0	1	46
11:45	30	21	6	9	2	0	0	68
1 Hr	89	76	26	29	3	1	3	227
12:00	30	15	9	7	1	0	0	63
12:15	38	22	4	10	1	0	0	75
12:30	45	25	8	4	2	0	0	84
12:45	38	12	8	6	1	0	0	65
1 Hr	151	75	29	27	5	0	0	283
13:00	35	19	13	8	0	0	0	75
13:15	40	21	8	3	2	0	0	74
13:30	42	12	10	6	0	0	0	70
13:45	40	15	8	10	2	0	0	75
1 Hr	157	67	39	27	5	0	0	290
14:00	29	15	6	10	1	1	0	62
14:15	35	21	8	3	1	1	0	69
14:30	23	13	4	11	1	0	0	52
14:45	33	28	6	7	1	0	0	69
1 Hr	120	69	24	31	4	3	0	251
15:00	31	18	14	9	1	0	0	73
15:15	38	15	5	12	2	0	0	72
15:30	43	23	9	5	1	1	0	87
15:45	28	17	6	7	1	0	0	59
1 Hr	140	78	35	37	6	0	2	298
16:00	28	24	10	12	1	1	1	77
16:15	36	19	5	10	1	0	0	71
16:30	33	15	3	13	0	2	0	65
16:45	51	17	6	9	0	0	0	85
1 Hr	148	75	24	44	4	3	2	300
17:00	61	16	7	8	1	1	1	95
17:15	47	16	4	5	1	2	1	76
17:30	54	54	7	1	1	0	0	116
17:45	49	8	9	5	2	1	1	75
1 Hr	211	54	27	24	5	4	6	331
18:00	35	6	5	1	3	1	3	54
18:15	33	11	3	5	2	1	1	56
18:30	27	7	3	4	0	1	1	44
18:45	19	6	2	1	1	1	1	31

TRAFFIC AND TRANSPORTATION 13

APPENDIX 13-B TRAFFIC FLOW SHEETS

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R135 / N2 Slip Road Priority Junction - AM Peak Hour

2016 Existing AM Peak

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	58	58
N2 Slip Road	396	0	128	524
R135 (South)	86	0	0	86
Totals	482	0	186	668

2017 AM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	58	58
N2 Slip Road	399	0	129	528
R135 (South)	87	0	0	87
Totals	485	0	187	673

2017 AM Peak Development Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	1	1
N2 Slip Road	0	0	2	2
R135 (South)	3	0	0	3
Totals	3	0	3	6

2017 AM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	59	59
N2 Slip Road	399	0	131	530
R135 (South)	90	0	0	90
Totals	488	0	190	679

2022 AM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	61	61
N2 Slip Road	413	0	134	547
R135 (South)	90	0	0	90
Totals	503	0	194	697

2022 AM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	62	62
N2 Slip Road	413	0	136	549
R135 (South)	93	0	0	93
Totals	506	0	197	703

2022 AM Peak Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	22	22
N2 Slip Road	0	0	22	22
R135 (South)	44	0	0	44
Totals	44	0	44	88

2022 AM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	84	84
N2 Slip Road	413	0	158	571
R135 (South)	137	0	0	137
Totals	550	0	241	791

2032 AM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	63	63
N2 Slip Road	430	0	139	569
R135 (South)	93	0	0	93
Totals	523	0	202	725

2032 AM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	64	64
N2 Slip Road	430	0	141	571
R135 (South)	96	0	0	96
Totals	526	0	205	731

2032 AM Peak Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	16	16
N2 Slip Road	0	0	16	16
R135 (South)	32	0	0	32
Totals	32	0	32	64

2032 AM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	80	80
N2 Slip Road	430	0	157	587
R135 (South)	128	0	0	128
Totals	558	0	237	795

R135 / N2 Slip Road Priority Junction - PM Peak Hour

2016 Existing PM Peak

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	20	20
N2 Slip Road	308	0	20	328
R135 (South)	116	0	0	116
Totals	424	0	40	464

2017 PM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	20	20
N2 Slip Road	310	0	20	330
R135 (South)	117	0	0	117
Totals	427	0	40	467

2017 PM Peak Development Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	1	1
N2 Slip Road	0	0	1	1
R135 (South)	3	0	0	3
Totals	3	0	2	5

2017 PM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	21	21
N2 Slip Road	310	0	21	331
R135 (South)	120	0	0	120
Totals	430	0	42	472

2022 PM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	21	21
N2 Slip Road	321	0	21	342
R135 (South)	121	0	0	121
Totals	442	0	42	484

2022 PM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	22	22
N2 Slip Road	321	0	22	343
R135 (South)	124	0	0	124
Totals	445	0	44	489

2022 PM Peak Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	22	22
N2 Slip Road	0	0	22	22
R135 (South)	44	0	0	44
Totals	44	0	44	88

2022 PM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	44	44
N2 Slip Road	321	0	44	365
R135 (South)	168	0	0	168
Totals	489	0	88	577

2032 PM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	22	22
N2 Slip Road	334	0	22	356
R135 (South)	126	0	0	126
Totals	460	0	43	504

2032 PM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	23	23
N2 Slip Road	334	0	23	357
R135 (South)	129	0	0	129
Totals	463	0	45	509

2032 PM Peak Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	16	16
N2 Slip Road	0	0	16	16
R135 (South)	32	0	0	32
Totals	32	0	32	64

2032 PM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	39	39
N2 Slip Road	334	0	39	373
R135 (South)	161	0	0	161
Totals	495	0	77	573

R135 / Elm Road Signalised Junction - AM Peak Hour

2016 Existing AM Peak

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	230	63	293
Elm Road	21	0	3	24
R135 (South)	319	122	0	441
Totals	340	352	66	758

2016 AM Peak PCU Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	283	71	354
Elm Road	22	0	4	26
R135 (South)	371	173	0	544
Totals	393	456	75	924

2017 AM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	285	72	357
Elm Road	22	0	4	26
R135 (South)	374	174	0	548
Totals	396	459	76	931

2017 AM Peak Development Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	0	1	1
Elm Road	0	0	0	0
R135 (South)	1	2	0	3
Totals	1	2	1	4

2017 AM Peak With Development

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	285	73	358
Elm Road	22	0	4	26
R135 (South)	375	176	0	551
Totals	397	461	77	935

2022 AM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	295	74	369
Elm Road	23	0	4	27
R135 (South)	387	181	0	568
Totals	410	476	78	964

2022 AM Peak With Development

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	295	75	370
Elm Road	23	0	4	27
R135 (South)	388	183	0	571
Totals	411	478	79	968

2022 AM Peak Sensitivity Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	0	22	22
Elm Road	0	0	0	0
R135 (South)	22	22	0	44
Totals	22	22	22	66

2022 AM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	295	97	392
Elm Road	23	0	4	27
R135 (South)	410	205	0	615
Totals	433	500	101	1034

2032 AM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	307	77	384
Elm Road	24	0	4	28
R135 (South)	403	188	0	591
Totals	427	495	81	1003

2032 AM Peak With Development

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	307	78	385
Elm Road	24	0	4	28
R135 (South)	404	190	0	594
Totals	428	497	82	1007

2032 AM Peak Sensitivity Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	0	16	16
Elm Road	0	0	0	0
R135 (South)	16	16	0	32
Totals	16	16	16	48

2032 AM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	307	94	401
Elm Road	24	0	4	28
R135 (South)	420	206	0	626
Totals	444	513	98	1055

R135 / Elm Road Signalised Junction - PM Peak Hour

2016 Existing PM Peak

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	313	21	334
Elm Road	29	0	2	31
R135 (South)	320	144	0	464
Totals	349	457	23	829

2016 PM Peak PCU Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	337	28	365
Elm Road	30	0	2	32
R135 (South)	378	164	0	542
Totals	408	501	30	939

2017 PM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	339	28	368
Elm Road	30	0	2	32
R135 (South)	381	165	0	546
Totals	411	505	30	946

2017 PM Peak Development Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	0	1	1
Elm Road	0	0	0	0
R135 (South)	1	2	0	3
Totals	1	2	1	4

2017 PM Peak With Development

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	339	29	369
Elm Road	30	0	2	32
R135 (South)	382	167	0	549
Totals	412	507	31	950

2022 PM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	352	29	381
Elm Road	31	0	2	33
R135 (South)	394	171	0	566
Totals	426	523	31	980

2022 PM Peak With Development

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	352	30	382
Elm Road	31	0	2	33
R135 (South)	395	173	0	569
Totals	427	525	32	984

2022 PM Peak Sensitivity Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	0	0	22
Elm Road	0	0	0	0
R135 (South)	22	22	0	44
Totals	22	22	22	66

2022 PM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	352	52	404
Elm Road	31	0	2	33
R135 (South)	417	195	0	613
Totals	449	547	54	1050

2032 PM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	366	30	396
Elm Road	33	0	2	35
R135 (South)	410	178	0	589
Totals	443	544	33	1020

2032 PM Peak With Development

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	366	31	397
Elm Road	33	0	2	35
R135 (South)	411	180	0	592
Totals	444	546	34	1024

2032 PM Peak Sensitivity Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	0	16	16
Elm Road	0	0	0	0
R135 (South)	16	16	0	32
Totals	16	16	16	48

2032 PM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	366	47	413
Elm Road	33	0	2	35
R135 (South)	427	196	0	624
Totals	460	562	50	1072

R135 / L3125 Signalised Junction - AM Peak Hour

2016 Existing AM Peak

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	219	117	29	365
L3125 (East)	82	0	90	320	492
R135 (South)	59	73	0	205	337
L3125 (West)	9	134	81	0	224
Totals	150	426	288	554	1418

2016 AM Peak PCU Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	234	137	30	401
L3125 (East)	93	0	101	331	525
R135 (South)	77	88	0	224	389
L3125 (West)	11	163	111	0	285
Totals	181	485	349	585	1600

2017 AM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	236	138	30	404
L3125 (East)	94	0	102	333	529
R135 (South)	78	89	0	226	392
L3125 (West)	11	164	112	0	287
Totals	182	488	351	589	1611

2017 AM Peak Development Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	1	0	1
L3125 (East)	0	0	0	0	0
R135 (South)	1	0	0	0	1
L3125 (West)	0	0	0	0	0
Totals	1	0	1	0	2

2017 AM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	236	139	30	405
L3125 (East)	94	0	102	333	529
R135 (South)	79	89	0	226	393
L3125 (West)	11	164	112	0	287
Totals	183	488	352	589	1613

2022 AM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	244	143	31	418
L3125 (East)	97	0	105	345	548
R135 (South)	80	92	0	234	406
L3125 (West)	11	170	116	0	297
Totals	189	506	364	611	1670

2022 AM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	244	144	31	419
L3125 (East)	97	0	105	345	548
R135 (South)	81	92	0	234	407
L3125 (West)	11	170	116	0	297
Totals	190	506	365	611	1672

2022 AM Peak Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	22	0	22
L3125 (East)	0	0	0	0	0
R135 (South)	22	0	0	0	22
L3125 (West)	0	0	0	0	0
Totals	22	0	22	0	44

2022 AM Peak With Development + Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	244	166	31	441
L3125 (East)	97	0	105	345	548
R135 (South)	103	92	0	234	429
L3125 (West)	11	170	116	0	297
Totals	212	506	387	611	1716

2032 AM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	254	149	33	435
L3125 (East)	101	0	110	359	570
R135 (South)	84	96	0	243	422
L3125 (West)	12	177	121	0	309
Totals	197	527	379	635	1737

2032 AM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	254	150	33	436
L3125 (East)	101	0	110	359	570
R135 (South)	85	96	0	243	423
L3125 (West)	12	177	121	0	309
Totals	198	527	380	635	1739

2032 AM Peak Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	16	0	16
L3125 (East)	0	0	0	0	0
R135 (South)	16	0	0	0	16
L3125 (West)	0	0	0	0	0
Totals	16	0	16	0	32

2032 AM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	254	166	33	452
L3125 (East)	101	0	110	359	570
R135 (South)	101	96	0	243	439
L3125 (West)	12	177	121	0	309
Totals	214	527	396	635	1771

R135 / L3125 Signalised Junction - PM Peak Hour

2016 Existing PM Peak

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	168	86	16	270
N2 Slip Road	204	0	85	199	488
R135 (South)	111	120	0	110	341
L3125 (West)	22	273	163	0	458
Totals	337	561	334	325	1557

2016 PM Peak PCU Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	177	97	18	292
N2 Slip Road	216	0	91	221	528
R135 (South)	127	136	0	135	398
L3125 (West)	24	179	175	0	378
Totals	367	492	363	374	1596

2017 PM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	178	98	18	294
N2 Slip Road	218	0	92	223	532
R135 (South)	128	137	0	136	401
L3125 (West)	24	180	176	0	381
Totals	370	495	366	377	1607

2017 PM Peak Development Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	1	0	1
N2 Slip Road	0	0	0	0	0
R135 (South)	1	0	0	0	1
L3125 (West)	0	0	0	0	0
Totals	1	0	1	0	2

2017 PM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	178	99	18	295
N2 Slip Road	218	0	92	223	532
R135 (South)	129	137	0	136	402
L3125 (West)	24	180	176	0	381
Totals	371	495	367	377	1609

2022 PM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	186	102	19	307
N2 Slip Road	227	0	96	232	555
R135 (South)	133	143	0	142	418
L3125 (West)	25	188	184	0	397
Totals	386	517	382	393	1677

2022 PM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	186	103	19	308
N2 Slip Road	227	0	96	232	555
R135 (South)	134	143	0	142	419
L3125 (West)	25	188	184	0	397
Totals	387	517	383	393	1679

2022 PM Peak Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	22	0	22
N2 Slip Road	0	0	0	0	0
R135 (South)	22	0	0	0	22
L3125 (West)	0	0	0	0	0
Totals	22	0	22	0	44

2022 PM Peak With Development + Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	186	125	19	330
N2 Slip Road	227	0	96	232	555
R135 (South)	156	143	0	142	441
L3125 (West)	25	188	184	0	397
Totals	409	517	405	393	1723

2032 PM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	192	105	20	317
N2 Slip Road	235	0	99	240	573
R135 (South)	138	148	0	147	433
L3125 (West)	26	194	190	0	410
Totals	399	534	394	406	1733

2032 PM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	192	106	20	318
N2 Slip Road	235	0	99	240	573
R135 (South)	139	148	0	147	433
L3125 (West)	26	194	190	0	410
Totals	400	534	395	406	1735

2032 PM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	16	0	16
N2 Slip Road	0	0	0	0	0
R135 (South)	16	0	0	0	16
L3125 (West)	0	0	0	0	0
Totals	16	0	16	0	32

2032 PM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	192	122	20	334
N2 Slip Road	235	0	99	240	573
R135 (South)	155	148	0	147	449
L3125 (West)	26	194	190	0	410
Totals	416	534	411	406	1767

R135 / N2 Roundabout Junction - AM Peak Hour

2016 Existing AM Peak

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	170	139	309
R135 (South)	48	0	88	136
N2 Slip Road	63	186	0	249
Totals	111	356	227	694

2017 AM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	171	140	311
R135 (South)	48	0	89	137
N2 Slip Road	63	187	0	251
Totals	112	359	229	699

2017 AM Peak Development Flows

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	0	1	1
R135 (South)	0	0	1	1
N2 Slip Road	0	0	0	0
Totals	0	0	2	2

2017 AM Peak With Development

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	171	141	312
R135 (South)	48	0	90	138
N2 Slip Road	63	187	0	251
Totals	112	359	231	701

2022 AM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	177	145	322
R135 (South)	50	0	92	142
N2 Slip Road	66	194	0	260
Totals	116	372	237	724

2022 AM Peak With Development

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	177	146	323
R135 (South)	50	0	93	143
N2 Slip Road	66	194	0	260
Totals	116	372	239	726

2022 AM Peak Sensitivity Flows

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	0	22	22
R135 (South)	0	0	22	22
N2 Slip Road	0	0	0	0
Totals	0	0	44	44

2022 AM Peak With Development + Sensitivity Flows

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	177	168	345
R135 (South)	50	0	115	165
N2 Slip Road	66	194	0	260
Totals	116	372	283	770

2032 AM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	185	151	336
R135 (South)	52	0	96	148
N2 Slip Road	68	202	0	270
Totals	121	387	246	754

2032 AM Peak With Development

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	185	152	337
R135 (South)	52	0	97	149
N2 Slip Road	68	202	0	270
Totals	121	387	248	756

2032 AM Peak Sensitivity Flows

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	0	16	16
R135 (South)	0	0	16	16
N2 Slip Road	0	0	0	0
Totals	0	0	32	32

2032 AM Peak With Development + Sensitivity Flows

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	185	168	353
R135 (South)	52	0	113	165
N2 Slip Road	68	202	0	270
Totals	121	387	280	788

R135 / N2 Roundabout Junction - PM Peak Hour

2016 Existing PM Peak

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	111	86	197
R135 (South)	185	0	175	360
N2 Slip Road	127	160	0	287
Totals	312	271	261	844

2017 PM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	112	87	198
R135 (South)	186	0	176	363
N2 Slip Road	128	161	0	289
Totals	314	273	263	850

2017 PM Peak Development Flows

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	0	1	1
R135 (South)	0	0	1	1
N2 Slip Road	0	0	0	0
Totals	0	0	2	2

2017 PM Peak With Development

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	112	88	199
R135 (South)	186	0	177	364
N2 Slip Road	128	161	0	289
Totals	314	273	265	852

2022 PM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	116	90	206
R135 (South)	193	0	183	376
N2 Slip Road	133	167	0	300
Totals	326	283	272	881

2022 PM Peak With Development

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	116	91	207
R135 (South)	193	0	184	377
N2 Slip Road	133	167	0	300
Totals	326	283	274	883

2022 PM Peak Sensitivity Flows

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	0	22	22
R135 (South)	0	0	22	22
N2 Slip Road	0	0	0	0
Totals	0	0	44	44

2022 PM Peak With Development + Sensitivity Flows

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	116	113	229
R135 (South)	193	0	206	399
N2 Slip Road	133	167	0	300
Totals	326	283	318	927

2022 PM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	121	93	214
R135 (South)	201	0	190	391
N2 Slip Road	138	174	0	312
Totals	339	294	283	916

2022 PM Peak With Development

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	121	94	215
R135 (South)	201	0	191	392
N2 Slip Road	138	174	0	312
Totals	339	294	285	918

2022 PM Peak Sensitivity Flows

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	0	16	16
R135 (South)	0	0	16	16
N2 Slip Road	0	0	0	0
Totals	0	0	32	32

2022 PM Peak With Development + Sensitivity Flows

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	121	110	231
R135 (South)	201	0	207	408
N2 Slip Road	138	174	0	312
Totals	339	294	317	950

R135 / L3125 Signalised Junction - AM Peak Hour

2016 Existing AM Peak

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	219	117	29	365
L3125 (East)	82	0	90	320	492
R135 (South)	59	73	0	205	337
L3125 (West)	9	134	81	0	224
Totals	150	426	288	554	1418

2016 AM Peak PCU Flows + Re-distributed Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	117	137	30	284
L3125 (East)	46	0	101	331	478
R135 (South)	77	88	0	224	389
L3125 (West)	11	163	111	0	285
Totals	134	368	349	585	1436

2017 AM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	118	138	30	286
L3125 (East)	46	0	102	333	481
R135 (South)	78	89	0	226	392
L3125 (West)	11	164	112	0	287
Totals	135	371	351	589	1446

2017 AM Peak Development Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	1	0	1
L3125 (East)	0	0	0	0	0
R135 (South)	1	0	0	0	1
L3125 (West)	0	0	0	0	0
Totals	1	0	1	0	2

2017 AM Peak With Development

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	118	139	30	287
L3125 (East)	46	0	102	333	481
R135 (South)	79	89	0	226	393
L3125 (West)	11	164	112	0	287
Totals	136	371	352	589	1448

2022 AM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	122	143	31	296
L3125 (East)	48	0	105	345	499
R135 (South)	80	92	0	234	406
L3125 (West)	11	170	116	0	297
Totals	140	384	364	611	1499

2022 AM Peak With Development + Opening of Airport Link Road

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	122	144	31	297
L3125 (East)	48	0	105	345	499
R135 (South)	81	92	0	234	407
L3125 (West)	11	170	116	0	297
Totals	141	384	365	611	1501

2022 AM Peak Sensitivity Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	22	0	22
L3125 (East)	0	0	0	0	0
R135 (South)	22	0	0	0	22
L3125 (West)	0	0	0	0	0
Totals	22	0	22	0	44

2022 AM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	122	166	31	319
L3125 (East)	48	0	105	345	499
R135 (South)	103	92	0	234	429
L3125 (West)	11	170	116	0	297
Totals	163	384	387	611	1545

2032 AM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	127	149	33	308
L3125 (East)	50	0	110	359	519
R135 (South)	84	96	0	243	422
L3125 (West)	12	177	121	0	309
Totals	146	400	379	635	1559

2032 AM Peak With Development + Opening of Airport Link Road

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	127	150	33	309
L3125 (East)	50	0	110	359	519
R135 (South)	85	96	0	243	423
L3125 (West)	12	177	121	0	309
Totals	147	400	380	635	1561

2032 AM Peak Sensitivity Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	16	0	16
L3125 (East)	0	0	0	0	0
R135 (South)	16	0	0	0	16
L3125 (West)	0	0	0	0	0
Totals	16	0	16	0	32

2032 AM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	127	166	33	325
L3125 (East)	50	0	110	359	519
R135 (South)	101	96	0	243	439
L3125 (West)	12	177	121	0	309
Totals	163	400	396	635	1593

R135 / L3125 Signalised Junction - PM Peak Hour

2016 Existing PM Peak

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	168	86	16	270
L3125 (East)	204	0	85	199	488
R135 (South)	111	120	0	110	341
L3125 (West)	22	273	163	0	458
Totals	337	561	334	325	1557

2016 PM Peak PCU Flows + Re-distributed Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	88	97	18	203
L3125 (East)	108	0	91	221	420
R135 (South)	127	136	0	135	398
L3125 (West)	24	179	175	0	378
Totals	259	403	363	374	1399

2017 PM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	89	98	18	204
L3125 (East)	109	0	92	223	423
R135 (South)	128	137	0	136	401
L3125 (West)	24	180	176	0	381
Totals	261	406	366	377	1409

2017 PM Peak Development Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	1	0	1
L3125 (East)	0	0	0	0	0
R135 (South)	1	0	0	0	1
L3125 (West)	0	0	0	0	0
Totals	1	0	1	0	2

2017 PM Peak With Development

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	89	99	18	205
L3125 (East)	109	0	92	223	423
R135 (South)	129	137	0	136	402
L3125 (West)	24	180	176	0	381
Totals	262	406	367	377	1411

2022 PM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	92	102	19	213
L3125 (East)	114	0	96	232	441
R135 (South)	133	143	0	142	418
L3125 (West)	25	188	184	0	397
Totals	272	424	382	393	1470

2022 PM Peak With Development + Opening of Airport Link Road

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	92	103	19	214
L3125 (East)	114	0	96	232	441
R135 (South)	134	143	0	142	419
L3125 (West)	25	188	184	0	397
Totals	273	424	383	393	1472

2022 PM Peak Sensitivity Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	22	0	22
L3125 (East)	0	0	0	0	0
R135 (South)	22	0	0	0	22
L3125 (West)	0	0	0	0	0
Totals	22	0	22	0	44

2022 PM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	92	125	19	236
L3125 (East)	114	0	96	232	441
R135 (South)	156	143	0	142	441
L3125 (West)	25	188	184	0	397
Totals	295	424	405	393	1516

2032 PM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	96	105	20	220
L3125 (East)	117	0	99	240	456
R135 (South)	138	148	0	147	432
L3125 (West)	26	194	190	0	410
Totals	281	438	394	406	1519

2032 PM Peak With Development + Opening of Airport Link Road

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	96	106	20	221
L3125 (East)	117	0	99	240	456
R135 (South)	139	148	0	147	433
L3125 (West)	26	194	190	0	410
Totals	282	438	395	406	1521

2032 PM Peak Sensitivity Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	16	0	16
L3125 (East)	0	0	0	0	0
R135 (South)	16	0	0	0	16
L3125 (West)	0	0	0	0	0
Totals	16	0	16	0	32

2032 PM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	96	122	20	237
L3125 (East)	117	0	99	240	456
R135 (South)	155	148	0	147	449
L3125 (West)	26	194	190	0	410
Totals	298	438	411	406	1553

TRAFFIC AND TRANSPORTATION 13

APPENDIX 13-C PICADY RESULTS

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THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
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Run with file:-
 "C:\PICADY\2016\16047-04\R135 - N2 Slip Road Priority Junction.vpi"
 (drive-on-the-left) at 11:50:34 on Friday, 2 December 2016

.RUN INFORMATION

RUN TITLE: Huntstown Quarry
 LOCATION: Huntstown
 DATE: 02/12/16
 CLIENT: Roadstone
 ENUMERATOR: Roadplan Consulting
 JOB NUMBER: 16047-04
 STATUS: TIA
 DESCRIPTION:

.MAJOR/MINOR JUNCTION CAPACITY AND DELAY

INPUT DATA

MAJOR ROAD (ARM C) ----- MAJOR ROAD (ARM A)

I
 I
 I
 I
 I
 I
 I

MINOR ROAD (ARM B)

ARM A IS R135 North
 ARM B IS N2 Slip Road
 ARM C IS R135 South

STREAM LABELLING CONVENTION

STREAM A-B CONTAINS TRAFFIC GOING FROM ARM A TO ARM B
 STREAM B-AC CONTAINS TRAFFIC GOING FROM ARM B TO ARM A AND TO ARM C ETC.

.GEOMETRIC DATA

I	DATA ITEM	I	MINOR ROAD B	I
I	TOTAL MAJOR ROAD CARRIAGEWAY WIDTH	I (W)	7.20 M.	I
I	CENTRAL RESERVE WIDTH	I (WCR)	0.00 M.	I
I		I		I
I	MAJOR ROAD RIGHT TURN - WIDTH	I (WC-B)	2.20 M.	I
I	- VISIBILITY	I (VC-B)	200.0 M.	I
I	- BLOCKS TRAFFIC	I	NO	I
I		I		I
I	MINOR ROAD - VISIBILITY TO LEFT	I (VB-C)	50.0 M.	I
I	- VISIBILITY TO RIGHT	I (VB-A)	50.0 M.	I
I	- LANE 1 WIDTH	I (WB-C)	-	I
I	- LANE 2 WIDTH	I (WB-A)	-	I
I	- WIDTH AT 0 M FROM JUNC.	I	6.00 M.	I
I	- WIDTH AT 5 M FROM JUNC.	I	6.00 M.	I
I	- WIDTH AT 10 M FROM JUNC.	I	6.00 M.	I
I	- WIDTH AT 15 M FROM JUNC.	I	6.00 M.	I
I	- WIDTH AT 20 M FROM JUNC.	I	6.00 M.	I
I	- LENGTH OF FLARED SECTION	I	10 VEHS	I

Consent of copyright owner required for any other use.
 For inspection purposes only.

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	602.92		0.22		0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
I	476.98		0.21		0.08		0.13		0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	689.79		0.25		0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2016 AM Peak - Existing Flows

TIME PERIOD BEGINS 07.15 AND ENDS 08.45

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.73	I	1.09	I	0.73
I	ARM B	I	15.00	I	45.00	I	75.00	I	6.55	I	9.83	I	6.55
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.08	I	1.61	I	1.08

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	07.15 - 08.45	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(14.0)	I
I		I	ARM B	I	0.756	I	0.000	I	0.244	I
I		I		I	396.0	I	0.0	I	128.0	I
I		I		I	(10.0)	I	(0.0)	I	(33.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	86.0	I	0.0	I	0.0	I
I		I		I	(60.0)	I	(0.0)	I	(0.0)	I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2016 AM Peak - Existing Flows
AND FOR TIME PERIOD 1

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.15-07.30									
B-C	1.61	5.84	0.275		0.00	0.37	5.2		0.23
B-A	4.97	8.61	0.577		0.00	1.31	17.9		0.26
C-A	1.08								
C-B	0.00	10.26	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.73								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.30-07.45									
B-C	1.92	5.55	0.345		0.37	0.51	7.4		0.27
B-A	5.93	8.52	0.696		1.31	2.13	29.3		0.37
C-A	1.29								
C-B	0.00	10.22	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.87								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.45-08.00									
B-C	2.35	4.70	0.500		0.51	0.95	13.1		0.42
B-A	7.27	8.40	0.865		2.13	4.28	60.4		0.68
C-A	1.58								
C-B	0.00	10.17	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.06								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
B-C	2.35	4.47	0.526		0.95	1.06	15.4		0.47
B-A	7.27	8.40	0.865		4.88	5.44	78.0		0.80
C-A	1.58								
C-B	0.00	10.17	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.06								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
B-C	1.92	5.47	0.351		1.06	0.56	8.9		0.29
B-A	5.93	8.52	0.696		5.44	2.47	42.7		0.45
C-A	1.29								
C-B	0.00	10.22	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.87								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
B-C	1.61	5.80	0.277		0.56	0.39	6.1		0.24
B-A	4.97	8.61	0.577		2.47	1.42	23.0		0.29
C-A	1.08								
C-B	0.00	10.26	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.73								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.4
07.45	0.5 *
08.00	0.9 *
08.15	1.1 *
08.30	0.6 *
08.45	0.4

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	1.3 *
07.45	2.1 **
08.00	4.9 *****
08.15	5.4 *****
08.30	2.5 **
08.45	1.4 *

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.0
07.45	0.0
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * DELAY (MIN)	* (MIN/VEH)	* INCLUSIVE QUEUEING * DELAY (MIN)	* (MIN/VEH)
B-C	176.2	117.5	56.1	0.32	56.1	0.32
B-A	545.1	363.4	251.3	0.46	251.4	0.46
C-A	118.4	78.9				
C-B	0.0	0.0	0.0	0.00	0.0	0.00
A-B	0.0	0.0				
A-C	79.8	53.2				
ALL	919.5	613.0	307.4	0.33	307.5	0.33

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	602.92	0.22	0.09	I

I	Intercept For Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	476.98	0.21	0.08	0.13	0.30	I

I	Intercept For Stream C-B	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	689.79	0.25	0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2017 AM Peak - No Development

TIME PERIOD BEGINS 07.15 AND ENDS 08.45

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	NUMBER OF MINUTES FROM START WHEN TOP OF PEAK IS REACHED	I	NUMBER OF MINUTES FROM START WHEN FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	I	RATE OF FLOW (VEH/MIN) AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.73	I	1.09	I	0.73
I	ARM B	I	15.00	I	45.00	I	75.00	I	6.60	I	9.90	I	6.60
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.09	I	1.63	I	1.09

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C
I	07.15 - 08.45	I	ARM A	I	0.000	I	0.000	I	1.000
I		I		I	(0.0)	I	(0.0)	I	(14.0)
I		I	ARM B	I	0.756	I	0.000	I	0.244
I		I		I	399.0	I	0.0	I	129.0
I		I		I	(10.0)	I	(0.0)	I	(33.0)
I		I	ARM C	I	1.000	I	0.000	I	0.000
I		I		I	87.0	I	0.0	I	0.0
I		I		I	(60.0)	I	(0.0)	I	(0.0)

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2017 AM Peak - No Development
AND FOR TIME PERIOD 1

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.15-07.30									
B-C	1.62	5.83	0.278		0.00	0.38	5.3		0.23
B-A	5.01	8.61	0.582		0.00	1.33	18.2		0.27
C-A	1.09								
C-B	0.00	10.26	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.73								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.30-07.45									
B-C	1.93	5.54	0.349		0.38	0.52	7.5		0.28
B-A	5.98	8.52	0.702		1.33	2.19	29.9		0.38
C-A	1.30								
C-B	0.00	10.22	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.87								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.45-08.00									
B-C	2.37	4.63	0.511		0.52	0.99	13.6		0.43
B-A	7.32	8.39	0.872		2.19	5.08	62.5		0.70
C-A	1.60								
C-B	0.00	10.17	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.06								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
B-C	2.37	4.37	0.542		0.99	1.13	16.2		0.49
B-A	7.32	8.39	0.872		5.08	5.70	81.5		0.84
C-A	1.60								
C-B	0.00	10.17	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.06								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
B-C	1.93	5.45	0.355		1.13	0.57	9.1		0.29
B-A	5.98	8.52	0.702		5.70	2.55	44.3		0.46
C-A	1.30								
C-B	0.00	10.22	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.87								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
B-C	1.62	5.79	0.279		0.57	0.40	6.2		0.24
B-A	5.01	8.61	0.582		2.55	1.45	23.5		0.29
C-A	1.09								
C-B	0.00	10.26	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.73								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.4
07.45	0.5 *
08.00	1.0 *
08.15	1.1 *
08.30	0.6 *
08.45	0.4

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	1.3 *
07.45	2.2 **
08.00	5.1 *****
08.15	5.7 *****
08.30	2.5 ***
08.45	1.4 *

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.0
07.45	0.0
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	INCLUSIVE QUEUEING (VEH/H)	* QUEUEING (MIN)	* INCLUSIVE QUEUEING (MIN/VEH)
B-C	177.6	118.4	58.0	0.33
B-A	549.2	366.1	260.0	0.47
C-A	119.7	79.8		
C-B	0.0	0.0	0.0	0.00
A-B	0.0	0.0		
A-C	79.8	53.2		
ALL	926.3	617.6	317.9	0.34

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	602.92		0.22		0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
I	476.98		0.21		0.08		0.13		0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	689.79		0.25		0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2017 AM Peak - With Development

TIME PERIOD BEGINS 07.15 AND ENDS 08.45

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.74	I	1.11	I	0.74
I	ARM B	I	15.00	I	45.00	I	75.00	I	6.63	I	9.94	I	6.63
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.13	I	1.69	I	1.13

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	07.15 - 08.45	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	0.0	I	0.0	I	59.0	I
I		I		I	(0.0)	I	(0.0)	I	(14.0)	I
I		I	ARM B	I	0.753	I	0.000	I	0.247	I
I		I		I	399.0	I	0.0	I	131.0	I
I		I		I	(10.0)	I	(0.0)	I	(33.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	90.0	I	0.0	I	0.0	I
I		I		I	(60.0)	I	(0.0)	I	(0.0)	I
I		I		I		I		I		I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2017 AM Peak - With Development
AND FOR TIME PERIOD 1

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.15-07.30									
B-C	1.64	5.83	0.282		0.00	0.38	5.4		0.24
B-A	5.01	8.58	0.583		0.00	1.34	18.3		0.27
C-A	1.13								
C-B	0.00	10.26	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.74								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.30-07.45									
B-C	1.96	5.54	0.354		0.38	0.53	7.6		0.28
B-A	5.98	8.49	0.704		1.34	2.21	30.2		0.38
C-A	1.35								
C-B	0.00	10.22	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.88								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.45-08.00									
B-C	2.40	4.61	0.522		0.53	1.03	14.1		0.44
B-A	7.32	8.36	0.876		2.21	5.17	63.4		0.71
C-A	1.65								
C-B	0.00	10.17	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.08								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
B-C	2.40	4.33	0.555		1.03	1.18	17.0		0.51
B-A	7.32	8.36	0.876		5.17	5.82	83.2		0.86
C-A	1.65								
C-B	0.00	10.17	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.08								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
B-C	1.96	5.44	0.360		1.18	0.58	9.3		0.29
B-A	5.98	8.49	0.704		5.82	2.58	45.0		0.47
C-A	1.35								
C-B	0.00	10.22	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.88								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
B-C	1.64	5.80	0.284		0.58	0.40	6.3		0.24
B-A	5.01	8.58	0.583		2.58	1.46	23.6		0.29
C-A	1.13								
C-B	0.00	10.26	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.74								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.4
07.45	0.5 *
08.00	1.0 *
08.15	1.2 *
08.30	0.6 *
08.45	0.4

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	1.3 *
07.45	2.2 **
08.00	5.2 *****
08.15	5.8 *****
08.30	2.6 ***
08.45	1.5 *

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.0
07.45	0.0
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * DELAY (MIN)	* (MIN/VEH)	* INCLUSIVE QUEUEING * DELAY (MIN)	* (MIN/VEH)
B-C	180.3	120.2	59.9	0.33	59.9	0.33
B-A	549.2	366.1	263.8	0.48	263.9	0.48
C-A	123.9	82.6				
C-B	0.0	0.0	0.0	0.00	0.0	0.00
A-B	0.0	0.0				
A-C	81.2	54.1				
ALL	934.6	623.1	323.7	0.35	323.8	0.35

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	602.92		0.22		0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
I	476.98		0.21		0.08		0.13		0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	689.79		0.25		0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2022 AM Peak - No Development

TIME PERIOD BEGINS 07.15 AND ENDS 08.45

LENGTH OF TIME PERIOD - 90 MINUTES.

LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.76	I	1.14	I	0.76
I	ARM B	I	15.00	I	45.00	I	75.00	I	6.84	I	10.26	I	6.84
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.13	I	1.69	I	1.13

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	07.15 - 08.45	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(14.0)	I
I		I	ARM B	I	0.755	I	0.000	I	0.245	I
I		I		I	413.0	I	0.0	I	134.0	I
I		I		I	(10.0)	I	(0.0)	I	(33.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	90.0	I	0.0	I	0.0	I
I		I		I	(60.0)	I	(0.0)	I	(0.0)	I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2022 AM Peak - No Development
AND FOR TIME PERIOD 1

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.15-07.30									
B-C	1.68	5.78	0.291		0.00	0.40	5.6		0.24
B-A	5.18	8.58	0.604		0.00	1.45	19.7		0.28
C-A	1.13								
C-B	0.00	10.25	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.77								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.30-07.45									
B-C	2.01	5.46	0.368		0.40	0.57	8.1		0.29
B-A	6.19	8.49	0.729		1.45	2.46	33.4		0.41
C-A	1.35								
C-B	0.00	10.21	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.91								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.45-08.00									
B-C	2.46	4.20	0.586		0.57	1.30	17.4		0.55
B-A	7.58	8.36	0.906		2.46	6.21	74.0		0.81
C-A	1.65								
C-B	0.00	10.16	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.12								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
B-C	2.46	3.70	0.665		1.30	1.77	24.4		0.76
B-A	7.58	8.36	0.906		6.21	7.26	102.0		1.03
C-A	1.65								
C-B	0.00	10.16	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.12								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
B-C	2.01	5.31	0.378		1.77	0.63	10.4		0.32
B-A	6.19	8.49	0.729		7.26	2.95	54.1		0.55
C-A	1.35								
C-B	0.00	10.21	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.91								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
B-C	1.68	5.74	0.293		0.63	0.42	6.7		0.25
B-A	5.18	8.58	0.604		2.95	1.59	26.0		0.31
C-A	1.13								
C-B	0.00	10.25	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.77								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.4
07.45	0.6 *
08.00	1.3 *
08.15	1.8 **
08.30	0.6 *
08.45	0.4

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	1.5 *
07.45	2.5 **
08.00	6.2 *****
08.15	7.3 *****
08.30	3.0 ***
08.45	1.6 **

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.0
07.45	0.0
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * DELAY (MIN)	* (MIN/VEH)	* INCLUSIVE QUEUEING * DELAY (MIN)	* (MIN/VEH)
B-C	184.4	123.0	72.6	0.39	72.6	0.39
B-A	568.5	379.0	309.1	0.54	309.2	0.54
C-A	123.9	82.6				
C-B	0.0	0.0	0.0	0.00	0.0	0.00
A-B	0.0	0.0				
A-C	84.0	56.0				
ALL	960.7	640.5	381.7	0.40	381.9	0.40

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	602.92		0.22		0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
I	476.98		0.21		0.08		0.13		0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	689.79		0.25		0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2022 AM Peak - With Development

TIME PERIOD BEGINS 07.15 AND ENDS 08.45

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.77	I	1.16	I	0.77
I	ARM B	I	15.00	I	45.00	I	75.00	I	6.86	I	10.29	I	6.86
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.16	I	1.74	I	1.16

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	07.15 - 08.45	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(14.0)	I
I		I	ARM B	I	0.752	I	0.000	I	0.248	I
I		I		I	413.0	I	0.0	I	136.0	I
I		I		I	(10.0)	I	(0.0)	I	(33.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	93.0	I	0.0	I	0.0	I
I		I		I	(60.0)	I	(0.0)	I	(0.0)	I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2022 AM Peak - With Development
AND FOR TIME PERIOD 1

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.15-07.30									
B-C	1.71	5.79	0.295		0.00	0.41	5.7		0.24
B-A	5.18	8.56	0.605		0.00	1.46	19.8		0.28
C-A	1.17								
C-B	0.00	10.25	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.78								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.30-07.45									
B-C	2.04	5.46	0.373		0.41	0.58	8.3		0.29
B-A	6.19	8.46	0.731		1.46	2.49	33.7		0.41
C-A	1.39								
C-B	0.00	10.21	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.93								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.45-08.00									
B-C	2.50	4.17	0.599		0.58	0.37	18.2		0.56
B-A	7.58	8.33	0.910		2.49	6.33	75.1		0.83
C-A	1.71								
C-B	0.00	10.15	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.14								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
B-C	2.50	3.63	0.687		1.37	1.93	26.3		0.82
B-A	7.58	8.33	0.910		6.33	7.43	104.3		1.06
C-A	1.71								
C-B	0.00	10.15	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.14								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
B-C	2.04	5.30	0.384		1.93	0.64	10.8		0.32
B-A	6.19	8.46	0.731		7.43	2.99	55.2		0.56
C-A	1.39								
C-B	0.00	10.21	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.93								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
B-C	1.71	5.75	0.297		0.64	0.43	6.8		0.25
B-A	5.18	8.56	0.605		2.99	1.61	26.2		0.31
C-A	1.17								
C-B	0.00	10.25	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.78								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	0.4	
07.45	0.6	*
08.00	1.4	*
08.15	1.9	**
08.30	0.6	*
08.45	0.4	

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	1.5	*
07.45	2.5	**
08.00	6.3	*****
08.15	7.4	*****
08.30	3.0	***
08.45	1.6	**

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	0.0	
07.45	0.0	
08.00	0.0	
08.15	0.0	
08.30	0.0	
08.45	0.0	

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * DELAY (MIN)	* (MIN/VEH)	* INCLUSIVE QUEUEING * DELAY (MIN)	* (MIN/VEH)
B-C	187.2	124.8	76.1	0.41	76.1	0.41
B-A	568.5	379.0	314.3	0.55	314.4	0.55
C-A	128.0	85.3				
C-B	0.0	0.0	0.0	0.00	0.0	0.00
A-B	0.0	0.0				
A-C	85.3	56.9				
ALL	969.0	646.0	390.4	0.40	390.5	0.40

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For	Slope For	Opposing	Slope For	Opposing	I
I	Stream B-C	Stream	A-C	Stream	A-B	I
I	602.92		0.22		0.09	I

I	Intercept For	Slope For	Opposing	Slope For	Opposing	Slope For	Opposing	I
I	Stream B-A	Stream	A-C	Stream	A-B	Stream	C-A	Stream
I								
I	476.98		0.21		0.08		0.13	
								0.30

I	Intercept For	Slope For	Opposing	Slope For	Opposing	I
I	Stream C-B	Stream	A-C	Stream	A-B	I
I	689.79		0.25		0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2016 PM Peak - Existing Flows

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.25	I	0.38	I	0.25
I	ARM B	I	15.00	I	45.00	I	75.00	I	4.10	I	6.15	I	4.10
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.45	I	2.18	I	1.45

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	16.30 - 18.00	I	ARM A	I	0.000	I	0.000	I	1.000	I
					(0.0)		(0.0)		(35.0)	
			ARM B		0.939		0.000		0.061	
					308.0		0.0		20.0	
					(19.0)		(0.0)		(50.0)	
			ARM C		1.000		0.000		0.000	
					116.0		0.0		0.0	
					(15.0)		(0.0)		(0.0)	

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2016 PM Peak - Existing Flows
AND FOR TIME PERIOD 2

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	0.25	5.12	0.049		0.00	0.05	0.7		0.21
B-A	3.86	8.40	0.460		0.00	0.83	11.6		0.22
C-A	1.46								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.25								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	0.30	4.96	0.060		0.05	0.06	0.9		0.21
B-A	4.61	8.34	0.554		0.83	1.20	17.0		0.27
C-A	1.74								
C-B	0.00	10.36	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.30								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	0.37	4.71	0.078		0.06	0.08	1.2		0.23
B-A	5.65	8.25	0.685		1.23	2.03	27.8		0.37
C-A	2.13								
C-B	0.00	10.34	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.37								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	0.37	4.69	0.078		0.08	0.08	1.3		0.23
B-A	5.65	8.25	0.685		2.03	2.09	31.0		0.38
C-A	2.13								
C-B	0.00	10.34	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.37								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	0.30	4.94	0.061		0.08	0.07	1.0		0.22
B-A	4.61	8.34	0.554		2.09	1.29	20.7		0.28
C-A	1.74								
C-B	0.00	10.36	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.30								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
B-C	0.25	5.10	0.049		0.07	0.05	0.8		0.21
B-A	3.86	8.40	0.460		1.29	0.87	13.8		0.22
C-A	1.46								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.25								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1
17.45	0.1
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.8 *
17.00	1.2 *
17.15	2.0 **
17.30	2.1 **
17.45	1.3 *
18.00	0.9 *

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * DELAY (MIN)	* INCLUSIVE QUEUEING * DELAY (MIN)
B-C	27.5	18.4	5.9	5.9
B-A	423.9	282.6	121.8	121.8
C-A	159.7	106.4		
C-B	0.0	0.0	0.0	0.0
A-B	0.0	0.0		
A-C	27.5	18.4		
ALL	638.7	425.8	127.7	127.8

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	602.92	0.22	0.09	I

I	Intercept For Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	476.98	0.21	0.08	0.13	0.30	I

I	Intercept For Stream C-B	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	689.79	0.25	0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2017 PM Peak - No Development

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.25	I	0.38	I	0.25
I	ARM B	I	15.00	I	45.00	I	75.00	I	4.13	I	6.19	I	4.13
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.46	I	2.19	I	1.46

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C
I	16.30 - 18.00	I	ARM A	I	0.000	I	0.000	I	1.000
I		I		I	(0.0)	I	(0.0)	I	(35.0)
I		I	ARM B	I	0.939	I	0.000	I	0.061
I		I		I	310.0	I	0.0	I	20.0
I		I		I	(19.0)	I	(0.0)	I	(50.0)
I		I	ARM C	I	1.000	I	0.000	I	0.000
I		I		I	117.0	I	0.0	I	0.0
I		I		I	(15.0)	I	(0.0)	I	(0.0)

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2017 PM Peak - No Development
AND FOR TIME PERIOD 2

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	0.25	5.11	0.049		0.00	0.05	0.7		0.21
B-A	3.89	8.40	0.463		0.00	0.84	11.7		0.22
C-A	1.47								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.25								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	0.30	4.95	0.060		0.05	0.06	0.9		0.21
B-A	4.64	8.33	0.557		0.84	1.21	17.2		0.27
C-A	1.75								
C-B	0.00	10.36	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.30								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	0.37	4.70	0.078		0.06	0.08	1.2		0.23
B-A	5.69	8.25	0.690		1.21	2.07	28.3		0.37
C-A	2.15								
C-B	0.00	10.34	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.37								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	0.37	4.68	0.078		0.08	0.08	1.3		0.23
B-A	5.69	8.25	0.690		2.07	2.14	31.6		0.39
C-A	2.15								
C-B	0.00	10.34	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.37								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	0.30	4.94	0.061		0.08	0.07	1.0		0.22
B-A	4.64	8.33	0.557		2.14	1.31	21.0		0.28
C-A	1.75								
C-B	0.00	10.36	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.30								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
B-C	0.25	5.10	0.049		0.07	0.05	0.8		0.21
B-A	3.89	8.40	0.463		1.31	0.89	14.0		0.22
C-A	1.47								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.25								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1
17.45	0.1
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	0.8	*
17.00	1.2	*
17.15	2.1	**
17.30	2.1	**
17.45	1.3	*
18.00	0.9	*

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * DELAY (MIN)	* (MIN/VEH)	* INCLUSIVE QUEUEING * DELAY (MIN)	* (MIN/VEH)
B-C	27.5	18.4	6.0	0.22	6.0	0.22
B-A	426.7	284.5	123.9	0.29	123.9	0.29
C-A	161.0	107.4				
C-B	0.0	0.0	0.0	0.00	0.0	0.00
A-B	0.0	0.0				
A-C	27.5	18.4				
ALL	642.8	428.5	129.8	0.20	129.9	0.20

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	602.92	0.22	0.09	I

I	Intercept For Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	476.98	0.21	0.08	0.13	0.30	I

I	Intercept For Stream C-B	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	689.79	0.25	0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2017 PM Peak - With Development

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	NUMBER OF MINUTES FROM START WHEN TOP OF PEAK IS REACHED	I	NUMBER OF MINUTES FROM START WHEN FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	I	RATE OF FLOW (VEH/MIN) AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.26	I	0.39	I	0.26
I	ARM B	I	15.00	I	45.00	I	75.00	I	4.14	I	6.21	I	4.14
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.50	I	2.25	I	1.50

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C
I	16.30 - 18.00	I	ARM A	I	0.000	I	0.000	I	1.000
I		I		I	(0.0)	I	(0.0)	I	(35.0)
I		I	ARM B	I	0.937	I	0.000	I	0.063
I		I		I	310.0	I	0.0	I	21.0
I		I		I	(19.0)	I	(0.0)	I	(50.0)
I		I	ARM C	I	1.000	I	0.000	I	0.000
I		I		I	120.0	I	0.0	I	0.0
I		I		I	(15.0)	I	(0.0)	I	(0.0)

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2017 PM Peak - With Development
AND FOR TIME PERIOD 2

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	0.26	5.11	0.052		0.00	0.05	0.8		0.21
B-A	3.89	8.39	0.464		0.00	0.84	11.8		0.22
C-A	1.51								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.26								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	0.31	4.95	0.064		0.05	0.07	1.0		0.22
B-A	4.64	8.32	0.558		0.84	1.22	17.2		0.27
C-A	1.80								
C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.31								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	0.39	4.69	0.082		0.07	0.09	1.3		0.23
B-A	5.69	8.23	0.691		1.22	2.08	28.4		0.38
C-A	2.20								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.39								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	0.39	4.67	0.082		0.09	0.09	1.3		0.23
B-A	5.69	8.23	0.691		2.08	2.15	31.8		0.39
C-A	2.20								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.39								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	0.31	4.93	0.064		0.09	0.07	1.1		0.22
B-A	4.64	8.32	0.558		2.15	1.31	21.1		0.28
C-A	1.80								
C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.31								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
B-C	0.26	5.09	0.052		0.07	0.06	0.9		0.21
B-A	3.89	8.39	0.464		1.31	0.89	14.0		0.23
C-A	1.51								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.26								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1
17.45	0.1
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.8 *
17.00	1.2 *
17.15	2.1 **
17.30	2.1 **
17.45	1.3 *
18.00	0.9 *

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * DELAY (MIN)	* (MIN/VEH)	* INCLUSIVE QUEUEING * DELAY (MIN)	* (MIN/VEH)
B-C	28.9	19.3	6.3	0.22	6.3	0.22
B-A	426.7	284.5	124.3	0.29	124.4	0.29
C-A	165.2	110.1				
C-B	0.0	0.0	0.0	0.00	0.0	0.00
A-B	0.0	0.0				
A-C	28.9	19.3				
ALL	649.7	433.1	130.6	0.20	130.7	0.20

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	602.92		0.22		0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
I	476.98		0.21		0.08		0.13		0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	689.79		0.25		0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2022 PM Peak - No Development

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.

LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.26	I	0.39	I	0.26
I	ARM B	I	15.00	I	45.00	I	75.00	I	4.28	I	6.41	I	4.28
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.51	I	2.27	I	1.51

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	16.30 - 18.00	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(35.0)	I
I		I	ARM B	I	0.939	I	0.000	I	0.061	I
I		I		I	321.0	I	0.0	I	21.0	I
I		I		I	(19.0)	I	(0.0)	I	(50.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	121.0	I	0.0	I	0.0	I
I		I		I	(15.0)	I	(0.0)	I	(0.0)	I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2022 PM Peak - No Development
AND FOR TIME PERIOD 2

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	0.26	5.08	0.052		0.00	0.05	0.8		0.21
B-A	4.03	8.38	0.480		0.00	0.90	12.5		0.22
C-A	1.52								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.26								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	0.31	4.92	0.064		0.05	0.07	1.0		0.22
B-A	4.81	8.32	0.578		0.90	1.32	18.6		0.28
C-A	1.81								
C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.31								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	0.39	4.62	0.083		0.07	0.09	1.3		0.24
B-A	5.89	8.23	0.716		1.32	2.31	31.3		0.40
C-A	2.22								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.39								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	0.39	4.60	0.084		0.09	0.09	1.4		0.24
B-A	5.89	8.23	0.716		2.31	2.40	35.5		0.42
C-A	2.22								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.39								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	0.31	4.90	0.064		0.09	0.07	1.1		0.22
B-A	4.81	8.32	0.578		2.40	1.43	23.1		0.30
C-A	1.81								
C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.31								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
B-C	0.26	5.07	0.052		0.07	0.06	0.9		0.21
B-A	4.03	8.38	0.480		1.43	0.95	15.1		0.23
C-A	1.52								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.26								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1
17.45	0.1
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.9 *
17.00	1.3 *
17.15	2.3 **
17.30	2.4 **
17.45	1.4 *
18.00	1.0 *

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * DELAY (MIN)	* (MIN/VEH)	* INCLUSIVE QUEUEING * DELAY (MIN)	* (MIN/VEH)
B-C	28.9	19.3	6.3	0.22	6.3	0.22
B-A	441.8	294.6	136.1	0.31	136.1	0.31
C-A	166.5	111.0				
C-B	0.0	0.0	0.0	0.00	0.0	0.00
A-B	0.0	0.0				
A-C	28.9	19.3				
ALL	666.2	444.1	142.4	0.21	142.5	0.21

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	602.92	0.22	0.09	I

I	Intercept For Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	476.98	0.21	0.08	0.13	0.30	I

I	Intercept For Stream C-B	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	689.79	0.25	0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2022 PM Peak - With Development

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	NUMBER OF MINUTES FROM START WHEN TOP OF PEAK IS REACHED	I	NUMBER OF MINUTES FROM START WHEN FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	I	RATE OF FLOW (VEH/MIN) AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.28	I	0.41	I	0.28
I	ARM B	I	15.00	I	45.00	I	75.00	I	4.29	I	6.43	I	4.29
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.55	I	2.32	I	1.55

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	16.30 - 18.00	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(35.0)	I
I		I	ARM B	I	0.936	I	0.000	I	0.064	I
I		I		I	321.0	I	0.0	I	22.0	I
I		I		I	(19.0)	I	(0.0)	I	(50.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	124.0	I	0.0	I	0.0	I
I		I		I	(15.0)	I	(0.0)	I	(0.0)	I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2022 PM Peak - With Development
AND FOR TIME PERIOD 2

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	0.28	5.08	0.054		0.00	0.06	0.8		0.21
B-A	4.03	8.37	0.481		0.00	0.90	12.5		0.22
C-A	1.56								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.28								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	0.33	4.91	0.067		0.06	0.07	1.0		0.22
B-A	4.81	8.31	0.579		0.90	1.32	18.6		0.28
C-A	1.86								
C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.33								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	0.40	4.62	0.087		0.07	0.09	1.4		0.24
B-A	5.89	8.22	0.717		1.32	2.33	31.5		0.41
C-A	2.28								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.40								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	0.40	4.59	0.088		0.09	0.10	1.4		0.24
B-A	5.89	8.22	0.717		2.33	2.42	35.7		0.43
C-A	2.28								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.40								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	0.33	4.90	0.067		0.10	0.07	1.1		0.22
B-A	4.81	8.31	0.579		2.42	1.43	23.2		0.30
C-A	1.86								
C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.33								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
B-C	0.28	5.06	0.055		0.07	0.06	0.9		0.21
B-A	4.03	8.37	0.481		1.43	0.95	15.1		0.23
C-A	1.56								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.28								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1
17.45	0.1
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.9 *
17.00	1.3 *
17.15	2.3 **
17.30	2.4 **
17.45	1.4 *
18.00	1.0 *

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * DELAY (MIN)	* INCLUSIVE QUEUEING * DELAY (MIN)
B-C	30.3	20.2	6.7	6.7
B-A	441.8	294.6	136.6	136.7
C-A	170.7	113.8		
C-B	0.0	0.0	0.0	0.0
A-B	0.0	0.0		
A-C	30.3	20.2		
ALL	673.1	448.7	143.3	143.4

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	602.92		0.22		0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
I	476.98		0.21		0.08		0.13		0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	689.79		0.25		0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2022 AM Peak - Development Flows + Sensitivity Flows

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	1.05	I	1.57	I	1.05
I	ARM B	I	15.00	I	45.00	I	75.00	I	7.14	I	10.71	I	7.14
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.71	I	2.57	I	1.71

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	16.30 - 18.00	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(14.0)	I
I		I	ARM B	I	0.723	I	0.000	I	0.277	I
I		I		I	413.0	I	0.0	I	158.0	I
I		I		I	(10.0)	I	(0.0)	I	(33.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	137.0	I	0.0	I	0.0	I
I		I		I	(60.0)	I	(0.0)	I	(0.0)	I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2022 AM Peak - Development Flows + Sensitivity Flows
AND FOR TIME PERIOD 435

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	1.98	5.82	0.341		0.00	0.50	7.0		0.26
B-A	5.18	8.23	0.630		0.00	1.61	21.6		0.31
C-A	1.72								
C-B	0.00	10.17	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.05								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	2.37	5.43	0.436		0.50	0.75	10.6		0.32
B-A	6.19	8.09	0.765		1.61	2.89	38.4		0.48
C-A	2.05								
C-B	0.00	10.12	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.26								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	2.90	3.39	0.856		0.75	1.67	41.4		1.23
B-A	7.58	7.90	0.959		2.89	8.49	94.9		1.07
C-A	2.51								
C-B	0.00	10.05	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.54								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	2.90	3.05	0.951		3.67	6.16	76.0		2.18
B-A	7.58	7.90	0.959		8.49	10.88	146.8		1.52
C-A	2.51								
C-B	0.00	10.05	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.54								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	2.37	5.08	0.466		6.16	0.92	21.5		0.48
B-A	6.19	8.09	0.765		10.88	3.73	79.4		0.83
C-A	2.05								
C-B	0.00	10.12	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.26								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
B-C	1.98	5.76	0.344		0.92	0.54	8.6		0.27
B-A	5.18	8.23	0.630		3.73	1.80	29.9		0.36
C-A	1.72								
C-B	0.00	10.17	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.05								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	0.5	*
17.00	0.7	*
17.15	3.7	****
17.30	6.2	*****
17.45	0.9	*
18.00	0.5	*

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	1.6	**
17.00	2.9	***
17.15	8.5	*****
17.30	10.9	*****
17.45	3.7	****
18.00	1.8	**

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * * DELAY *	(MIN)	(MIN/VEH)	* INCLUSIVE QUEUEING * * DELAY *	(MIN)	(MIN/VEH)
B-C	217.5	145.0	165.1	0.76	165.1	0.76		
B-A	568.5	379.0	411.1	0.72	411.3	0.72		
C-A	188.6	125.7						
C-B	0.0	0.0	0.0	0.00	0.0	0.00		
A-B	0.0	0.0						
A-C	115.6	77.1						
ALL	1090.1	726.8	576.2	0.53	576.4	0.53		

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	602.92		0.22		0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
I	476.98		0.21		0.08		0.13		0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	689.79		0.25		0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2022 PM Peak - Development Flows + Sensitivity Flows

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.55	I	0.83	I	0.55
I	ARM B	I	15.00	I	45.00	I	75.00	I	4.15	I	6.23	I	4.15
I	ARM C	I	15.00	I	45.00	I	75.00	I	2.10	I	3.15	I	2.10

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C
I	16.30 - 18.00	I	ARM A	I	0.000	I	0.000	I	1.000
I		I		I	(0.0)	I	(0.0)	I	(35.0)
I		I	ARM B	I	0.967	I	0.000	I	0.033
I		I		I	321.0	I	0.0	I	11.0
I		I		I	(19.0)	I	(0.0)	I	(50.0)
I		I	ARM C	I	1.000	I	0.000	I	0.000
I		I		I	168.0	I	0.0	I	0.0
I		I		I	(15.0)	I	(0.0)	I	(0.0)

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2022 PM Peak - Development Flows + Sensitivity Flows
AND FOR TIME PERIOD 990

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	0.14	5.02	0.027		0.00	0.03	0.4		0.20
B-A	4.03	8.20	0.491		0.00	0.94	13.0		0.23
C-A	2.11								
C-B	0.00	10.28	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.55								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	0.16	4.84	0.034		0.03	0.03	0.5		0.21
B-A	4.81	8.10	0.594		0.94	1.40	19.7		0.30
C-A	2.52								
C-B	0.00	10.25	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.66								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	0.20	4.47	0.045		0.03	0.05	0.7		0.23
B-A	5.89	7.96	0.740		1.40	2.57	34.4		0.45
C-A	3.08								
C-B	0.00	10.20	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.81								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	0.20	4.44	0.045		0.05	0.05	0.7		0.24
B-A	5.89	7.96	0.740		2.57	2.69	39.7		0.48
C-A	3.08								
C-B	0.00	10.20	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.81								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	0.16	4.82	0.034		0.05	0.04	0.6		0.21
B-A	4.81	8.10	0.594		2.69	1.53	24.9		0.32
C-A	2.52								
C-B	0.00	10.25	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.66								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
B-C	0.14	5.01	0.028		0.04	0.03	0.4		0.21
B-A	4.03	8.20	0.491		1.53	0.99	15.8		0.24
C-A	2.11								
C-B	0.00	10.28	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.55								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.9 *
17.00	1.4 *
17.15	2.6 ***
17.30	2.7 ***
17.45	1.5 **
18.00	1.0 *

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * DELAY (MIN)	* INCLUSIVE QUEUEING * DELAY (MIN)
B-C	15.1	10.1	3.3	3.3
B-A	441.8	294.6	147.5	147.5
C-A	231.2	154.2		
C-B	0.0	0.0	0.0	0.0
A-B	0.0	0.0		
A-C	60.6	40.4		
ALL	748.8	499.2	150.8	150.8

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	602.92		0.22		0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
I	476.98		0.21		0.08		0.13		0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	689.79		0.25		0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2032 AM Peak - No Development

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.

LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.79	I	1.18	I	0.79
I	ARM B	I	15.00	I	45.00	I	75.00	I	7.11	I	10.67	I	7.11
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.13	I	1.69	I	1.13

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	16.30 - 18.00	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(14.0)	I
I		I	ARM B	I	0.756	I	0.000	I	0.244	I
I		I		I	430.0	I	0.0	I	139.0	I
I		I		I	(10.0)	I	(0.0)	I	(33.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	90.0	I	0.0	I	0.0	I
I		I		I	(60.0)	I	(0.0)	I	(0.0)	I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2032 AM Peak - No Development
AND FOR TIME PERIOD 990

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	1.74	5.73	0.304		0.00	0.43	6.0		0.25
B-A	5.40	8.58	0.629		0.00	1.61	21.6		0.30
C-A	1.13								
C-B	0.00	10.24	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.79								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	2.08	5.35	0.389		0.43	0.62	8.8		0.30
B-A	6.44	8.49	0.759		1.61	2.83	37.9		0.45
C-A	1.35								
C-B	0.00	10.20	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.94								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	2.55	3.48	0.732		0.62	0.24	27.5		0.89
B-A	7.89	8.35	0.945		2.83	7.91	90.2		0.97
C-A	1.65								
C-B	0.00	10.15	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.16								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	2.55	2.68	0.952		2.24	5.35	60.8		2.06
B-A	7.89	8.35	0.945		7.91	9.82	134.3		1.33
C-A	1.65								
C-B	0.00	10.15	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.16								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	2.08	5.08	0.410		5.35	0.72	16.4		0.41
B-A	6.44	8.48	0.759		9.82	3.57	71.7		0.71
C-A	1.35								
C-B	0.00	10.20	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.94								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
B-C	1.74	5.68	0.307		0.72	0.45	7.2		0.26
B-A	5.40	8.58	0.629		3.57	1.78	29.4		0.34
C-A	1.13								
C-B	0.00	10.24	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.79								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	0.4	
17.00	0.6	*
17.15	2.2	**
17.30	5.3	*****
17.45	0.7	*
18.00	0.5	

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	1.6	**
17.00	2.8	***
17.15	7.9	*****
17.30	9.8	*****
17.45	3.6	****
18.00	1.8	**

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	0.0	
17.00	0.0	
17.15	0.0	
17.30	0.0	
17.45	0.0	
18.00	0.0	

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * * DELAY * (MIN)	* INCLUSIVE QUEUEING * * DELAY * (MIN)
B-C	191.3	127.5	126.7	0.66
B-A	591.9	394.6	385.1	0.65
C-A	123.9	82.6		
C-B	0.0	0.0	0.0	0.00
A-B	0.0	0.0		
A-C	86.7	57.8		
ALL	993.8	662.5	511.8	0.52

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	602.92	0.22	0.09	I

I	Intercept For Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	476.98	0.21	0.08	0.13	0.30	I

I	Intercept For Stream C-B	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	689.79	0.25	0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2032 AM Peak - With Development

TIME PERIOD BEGINS 07.15 AND ENDS 08.45

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK	I
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.80	I	1.20	I	0.80	I
I	ARM B	I	15.00	I	45.00	I	75.00	I	7.14	I	10.71	I	7.14	I
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.20	I	1.80	I	1.20	I

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	07.15 - 08.45	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(14.0)	I
I		I	ARM B	I	0.753	I	0.000	I	0.247	I
I		I		I	430.0	I	0.0	I	141.0	I
I		I		I	(10.0)	I	(0.0)	I	(33.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	96.0	I	0.0	I	0.0	I
I		I		I	(60.0)	I	(0.0)	I	(0.0)	I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2032 AM Peak - With Development
AND FOR TIME PERIOD 1

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.15-07.30									
B-C	1.77	5.74	0.308		0.00	0.44	6.1		0.25
B-A	5.40	8.55	0.631		0.00	1.62	21.8		0.30
C-A	1.20								
C-B	0.00	10.24	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.80								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.30-07.45									
B-C	2.11	5.35	0.395		0.44	0.63	9.0		0.31
B-A	6.44	8.45	0.763		1.62	2.87	38.4		0.46
C-A	1.44								
C-B	0.00	10.20	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.96								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.45-08.00									
B-C	2.59	3.39	0.764		0.63	0.52	30.3		0.98
B-A	7.89	8.31	0.949		2.87	8.15	92.3		0.99
C-A	1.76								
C-B	0.00	10.14	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.17								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
B-C	2.59	2.71	0.953		2.52	5.50	63.6		2.11
B-A	7.89	8.31	0.950		8.15	10.21	139.1		1.38
C-A	1.76								
C-B	0.00	10.14	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.17								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
B-C	2.11	5.05	0.418		5.50	0.75	17.2		0.42
B-A	6.44	8.45	0.763		10.21	3.65	74.6		0.74
C-A	1.44								
C-B	0.00	10.20	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.96								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
B-C	1.77	5.68	0.312		0.75	0.46	7.3		0.26
B-A	5.40	8.55	0.631		3.65	1.80	29.9		0.34
C-A	1.20								
C-B	0.00	10.24	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.80								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	0.4	
07.45	0.6	*
08.00	2.5	***
08.15	5.5	*****
08.30	0.7	*
08.45	0.5	

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	1.6	**
07.45	2.9	***
08.00	8.1	*****
08.15	10.2	*****
08.30	3.6	****
08.45	1.8	**

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	0.0	
07.45	0.0	
08.00	0.0	
08.15	0.0	
08.30	0.0	
08.45	0.0	

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * * DELAY *	(MIN)	(MIN/VEH)	* INCLUSIVE QUEUEING * * DELAY *	(MIN)	(MIN/VEH)
B-C	194.1	129.4	133.6	0.69	133.6	0.69		
B-A	591.9	394.6	396.1	0.67	396.3	0.67		
C-A	132.1	88.1						
C-B	0.0	0.0	0.0	0.00	0.0	0.00		
A-B	0.0	0.0						
A-C	88.1	58.7						
ALL	1006.2	670.8	529.7	0.53	529.9	0.53		

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	602.92		0.22		0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
I	476.98		0.21		0.08		0.13		0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	689.79		0.25		0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2032 AM Peak - Development Flows + Sensitivity Flows

TIME PERIOD BEGINS 07.15 AND ENDS 08.45

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	1.00	I	1.50	I	1.00
I	ARM B	I	15.00	I	45.00	I	75.00	I	7.34	I	11.01	I	7.34
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.60	I	2.40	I	1.60

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	07.15 - 08.45	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(14.0)	I
I		I	ARM B	I	0.733	I	0.000	I	0.267	I
I		I		I	430.0	I	0.0	I	157.0	I
I		I		I	(10.0)	I	(0.0)	I	(33.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	128.0	I	0.0	I	0.0	I
I		I		I	(60.0)	I	(0.0)	I	(0.0)	I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2032 AM Peak - Development Flows + Sensitivity Flows
AND FOR TIME PERIOD 1

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.15-07.30									
B-C	1.97	5.75	0.342		0.00	0.51	7.1		0.26
B-A	5.40	8.31	0.650		0.00	1.74	23.3		0.32
C-A	1.61								
C-B	0.00	10.19	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.00								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.30-07.45									
B-C	2.35	5.30	0.444		0.51	0.77	10.9		0.34
B-A	6.44	8.18	0.788		1.74	3.23	42.5		0.52
C-A	1.92								
C-B	0.00	10.14	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.20								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.45-08.00									
B-C	2.88	2.89	0.982		0.77	1.10	60.8		1.92
B-A	7.89	8.00	0.982		3.23	10.18	110.3		1.20
C-A	2.35								
C-B	0.00	10.07	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.47								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
B-C	2.88	3.07	0.939		6.10	7.30	101.1		2.66
B-A	7.89	8.00	0.987		10.18	13.80	181.6		1.81
C-A	2.35								
C-B	0.00	10.07	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.47								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
B-C	2.35	4.69	0.501		7.30	1.07	28.1		0.62
B-A	6.44	8.18	0.788		13.80	4.43	105.7		1.07
C-A	1.92								
C-B	0.00	10.14	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.20								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
B-C	1.97	5.68	0.347		1.07	0.55	8.7		0.27
B-A	5.40	8.31	0.650		4.43	1.97	33.5		0.38
C-A	1.61								
C-B	0.00	10.19	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.00								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	0.5	*
07.45	0.8	*
08.00	6.1	*****
08.15	7.3	*****
08.30	1.1	*
08.45	0.5	*

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	1.7	**
07.45	3.2	***
08.00	10.2	*****
08.15	13.8	*****
08.30	4.4	****
08.45	2.0	**

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.0
07.45	0.0
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * DELAY (MIN)	* (MIN/VEH)	* INCLUSIVE QUEUEING * DELAY (MIN)	* (MIN/VEH)
B-C	216.1	144.1	216.7	1.00	216.7	1.00
B-A	591.9	394.6	496.8	0.84	497.0	0.84
C-A	176.2	117.5				
C-B	0.0	0.0	0.0	0.00	0.0	0.00
A-B	0.0	0.0				
A-C	110.1	73.4				
ALL	1094.3	729.5	713.4	0.65	713.7	0.65

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	602.92	0.22	0.09	I

I	Intercept For Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	476.98	0.21	0.08	0.13	0.30	I

I	Intercept For Stream C-B	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	689.79	0.25	0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2032 PM Peak - No Development

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	NUMBER OF MINUTES FROM START WHEN TOP OF PEAK IS REACHED	I	NUMBER OF MINUTES FROM START WHEN FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	I	RATE OF FLOW (VEH/MIN) AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.28	I	0.41	I	0.28
I	ARM B	I	15.00	I	45.00	I	75.00	I	4.45	I	6.67	I	4.45
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.58	I	2.36	I	1.58

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	16.30 - 18.00	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(35.0)	I
I		I	ARM B	I	0.938	I	0.000	I	0.062	I
I		I		I	334.0	I	0.0	I	22.0	I
I		I		I	(19.0)	I	(0.0)	I	(50.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	126.0	I	0.0	I	0.0	I
I		I		I	(15.0)	I	(0.0)	I	(0.0)	I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2032 PM Peak - No Development
AND FOR TIME PERIOD 2

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	0.28	5.05	0.055		0.00	0.06	0.8		0.21
B-A	4.19	8.37	0.501		0.00	0.97	13.5		0.23
C-A	1.58								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.28								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	0.33	4.88	0.068		0.06	0.07	1.0		0.22
B-A	5.00	8.30	0.603		0.97	1.45	20.4		0.30
C-A	1.89								
C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.33								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	0.40	4.51	0.089		0.07	1.10	1.4		0.24
B-A	6.13	8.21	0.747		1.45	2.66	35.5		0.45
C-A	2.31								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.40								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	0.40	4.48	0.090		0.10	0.10	1.5		0.25
B-A	6.13	8.21	0.747		2.66	2.78	41.0		0.47
C-A	2.31								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.40								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	0.33	4.85	0.068		0.10	0.07	1.1		0.22
B-A	5.00	8.30	0.603		2.78	1.59	25.8		0.32
C-A	1.89								
C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.33								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
B-C	0.28	5.03	0.055		0.07	0.06	0.9		0.21
B-A	4.19	8.37	0.501		1.59	1.03	16.4		0.24
C-A	1.58								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.28								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1
17.45	0.1
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	1.0 *
17.00	1.5 *
17.15	2.7 ***
17.30	2.8 ***
17.45	1.6 **
18.00	1.0 *

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * * DELAY * (MIN)	* INCLUSIVE QUEUEING * * DELAY * (MIN)
B-C	30.3	20.2	6.8	6.8
B-A	459.7	306.5	152.6	152.7
C-A	173.4	115.6		
C-B	0.0	0.0	0.0	0.0
A-B	0.0	0.0		
A-C	30.3	20.2		
ALL	693.7	462.5	159.4	159.5

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	602.92	0.22	0.09	I

I	Intercept For Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	476.98	0.21	0.08	0.13	0.30	I

I	Intercept For Stream C-B	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	689.79	0.25	0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2032 PM Peak - With Development

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	NUMBER OF MINUTES FROM START WHEN TOP OF PEAK IS REACHED	I	NUMBER OF MINUTES FROM START WHEN FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	I	RATE OF FLOW (VEH/MIN) AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.29	I	0.43	I	0.29
I	ARM B	I	15.00	I	45.00	I	75.00	I	4.46	I	6.69	I	4.46
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.61	I	2.42	I	1.61

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	16.30 - 18.00	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(35.0)	I
I		I	ARM B	I	0.936	I	0.000	I	0.064	I
I		I		I	334.0	I	0.0	I	23.0	I
I		I		I	(19.0)	I	(0.0)	I	(50.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	129.0	I	0.0	I	0.0	I
I		I		I	(15.0)	I	(0.0)	I	(0.0)	I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2032 PM Peak - With Development
AND FOR TIME PERIOD 2

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	0.29	5.05	0.057		0.00	0.06	0.9		0.21
B-A	4.19	8.36	0.501		0.00	0.97	13.5		0.23
C-A	1.62								
C-B	0.00	10.36	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.29								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	0.34	4.87	0.071		0.06	0.08	1.1		0.22
B-A	5.00	8.29	0.604		0.97	1.46	20.4		0.30
C-A	1.93								
C-B	0.00	10.34	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.34								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	0.42	4.51	0.094		0.08	0.10	1.5		0.24
B-A	6.13	8.20	0.748		1.47	2.67	35.7		0.45
C-A	2.37								
C-B	0.00	10.32	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.42								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	0.42	4.47	0.094		0.10	0.10	1.5		0.25
B-A	6.13	8.20	0.748		2.67	2.80	41.2		0.48
C-A	2.37								
C-B	0.00	10.32	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.42								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	0.34	4.85	0.071		0.10	0.08	1.2		0.22
B-A	5.00	8.29	0.604		2.80	1.59	26.0		0.32
C-A	1.93								
C-B	0.00	10.34	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.34								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
B-C	0.29	5.03	0.057		0.08	0.06	1.0		0.21
B-A	4.19	8.36	0.501		1.59	1.04	16.5		0.24
C-A	1.62								
C-B	0.00	10.36	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.29								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1
17.45	0.1
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	1.0	*
17.00	1.5	*
17.15	2.7	***
17.30	2.8	***
17.45	1.6	**
18.00	1.0	*

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * DELAY (MIN)	* (MIN/VEH)	* INCLUSIVE QUEUEING * DELAY (MIN)	* (MIN/VEH)
B-C	31.7	21.1	7.1	0.22	7.1	0.22
B-A	459.7	306.5	153.3	0.33	153.4	0.33
C-A	177.6	118.4				
C-B	0.0	0.0	0.0	0.00	0.0	0.00
A-B	0.0	0.0				
A-C	31.7	21.1				
ALL	700.6	467.1	160.4	0.23	160.5	0.23

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	602.92		0.22		0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
I	476.98		0.21		0.08		0.13		0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	689.79		0.25		0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2032 PM Peak - Development Flows + Sensitivity Flows

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.49	I	0.73	I	0.49
I	ARM B	I	15.00	I	45.00	I	75.00	I	4.66	I	6.99	I	4.66
I	ARM C	I	15.00	I	45.00	I	75.00	I	2.01	I	3.02	I	2.01

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	16.30 - 18.00	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(35.0)	I
I		I	ARM B	I	0.895	I	0.000	I	0.105	I
I		I		I	334.0	I	0.0	I	39.0	I
I		I		I	(19.0)	I	(0.0)	I	(50.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	161.0	I	0.0	I	0.0	I
I		I		I	(15.0)	I	(0.0)	I	(0.0)	I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

Appendix C – PICADY Results

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2032 PM Peak - Development Flows + Sensitivity Flows
AND FOR TIME PERIOD *****

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	0.49	5.01	0.098		0.00	0.11	1.5		0.22
B-A	4.19	8.23	0.509		0.00	1.00	13.9		0.24
C-A	2.02								
C-B	0.00	10.30	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.49								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	0.58	4.82	0.121		0.11	0.14	2.0		0.24
B-A	5.00	8.14	0.615		1.00	1.52	21.3		0.31
C-A	2.41								
C-B	0.00	10.27	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.58								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	0.72	4.41	0.162		0.14	0.19	2.7		0.27
B-A	6.13	8.01	0.765		1.51	2.89	38.2		0.48
C-A	2.95								
C-B	0.00	10.23	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.72								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	0.72	4.36	0.164		0.19	0.19	2.9		0.27
B-A	6.13	8.01	0.765		2.89	3.05	44.8		0.52
C-A	2.95								
C-B	0.00	10.23	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.72								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	0.58	4.79	0.122		0.19	0.14	2.2		0.24
B-A	5.00	8.14	0.615		3.05	1.68	27.4		0.34
C-A	2.41								
C-B	0.00	10.27	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.58								

Appendix C – PICADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
B-C	0.49	4.99	0.098		0.14	0.11	1.7		0.22
B-A	4.19	8.23	0.509		1.68	1.07	17.1		0.25
C-A	2.02								
C-B	0.00	10.30	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.49								

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	1.0	*
17.00	1.5	**
17.15	2.9	***
17.30	3.1	***
17.45	1.7	**
18.00	1.1	*

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * DELAY (MIN)	* (MIN/VEH)	* INCLUSIVE QUEUEING * DELAY (MIN)	* (MIN/VEH)
B-C	53.7	35.8	13.0	0.24	13.0	0.24
B-A	459.7	306.5	162.7	0.35	162.8	0.35
C-A	221.6	147.7				
C-B	0.0	0.0	0.0	0.00	0.0	0.00
A-B	0.0	0.0				
A-C	53.7	35.8				
ALL	788.7	525.8	175.7	0.22	175.8	0.22

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

TRAFFIC AND TRANSPORTATION 13

APPENDIX 13-D ARCADY RESULTS

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ARCADY 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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TRL Limited Tel: +44 (0) 1344 770018
Crowthorne House Fax: +44 (0) 1344 770864
Nine Mile Ride Email: softwarebureau@trl.co.uk
Wokingham, Berks. Web: www.trlsoftware.co.uk
RG40 3GA, UK

THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\AM Peak\2016 AM Peak - Existing Flows.vai"
(drive-on-the-left) at 15:08:17 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
For inspection purposes only.

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT	PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623		I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767		I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086		I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW	SCALE (%)	I
I	A	I	100		I
I	B	I	100		I
I	C	I	100		I

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2016 AM Peak - Existing Flows

I	I	NUMBER OF MINUTES FROM START WHEN			RATE OF FLOW (VEH/MIN)		
		I	I	I	I	I	I
I	ARM	FLOW STARTS	TOP OF PEAK	FLOW STOPS	BEFORE	AT TOP	AFTER
I	I	TO RISE	IS REACHED	IF FALLING	PEAK	OF PEAK	PEAK
I	ARM A	I 15.00	I 45.00	I 75.00	I 3.86	I 5.79	I 3.86
I	ARM B	I 15.00	I 45.00	I 75.00	I 1.70	I 2.55	I 1.70
I	ARM C	I 15.00	I 45.00	I 75.00	I 3.11	I 4.67	I 3.11

DEMAND SET TITLE: 2016 AM Peak - Existing Flows

I	I	TURNING PROPORTIONS			
		I	I	I	
I		TURNING COUNTS (VEH/HR)			
I		(PERCENTAGE OF H.V.S)			
I	TIME	FROM/TO	ARM A	ARM B	ARM C
I	07.15 - 08.45	I	I	I	I
I		I ARM A	I 0.000	I 0.550	I 0.450
I		I	I 0.0	I 170.0	I 139.0
I		I	I (0.0)	I (7.0)	I (16.0)
I		I	I	I	I
I		I ARM B	I 0.353	I 0.000	I 0.647
I		I	I 48.0	I 0.0	I 88.0
I		I	I (15.0)	I (0.0)	I (24.0)
I		I	I	I	I
I		I ARM C	I 0.253	I 0.747	I 0.000
I		I	I 63.0	I 186.0	I 0
I		I	I (2.0)	I (10.0)	I (0.0)
I		I	I	I	I

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	07.15-07.30									
I	ARM A	3.86	26.28	0.147		0.0	0.2	2.5		0.04
I	ARM B	1.70	26.95	0.063		0.0	0.1	1.0		0.04
I	ARM C	3.11	25.66	0.121		0.0	0.1	2.0		0.04

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	07.30-07.45									
I	ARM A	4.61	26.03	0.177		0.2	0.2	3.2		0.05
I	ARM B	2.03	26.76	0.076		0.1	0.1	1.2		0.04
I	ARM C	3.72	25.59	0.145		0.1	0.2	2.5		0.05

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	07.45-08.00									
I	ARM A	5.65	25.68	0.220		0.2	0.3	4.2		0.05
I	ARM B	2.49	26.49	0.094		0.1	0.1	1.5		0.04
I	ARM C	4.55	25.50	0.179		0.2	0.2	3.2		0.05

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
ARM A	5.65	25.68	0.220		0.3	0.3	4.2		0.05
ARM B	2.49	26.49	0.094		0.1	0.1	1.6		0.04
ARM C	4.55	25.50	0.179		0.2	0.2	3.3		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
ARM A	4.61	26.03	0.177		0.3	0.2	3.3		0.05
ARM B	2.03	26.76	0.076		0.1	0.1	1.2		0.04
ARM C	3.72	25.59	0.145		0.2	0.2	2.6		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
ARM A	3.86	26.28	0.147		0.2	0.2	2.6		0.04
ARM B	1.70	26.95	0.063		0.1	0.1	1.0		0.04
ARM C	3.11	25.66	0.121		0.2	0.1	2.1		0.04

.QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.3
08.15	0.3
08.30	0.2
08.45	0.2

.QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

.QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.1

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 QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
(VEH)	(VEH/H)	(MIN)	(MIN)
		(MIN/VEH)	(MIN/VEH)
A	423.7	282.5	20.0
B	186.5	124.3	7.6
C	341.4	227.6	15.7
ALL	951.6	634.4	43.3

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\AM Peak\2017 AM Peak - No Development.vai"
(drive-on-the-left) at 15:08:54 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
For inspection purposes only.

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2017 AM Peak - No Development

I I I	I I I	I NUMBER OF MINUTES FROM START WHEN			I RATE OF FLOW (VEH/MIN) I		
		I FLOW STARTS	I TOP OF PEAK	I FLOW STOPS	I BEFORE	I AT TOP	I AFTER
		I TO RISE	I IS REACHED	I IF FALLING	I PEAK	I OF PEAK	I PEAK
I ARM A	I	15.00	I 45.00	I 75.00	I 3.89	I 5.83	I 3.89
I ARM B	I	15.00	I 45.00	I 75.00	I 1.71	I 2.57	I 1.71
I ARM C	I	15.00	I 45.00	I 75.00	I 3.13	I 4.69	I 3.13

DEMAND SET TITLE: 2017 AM Peak - No Development

I I I I I	I I I I I	I TURNING PROPORTIONS			I
		I TURNING COUNTS (VEH/HR)			
		I (PERCENTAGE OF H.V.S)			I
I TIME		I FROM/TO	I ARM A	I ARM B	I ARM C
I	07.15 - 08.45	I	I	I	I
I		I ARM A	I 0.000	I 0.550	I 0.450
I		I	I 0.0	I 171.0	I 140.0
I		I	I (0.0)	I (7.0)	I (16.0)
I		I	I	I	I
I		I ARM B	I 0.350	I 0.000	I 0.650
I		I	I 48.0	I 0.0	I 89.0
I		I	I (15.0)	I (0.0)	I (24.0)
I		I	I	I	I
I		I ARM C	I 0.252	I 0.748	I 0.000
I		I	I 63.0	I 187.0	I 0.0
I		I	I (2.0)	I (10.0)	I (0.0)
I		I	I	I	I

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QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I TIME	I DEMAND (VEH/MIN)	I CAPACITY (VEH/MIN)	I DEMAND/CAPACITY (RFC)	I PEDESTRIAN FLOW (PEDS/MIN)	I START QUEUE (VEHS)	I END QUEUE (VEHS)	I DELAY (VEH.MIN/ TIME SEGMENT)	I GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	I AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I 07.15-07.30									
I ARM A	3.89	26.27	0.148		0.0	0.2	2.6		0.04
I ARM B	1.71	26.94	0.064		0.0	0.1	1.0		0.04
I ARM C	3.13	25.66	0.122		0.0	0.1	2.0		0.04

I TIME	I DEMAND (VEH/MIN)	I CAPACITY (VEH/MIN)	I DEMAND/CAPACITY (RFC)	I PEDESTRIAN FLOW (PEDS/MIN)	I START QUEUE (VEHS)	I END QUEUE (VEHS)	I DELAY (VEH.MIN/ TIME SEGMENT)	I GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	I AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I 07.30-07.45									
I ARM A	4.64	26.02	0.178		0.2	0.2	3.2		0.05
I ARM B	2.04	26.74	0.076		0.1	0.1	1.2		0.04
I ARM C	3.73	25.59	0.146		0.1	0.2	2.5		0.05

I TIME	I DEMAND (VEH/MIN)	I CAPACITY (VEH/MIN)	I DEMAND/CAPACITY (RFC)	I PEDESTRIAN FLOW (PEDS/MIN)	I START QUEUE (VEHS)	I END QUEUE (VEHS)	I DELAY (VEH.MIN/ TIME SEGMENT)	I GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	I AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I 07.45-08.00									
I ARM A	5.69	25.67	0.221		0.2	0.3	4.2		0.05
I ARM B	2.50	26.47	0.095		0.1	0.1	1.5		0.04
I ARM C	4.57	25.49	0.179		0.2	0.2	3.2		0.05

Appendix D – ARCADY Results

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	ARM A	5.69	25.67	0.222		0.3	0.3	4.3		0.05	I
I	ARM B	2.50	26.47	0.095		0.1	0.1	1.6		0.04	I
I	ARM C	4.57	25.49	0.179		0.2	0.2	3.3		0.05	I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	ARM A	4.64	26.02	0.178		0.3	0.2	3.3		0.05	I
I	ARM B	2.04	26.74	0.076		0.1	0.1	1.3		0.04	I
I	ARM C	3.73	25.59	0.146		0.2	0.2	2.6		0.05	I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	ARM A	3.89	26.27	0.148		0.2	0.2	2.6		0.04	I
I	ARM B	1.71	26.94	0.064		0.1	0.1	1.0		0.04	I
I	ARM C	3.13	25.66	0.122		0.2	0.1	2.1		0.04	I

.QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.3
08.15	0.3
08.30	0.2
08.45	0.2

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.QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

.QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
(VEH)	(VEH/H)	(MIN)	(MIN)
		(MIN/VEH)	(MIN/VEH)
A	426.4	284.3	20.2
B	187.9	125.2	7.6
C	342.8	228.5	15.8
ALL	957.1	638.1	43.6

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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_____ A R C A D Y 6 _____

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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RG40 3GA, UK

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\AM Peak\2017 AM Peak - With Development.vai"
(drive-on-the-left) at 15:09:30 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

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.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2017 AM Peak - With Development

I I I	I I I	I NUMBER OF MINUTES FROM START WHEN			I RATE OF FLOW (VEH/MIN) I									
		I I I	I I I	I I I	I I I	I I I	I I I							
I	I	I	I	I	I	I	I							
I	I	I	I	I	I	I	I							
I	ARM A	I	15.00	I	45.00	I	75.00	I	3.90	I	5.85	I	3.90	I
I	ARM B	I	15.00	I	45.00	I	75.00	I	2.91	I	4.37	I	2.91	I
I	ARM C	I	15.00	I	45.00	I	75.00	I	3.13	I	4.69	I	3.13	I

DEMAND SET TITLE: 2017 AM Peak - With Development

I I I I I	I I I I I	I TURNING PROPORTIONS			I I I I I
		I TURNING COUNTS (VEH/HR)			
		I (PERCENTAGE OF H.V.S)			
I		I			
I	I	I	I	I	I
I	I	I	I	I	I
I	07.15 - 08.45	I	I	I	I
I	ARM A	I	0.000	I	0.548
I		I	0.0	I	171.0
I		I	(0.0)	I	(7.0)
I		I		I	
I	ARM B	I	0.206	I	0.386
I		I	48.0	I	90.0
I		I	(15.0)	I	(0.0)
I		I		I	
I	ARM C	I	0.252	I	0.748
I		I	63.0	I	187.0
I		I	(2.0)	I	(10.0)
I		I		I	

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I	TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY
I		(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING
I				(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)
I	07.15-07.30									
I	ARM A	3.90	25.70	0.152		0.0	0.2	2.6		0.05
I	ARM B	2.91	28.84	0.101		0.0	0.1	1.7		0.04
I	ARM C	3.13	25.08	0.125		0.0	0.1	2.1		0.05

I	TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY
I		(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING
I				(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)
I	07.30-07.45									
I	ARM A	4.66	25.33	0.184		0.2	0.2	3.3		0.05
I	ARM B	3.48	28.62	0.122		0.1	0.1	2.0		0.04
I	ARM C	3.73	24.90	0.150		0.1	0.2	2.6		0.05

I	TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY
I		(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING
I				(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)
I	07.45-08.00									
I	ARM A	5.70	24.83	0.230		0.2	0.3	4.4		0.05
I	ARM B	4.26	28.33	0.150		0.1	0.2	2.6		0.04
I	ARM C	4.57	24.65	0.185		0.2	0.2	3.4		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
ARM A	5.70	24.83	0.230		0.3	0.3	4.5		0.05
ARM B	4.26	28.33	0.150		0.2	0.2	2.6		0.04
ARM C	4.57	24.65	0.185		0.2	0.2	3.4		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
ARM A	4.66	25.33	0.184		0.3	0.2	3.4		0.05
ARM B	3.48	28.62	0.122		0.2	0.1	2.1		0.04
ARM C	3.73	24.90	0.150		0.2	0.2	2.7		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
ARM A	3.90	25.70	0.152		0.2	0.2	2.7		0.05
ARM B	2.91	28.83	0.101		0.1	0.1	1.7		0.04
ARM C	3.13	25.08	0.125		0.2	0.1	2.2		0.05

.QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.3
08.15	0.3
08.30	0.2
08.45	0.2

.QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.2
08.15	0.2
08.30	0.1
08.45	0.1

.QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.1

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 QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
		(VEH) (VEH/H)	(MIN) (MIN/VEH)	(MIN) (MIN/VEH)
I	A	427.8 I 285.2 I	21.0 I 0.05 I	21.0 I 0.05 I
I	B	319.5 I 213.0 I	12.8 I 0.04 I	12.8 I 0.04 I
I	C	342.8 I 228.5 I	16.3 I 0.05 I	16.3 I 0.05 I
I	ALL	1090.1 I 726.7 I	50.1 I 0.05 I	50.1 I 0.05 I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\AM Peak\2022 AM Peak - No Development.vai"
(drive-on-the-left) at 15:10:19 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
For inspection purposes only.

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2022 AM Peak - No Development

I I I	I I I	NUMBER OF MINUTES FROM START WHEN			RATE OF FLOW (VEH/MIN)		
		I I I	I I I	I I I	I I I	I I I	I I I
ARM	FLOW STARTS TO RISE	TOP OF PEAK IS REACHED	FLOW STOPS IF FALLING	BEFORE PEAK	AT TOP OF PEAK	AFTER PEAK	
ARM A	15.00	45.00	75.00	4.03	6.04	4.03	
ARM B	15.00	45.00	75.00	1.77	2.66	1.77	
ARM C	15.00	45.00	75.00	3.25	4.88	3.25	

DEMAND SET TITLE: 2022 AM Peak - No Development

I I I I I	I I I I I	TURNING PROPORTIONS			I I I I I
		TURNING COUNTS (VEH/HR)			
			(PERCENTAGE OF H.V.S)		
TIME	FROM/TO	ARM A	ARM B	ARM C	
07.15 - 08.45	ARM A	0.000	0.550	0.450	
		0.0	177.0	145.0	
		(0.0)	(7.0)	(16.0)	
	ARM B	0.352	0.000	0.648	
		50.0	0.0	92.0	
		(15.0)	(0.0)	(24.0)	
	ARM C	0.254	0.746	0.000	
		66.0	194.0	0.0	
		(2.0)	(10.0)	(0.0)	

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QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I I I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
	07.15-07.30									
	ARM A	4.03	26.23	0.153		0.0	0.2	2.7		0.04
	ARM B	1.77	26.91	0.066		0.0	0.1	1.0		0.04
	ARM C	3.25	25.65	0.127		0.0	0.1	2.1		0.04

I I I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
	07.30-07.45									
	ARM A	4.81	25.96	0.185		0.2	0.2	3.4		0.05
	ARM B	2.12	26.70	0.079		0.1	0.1	1.3		0.04
	ARM C	3.88	25.57	0.152		0.1	0.2	2.6		0.05

I I I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
	07.45-08.00									
	ARM A	5.89	25.60	0.230		0.2	0.3	4.4		0.05
	ARM B	2.60	26.42	0.098		0.1	0.1	1.6		0.04
	ARM C	4.75	25.48	0.187		0.2	0.2	3.4		0.05

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
ARM A	5.89	25.60	0.230		0.3	0.3	4.5		0.05
ARM B	2.60	26.42	0.098		0.1	0.1	1.6		0.04
ARM C	4.75	25.48	0.187		0.2	0.2	3.4		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
ARM A	4.81	25.96	0.185		0.3	0.2	3.5		0.05
ARM B	2.12	26.70	0.079		0.1	0.1	1.3		0.04
ARM C	3.88	25.57	0.152		0.2	0.2	2.7		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
ARM A	4.03	26.22	0.154		0.2	0.2	2.8		0.05
ARM B	1.77	26.90	0.066		0.1	0.1	1.1		0.04
ARM C	3.25	25.65	0.127		0.2	0.1	2.2		0.04

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.3
08.15	0.3
08.30	0.2
08.45	0.2

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.1

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 QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
		(VEH) (VEH/H)	(MIN) (MIN/VEH)	(MIN) (MIN/VEH)
A	441.5	294.4	21.1 0.05	21.1 0.05
B	194.7	129.8	7.9 0.04	7.9 0.04
C	356.5	237.7	16.5 0.05	16.5 0.05
ALL	992.8	661.8	45.6 0.05	45.6 0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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ARCADY 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\AM Peak\2022 AM Peak - With Development.vai"
(drive-on-the-left) at 15:11:06 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
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.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2022 AM Peak - With Development

I I I	I I I	NUMBER OF MINUTES FROM START WHEN			RATE OF FLOW (VEH/MIN)		
		I I I	I I I	I I I	I I I	I I I	I I I
ARM	FLOW STARTS TO RISE	TOP OF PEAK IS REACHED	FLOW STOPS IF FALLING	BEFORE PEAK	AT TOP OF PEAK	AFTER PEAK	
ARM A	15.00	45.00	75.00	4.03	6.04	4.03	
ARM B	15.00	45.00	75.00	1.77	2.66	1.77	
ARM C	15.00	45.00	75.00	3.25	4.88	3.25	

DEMAND SET TITLE: 2022 AM Peak - With Development

I I I I I	I I I I I	TURNING PROPORTIONS			I I I I I
		TURNING COUNTS (VEH/HR)			
			(PERCENTAGE OF H.V.S)		
TIME	FROM/TO	ARM A	ARM B	ARM C	
07.15 - 08.45	ARM A	0.000	0.550	0.450	
		0.0	177.0	145.0	
		(0.0)	(7.0)	(16.0)	
	ARM B	0.352	0.000	0.648	
		50.0	0.0	92.0	
		(15.0)	(0.0)	(24.0)	
	ARM C	0.254	0.746	0.000	
		66.0	194.0	0.0	
		(2.0)	(10.0)	(0.0)	

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QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I I I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
	07.15-07.30									
	ARM A	4.03	26.23	0.153		0.0	0.2	2.7		0.04
	ARM B	1.77	26.91	0.066		0.0	0.1	1.0		0.04
	ARM C	3.25	25.65	0.127		0.0	0.1	2.1		0.04

I I I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
	07.30-07.45									
	ARM A	4.81	25.96	0.185		0.2	0.2	3.4		0.05
	ARM B	2.12	26.70	0.079		0.1	0.1	1.3		0.04
	ARM C	3.88	25.57	0.152		0.1	0.2	2.6		0.05

I I I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
	07.45-08.00									
	ARM A	5.89	25.60	0.230		0.2	0.3	4.4		0.05
	ARM B	2.60	26.42	0.098		0.1	0.1	1.6		0.04
	ARM C	4.75	25.48	0.187		0.2	0.2	3.4		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
ARM A	5.89	25.60	0.230		0.3	0.3	4.5		0.05
ARM B	2.60	26.42	0.098		0.1	0.1	1.6		0.04
ARM C	4.75	25.48	0.187		0.2	0.2	3.4		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
ARM A	4.81	25.96	0.185		0.3	0.2	3.5		0.05
ARM B	2.12	26.70	0.079		0.1	0.1	1.3		0.04
ARM C	3.88	25.57	0.152		0.2	0.2	2.7		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
ARM A	4.03	26.22	0.154		0.2	0.2	2.8		0.05
ARM B	1.77	26.90	0.066		0.1	0.1	1.1		0.04
ARM C	3.25	25.65	0.127		0.2	0.1	2.2		0.04

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.3
08.15	0.3
08.30	0.2
08.45	0.2

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.1

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
		(VEH) (VEH/H)	(MIN) (MIN/VEH)	(MIN) (MIN/VEH)
A	441.5	294.4	21.1 0.05	21.1 0.05
B	194.7	129.8	7.9 0.04	7.9 0.04
C	356.5	237.7	16.5 0.05	16.5 0.05
ALL	992.8	661.8	45.6 0.05	45.6 0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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_____ A R C A D Y 6 _____

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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Run with file:-

"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\AM Peak\2022 AM Peak - With Development + Sensitivity Flows.vai"
(drive-on-the-left) at 15:12:09 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

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.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2022 AM Peak - With Development + Sensitivity Flows

I I I	I I I	I NUMBER OF MINUTES FROM START WHEN I RATE OF FLOW (VEH/MIN) I		
		I I I	I I I	I I I
I I I	I I I	I I I	I I I	I I I
I	I	I	I	I
I	I	I	I	I
I	ARM A	I 15.00	I 45.00	I 75.00 I 4.31 I 6.47 I 4.31 I
I	ARM B	I 15.00	I 45.00	I 75.00 I 2.06 I 3.09 I 2.06 I
I	ARM C	I 15.00	I 45.00	I 75.00 I 3.25 I 4.88 I 3.25 I

DEMAND SET TITLE: 2022 AM Peak - With Development + Sensitivity Flows

I I I I I	I I I I I	I TURNING PROPORTIONS I		
		I I I	I I I	I I I
I	I	I	I	I
I	I	I	I	I
I	I	I	I	I
I	07.15 - 08.45	I	I	I
I	ARM A	I 0.000 I 0.513 I 0.487 I	I	I
I		I 0.0 I 177.0 I 168.0 I	I	I
I		I (0.0) I (7.0) I (16.0) I	I	I
I	ARM B	I 0.303 I 0.000 I 0.697 I	I	I
I		I 50.0 I 0.0 I 115.0 I	I	I
I		I (15.0) I (0.0) I (24.0) I	I	I
I	ARM C	I 0.254 I 0.746 I 0.000 I	I	I
I		I 66.0 I 194.0 I 0.0 I	I	I
I		I (2.0) I (10.0) I (0.0) I	I	I

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QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
I	I	I	I	I	I	I	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I
I	07.15-07.30	I	I	I	I	I	I	I	I	I
I	ARM A	I 4.31	I 26.15	I 0.165	I	I 0.0	I 0.2	I 2.9	I	I 0.05
I	ARM B	I 2.06	I 26.65	I 0.077	I	I 0.0	I 0.1	I 1.2	I	I 0.04
I	ARM C	I 3.25	I 25.65	I 0.127	I	I 0.0	I 0.1	I 2.1	I	I 0.04

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
I	I	I	I	I	I	I	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I
I	07.30-07.45	I	I	I	I	I	I	I	I	I
I	ARM A	I 5.15	I 25.88	I 0.199	I	I 0.2	I 0.2	I 3.7	I	I 0.05
I	ARM B	I 2.46	I 26.41	I 0.093	I	I 0.1	I 0.1	I 1.5	I	I 0.04
I	ARM C	I 3.88	I 25.57	I 0.152	I	I 0.1	I 0.2	I 2.6	I	I 0.05

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
I	I	I	I	I	I	I	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I
I	07.45-08.00	I	I	I	I	I	I	I	I	I
I	ARM A	I 6.31	I 25.53	I 0.247	I	I 0.2	I 0.3	I 4.8	I	I 0.05
I	ARM B	I 3.02	I 26.09	I 0.116	I	I 0.1	I 0.1	I 1.9	I	I 0.04
I	ARM C	I 4.75	I 25.48	I 0.187	I	I 0.2	I 0.2	I 3.4	I	I 0.05

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
ARM A	6.31	25.53	0.247		0.3	0.3	4.9		0.05
ARM B	3.02	26.09	0.116		0.1	0.1	2.0		0.04
ARM C	4.75	25.48	0.187		0.2	0.2	3.4		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
ARM A	5.15	25.88	0.199		0.3	0.2	3.8		0.05
ARM B	2.46	26.41	0.093		0.1	0.1	1.6		0.04
ARM C	3.88	25.57	0.152		0.2	0.2	2.7		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
ARM A	4.31	26.14	0.165		0.2	0.2	3.0		0.05
ARM B	2.06	26.64	0.077		0.1	0.1	1.3		0.04
ARM C	3.25	25.65	0.127		0.2	0.1	2.2		0.04

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.3
08.15	0.3
08.30	0.2
08.45	0.2

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.1

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 QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
		(VEH) (VEH/H)	(MIN) (MIN/VEH)	(MIN) (MIN/VEH)
A	473.1	315.4	23.1 0.05	23.1 0.05
B	226.2	150.8	9.5 0.04	9.5 0.04
C	356.5	237.7	16.5 0.05	16.5 0.05
ALL	1055.8	703.9	49.1 0.05	49.1 0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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Appendix D – ARCADY Results

A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\AM Peak\2032 AM Peak - No Development.vai"
(drive-on-the-left) at 15:14:59 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

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.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Appendix D – ARCADY Results

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2032 AM Peak - No Development

I I I	I I I	I NUMBER OF MINUTES FROM START WHEN			I RATE OF FLOW (VEH/MIN) I		
		I FLOW STARTS	I TOP OF PEAK	I FLOW STOPS	I BEFORE	I AT TOP	I AFTER
I	I	I TO RISE	I IS REACHED	I IF FALLING	I PEAK	I OF PEAK	I PEAK
I ARM A	I	I 15.00	I 45.00	I 75.00	I 4.20	I 6.30	I 4.20
I ARM B	I	I 15.00	I 45.00	I 75.00	I 1.85	I 2.78	I 1.85
I ARM C	I	I 15.00	I 45.00	I 75.00	I 3.38	I 5.06	I 3.38

DEMAND SET TITLE: 2032 AM Peak - No Development

I I I I I	I I I I I	I TURNING PROPORTIONS			I TURNING COUNTS (VEH/HR)			I (PERCENTAGE OF H.V.S)					
		I FROM/TO	I ARM A	I ARM B	I ARM C	I FROM/TO	I ARM A	I ARM B	I ARM C	I FROM/TO	I ARM A	I ARM B	I ARM C
I	I	I 07.15 - 08.45	I	I	I	I	I	I	I	I	I	I	I
I	I	I	I ARM A	I	I 0.000	I	I 0.551	I	I 0.449	I	I	I	I
I	I	I	I	I	I 0.0	I	I 185.0	I	I 151.0	I	I	I	I
I	I	I	I	I	I (0.0)	I	I (7.0)	I	I (16.0)	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I	I	I	I
I	I	I	I ARM B	I	I 0.351	I	I 0.000	I	I 0.649	I	I	I	I
I	I	I	I	I	I 52.0	I	I 0.0	I	I 96.0	I	I	I	I
I	I	I	I	I	I (15.0)	I	I (0.0)	I	I (24.0)	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I	I	I	I
I	I	I	I ARM C	I	I 0.252	I	I 0.748	I	I 0.000	I	I	I	I
I	I	I	I	I	I 68.0	I	I 202.0	I	I 151.0	I	I	I	I
I	I	I	I	I	I (2.0)	I	I (10.0)	I	I (0.0)	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I	I	I	I

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	
	(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	
			(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	
I 07.15-07.30										
I ARM A	4.20	26.17	0.160		0.0	0.2	2.8		0.05	
I ARM B	1.85	26.86	0.069		0.0	0.1	1.1		0.04	
I ARM C	3.38	25.63	0.132		0.0	0.2	2.2		0.04	

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	
	(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	
			(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	
I 07.30-07.45										
I ARM A	5.02	25.90	0.194		0.2	0.2	3.5		0.05	
I ARM B	2.21	26.65	0.083		0.1	0.1	1.3		0.04	
I ARM C	4.03	25.55	0.158		0.2	0.2	2.8		0.05	

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	
	(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	
			(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	
I 07.45-08.00										
I ARM A	6.14	25.52	0.241		0.2	0.3	4.7		0.05	
I ARM B	2.71	26.37	0.103		0.1	0.1	1.7		0.04	
I ARM C	4.94	25.45	0.194		0.2	0.2	3.6		0.05	

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
ARM A	6.14	25.52	0.241		0.3	0.3	4.7		0.05
ARM B	2.71	26.36	0.103		0.1	0.1	1.7		0.04
ARM C	4.94	25.45	0.194		0.2	0.2	3.6		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
ARM A	5.02	25.89	0.194		0.3	0.2	3.7		0.05
ARM B	2.21	26.65	0.083		0.1	0.1	1.4		0.04
ARM C	4.03	25.55	0.158		0.2	0.2	2.9		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
ARM A	4.20	26.17	0.161		0.2	0.2	2.9		0.05
ARM B	1.85	26.86	0.069		0.1	0.1	1.1		0.04
ARM C	3.38	25.63	0.132		0.2	0.2	2.3		0.04

.QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.3
08.15	0.3
08.30	0.2
08.45	0.2

.QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

.QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.2

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Appendix D – ARCADY Results

 QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
		(VEH) (VEH/H)	(MIN) (MIN/VEH)	(MIN) (MIN/VEH)
A	460.7	307.2	22.3 0.05	22.3 0.05
B	202.9	135.3	8.3 0.04	8.3 0.04
C	370.2	246.8	17.3 0.05	17.3 0.05
ALL	1033.9	689.3	48.0 0.05	48.0 0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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Nine Mile Ride Email: softwarebureau@trl.co.uk
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RG40 3GA, UK

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\AM Peak\2032 AM Peak - With Development.vai"
(drive-on-the-left) at 15:14:38 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

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.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2032 AM Peak - With Development

I I I	I I I	I NUMBER OF MINUTES FROM START WHEN			I RATE OF FLOW (VEH/MIN)		
		I FLOW STARTS	I TOP OF PEAK	I FLOW STOPS	I BEFORE	I AT TOP	I AFTER
I	I	I TO RISE	I IS REACHED	I IF FALLING	I PEAK	I OF PEAK	I PEAK
I ARM A	I	I 15.00	I 45.00	I 75.00	I 4.21	I 6.32	I 4.21
I ARM B	I	I 15.00	I 45.00	I 75.00	I 1.86	I 2.79	I 1.86
I ARM C	I	I 15.00	I 45.00	I 75.00	I 3.38	I 5.06	I 3.38

DEMAND SET TITLE: 2032 AM Peak - With Development

I I I I I	I I I I I	I TURNING PROPORTIONS			I TURNING COUNTS (VEH/HR)			I (PERCENTAGE OF H.V.S)					
		I FROM/TO	I ARM A	I ARM B	I ARM C	I FROM/TO	I ARM A	I ARM B	I ARM C	I FROM/TO	I ARM A	I ARM B	I ARM C
I	I	I 07.15 - 08.45	I	I	I	I	I	I	I	I	I	I	I
I	I	I	I ARM A	I	I 0.000	I	I 0.549	I	I 0.451	I	I	I	I
I	I	I	I	I	I 0.0	I	I 185.0	I	I 152.0	I	I	I	I
I	I	I	I	I	I (0.0)	I	I (7.0)	I	I (16.0)	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I	I	I	I
I	I	I	I ARM B	I	I 0.349	I	I 0.000	I	I 0.651	I	I	I	I
I	I	I	I	I	I 52.0	I	I 0.0	I	I 97.0	I	I	I	I
I	I	I	I	I	I (15.0)	I	I (0.0)	I	I (24.0)	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I	I	I	I
I	I	I	I ARM C	I	I 0.252	I	I 0.748	I	I 0.000	I	I	I	I
I	I	I	I	I	I 68.0	I	I 202.0	I	I 152.0	I	I	I	I
I	I	I	I	I	I (2.0)	I	I (10.0)	I	I (0.0)	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I	I	I	I

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	
I 07.15-07.30										
I ARM A	I 4.21	I 26.17	I 0.161		I 0.0	I 0.2	I 2.8		I 0.05	I
I ARM B	I 1.86	I 26.85	I 0.069		I 0.0	I 0.1	I 1.1		I 0.04	I
I ARM C	I 3.38	I 25.63	I 0.132		I 0.0	I 0.2	I 2.2		I 0.04	I

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	
I 07.30-07.45										
I ARM A	I 5.03	I 25.89	I 0.194		I 0.2	I 0.2	I 3.6		I 0.05	I
I ARM B	I 2.22	I 26.64	I 0.083		I 0.1	I 0.1	I 1.3		I 0.04	I
I ARM C	I 4.03	I 25.55	I 0.158		I 0.2	I 0.2	I 2.8		I 0.05	I

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	
I 07.45-08.00										
I ARM A	I 6.16	I 25.52	I 0.241		I 0.2	I 0.3	I 4.7		I 0.05	I
I ARM B	I 2.72	I 26.35	I 0.103		I 0.1	I 0.1	I 1.7		I 0.04	I
I ARM C	I 4.94	I 25.45	I 0.194		I 0.2	I 0.2	I 3.6		I 0.05	I

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
ARM A	6.16	25.52	0.241		0.3	0.3	4.8		0.05
ARM B	2.72	26.35	0.103		0.1	0.1	1.7		0.04
ARM C	4.94	25.45	0.194		0.2	0.2	3.6		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
ARM A	5.03	25.89	0.194		0.3	0.2	3.7		0.05
ARM B	2.22	26.64	0.083		0.1	0.1	1.4		0.04
ARM C	4.03	25.55	0.158		0.2	0.2	2.9		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
ARM A	4.21	26.16	0.161		0.2	0.2	2.9		0.05
ARM B	1.86	26.85	0.069		0.1	0.1	1.1		0.04
ARM C	3.38	25.63	0.132		0.2	0.2	2.3		0.04

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.3
08.15	0.3
08.30	0.2
08.45	0.2

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.2

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
	(VEH)	(VEH/H)	(MIN)	(MIN/VEH)
A	462.1	308.1	22.4	0.05
B	204.3	136.2	8.4	0.04
C	370.2	246.8	17.3	0.05
ALL	1036.6	691.1	48.1	0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\AM Peak\2032 AM Peak - With Development + Sensitivity Flows.vai"
(drive-on-the-left) at 15:15:39 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

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.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2032 AM Peak - With Development + Sensitivity Flows

I	I	NUMBER OF MINUTES FROM START WHEN			RATE OF FLOW (VEH/MIN)			I
I	ARM	FLOW STARTS	TOP OF PEAK	FLOW STOPS	BEFORE	AT TOP	AFTER	I
I	I	TO RISE	IS REACHED	IF FALLING	PEAK	OF PEAK	PEAK	I
I	ARM A	I 15.00	I 45.00	I 75.00	I 4.41	I 6.62	I 4.41	I
I	ARM B	I 15.00	I 45.00	I 75.00	I 2.06	I 3.09	I 2.06	I
I	ARM C	I 15.00	I 45.00	I 75.00	I 3.38	I 5.06	I 3.38	I

DEMAND SET TITLE: 2032 AM Peak - With Development + Sensitivity Flows

I	TURNING PROPORTIONS						I
I	TURNING COUNTS (VEH/HR)						I
I	(PERCENTAGE OF H.V.S)						I
I	TIME	FROM/TO	ARM A	ARM B	ARM C	I	I
I	07.15 - 08.45	I	I	I	I	I	I
I		I ARM A	I 0.000	I 0.524	I 0.476	I	I
I		I	I 0.0	I 185.0	I 168.0	I	I
I		I	I (0.0)	I (7.0)	I (16.0)	I	I
I		I	I	I	I	I	I
I		I ARM B	I 0.315	I 0.000	I 0.685	I	I
I		I	I 52.0	I 0.0	I 113.0	I	I
I		I	I (15.0)	I (0.0)	I (24.0)	I	I
I		I	I	I	I	I	I
I		I ARM C	I 0.252	I 0.748	I 0.000	I	I
I		I	I 68.0	I 202.0	I 0.0	I	I
I		I	I (2.0)	I (10.0)	I (0.0)	I	I
I		I	I	I	I	I	I

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QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.15-07.30										I
I	ARM A	4.41	26.12	0.169		0.0	0.2	3.0		0.05	I
I	ARM B	2.06	26.67	0.077		0.0	0.1	1.2		0.04	I
I	ARM C	3.38	25.63	0.132		0.0	0.2	2.2		0.04	I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.30-07.45										I
I	ARM A	5.27	25.84	0.204		0.2	0.3	3.8		0.05	I
I	ARM B	2.46	26.43	0.093		0.1	0.1	1.5		0.04	I
I	ARM C	4.03	25.55	0.158		0.2	0.2	2.8		0.05	I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	ARM A	6.45	25.47	0.253		0.3	0.3	5.0		0.05	I
I	ARM B	3.02	26.12	0.115		0.1	0.1	1.9		0.04	I
I	ARM C	4.94	25.45	0.194		0.2	0.2	3.6		0.05	I

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
ARM A	6.45	25.47	0.253		0.3	0.3	5.1		0.05
ARM B	3.02	26.12	0.115		0.1	0.1	2.0		0.04
ARM C	4.94	25.45	0.194		0.2	0.2	3.6		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
ARM A	5.27	25.84	0.204		0.3	0.3	3.9		0.05
ARM B	2.46	26.43	0.093		0.1	0.1	1.6		0.04
ARM C	4.03	25.55	0.158		0.2	0.2	2.9		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
ARM A	4.41	26.11	0.169		0.3	0.2	3.1		0.05
ARM B	2.06	26.67	0.077		0.1	0.1	1.3		0.04
ARM C	3.38	25.63	0.132		0.2	0.2	2.3		0.04

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.3
08.00	0.3
08.15	0.3
08.30	0.3
08.45	0.2

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.2

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 QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
		(VEH) (VEH/H)	(MIN) (MIN/VEH)	(MIN) (MIN/VEH)
A	484.0	322.7	23.8 0.05	23.8 0.05
B	226.2	150.8	9.5 0.04	9.5 0.04
C	370.2	246.8	17.3 0.05	17.3 0.05
ALL	1080.5	720.3	50.6 0.05	50.6 0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\PM Peak\2016 PM Peak - Existing Flows.vai"
(drive-on-the-left) at 15:20:14 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

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.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 16.30 AND ENDS 18.00
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2016 PM Peak - Existing Flows

I I I	I I I	NUMBER OF MINUTES FROM START WHEN			RATE OF FLOW (VEH/MIN)		
		I I I	I I I	I I I	I I I	I I I	I I I
ARM	FLOW STARTS TO RISE	TOP OF PEAK IS REACHED	FLOW STOPS IF FALLING	BEFORE PEAK	AT TOP OF PEAK	AFTER PEAK	
ARM A	15.00	45.00	75.00	2.46	3.69	2.46	
ARM B	15.00	45.00	75.00	4.50	6.75	4.50	
ARM C	15.00	45.00	75.00	3.59	5.38	3.59	

DEMAND SET TITLE: 2016 PM Peak - Existing Flows

I I I I	I I I I	TURNING PROPORTIONS			I I I I
		TURNING COUNTS (VEH/HR)			
			(PERCENTAGE OF H.V.S)		
TIME	FROM/TO	ARM A	ARM B	ARM C	
16.30 - 18.00	ARM A	0.000	0.563	0.437	
		0.0	111.0	86.0	
		(0.0)	(8.0)	(5.0)	
	ARM B	0.514	0.000	0.486	
		185.0	0.0	175.0	
		(9.0)	(0.0)	(9.0)	
	ARM C	0.443	0.557	0.000	
		127.0	160.0	0	
		(9.0)	(10.0)	(0.0)	

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	16.30-16.45									
I	ARM A	2.46	27.54	0.089		0.0	0.1	1.4		0.04
I	ARM B	4.50	30.36	0.148		0.0	0.2	2.6		0.04
I	ARM C	3.59	24.37	0.147		0.0	0.2	2.5		0.05

I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	16.45-17.00									
I	ARM A	2.94	27.32	0.108		0.1	0.1	1.8		0.04
I	ARM B	5.37	30.24	0.178		0.2	0.2	3.2		0.04
I	ARM C	4.28	24.12	0.178		0.2	0.2	3.2		0.05

I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	17.00-17.15									
I	ARM A	3.60	27.00	0.133		0.1	0.2	2.3		0.04
I	ARM B	6.58	30.07	0.219		0.2	0.3	4.1		0.04
I	ARM C	5.25	23.78	0.221		0.2	0.3	4.2		0.05

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
ARM A	3.60	27.00	0.133		0.2	0.2	2.3		0.04
ARM B	6.58	30.07	0.219		0.3	0.3	4.2		0.04
ARM C	5.25	23.78	0.221		0.3	0.3	4.2		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
ARM A	2.94	27.31	0.108		0.2	0.1	1.8		0.04
ARM B	5.37	30.24	0.178		0.3	0.2	3.3		0.04
ARM C	4.28	24.12	0.178		0.3	0.2	3.3		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
ARM A	2.46	27.54	0.089		0.1	0.1	1.5		0.04
ARM B	4.50	30.36	0.148		0.2	0.2	2.6		0.04
ARM C	3.59	24.36	0.147		0.2	0.2	2.6		0.05

.QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

.QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

.QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

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 QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
		(VEH) (VEH/H)	(MIN) (MIN/VEH)	(MIN) (MIN/VEH)
I	A	270.1 I 180.1	11.1 I 0.04	11.1 I 0.04
I	B	493.6 I 329.1	20.0 I 0.04	20.0 I 0.04
I	C	393.5 I 262.4	20.1 I 0.05	20.1 I 0.05
I	ALL	1157.3 I 771.5	51.2 I 0.04	51.2 I 0.04

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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RG40 3GA, UK

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\PM Peak\2017 PM Peak - No Development.vai"
(drive-on-the-left) at 15:20:32 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
For inspection purposes only.

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 16.30 AND ENDS 18.00
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2017 PM Peak - No Development

I I I	I I I	I NUMBER OF MINUTES FROM START WHEN			I RATE OF FLOW (VEH/MIN) I		
		I FLOW STARTS	I TOP OF PEAK	I FLOW STOPS	I BEFORE	I AT TOP	I AFTER
I	I	I TO RISE	I IS REACHED	I IF FALLING	I PEAK	I OF PEAK	I PEAK
I ARM A	I	I 15.00	I 45.00	I 75.00	I 2.49	I 3.73	I 2.49
I ARM B	I	I 15.00	I 45.00	I 75.00	I 4.53	I 6.79	I 4.53
I ARM C	I	I 15.00	I 45.00	I 75.00	I 3.61	I 5.42	I 3.61

DEMAND SET TITLE: 2017 PM Peak - No Development

I I I I I	I I I I I	I TURNING PROPORTIONS			I
		I TURNING COUNTS (VEH/HR)			
		I (PERCENTAGE OF H.V.S)			I
I	I	I FROM/TO	I ARM A	I ARM B	I ARM C
I 16.30 - 18.00	I	I	I	I	I
I	I	I ARM A	I 0.000	I 0.563	I 0.437
I	I	I	I 0.0	I 112.0	I 87.0
I	I	I	I (0.0)	I (8.0)	I (5.0)
I	I	I	I	I	I
I	I	I ARM B	I 0.514	I 0.000	I 0.486
I	I	I	I 186.0	I 0.0	I 176.0
I	I	I	I (9.0)	I (0.0)	I (9.0)
I	I	I	I	I	I
I	I	I ARM C	I 0.443	I 0.557	I 0.000
I	I	I	I 128.0	I 161.0	I 0.0
I	I	I	I (9.0)	I (10.0)	I (0.0)
I	I	I	I	I	I

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I
TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	
	(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	
			(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	
I 16.30-16.45										
I ARM A	2.49	27.54	0.090		0.0	0.1	1.5		0.04	
I ARM B	4.53	30.35	0.149		0.0	0.2	2.6		0.04	
I ARM C	3.61	24.36	0.148		0.0	0.2	2.6		0.05	

I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I
TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	
	(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	
			(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	
I 16.45-17.00										
I ARM A	2.97	27.31	0.109		0.1	0.1	1.8		0.04	
I ARM B	5.40	30.23	0.179		0.2	0.2	3.2		0.04	
I ARM C	4.31	24.11	0.179		0.2	0.2	3.2		0.05	

I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I	I I I I
TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	
	(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	
			(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	
I 17.00-17.15										
I ARM A	3.64	26.99	0.135		0.1	0.2	2.3		0.04	
I ARM B	6.62	30.06	0.220		0.2	0.3	4.2		0.04	
I ARM C	5.28	23.77	0.222		0.2	0.3	4.2		0.05	

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
ARM A	3.64	26.99	0.135		0.2	0.2	2.3		0.04
ARM B	6.62	30.06	0.220		0.3	0.3	4.2		0.04
ARM C	5.28	23.77	0.222		0.3	0.3	4.3		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
ARM A	2.97	27.31	0.109		0.2	0.1	1.9		0.04
ARM B	5.40	30.23	0.179		0.3	0.2	3.3		0.04
ARM C	4.31	24.11	0.179		0.3	0.2	3.3		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
ARM A	2.49	27.53	0.090		0.1	0.1	1.5		0.04
ARM B	4.53	30.35	0.149		0.2	0.2	2.7		0.04
ARM C	3.61	24.36	0.148		0.2	0.2	2.7		0.05

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
		(VEH) (VEH/H)	(MIN) (MIN/VEH)	(MIN) (MIN/VEH)
I	A	272.9 I 181.9 I	11.3 I 0.04 I	11.3 I 0.04 I
I	B	496.4 I 330.9 I	20.2 I 0.04 I	20.2 I 0.04 I
I	C	396.3 I 264.2 I	20.2 I 0.05 I	20.2 I 0.05 I
I	ALL	1165.5 I 777.0 I	51.7 I 0.04 I	51.7 I 0.04 I

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END OF JOB

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A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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(drive-on-the-left) at 15:20:55 on Thursday, 1 December 2016

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DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

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.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

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(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 16.30 AND ENDS 18.00
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2017 PM Peak - With Development

ARM	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	MINUTES FROM START WHEN TOP OF PEAK IS REACHED	MINUTES FROM START WHEN FLOW STOPS IF FALLING	RATE OF FLOW (VEH/MIN) BEFORE PEAK	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	RATE OF FLOW (VEH/MIN) AFTER PEAK
ARM A	15.00	45.00	75.00	2.50	3.75	2.50
ARM B	15.00	45.00	75.00	4.54	6.81	4.54
ARM C	15.00	45.00	75.00	3.61	5.42	3.61

DEMAND SET TITLE: 2017 PM Peak - With Development

TIME	FROM/TO	TURNING PROPORTIONS (PERCENTAGE OF H.V.S)		
		ARM A	ARM B	ARM C
16.30 - 18.00	ARM A	0.000	0.560	0.440
		0.0	112.0	88.0
		(0.0)	(8.0)	(5.0)
	ARM B	0.512	0.000	0.488
		186.0	0.0	177.0
		(9.0)	(0.0)	(9.0)
	ARM C	0.443	0.557	0.000
		128.0	161.0	0.0
		(9.0)	(10.0)	(0.0)

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
ARM A	2.50	27.54	0.091		0.0	0.1	1.5		0.04
ARM B	4.54	30.35	0.150		0.0	0.2	2.6		0.04
ARM C	3.61	24.36	0.148		0.0	0.2	2.6		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
ARM A	2.99	27.31	0.109		0.1	0.1	1.8		0.04
ARM B	5.42	30.22	0.179		0.2	0.2	3.2		0.04
ARM C	4.31	24.11	0.179		0.2	0.2	3.2		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
ARM A	3.66	27.00	0.135		0.1	0.2	2.3		0.04
ARM B	6.64	30.05	0.221		0.2	0.3	4.2		0.04
ARM C	5.28	23.77	0.222		0.2	0.3	4.2		0.05

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
ARM A	3.66	26.99	0.135		0.2	0.2	2.3		0.04
ARM B	6.64	30.05	0.221		0.3	0.3	4.2		0.04
ARM C	5.28	23.77	0.222		0.3	0.3	4.3		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
ARM A	2.99	27.31	0.109		0.2	0.1	1.9		0.04
ARM B	5.42	30.22	0.179		0.3	0.2	3.3		0.04
ARM C	4.31	24.11	0.179		0.3	0.2	3.3		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
ARM A	2.50	27.53	0.091		0.1	0.1	1.5		0.04
ARM B	4.54	30.35	0.150		0.2	0.2	2.7		0.04
ARM C	3.61	24.36	0.148		0.2	0.2	2.7		0.05

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

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 QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	ARM	I	TOTAL DEMAND		I	* QUEUEING *		I	* INCLUSIVE QUEUEING *		I
I		I			I	* DELAY *		I	* DELAY *		I
I		I			I			I			I
I		I	(VEH)	(VEH/H)	I	(MIN)	(MIN/VEH)	I	(MIN)	(MIN/VEH)	I
I	A	I	274.2	I 182.8	I	11.3	I 0.04	I	11.3	I 0.04	I
I	B	I	497.7	I 331.8	I	20.2	I 0.04	I	20.2	I 0.04	I
I	C	I	396.3	I 264.2	I	20.2	I 0.05	I	20.2	I 0.05	I
I	ALL	I	1168.3	I 778.8	I	51.8	I 0.04	I	51.8	I 0.04	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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ARCADY 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\PM Peak\2022 PM Peak - No Development.vai"
(drive-on-the-left) at 15:21:55 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
For inspection purposes only.

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 16.30 AND ENDS 18.00
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2022 PM Peak - No Development

I I I	I I I	I NUMBER OF MINUTES FROM START WHEN			I RATE OF FLOW (VEH/MIN) I		
		I FLOW STARTS	I TOP OF PEAK	I FLOW STOPS	I BEFORE	I AT TOP	I AFTER
I	I	I TO RISE	I IS REACHED	I IF FALLING	I PEAK	I OF PEAK	I PEAK
I ARM A	I	I 15.00	I 45.00	I 75.00	I 2.58	I 3.86	I 2.58
I ARM B	I	I 15.00	I 45.00	I 75.00	I 4.70	I 7.05	I 4.70
I ARM C	I	I 15.00	I 45.00	I 75.00	I 3.75	I 5.63	I 3.75

DEMAND SET TITLE: 2022 PM Peak - No Development

I I I I I	I I I I I	I TURNING PROPORTIONS			I
		I TURNING COUNTS (VEH/HR)			
		I (PERCENTAGE OF H.V.S)			I
I	I	I FROM/TO	I ARM A	I ARM B	I ARM C
I	I	I 16.30 - 18.00	I	I	I
I	I	I ARM A	I 0.000	I 0.563	I 0.437
I	I	I	I 0.0	I 116.0	I 90.0
I	I	I	I (0.0)	I (8.0)	I (5.0)
I	I	I	I	I	I
I	I	I ARM B	I 0.513	I 0.000	I 0.487
I	I	I	I 193.0	I 0.0	I 183.0
I	I	I	I (9.0)	I (0.0)	I (9.0)
I	I	I	I	I	I
I	I	I ARM C	I 0.443	I 0.557	I 0.000
I	I	I	I 133.0	I 167.0	I 0.0
I	I	I	I (9.0)	I (10.0)	I (0.0)
I	I	I	I	I	I

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	
	(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	
			(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	
I 16.30-16.45										
I ARM A	2.58	27.49	0.094		0.0	0.1	1.5		0.04	
I ARM B	4.70	30.33	0.155		0.0	0.2	2.7		0.04	
I ARM C	3.75	24.31	0.154		0.0	0.2	2.7		0.05	

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	
	(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	
			(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	
I 16.45-17.00										
I ARM A	3.07	27.26	0.113		0.1	0.1	1.9		0.04	
I ARM B	5.61	30.21	0.186		0.2	0.2	3.4		0.04	
I ARM C	4.48	24.05	0.186		0.2	0.2	3.4		0.05	

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	
	(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	
			(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	
I 17.00-17.15										
I ARM A	3.77	26.93	0.140		0.1	0.2	2.4		0.04	
I ARM B	6.87	30.03	0.229		0.2	0.3	4.4		0.04	
I ARM C	5.48	23.70	0.231		0.2	0.3	4.4		0.05	

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
ARM A	3.77	26.93	0.140		0.2	0.2	2.4		0.04
ARM B	6.87	30.03	0.229		0.3	0.3	4.4		0.04
ARM C	5.48	23.70	0.231		0.3	0.3	4.5		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
ARM A	3.07	27.25	0.113		0.2	0.1	1.9		0.04
ARM B	5.61	30.20	0.186		0.3	0.2	3.5		0.04
ARM C	4.48	24.05	0.186		0.3	0.2	3.5		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
ARM A	2.58	27.49	0.094		0.1	0.1	1.6		0.04
ARM B	4.70	30.33	0.155		0.2	0.2	2.8		0.04
ARM C	3.75	24.31	0.154		0.2	0.2	2.8		0.05

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

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 QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	ARM	I	TOTAL DEMAND		I	* QUEUEING *		I	* INCLUSIVE QUEUEING *		I
I		I		I	I	* DELAY *		I	* DELAY *		I
I		I		I	I			I			I
I		I	(VEH)	(VEH/H)	I	(MIN)	(MIN/VEH)	I	(MIN)	(MIN/VEH)	I
I	A	I	282.5	I 188.3	I	11.7	I 0.04	I	11.7	I 0.04	I
I	B	I	515.6	I 343.7	I	21.2	I 0.04	I	21.2	I 0.04	I
I	C	I	411.4	I 274.2	I	21.3	I 0.05	I	21.3	I 0.05	I
I	ALL	I	1209.4	I 806.3	I	54.2	I 0.04	I	54.2	I 0.04	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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ARCADY 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\PM Peak\2022 PM Peak - With Development.vai"
(drive-on-the-left) at 15:22:48 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
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.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 16.30 AND ENDS 18.00
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2022 PM Peak - With Development

I I I	I I I	I NUMBER OF MINUTES FROM START WHEN			I RATE OF FLOW (VEH/MIN) I		
		I FLOW STARTS I TO RISE	I TOP OF PEAK I IS REACHED	I FLOW STOPS IFALLING	I BEFORE I PEAK	I AT TOP I OF PEAK	I AFTER I PEAK
I ARM A	I	I 15.00	I 45.00	I 75.00	I 2.59	I 3.88	I 2.59
I ARM B	I	I 15.00	I 45.00	I 75.00	I 4.71	I 7.07	I 4.71
I ARM C	I	I 15.00	I 45.00	I 75.00	I 3.75	I 5.63	I 3.75

DEMAND SET TITLE: 2022 PM Peak - With Development

I I I I I	I I I I I	I TURNING PROPORTIONS			I I I I I
		I TURNING COUNTS (VEH/HR)			
		I (PERCENTAGE OF H.V.S)			
I I	I I	I I	I I	I I	I I
TIME	FROM/TO	ARM A	ARM B	ARM C	
I 16.30 - 18.00	I	I	I	I	I
I	I ARM A	I 0.000	I 0.560	I 0.440	I
I	I	I 0.0	I 116.0	I 91.0	I
I	I	I (0.0)	I (8.0)	I (5.0)	I
I	I	I	I	I	I
I	I ARM B	I 0.512	I 0.000	I 0.488	I
I	I	I 193.0	I 0.0	I 184.0	I
I	I	I (9.0)	I (0.0)	I (9.0)	I
I	I	I	I	I	I
I	I ARM C	I 0.443	I 0.557	I 0.000	I
I	I	I 133.0	I 167.0	I 0.0	I
I	I	I (9.0)	I (10.0)	I (0.0)	I
I	I	I	I	I	I

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	I 16.30-16.45									
I	I ARM A	I 2.59	I 27.50	I 0.094		I 0.0	I 0.1	I 1.5		I 0.04
I	I ARM B	I 4.71	I 30.33	I 0.155		I 0.0	I 0.2	I 2.7		I 0.04
I	I ARM C	I 3.75	I 24.31	I 0.154		I 0.0	I 0.2	I 2.7		I 0.05

I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	I 16.45-17.00									
I	I ARM A	I 3.09	I 27.26	I 0.113		I 0.1	I 0.1	I 1.9		I 0.04
I	I ARM B	I 5.63	I 30.20	I 0.186		I 0.2	I 0.2	I 3.4		I 0.04
I	I ARM C	I 4.48	I 24.05	I 0.186		I 0.2	I 0.2	I 3.4		I 0.05

I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	I 17.00-17.15									
I	I ARM A	I 3.78	I 26.93	I 0.141		I 0.1	I 0.2	I 2.4		I 0.04
I	I ARM B	I 6.89	I 30.02	I 0.230		I 0.2	I 0.3	I 4.4		I 0.04
I	I ARM C	I 5.48	I 23.70	I 0.231		I 0.2	I 0.3	I 4.4		I 0.05

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
ARM A	3.78	26.94	0.140		0.2	0.2	2.4		0.04
ARM B	6.89	30.02	0.230		0.3	0.3	4.5		0.04
ARM C	5.48	23.70	0.231		0.3	0.3	4.5		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
ARM A	3.09	27.25	0.113		0.2	0.1	1.9		0.04
ARM B	5.63	30.20	0.186		0.3	0.2	3.5		0.04
ARM C	4.48	24.05	0.186		0.3	0.2	3.5		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
ARM A	2.59	27.49	0.094		0.1	0.1	1.6		0.04
ARM B	4.71	30.32	0.155		0.2	0.2	2.8		0.04
ARM C	3.75	24.31	0.154		0.2	0.2	2.8		0.05

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
		(VEH) (VEH/H)	(MIN) (MIN/VEH)	(MIN) (MIN/VEH)
I	A	283.8 I 189.2 I	11.8 I 0.04 I	11.8 I 0.04 I
I	B	516.9 I 344.6 I	21.2 I 0.04 I	21.2 I 0.04 I
I	C	411.4 I 274.2 I	21.3 I 0.05 I	21.3 I 0.05 I
I	ALL	1212.2 I 808.1 I	54.3 I 0.04 I	54.3 I 0.04 I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\PM Peak\2022 PM Peak - With Development + Sensitivity Flows.vai"
(drive-on-the-left) at 15:31:53 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

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.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 16.30 AND ENDS 18.00
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2022 PM Peak - With Development + Sensitivity Flows

I I I	I I I	NUMBER OF MINUTES FROM START WHEN			RATE OF FLOW (VEH/MIN)		
		I I I	I I I	I I I	I I I	I I I	I I I
ARM	FLOW STARTS TO RISE	TOP OF PEAK IS REACHED	FLOW STOPS IF FALLING	BEFORE PEAK	AT TOP OF PEAK	AFTER PEAK	
ARM A	15.00	45.00	75.00	2.86	4.29	2.86	
ARM B	15.00	45.00	75.00	4.99	7.48	4.99	
ARM C	15.00	45.00	75.00	3.75	5.63	3.75	

DEMAND SET TITLE: 2022 PM Peak - With Development + Sensitivity Flows

I I I I I	I I I I I	TURNING PROPORTIONS			TURNING COUNTS (VEH/HR)			(PERCENTAGE OF H.V.S)		
		I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	
TIME	FROM/TO	ARM A	ARM B	ARM C						
16.30 - 18.00	ARM A	0.000	0.507	0.493	0.0	116.0	113.0	(0.0)	(8.0)	(5.0)
	ARM B	0.484	0.000	0.516	193.0	0.0	206.0	(9.0)	(0.0)	(9.0)
	ARM C	0.443	0.557	0.000	133.0	167.0	0.0	(9.0)	(10.0)	(0.0)

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	16.30-16.45									
I	ARM A	2.86	27.54	0.104		0.0	0.1	1.7		0.04
I	ARM B	4.99	30.17	0.165		0.0	0.2	2.9		0.04
I	ARM C	3.75	24.31	0.154		0.0	0.2	2.7		0.05

I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	16.45-17.00									
I	ARM A	3.42	27.30	0.125		0.1	0.1	2.1		0.04
I	ARM B	5.96	30.01	0.198		0.2	0.2	3.7		0.04
I	ARM C	4.48	24.05	0.186		0.2	0.2	3.4		0.05

I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	17.00-17.15									
I	ARM A	4.19	26.98	0.155		0.1	0.2	2.7		0.04
I	ARM B	7.29	29.79	0.245		0.2	0.3	4.8		0.04
I	ARM C	5.48	23.70	0.231		0.2	0.3	4.4		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
ARM A	4.19	26.98	0.155		0.2	0.2	2.7		0.04
ARM B	7.29	29.79	0.245		0.3	0.3	4.8		0.04
ARM C	5.48	23.70	0.231		0.3	0.3	4.5		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
ARM A	3.42	27.30	0.125		0.2	0.1	2.2		0.04
ARM B	5.96	30.01	0.198		0.3	0.2	3.8		0.04
ARM C	4.48	24.05	0.186		0.3	0.2	3.5		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
ARM A	2.86	27.53	0.104		0.1	0.1	1.8		0.04
ARM B	4.99	30.17	0.165		0.2	0.2	3.0		0.04
ARM C	3.75	24.31	0.154		0.2	0.2	2.8		0.05

.QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

.QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

.QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
		(VEH) (VEH/H)	(MIN) (MIN/VEH)	(MIN) (MIN/VEH)
I	A	I 314.0 I 209.3	I 13.2 I 0.04	I 13.2 I 0.04
I	B	I 547.1 I 364.7	I 23.0 I 0.04	I 23.0 I 0.04
I	C	I 411.4 I 274.2	I 21.3 I 0.05	I 21.3 I 0.05
I	ALL	I 1272.5 I 848.3	I 57.5 I 0.05	I 57.5 I 0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\PM Peak\2032 PM Peak - No Development.vai"
(drive-on-the-left) at 15:33:09 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

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.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 16.30 AND ENDS 18.00
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2032 PM Peak - No Development

I I I	I I I	I NUMBER OF MINUTES FROM START WHEN			I RATE OF FLOW (VEH/MIN) I		
		I FLOW STARTS	I TOP OF PEAK	I FLOW STOPS	I BEFORE	I AT TOP	I AFTER
I	I	I TO RISE	I IS REACHED	I IF FALLING	I PEAK	I OF PEAK	I PEAK
I ARM A	I	I 15.00	I 45.00	I 75.00	I 2.67	I 4.01	I 2.67
I ARM B	I	I 15.00	I 45.00	I 75.00	I 4.90	I 7.35	I 4.90
I ARM C	I	I 15.00	I 45.00	I 75.00	I 3.90	I 5.85	I 3.90

DEMAND SET TITLE: 2032 PM Peak - No Development

I I I I	I I I	I TURNING PROPORTIONS			I
		I TURNING COUNTS (VEH/HR)			
		I (PERCENTAGE OF H.V.S)			I
I	I	I FROM/TO	I ARM A	I ARM B	I ARM C
I	I	I 16.30 - 18.00	I	I	I
I	I	I ARM A	I 0.000	I 0.565	I 0.435
I	I	I	I 0.0	I 121.0	I 93.0
I	I	I	I (0.0)	I (8.0)	I (5.0)
I	I	I	I	I	I
I	I	I ARM B	I 0.513	I 0.000	I 0.487
I	I	I	I 201.0	I 0.0	I 191.0
I	I	I	I (9.0)	I (0.0)	I (9.0)
I	I	I	I	I	I
I	I	I ARM C	I 0.442	I 0.558	I 0.000
I	I	I	I 138.0	I 174.0	I 0.0
I	I	I	I (9.0)	I (10.0)	I (0.0)
I	I	I	I	I	I

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	
	(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	
			(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	
I 16.30-16.45										
I ARM A	2.67	27.44	0.097		0.0	0.1	1.6		0.04	
I ARM B	4.90	30.31	0.162		0.0	0.2	2.8		0.04	
I ARM C	3.90	24.26	0.161		0.0	0.2	2.8		0.05	

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	
	(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	
			(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	
I 16.45-17.00										
I ARM A	3.19	27.19	0.117		0.1	0.1	2.0		0.04	
I ARM B	5.85	30.18	0.194		0.2	0.2	3.6		0.04	
I ARM C	4.66	23.99	0.194		0.2	0.2	3.6		0.05	

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	
	(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	
			(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	
I 17.00-17.15										
I ARM A	3.91	26.86	0.146		0.1	0.2	2.5		0.04	
I ARM B	7.17	30.00	0.239		0.2	0.3	4.6		0.04	
I ARM C	5.70	23.62	0.241		0.2	0.3	4.7		0.06	

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
ARM A	3.91	26.86	0.146		0.2	0.2	2.6		0.04
ARM B	7.17	30.00	0.239		0.3	0.3	4.7		0.04
ARM C	5.70	23.62	0.241		0.3	0.3	4.8		0.06

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
ARM A	3.19	27.19	0.117		0.2	0.1	2.0		0.04
ARM B	5.85	30.18	0.194		0.3	0.2	3.7		0.04
ARM C	4.66	23.98	0.194		0.3	0.2	3.7		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
ARM A	2.67	27.44	0.097		0.1	0.1	1.6		0.04
ARM B	4.90	30.31	0.162		0.2	0.2	2.9		0.04
ARM C	3.90	24.25	0.161		0.2	0.2	2.9		0.05

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

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 QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
	(VEH)	(VEH/H)	(MIN)	(MIN/VEH)
A	293.4	195.6	12.3	0.04
B	537.5	358.3	22.3	0.04
C	427.8	285.2	22.4	0.05
ALL	1258.8	839.2	57.0	0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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ARCADY 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\PM Peak\2032 PM Peak - With Development.vai"
(drive-on-the-left) at 15:34:09 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 01/12/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-04
STATUS: TIA
DESCRIPTION:

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.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 16.30 AND ENDS 18.00
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2032 PM Peak - With Development

I I I	I I I	I NUMBER OF MINUTES FROM START WHEN			I RATE OF FLOW (VEH/MIN) I		
		I FLOW STARTS	I TOP OF PEAK	I FLOW STOPS	I BEFORE	I AT TOP	I AFTER
		I TO RISE	I IS REACHED	I IF FALLING	I PEAK	I OF PEAK	I PEAK
I ARM A	I	15.00	45.00	75.00	2.69	4.03	2.69
I ARM B	I	15.00	45.00	75.00	4.90	7.35	4.90
I ARM C	I	15.00	45.00	75.00	3.90	5.85	3.90

DEMAND SET TITLE: 2032 PM Peak - With Development

I I I I I	I I I I I	I TURNING PROPORTIONS			I I I I I
		I TURNING COUNTS (VEH/HR)			
		I (PERCENTAGE OF H.V.S)			
I TIME		I FROM/TO	I ARM A	I ARM B	I ARM C
I	I 16.30 - 18.00	I	I	I	I
I	I	I ARM A	I 0.000	I 0.563	I 0.437
I	I	I	I 0.0	I 121.0	I 94.0
I	I	I	I (0.0)	I (8.0)	I (5.0)
I	I	I	I	I	I
I	I	I ARM B	I 0.513	I 0.000	I 0.487
I	I	I	I 201.0	I 0.0	I 191.0
I	I	I	I (9.0)	I (0.0)	I (9.0)
I	I	I	I	I	I
I	I	I ARM C	I 0.442	I 0.558	I 0.000
I	I	I	I 138.0	I 174.0	I 0.0
I	I	I	I (9.0)	I (10.0)	I (0.0)
I	I	I	I	I	I

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QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	
I 16.30-16.45										
I ARM A	2.69	27.44	0.098		0.0	0.1	1.6		0.04	
I ARM B	4.90	30.30	0.162		0.0	0.2	2.8		0.04	
I ARM C	3.90	24.26	0.161		0.0	0.2	2.8		0.05	

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	
I 16.45-17.00										
I ARM A	3.21	27.19	0.118		0.1	0.1	2.0		0.04	
I ARM B	5.85	30.17	0.194		0.2	0.2	3.6		0.04	
I ARM C	4.66	23.99	0.194		0.2	0.2	3.6		0.05	

I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I
TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	
I 17.00-17.15										
I ARM A	3.93	26.86	0.146		0.1	0.2	2.5		0.04	
I ARM B	7.17	29.99	0.239		0.2	0.3	4.6		0.04	
I ARM C	5.70	23.62	0.241		0.2	0.3	4.7		0.06	

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
ARM A	3.93	26.86	0.146		0.2	0.2	2.6		0.04
ARM B	7.17	29.99	0.239		0.3	0.3	4.7		0.04
ARM C	5.70	23.62	0.241		0.3	0.3	4.8		0.06

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
ARM A	3.21	27.19	0.118		0.2	0.1	2.0		0.04
ARM B	5.85	30.17	0.194		0.3	0.2	3.7		0.04
ARM C	4.66	23.98	0.194		0.3	0.2	3.7		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
ARM A	2.69	27.44	0.098		0.1	0.1	1.7		0.04
ARM B	4.90	30.30	0.162		0.2	0.2	2.9		0.04
ARM C	3.90	24.25	0.161		0.2	0.2	2.9		0.05

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
		(VEH) (VEH/H)	(MIN) (MIN/VEH)	(MIN) (MIN/VEH)
A	294.8	196.5	12.4 0.04	12.4 0.04
B	537.5	358.3	22.3 0.04	22.3 0.04
C	427.8	285.2	22.4 0.05	22.4 0.05
ALL	1260.1	840.1	57.1 0.05	57.1 0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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ARCADY 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

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Run with file:-
"C:\ARCADY\2016\16047-04\R135 - N2 Roundabout\PM Peak\2032 PM Peak - With Development + Sensitivity Flows.vai"
(drive-on-the-left) at 15:35:10 on Thursday, 1 December 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

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.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 16.30 AND ENDS 18.00
 .LENGTH OF TIME PERIOD - 90 MINUTES.
 .LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2032 PM Peak - With Development + Sensitivity Flows

I I I	I I I	NUMBER OF MINUTES FROM START WHEN			RATE OF FLOW (VEH/MIN)		
		I I I	I I I	I I I	I I I	I I I	I I I
ARM	FLOW STARTS TO RISE	TOP OF PEAK IS REACHED	FLOW STOPS IF FALLING	BEFORE PEAK	AT TOP OF PEAK	AFTER PEAK	
ARM A	15.00	45.00	75.00	2.89	4.33	2.89	
ARM B	15.00	45.00	75.00	5.10	7.65	5.10	
ARM C	15.00	45.00	75.00	3.90	5.85	3.90	

DEMAND SET TITLE: 2032 PM Peak - With Development + Sensitivity Flows

I I I I I	I I I I I	TURNING PROPORTIONS			TURNING COUNTS (VEH/HR)			(PERCENTAGE OF H.V.S)		
		I I I	I I I	I I I	I I I	I I I	I I I	I I I	I I I	
TIME	FROM/TO	ARM A	ARM B	ARM C						
16.30 - 18.00	ARM A	0.000	0.524	0.476	0.0	121.0	110.0	(0.0)	(8.0)	(5.0)
	ARM B	0.493	0.000	0.507	201.0	0.0	207.0	(9.0)	(0.0)	(9.0)
	ARM C	0.442	0.558	0.000	138.0	174.0	0.0	(9.0)	(10.0)	(0.0)

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	16.30-16.45									
I	ARM A	2.89	27.47	0.105		0.0	0.1	1.7		0.04
I	ARM B	5.10	30.19	0.169		0.0	0.2	3.0		0.04
I	ARM C	3.90	24.26	0.161		0.0	0.2	2.8		0.05

I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	16.45-17.00									
I	ARM A	3.45	27.22	0.127		0.1	0.1	2.1		0.04
I	ARM B	6.09	30.03	0.203		0.2	0.3	3.8		0.04
I	ARM C	4.66	23.99	0.194		0.2	0.2	3.6		0.05

I I I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	17.00-17.15									
I	ARM A	4.22	26.89	0.157		0.1	0.2	2.8		0.04
I	ARM B	7.46	29.82	0.250		0.3	0.3	4.9		0.04
I	ARM C	5.70	23.62	0.241		0.2	0.3	4.7		0.06

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
ARM A	4.22	26.89	0.157		0.2	0.2	2.8		0.04
ARM B	7.46	29.82	0.250		0.3	0.3	5.0		0.04
ARM C	5.70	23.62	0.241		0.3	0.3	4.8		0.06

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
ARM A	3.45	27.22	0.127		0.2	0.1	2.2		0.04
ARM B	6.09	30.03	0.203		0.3	0.3	3.9		0.04
ARM C	4.66	23.98	0.194		0.3	0.2	3.7		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
ARM A	2.89	27.47	0.105		0.1	0.1	1.8		0.04
ARM B	5.10	30.19	0.169		0.3	0.2	3.1		0.04
ARM C	3.90	24.25	0.161		0.2	0.2	2.9		0.05

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.3
17.15	0.3
17.30	0.3
17.45	0.3
18.00	0.2

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

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QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

	ARM	TOTAL DEMAND	* QUEUEING * * DELAY *	* INCLUSIVE QUEUEING * * DELAY *
		(VEH) (VEH/H)	(MIN) (MIN/VEH)	(MIN) (MIN/VEH)
A	316.7	211.2	13.4 0.04	13.4 0.04
B	559.5	373.0	23.6 0.04	23.6 0.04
C	427.8	285.2	22.4 0.05	22.4 0.05
ALL	1304.0	869.3	59.5 0.05	59.5 0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

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TRAFFIC AND TRANSPORTATION 13

APPENDIX 13-E OSCADY PRO RESULTS

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads.osc
Report generation date: 05/12/2016 16:40:52

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

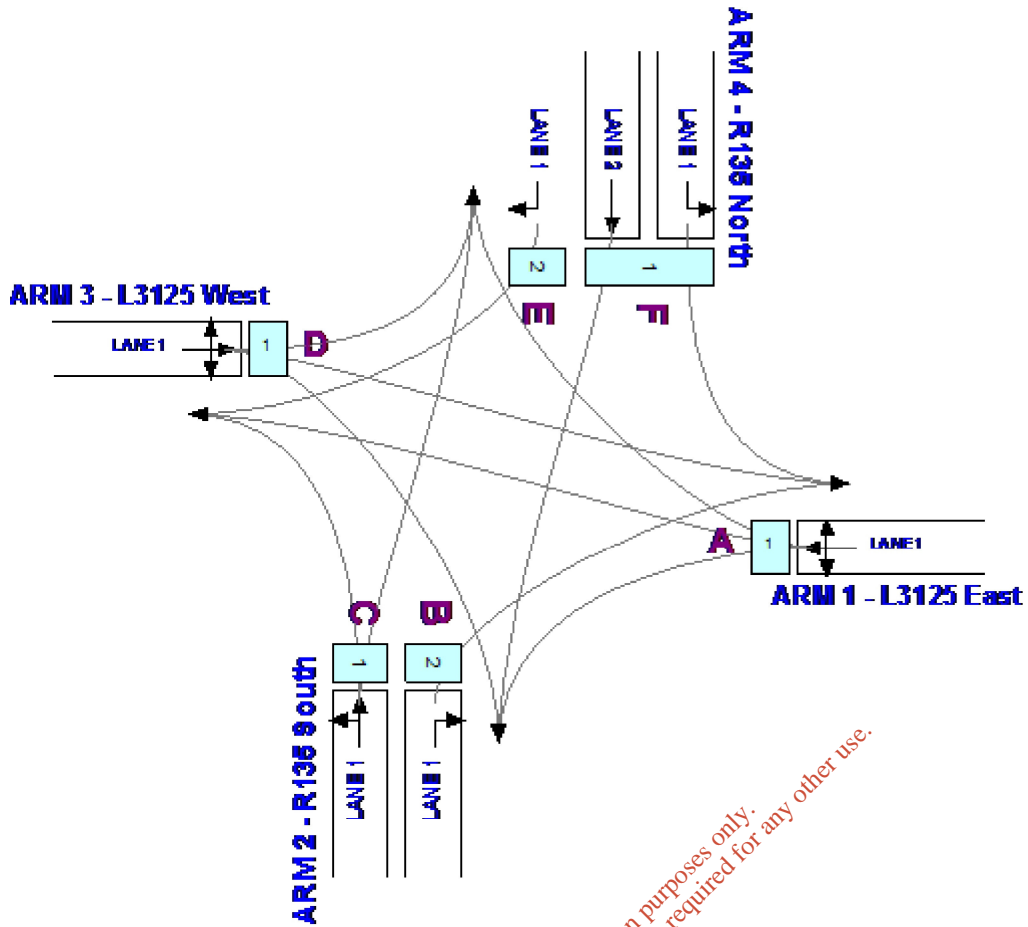
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

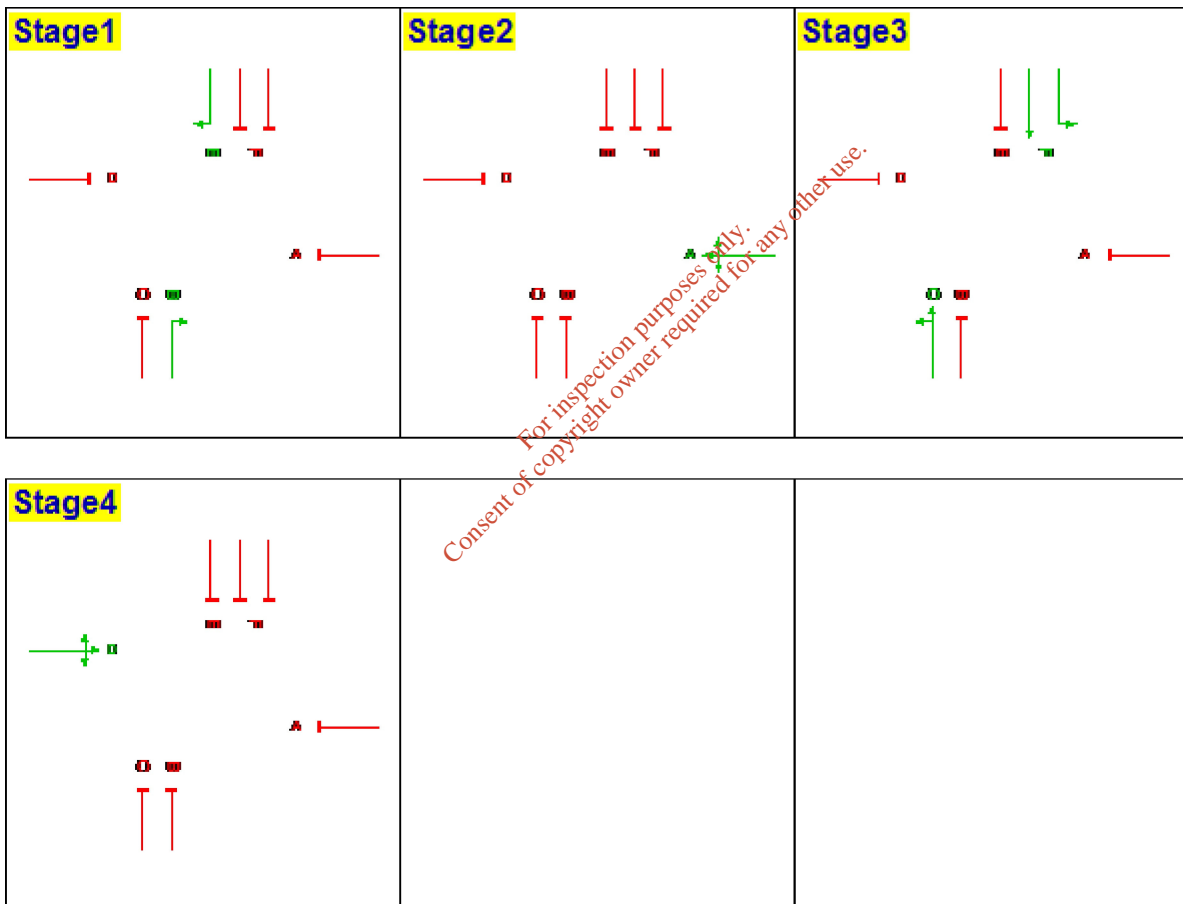
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	Yes	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak + Link	No	07:15	08:45	ODTAB	No	D1

Demand Set1 - 2016 AM Peak Existing

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	101	331	93
	Arm 2	88	-	224	77
	Arm 3	163	111	-	11
	Arm 4	234	137	30	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	394	470	576	576	470	394
2 - R135 South	1	C	228	272	333	333	272	228
2 - R135 South	2	B	67	80	98	98	80	67
3 - L3125 West	1	D	214	255	313	313	255	214
4 - R135 North	1	F	277	330	405	405	330	277
4 - R135 North	2	E	21	25	31	31	25	21

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	19	63	18
2 - R135 South	58	20	23
3 - L3125 West	4	57	39
4 - R135 North	58	34	7

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	21.48	20.44	20.44	68.8

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	1.30	21.71	21.71	66.40

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

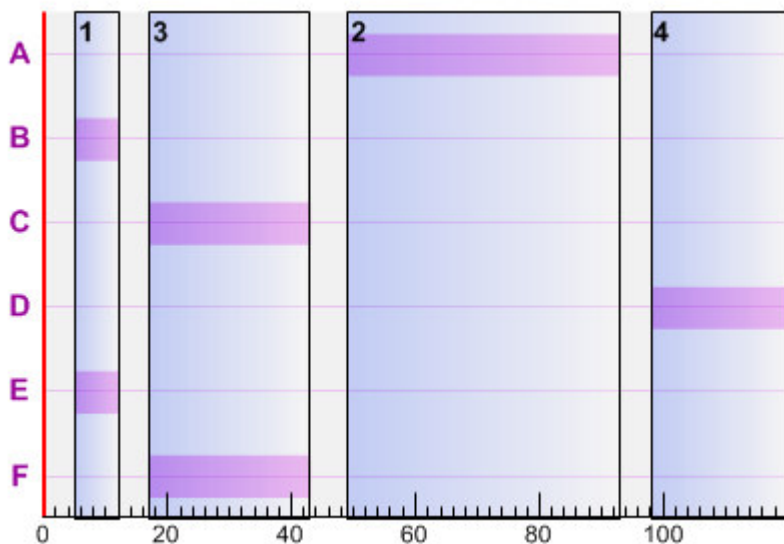
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	26.0	43.0
2	49.0	44.0	93.0
4	98.0	22.0	0.0

Phase Timings

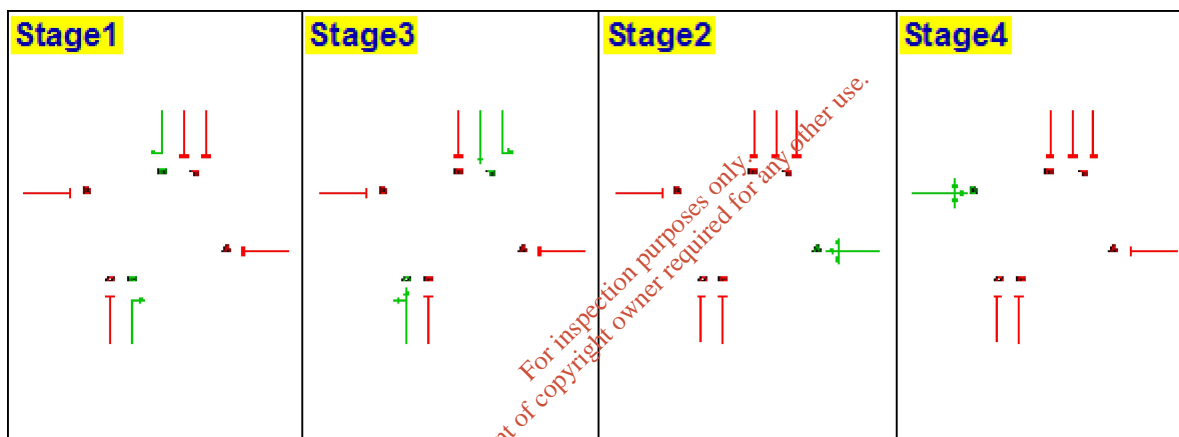
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	49	44.0	93						
B	5	7.0	12						
C	17	26.0	43						
D	98	22.0	0						
E	5	7.0	12						
F	17	26.0	43						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	480	A	45.50	39.92	5.32	70.68	27.33	1.24	12.22	10.99	28.10
2	1	278	C	27.50	56.47	4.36	70.36	27.90	1.15	8.72	7.56	10.30
2	2	82	B	8.50	82.36	1.88	61.97	45.23	0.66	3.25	2.58	1.70
3	1	261	D	23.50	64.03	4.64	74.08	21.48	1.45	8.83	7.38	8.00
4	1	337	F	27.50	40.87	3.83	38.85	131.65	0.17	9.46	9.28	19.90
4	2	26	E	8.50	56.91	0.41	19.65	358.02	0.03	0.84	0.81	0.80

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	394	1	45.50	34.32	3.76	58.02	55.12	0.56	9.42	8.85	4.90
2	1	228	3	27.50	48.83	3.09	57.71	55.95	0.53	6.67	6.14	1.80
2	2	67	2	8.50	70.20	1.31	50.64	77.74	0.33	2.43	2.10	0.30
3	1	214	4	23.50	53.71	3.19	60.74	48.17	0.62	6.62	5.99	1.40
4	1	277	6	27.50	39.74	3.06	31.93	181.83	0.10	7.64	7.53	2.90
4	2	21	5	8.50	55.72	0.33	15.87	467.07	0.02	0.67	0.65	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	470	1	45.50	38.93	5.08	69.21	30.04	1.09	11.83	10.74	4.70
2	1	272	3	27.50	54.85	4.14	68.85	30.73	1.00	8.39	7.39	1.70
2	2	80	2	8.50	79.12	1.76	60.46	48.86	0.57	3.08	2.52	0.30
3	1	255	4	23.50	61.41	4.35	72.38	24.34	1.22	8.42	7.20	1.30
4	1	330	6	27.50	40.73	3.73	38.04	136.56	0.16	9.24	9.08	3.30
4	2	25	5	8.50	56.66	0.39	18.89	376.34	0.03	0.81	0.78	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	576	1	45.50	50.51	8.08	84.82	6.11	3.09	16.56	13.47	3.70
2	1	333	3	27.50	69.41	6.42	84.29	6.78	2.62	11.79	9.17	1.50
2	2	98	2	8.50	95.38	2.60	74.06	21.52	1.16	4.26	3.10	0.30
3	1	313	4	23.50	81.61	7.10	88.84	1.30	3.50	12.45	8.95	1.10
4	1	405	6	27.50	42.30	4.76	46.69	92.75	0.29	11.61	11.32	3.60
4	2	31	5	8.50	58.11	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	576	1	45.50	52.66	8.43	84.82	6.11	3.21	16.69	13.47	3.70
2	1	333	3	27.50	73.75	6.82	84.29	6.78	2.79	11.96	9.17	1.50
2	2	98	2	8.50	101.46	2.76	74.06	21.52	1.24	4.33	3.10	0.30
3	1	313	4	23.50	90.90	7.90	88.84	1.30	3.89	12.84	8.95	1.10
4	1	405	6	27.50	42.32	4.76	46.69	92.75	0.29	11.62	11.32	3.60
4	2	31	5	8.50	58.15	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	470	1	45.50	40.09	5.23	69.21	30.04	1.17	11.91	10.74	4.70
2	1	272	3	27.50	57.78	4.37	68.85	30.73	1.12	8.51	7.39	1.70
2	2	80	2	8.50	85.73	1.91	60.46	48.86	0.68	3.20	2.52	0.30
3	1	255	4	23.50	67.99	4.82	72.38	24.34	1.43	8.63	7.20	1.30
4	1	330	6	27.50	40.75	3.74	38.04	136.56	0.17	9.24	9.08	3.30
4	2	25	5	8.50	56.72	0.39	18.89	376.34	0.03	0.81	0.78	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	394	1	45.50	34.63	3.79	58.02	55.12	0.58	9.44	8.85	4.90
2	1	228	3	27.50	49.71	3.15	57.71	55.95	0.56	6.70	6.14	1.80
2	2	67	2	8.50	73.79	1.37	50.64	77.74	0.37	2.47	2.10	0.30
3	1	214	4	23.50	55.28	3.29	60.74	48.17	0.67	6.67	5.99	1.40
4	1	277	6	27.50	39.76	3.06	31.93	181.83	0.11	7.64	7.53	2.90
4	2	21	5	8.50	55.81	0.33	15.87	467.07	0.02	0.67	0.65	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads.osc
Report generation date: 05/12/2016 16:42:10

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

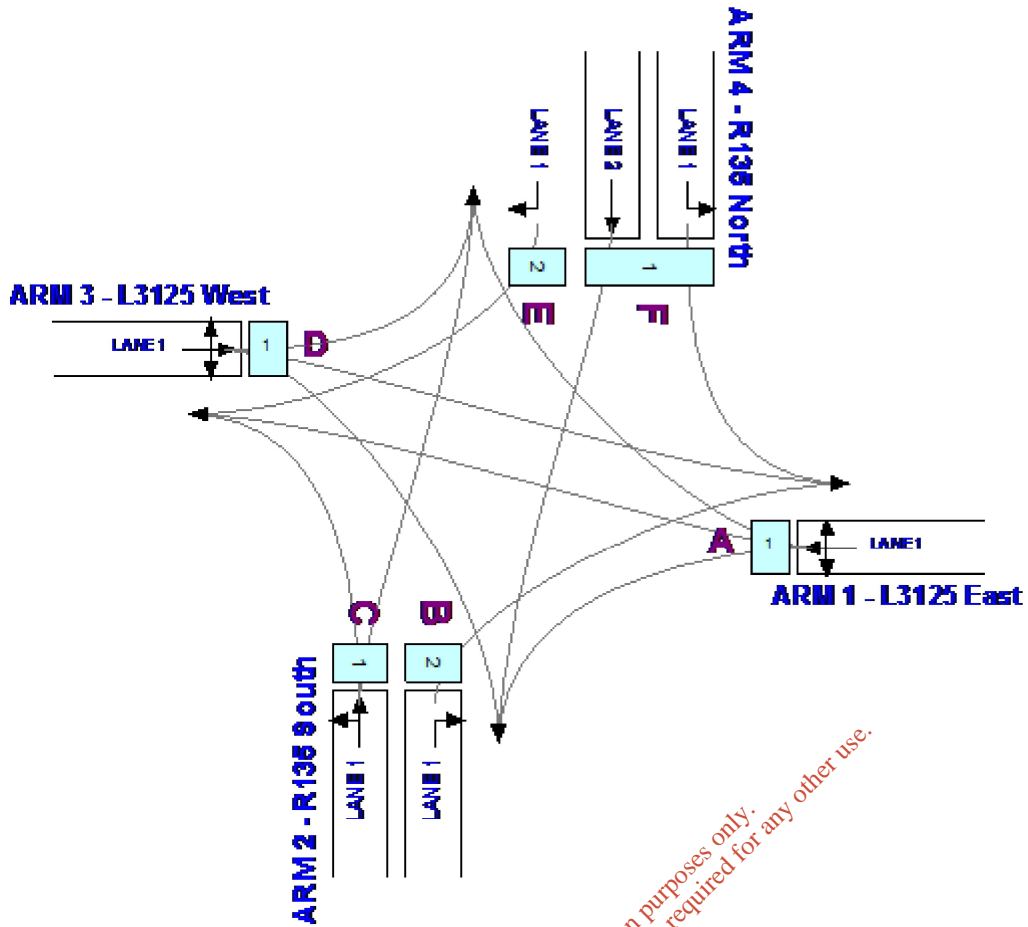
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

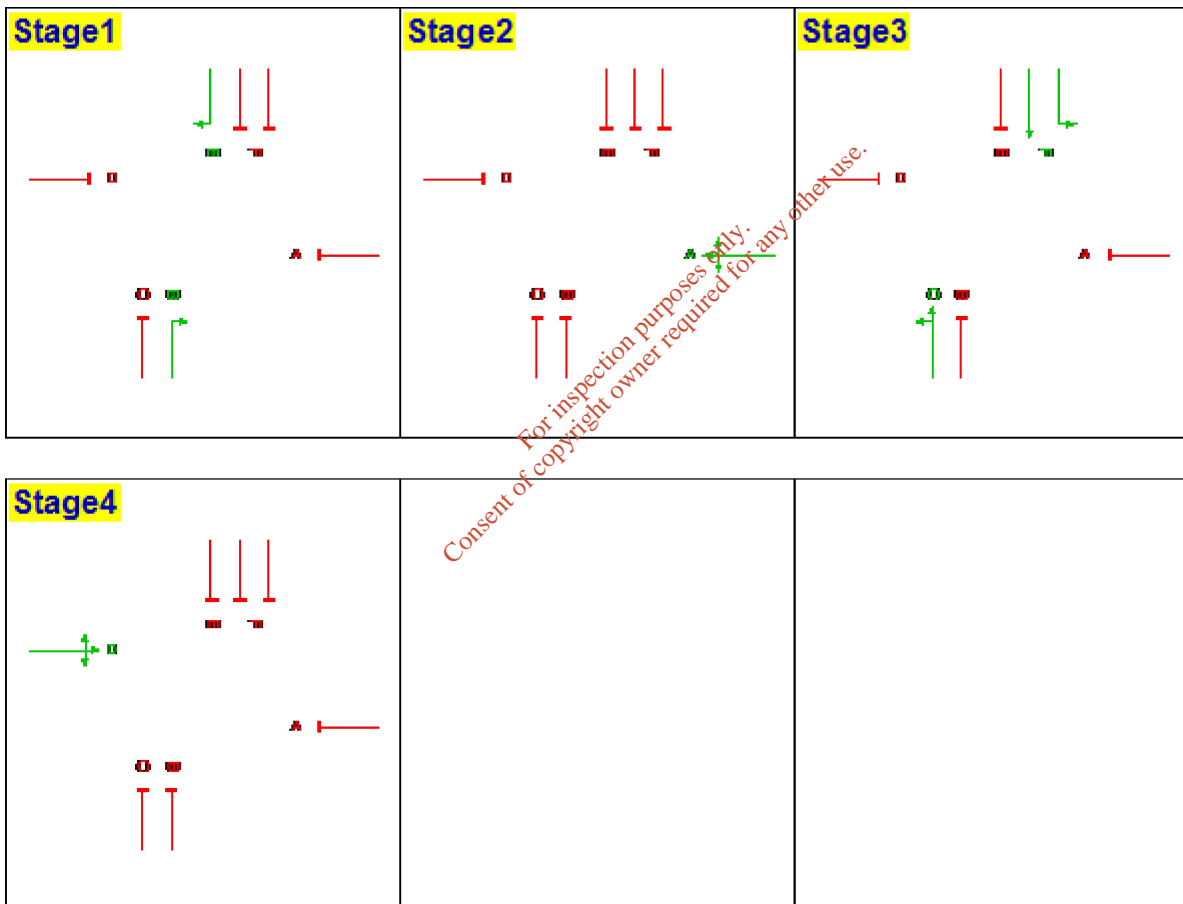
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	Yes	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak + Link	No	07:15	08:45	ODTAB	No	D1

Demand Set2 - 2017 AM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	102	333	94
	Arm 2	89	-	226	78
	Arm 3	164	112	-	11
	Arm 4	236	138	30	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	397	474	580	580	474	397
2 - R135 South	1	C	230	275	336	336	275	230
2 - R135 South	2	B	68	81	99	99	81	68
3 - L3125 West	1	D	215	257	315	315	257	215
4 - R135 North	1	F	279	333	408	408	333	279
4 - R135 North	2	E	21	25	31	31	25	21

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	19	63	18
2 - R135 South	58	20	23
3 - L3125 West	4	57	39
4 - R135 North	58	34	7

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	21.02	20.71	20.71	68.8

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	0.66	22.10	22.10	66.20

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

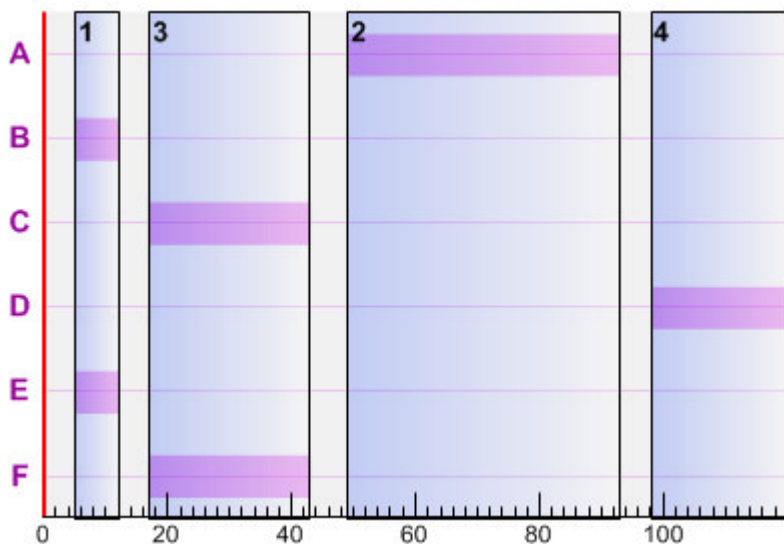
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	26.0	43.0
2	49.0	44.0	93.0
4	98.0	22.0	0.0

Phase Timings

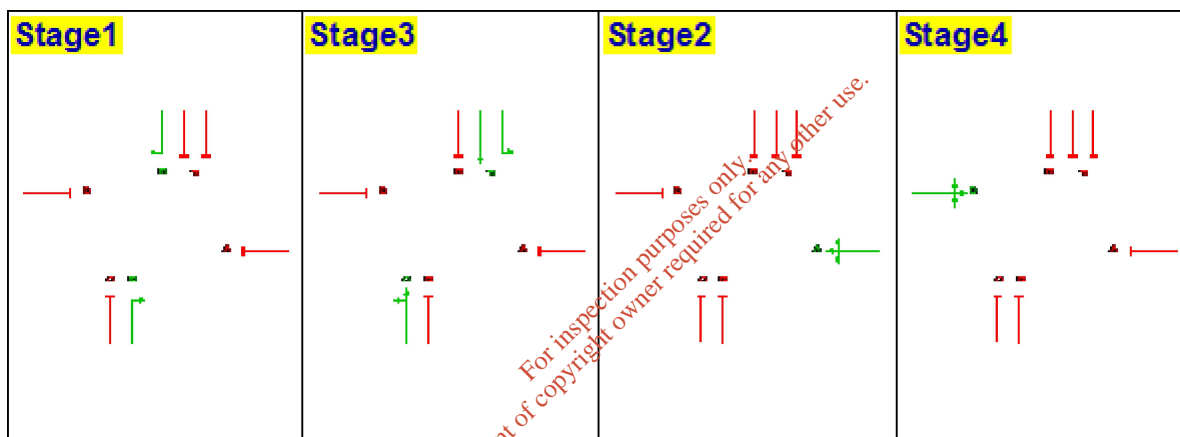
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	49	44.0	93						
B	5	7.0	12						
C	17	26.0	43						
D	98	22.0	0						
E	5	7.0	12						
F	17	26.0	43						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	484	A	45.50	40.25	5.41	71.27	26.28	1.28	12.37	11.09	28.00
2	1	280	C	27.50	56.86	4.42	70.87	26.99	1.19	8.81	7.62	10.30
2	2	83	B	8.50	83.26	1.92	62.73	43.48	0.69	3.31	2.61	1.70
3	1	262	D	23.50	64.32	4.68	74.37	21.02	1.48	8.89	7.41	8.00
4	1	340	F	27.50	40.93	3.87	39.20	129.60	0.18	9.55	9.37	20.00
4	2	26	E	8.50	56.91	0.41	19.65	358.02	0.03	0.84	0.81	0.80

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	397	1	45.50	34.46	3.80	58.46	53.95	0.58	9.50	8.92	4.90
2	1	230	3	27.50	49.03	3.13	58.22	54.60	0.54	6.74	6.19	1.80
2	2	68	2	8.50	70.67	1.33	51.39	75.12	0.34	2.48	2.14	0.30
3	1	215	4	23.50	53.84	3.22	61.03	47.48	0.63	6.66	6.02	1.40
4	1	279	6	27.50	39.78	3.08	32.17	179.81	0.11	7.70	7.59	2.90
4	2	21	5	8.50	55.72	0.33	15.87	467.07	0.02	0.67	0.65	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	474	1	45.50	39.22	5.16	69.80	28.94	1.13	11.97	10.84	4.70
2	1	275	3	27.50	55.33	4.23	69.61	29.30	1.05	8.53	7.48	1.70
2	2	81	2	8.50	79.92	1.80	61.22	47.02	0.59	3.14	2.55	0.30
3	1	257	4	23.50	61.86	4.42	72.95	23.37	1.26	8.52	7.26	1.30
4	1	333	6	27.50	40.79	3.77	38.39	134.43	0.17	9.34	9.17	3.30
4	2	25	5	8.50	56.66	0.39	18.89	376.34	0.03	0.81	0.78	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	580	1	45.50	51.19	8.23	85.41	5.38	3.23	16.81	13.58	3.60
2	1	336	3	27.50	70.51	6.58	85.05	5.83	2.77	12.02	9.26	1.40
2	2	99	2	8.50	96.58	2.66	74.82	20.29	1.21	4.34	3.13	0.30
3	1	315	4	23.50	82.71	7.24	89.41	0.66	3.64	12.65	9.01	1.10
4	1	408	6	27.50	42.37	4.80	47.04	91.34	0.30	11.71	11.41	3.70
4	2	31	5	8.50	58.11	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	580	1	45.50	53.57	8.63	85.41	5.38	3.37	16.95	13.58	3.60
2	1	336	3	27.50	75.27	7.03	85.05	5.83	2.95	12.21	9.26	1.40
2	2	99	2	8.50	103.04	2.83	74.82	20.29	1.29	4.42	3.13	0.30
3	1	315	4	23.50	92.68	8.11	89.41	0.66	4.07	13.09	9.01	1.10
4	1	408	6	27.50	42.39	4.80	47.04	91.34	0.30	11.71	11.41	3.70
4	2	31	5	8.50	58.15	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	474	1	45.50	40.49	5.33	69.80	28.94	1.22	12.06	10.84	4.70
2	1	275	3	27.50	58.58	4.48	69.61	29.30	1.18	8.66	7.48	1.70
2	2	81	2	8.50	86.95	1.96	61.22	47.02	0.71	3.26	2.55	0.30
3	1	257	4	23.50	69.05	4.93	72.95	23.37	1.49	8.75	7.26	1.30
4	1	333	6	27.50	40.81	3.77	38.39	134.43	0.17	9.34	9.17	3.30
4	2	25	5	8.50	56.72	0.39	18.89	376.34	0.03	0.81	0.78	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	397	1	45.50	34.79	3.84	58.46	53.95	0.60	9.52	8.92	4.90
2	1	230	3	27.50	49.97	3.19	58.22	54.60	0.58	6.77	6.19	1.80
2	2	68	2	8.50	74.51	1.41	51.39	75.12	0.38	2.52	2.14	0.30
3	1	215	4	23.50	55.50	3.31	61.03	47.48	0.68	6.71	6.02	1.40
4	1	279	6	27.50	39.79	3.08	32.17	179.81	0.11	7.70	7.59	2.90
4	2	21	5	8.50	55.81	0.33	15.87	467.07	0.02	0.67	0.65	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads.osc
Report generation date: 05/12/2016 16:43:17

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

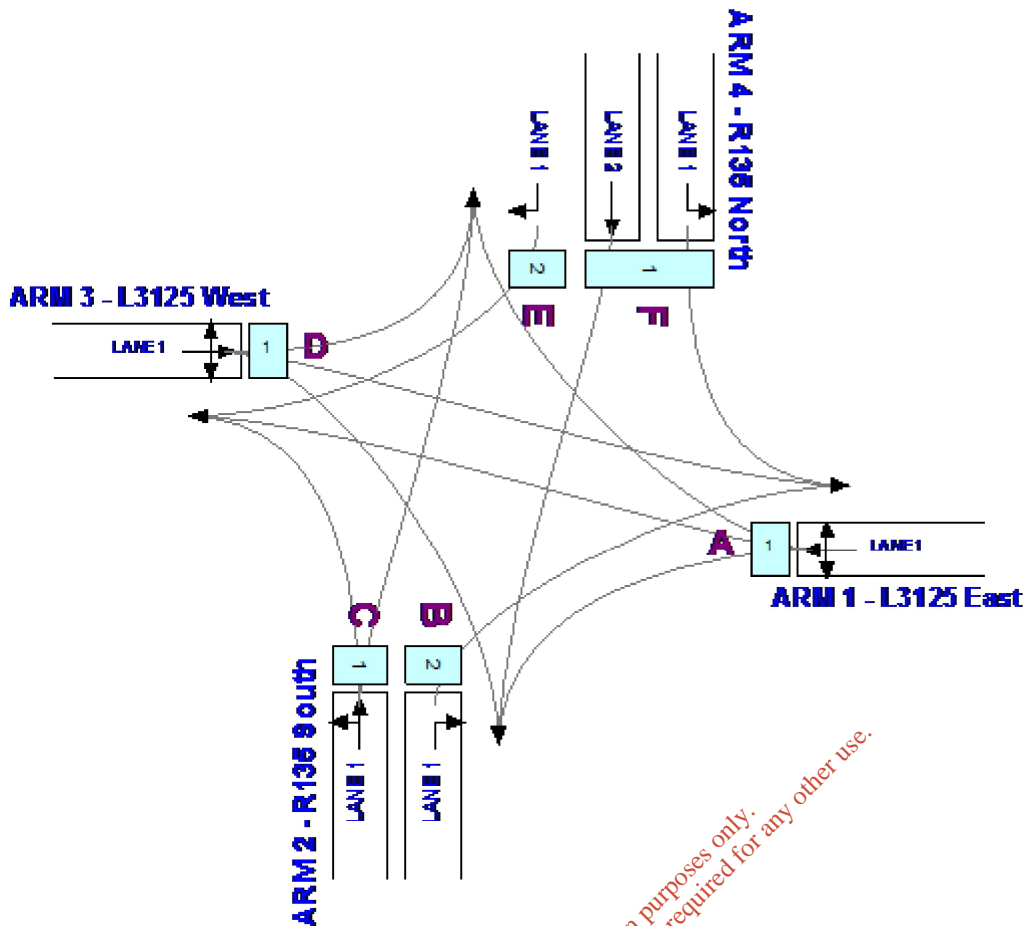
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

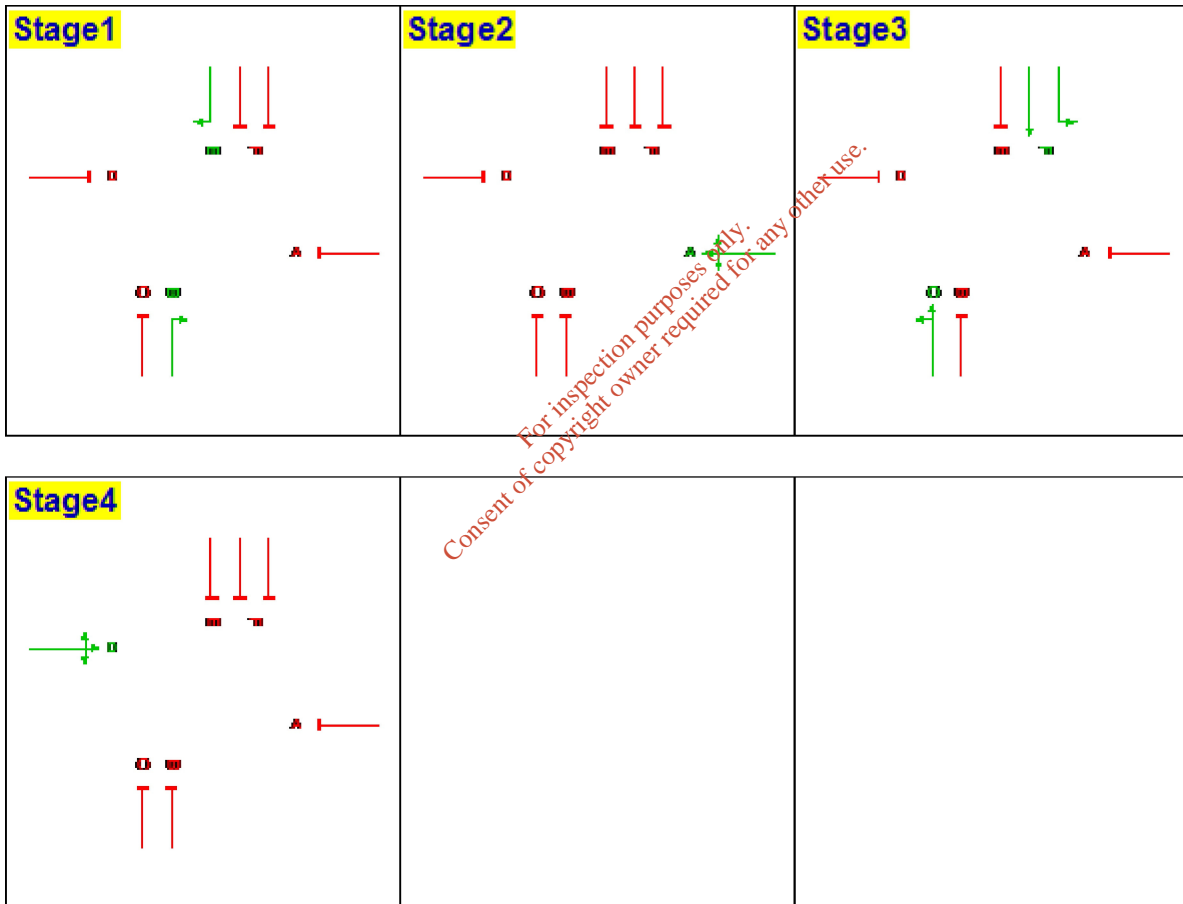
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	Yes	07:15	08:45	ODTAB	No	D1
2022 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak + Link	No	07:15	08:45	ODTAB	No	D1

Demand Set3 - 2017 AM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	102	333	94
	Arm 2	89	-	226	79
	Arm 3	164	112	-	11
	Arm 4	236	139	30	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	397	474	580	580	474	397
2 - R135 South	1	C	228	272	333	333	272	228
2 - R135 South	2	B	68	81	99	99	81	68
3 - L3125 West	1	D	215	257	315	315	257	215
4 - R135 North	1	F	279	334	409	409	334	279
4 - R135 North	2	E	21	25	31	31	25	21

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	19	63	18
2 - R135 South	57	20	23
3 - L3125 West	4	57	39
4 - R135 North	58	34	7

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	23.25	20.72	20.72	67.9

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	2.90	22.02	22.02	65.40

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

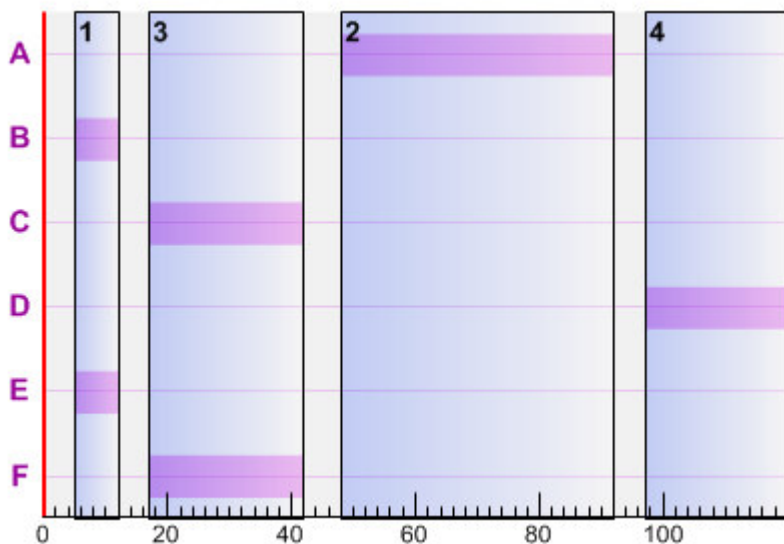
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	25.0	42.0
2	48.0	44.0	92.0
4	97.0	23.0	0.0

Phase Timings

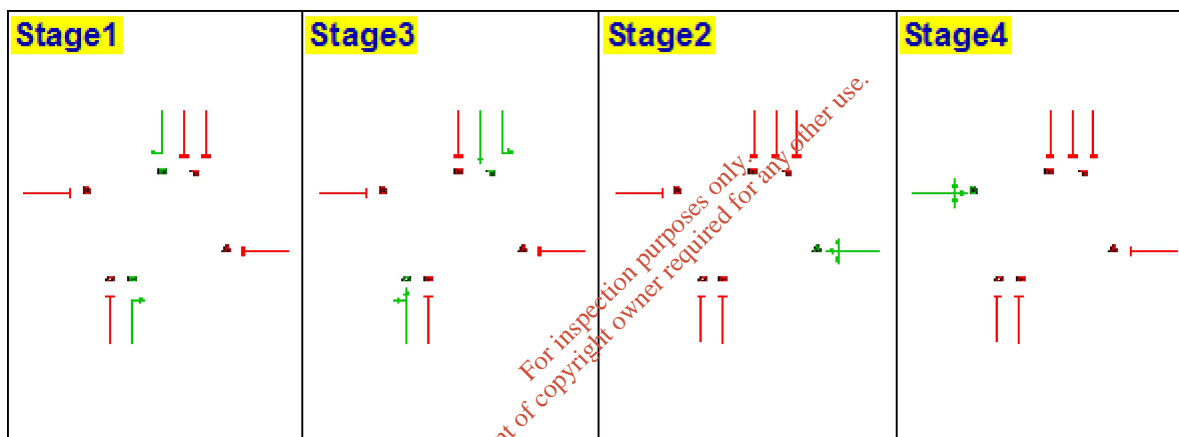
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	48	44.0	92						
B	5	7.0	12						
C	17	25.0	42						
D	97	23.0	0						
E	5	7.0	12						
F	17	25.0	42						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	484	A	45.50	40.25	5.41	71.27	26.28	1.28	12.37	11.09	28.00
2	1	278	C	26.50	59.84	4.62	73.02	23.25	1.36	9.01	7.65	9.50
2	2	83	B	8.50	83.26	1.92	62.73	43.48	0.69	3.31	2.61	1.70
3	1	262	D	24.50	60.16	4.38	71.33	26.17	1.22	8.55	7.33	8.80
4	1	341	F	26.50	41.99	3.98	40.80	120.61	0.20	9.70	9.50	19.10
4	2	26	E	8.50	56.91	0.41	19.65	358.02	0.03	0.84	0.81	0.80

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	397	1	45.50	34.46	3.80	58.46	53.95	0.58	9.50	8.92	4.90
2	1	228	3	26.50	50.69	3.21	59.89	50.28	0.60	6.80	6.20	1.70
2	2	68	2	8.50	70.67	1.33	51.39	75.12	0.34	2.48	2.14	0.30
3	1	215	4	24.50	51.72	3.09	58.54	53.75	0.55	6.51	5.96	1.50
4	1	279	6	26.50	40.74	3.16	33.38	169.63	0.12	7.79	7.67	2.80
4	2	21	5	8.50	55.72	0.33	15.87	467.07	0.02	0.67	0.65	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	474	1	45.50	39.22	5.16	69.80	28.94	1.13	11.97	10.84	4.70
2	1	272	3	26.50	57.73	4.36	71.44	25.97	1.16	8.63	7.47	1.60
2	2	81	2	8.50	79.92	1.80	61.22	47.02	0.59	3.14	2.55	0.30
3	1	257	4	24.50	58.39	4.17	69.97	28.62	1.06	8.25	7.19	1.50
4	1	334	6	26.50	41.83	3.88	39.96	125.23	0.19	9.48	9.30	3.10
4	2	25	5	8.50	56.66	0.39	18.89	376.34	0.03	0.81	0.78	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	580	1	45.50	51.19	8.22	85.41	5.38	3.23	16.81	13.58	3.60
2	1	333	3	26.50	75.65	7.00	87.47	2.90	3.24	12.51	9.27	1.30
2	2	99	2	8.50	96.58	2.66	74.82	20.29	1.21	4.34	3.13	0.30
3	1	315	4	24.50	74.87	6.55	85.76	4.94	2.85	11.77	8.92	1.20
4	1	409	6	26.50	43.54	4.95	48.93	83.93	0.33	11.90	11.57	3.50
4	2	31	5	8.50	58.11	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	580	1	45.50	53.57	8.63	85.41	5.38	3.37	16.95	13.58	3.60
2	1	333	3	26.50	82.67	7.65	87.47	2.90	3.54	12.81	9.27	1.30
2	2	99	2	8.50	103.04	2.83	74.82	20.29	1.29	4.42	3.13	0.30
3	1	315	4	24.50	80.68	7.06	85.76	4.94	3.08	11.99	8.92	1.20
4	1	409	6	26.50	43.56	4.95	48.93	83.93	0.33	11.90	11.57	3.50
4	2	31	5	8.50	58.15	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	474	1	45.50	40.49	5.33	69.80	28.94	1.22	12.06	10.84	4.70
2	1	272	3	26.50	62.50	4.72	71.44	25.97	1.34	8.81	7.47	1.60
2	2	81	2	8.50	86.95	1.96	61.22	47.02	0.71	3.26	2.55	0.30
3	1	257	4	24.50	62.42	4.46	69.97	28.62	1.21	8.40	7.19	1.50
4	1	334	6	26.50	41.86	3.88	39.96	125.23	0.19	9.48	9.30	3.10
4	2	25	5	8.50	56.72	0.39	18.89	376.34	0.03	0.81	0.78	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	397	1	45.50	34.79	3.84	58.46	53.95	0.60	9.52	8.92	4.90
2	1	228	3	26.50	51.91	3.29	59.89	50.28	0.64	6.84	6.20	1.70
2	2	68	2	8.50	74.51	1.41	51.39	75.12	0.38	2.52	2.14	0.30
3	1	215	4	24.50	52.84	3.16	58.54	53.75	0.59	6.55	5.96	1.50
4	1	279	6	26.50	40.75	3.16	33.38	169.63	0.12	7.79	7.67	2.80
4	2	21	5	8.50	55.81	0.33	15.87	467.07	0.02	0.67	0.65	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads.osc
Report generation date: 05/12/2016 16:43:49

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

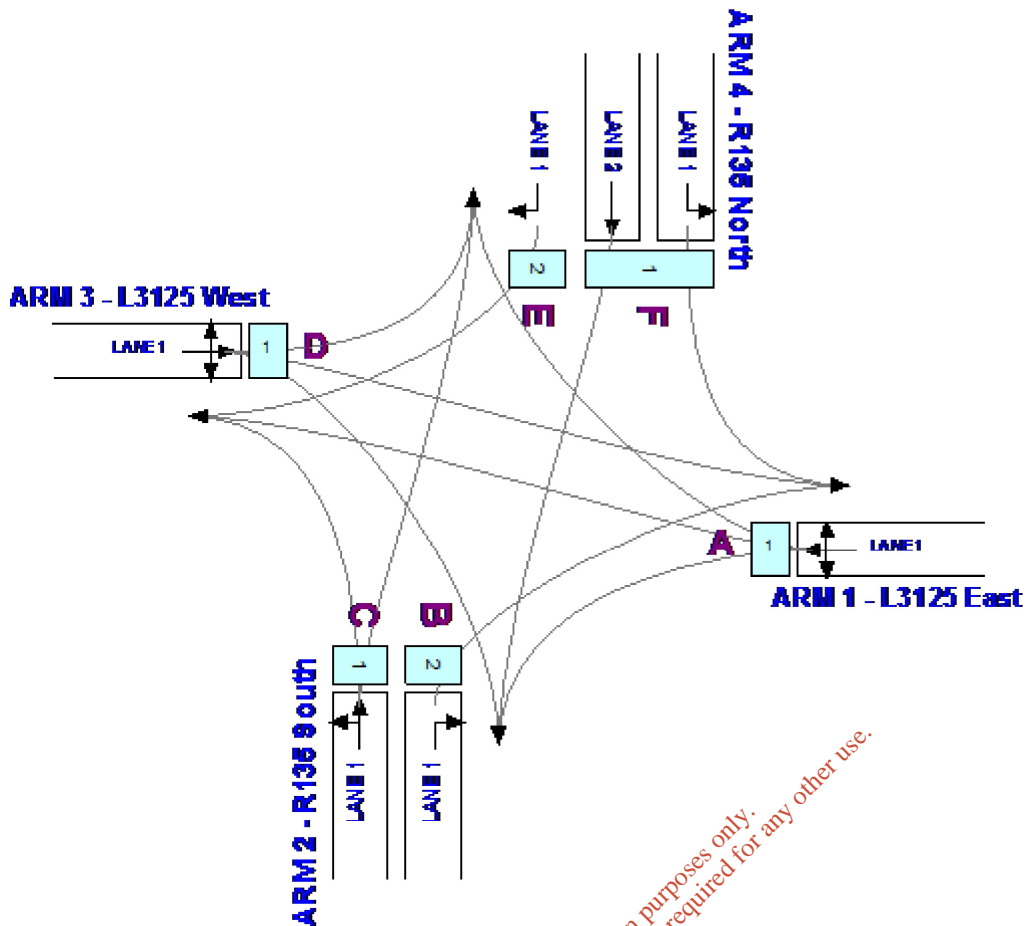
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

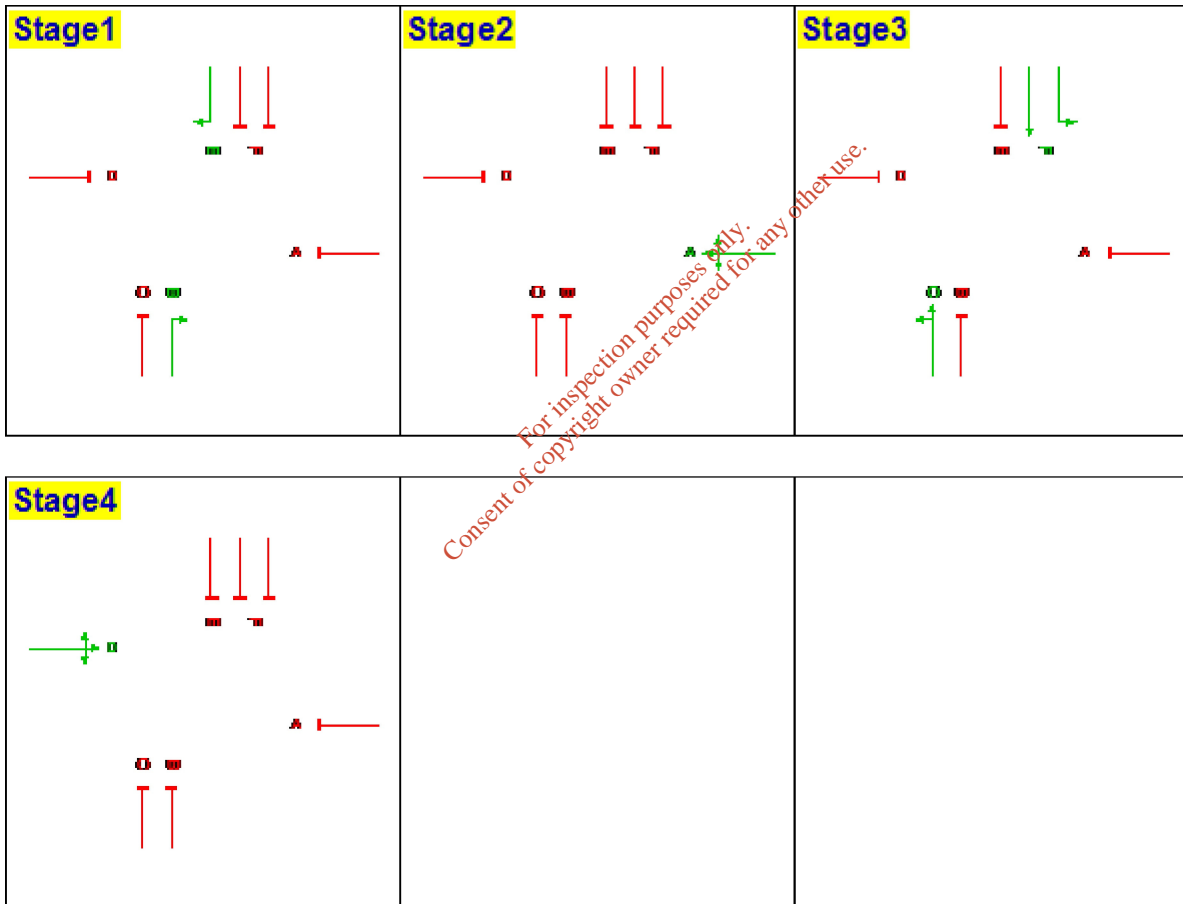
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak No Dev	Yes	07:15	08:45	ODTAB	No	D1
2022 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak + Link	No	07:15	08:45	ODTAB	No	D1

Demand Set4 - 2022 AM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	105	345	97
	Arm 2	92	-	234	80
	Arm 3	170	116	-	12
	Arm 4	244	143	31	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	410	490	600	600	490	410
2 - R135 South	1	C	238	284	347	347	284	238
2 - R135 South	2	B	70	84	102	102	84	70
3 - L3125 West	1	D	224	267	327	327	267	224
4 - R135 North	1	F	288	344	422	422	344	288
4 - R135 North	2	E	22	26	32	32	26	22

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	19	63	18
2 - R135 South	58	20	23
3 - L3125 West	4	57	39
4 - R135 North	58	34	7

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	19.55	22.04	22.04	67.3

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-0.38	23.85	23.85	64.50

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

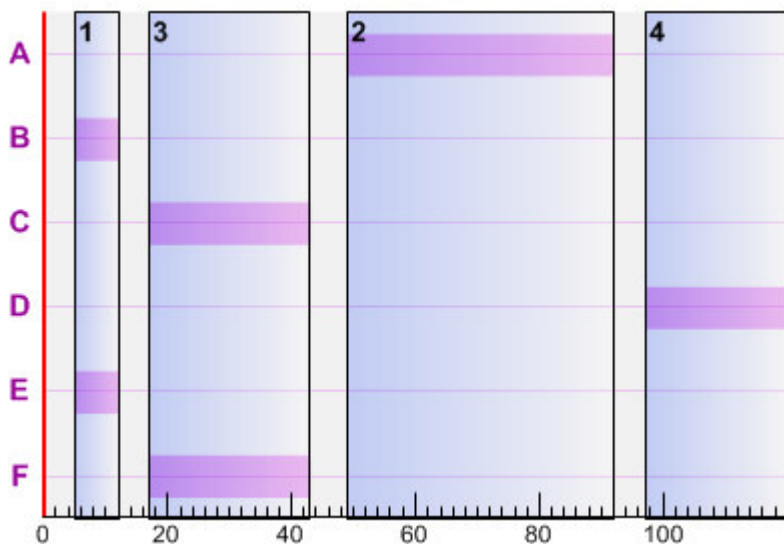
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	26.0	43.0
2	49.0	43.0	92.0
4	97.0	23.0	0.0

Phase Timings

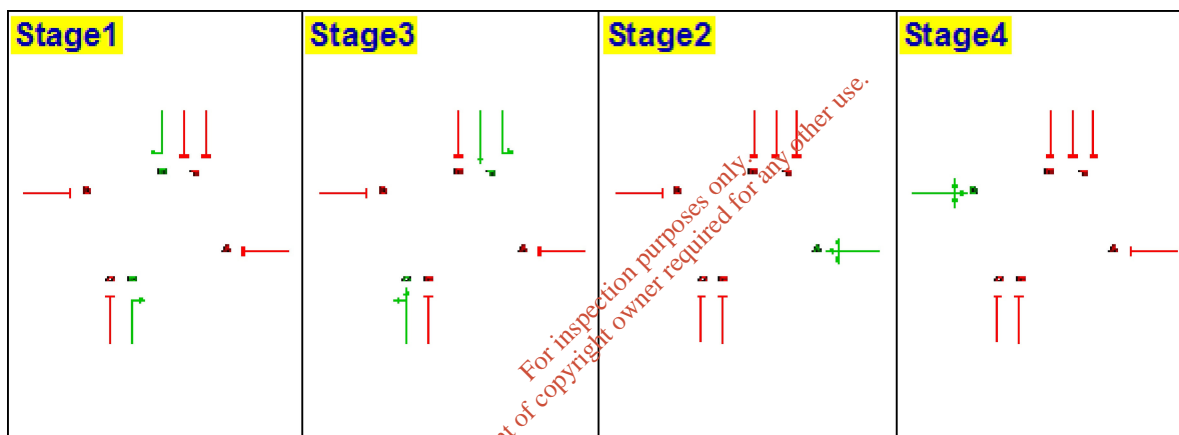
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	49	43.0	92						
B	5	7.0	12						
C	17	26.0	43						
D	97	23.0	0						
E	5	7.0	12						
F	17	26.0	43						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	500	A	44.50	43.63	6.06	75.28	19.55	1.66	13.32	11.65	25.60
2	1	290	C	27.50	58.99	4.75	73.40	22.61	1.40	9.31	7.91	10.10
2	2	85	B	8.50	85.16	2.01	64.24	40.10	0.76	3.44	2.68	1.80
3	1	273	D	24.50	62.92	4.77	74.33	21.09	1.48	9.14	7.66	8.60
4	1	351	F	27.50	41.15	4.01	40.47	122.41	0.20	9.89	9.70	20.40
4	2	27	E	8.50	57.15	0.43	20.41	341.06	0.03	0.88	0.84	0.80

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	410	1	44.50	36.32	4.14	61.73	45.79	0.70	10.07	9.37	4.70
2	1	238	3	27.50	49.89	3.30	60.24	49.40	0.61	7.03	6.42	1.80
2	2	70	2	8.50	71.63	1.39	52.90	70.12	0.37	2.57	2.20	0.30
3	1	224	4	24.50	52.79	3.28	60.99	47.57	0.63	6.85	6.22	1.50
4	1	288	6	27.50	39.94	3.20	33.20	171.06	0.12	7.97	7.85	3.00
4	2	22	5	8.50	55.94	0.34	16.63	441.30	0.02	0.71	0.68	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	490	1	44.50	42.21	5.75	73.78	21.99	1.45	12.84	11.39	4.30
2	1	284	3	27.50	56.94	4.49	71.88	25.20	1.20	8.94	7.74	1.70
2	2	84	2	8.50	82.11	1.92	63.48	41.77	0.67	3.31	2.65	0.30
3	1	267	4	24.50	60.43	4.48	72.69	23.81	1.25	8.73	7.48	1.40
4	1	344	6	27.50	41.01	3.92	39.66	126.93	0.18	9.68	9.49	3.40
4	2	26	5	8.50	56.89	0.41	19.65	358.02	0.03	0.84	0.81	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	600	1	44.50	59.00	9.83	90.34	-0.38	4.79	19.09	14.30	3.00
2	1	347	3	27.50	74.87	7.22	87.83	2.47	3.36	12.94	9.58	1.40
2	2	102	2	8.50	100.34	2.84	77.09	16.75	1.37	4.59	3.22	0.30
3	1	327	4	24.50	80.36	7.30	89.03	1.09	3.59	12.87	9.28	1.20
4	1	422	6	27.50	42.70	5.01	48.65	84.99	0.33	12.17	11.84	3.70
4	2	32	5	8.50	58.36	0.52	24.18	272.14	0.05	1.05	1.00	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	600	1	44.50	64.68	10.78	90.34	-0.38	5.22	19.52	14.30	2.90
2	1	347	3	27.50	81.93	7.90	87.83	2.47	3.68	13.26	9.58	1.30
2	2	102	2	8.50	108.27	3.07	77.09	16.75	1.47	4.70	3.22	0.30
3	1	327	4	24.50	89.45	8.12	89.03	1.09	3.99	13.27	9.28	1.10
4	1	422	6	27.50	42.72	5.01	48.65	84.99	0.33	12.17	11.84	3.70
4	2	32	5	8.50	58.43	0.52	24.18	272.14	0.05	1.05	1.00	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	490	1	44.50	45.02	6.13	73.78	21.99	1.60	12.99	11.39	4.30
2	1	284	3	27.50	61.69	4.87	71.88	25.20	1.38	9.11	7.74	1.70
2	2	84	2	8.50	90.97	2.12	63.48	41.77	0.82	3.47	2.65	0.30
3	1	267	4	24.50	66.78	4.95	72.69	23.81	1.46	8.94	7.48	1.40
4	1	344	6	27.50	41.03	3.92	39.66	126.93	0.19	9.68	9.49	3.40
4	2	26	5	8.50	56.98	0.41	19.65	358.02	0.03	0.84	0.81	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	410	1	44.50	36.83	4.19	61.73	45.79	0.73	10.10	9.37	4.70
2	1	238	3	27.50	51.07	3.38	60.24	49.40	0.65	7.07	6.42	1.80
2	2	70	2	8.50	76.22	1.48	52.90	70.12	0.42	2.62	2.20	0.30
3	1	224	4	24.50	54.28	3.38	60.99	47.57	0.68	6.90	6.22	1.50
4	1	288	6	27.50	39.95	3.20	33.20	171.06	0.12	7.97	7.85	3.00
4	2	22	5	8.50	56.02	0.34	16.63	441.30	0.02	0.71	0.68	0.10

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GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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Summary

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Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

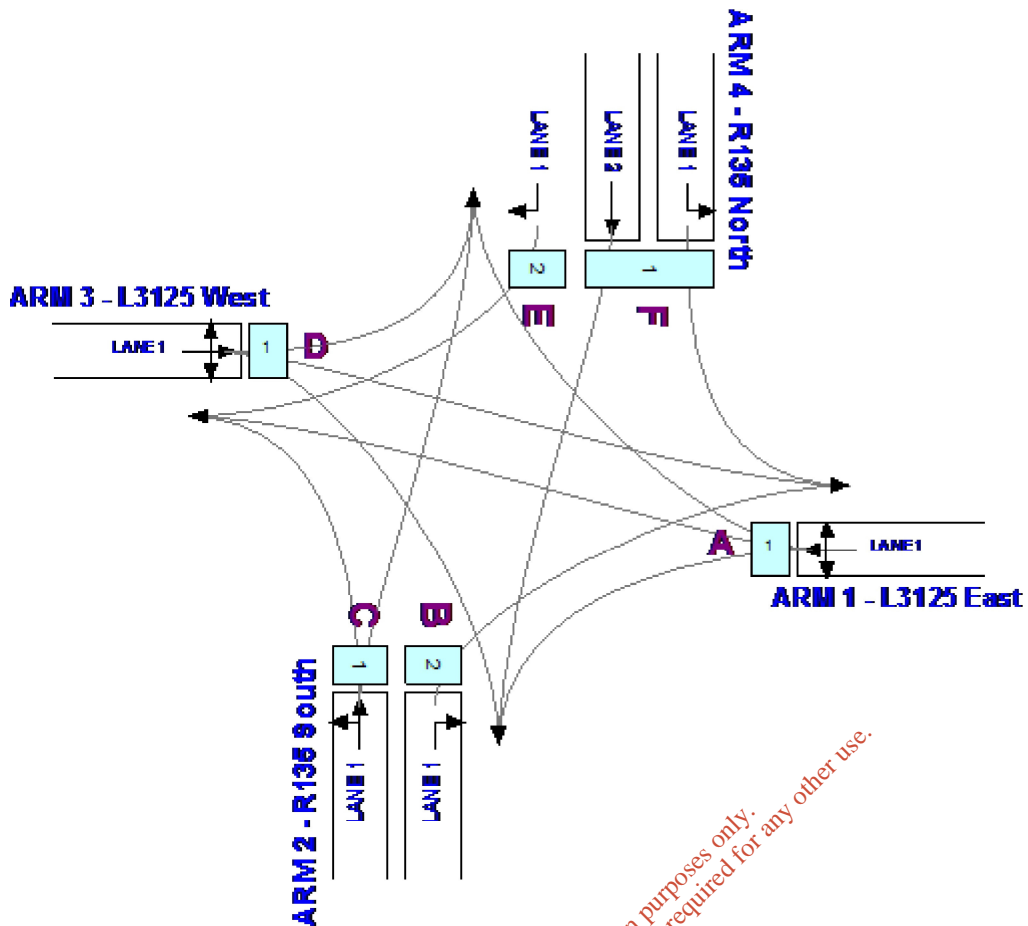
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

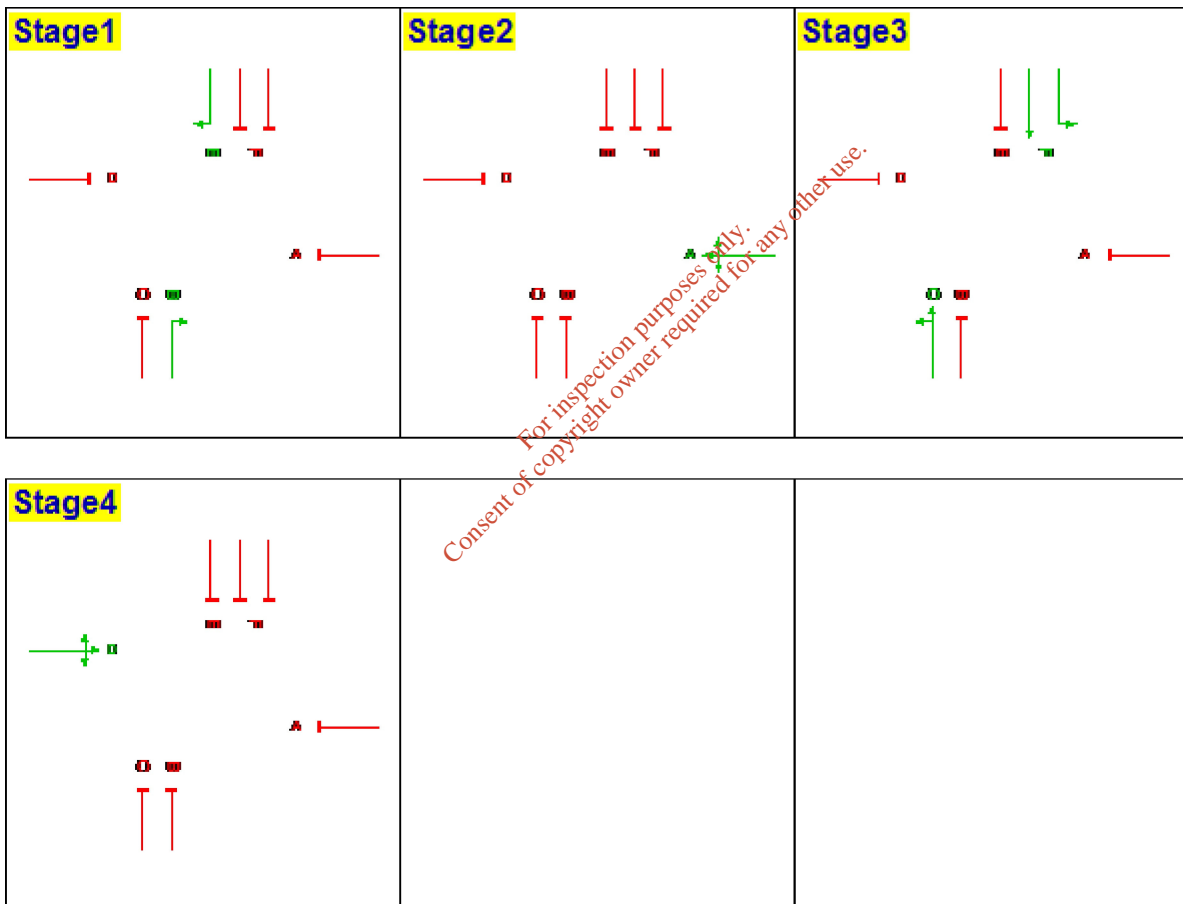
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2022 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2022 PM Peak With Dev	Yes	16:30	18:00	ODTAB	No	D1

Demand Set10 - 2022 PM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	96	232	227
	Arm 2	143	-	142	135
	Arm 3	188	184	-	25
	Arm 4	186	103	19	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	416	497	609	609	497	416
2 - R135 South	1	C	208	248	304	304	248	208
2 - R135 South	2	B	107	128	157	157	128	107
3 - L3125 West	1	D	295	352	431	431	352	295
4 - R135 North	1	F	215	257	314	314	257	215
4 - R135 North	2	E	14	17	20	20	17	14

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	34	32	34
3 - L3125 West	6	47	46
4 - R135 North	60	33	6

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	10.87	25.77	25.77	55.9

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-7.65	30.05	30.05	52.90

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

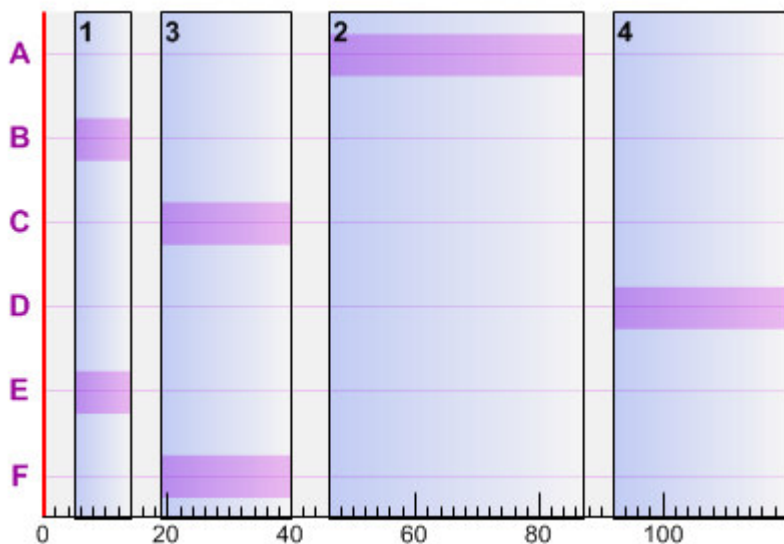
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	9.0	14.0
3	19.0	21.0	40.0
2	46.0	41.0	87.0
4	92.0	28.0	0.0

Phase Timings

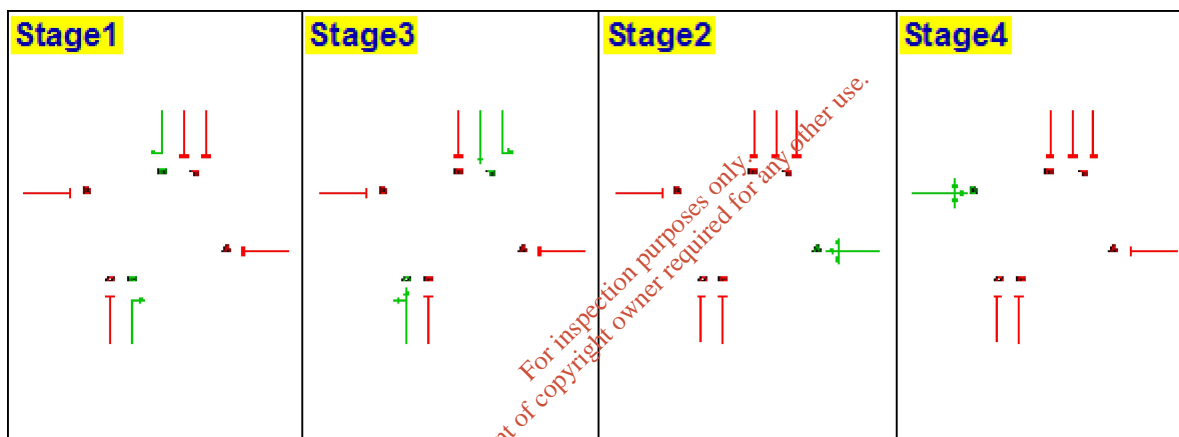
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	46	41.0	87						
B	5	9.0	14						
C	19	21.0	40						
D	92	28.0	0						
E	5	9.0	14						
F	19	21.0	40						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	507	A	42.50	49.36	6.95	79.93	12.60	2.30	14.45	12.15	21.70
2	1	253	C	22.50	71.56	5.03	78.27	14.99	1.91	9.13	7.22	6.90
2	2	131	B	10.50	106.06	3.86	80.15	12.29	2.06	6.15	4.09	2.40
3	1	359	D	29.50	64.65	6.45	81.18	10.87	2.42	12.15	9.72	10.90
4	1	262	F	22.50	44.51	3.24	36.92	143.79	0.15	7.64	7.49	13.40
4	2	17	E	10.50	52.10	0.25	10.40	765.32	0.01	0.53	0.52	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	416	1	42.50	39.33	4.54	65.58	37.23	0.87	10.64	9.77	4.20
2	1	208	3	22.50	57.04	3.30	64.35	39.87	0.76	6.64	5.88	1.20
2	2	107	2	10.50	75.67	2.25	65.46	37.48	0.75	4.08	3.33	0.40
3	1	295	4	29.50	50.68	4.15	66.70	34.92	0.90	8.78	7.88	2.10
4	1	215	6	22.50	43.44	2.59	30.30	197.08	0.09	6.18	6.08	2.00
4	2	14	5	10.50	51.69	0.20	8.57	950.75	0.01	0.43	0.43	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	42.50	47.06	6.50	78.35	14.87	1.93	13.81	11.88	3.70
2	1	248	3	22.50	67.45	4.65	76.72	17.31	1.56	8.63	7.07	1.20
2	2	128	2	10.50	94.15	3.35	78.31	14.93	1.52	5.52	4.00	0.40
3	1	352	4	29.50	60.76	5.94	79.59	13.08	1.97	11.48	9.52	1.90
4	1	257	6	22.50	44.39	3.17	36.21	148.53	0.14	7.48	7.34	2.20
4	2	17	5	10.50	52.09	0.25	10.40	765.32	0.01	0.53	0.52	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	609	1	42.50	72.29	12.23	96.01	-6.26	7.82	22.75	14.93	2.10
2	1	304	3	22.50	95.67	6.08	94.04	-4.30	4.92	13.69	8.77	0.90
2	2	157	2	10.50	133.33	5.81	96.05	-6.30	4.16	9.09	4.93	0.40
3	1	431	4	29.50	91.53	10.96	97.46	-7.65	7.38	19.24	11.86	1.20
4	1	314	6	22.50	45.83	4.00	44.24	103.41	0.25	9.32	9.07	2.50
4	2	20	5	10.50	52.52	0.29	12.24	635.52	0.01	0.62	0.61	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	609	1	42.50	87.90	14.87	96.01	-6.26	9.48	24.41	14.93	2.00
2	1	304	3	22.50	115.03	9.71	94.04	-4.30	5.92	14.68	8.77	0.90
2	2	157	2	10.50	169.38	7.39	96.05	-6.30	5.27	10.20	4.93	0.40
3	1	431	4	29.50	115.61	13.84	97.46	-7.65	9.39	21.24	11.86	1.10
4	1	314	6	22.50	45.85	4.00	44.24	103.41	0.25	9.32	9.07	2.50
4	2	20	5	10.50	52.52	0.29	12.24	635.52	0.01	0.62	0.61	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	42.50	56.04	7.74	78.35	14.87	2.25	14.13	11.88	3.60
2	1	248	3	22.50	84.27	5.80	76.72	17.31	1.98	9.04	7.07	1.10
2	2	128	2	10.50	144.60	5.14	78.31	14.93	2.36	6.36	4.00	0.40
3	1	352	4	29.50	81.15	7.94	79.59	13.08	2.49	12.01	9.52	1.80
4	1	257	6	22.50	44.41	3.17	36.21	148.53	0.14	7.48	7.34	2.20
4	2	17	5	10.50	52.11	0.25	10.40	765.32	0.01	0.53	0.52	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	416	1	42.50	40.27	4.65	65.58	37.23	0.93	10.69	9.77	4.20
2	1	208	3	22.50	60.06	3.47	64.35	39.87	0.84	6.72	5.88	1.20
2	2	107	2	10.50	89.98	2.67	65.46	37.48	0.92	4.25	3.33	0.40
3	1	295	4	29.50	52.97	4.34	66.70	34.92	0.98	8.86	7.88	2.10
4	1	215	6	22.50	43.45	2.60	30.30	197.08	0.09	6.18	6.08	2.00
4	2	14	5	10.50	51.71	0.20	8.57	950.75	0.01	0.43	0.43	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads.osc
Report generation date: 05/12/2016 16:44:59

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

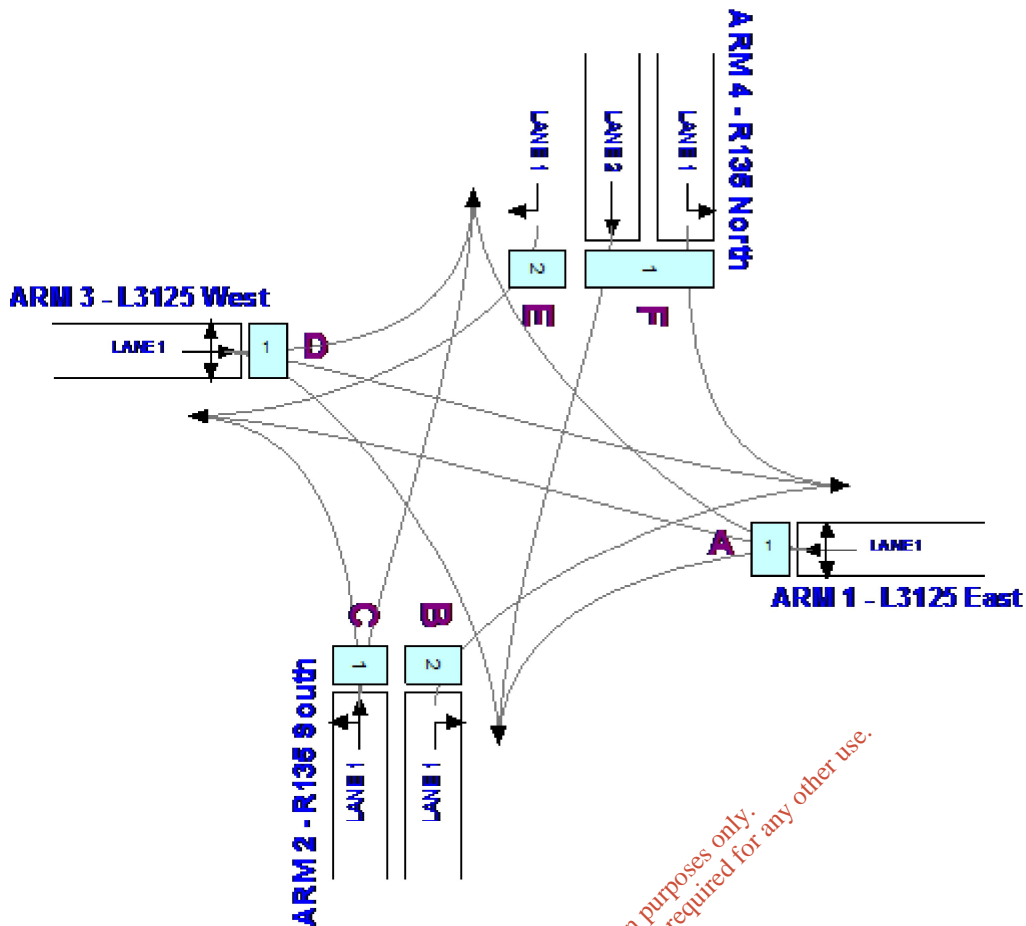
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

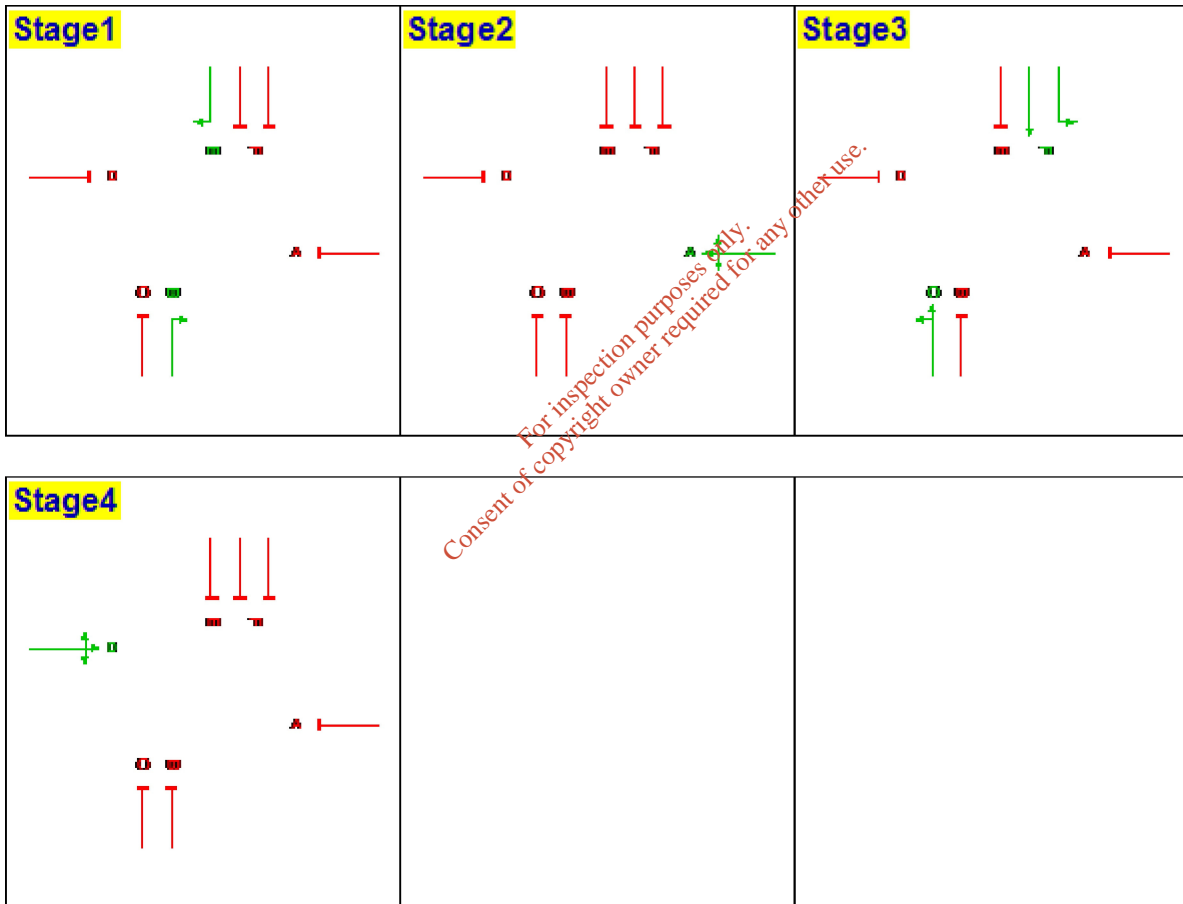
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak Sensitivity	Yes	07:15	08:45	ODTAB	No	D1
2032 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak + Link	No	07:15	08:45	ODTAB	No	D1

Demand Set6 - 2022 AM Peak Sensitivity

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	105	345	97
	Arm 2	92	-	234	103
	Arm 3	170	116	-	11
	Arm 4	244	166	31	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	410	490	600	600	490	410
2 - R135 South	1	C	254	304	372	372	304	254
2 - R135 South	2	B	68	81	99	99	81	68
3 - L3125 West	1	D	223	266	326	326	266	223
4 - R135 North	1	F	308	367	450	450	367	308
4 - R135 North	2	E	23	28	34	34	28	23

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	19	63	18
2 - R135 South	55	24	21
3 - L3125 West	4	57	39
4 - R135 North	55	38	7

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	16.57	22.93	22.93	68.5

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-2.74	25.12	25.12	65.70

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

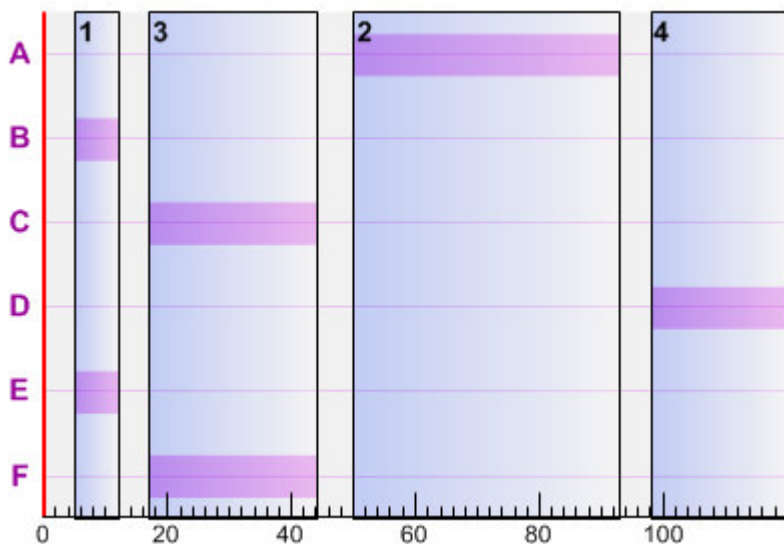
Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	27.0	44.0
2	50.0	43.0	93.0
4	98.0	22.0	0.0

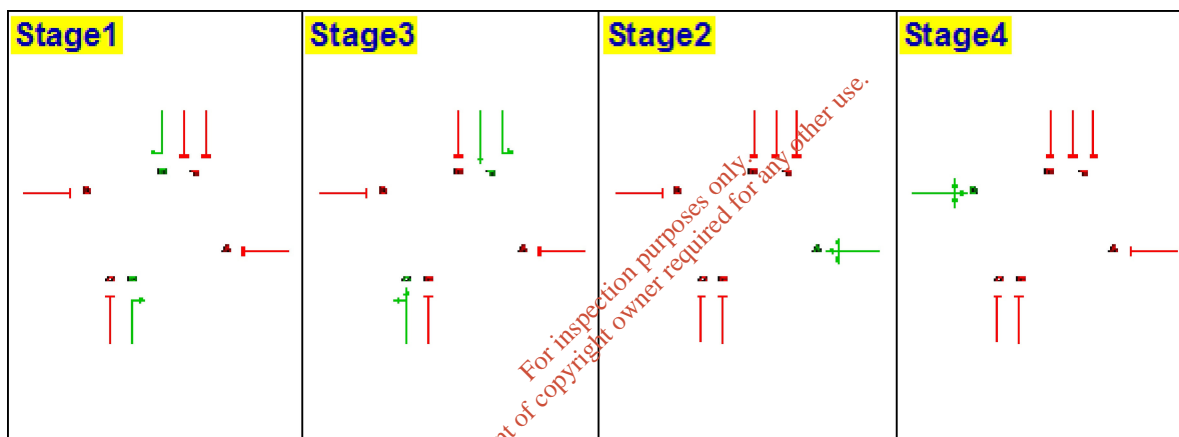
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	50	43.0	93						
B	5	7.0	12						
C	17	27.0	44						
D	98	22.0	0						
E	5	7.0	12						
F	17	27.0	44						

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	500	A	44.50	43.63	6.06	75.28	19.55	1.66	13.32	11.65	25.60
2	1	310	C	28.50	60.03	5.17	75.71	18.87	1.63	10.03	8.40	10.50
2	2	83	B	8.50	83.26	1.92	62.73	43.48	0.69	3.31	2.61	1.70
3	1	272	D	23.50	67.59	5.11	77.21	16.57	1.79	9.50	7.71	7.80
4	1	375	F	28.50	40.61	4.23	41.72	115.74	0.21	10.52	10.30	22.10
4	2	28	E	8.50	57.39	0.45	21.16	325.30	0.04	0.91	0.87	0.80

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	410	1	44.50	36.32	4.14	61.73	45.79	0.70	10.07	9.37	4.70
2	1	254	3	28.50	49.74	3.51	62.03	45.08	0.68	7.48	6.80	1.90
2	2	68	2	8.50	70.67	1.33	51.39	75.12	0.34	2.48	2.14	0.30
3	1	223	4	23.50	54.94	3.40	63.30	42.19	0.72	6.98	6.26	1.40
4	1	308	6	28.50	39.34	3.37	34.26	162.68	0.13	8.47	8.34	3.30
4	2	23	5	8.50	56.16	0.36	17.38	417.76	0.02	0.74	0.72	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	490	1	44.50	42.21	5.75	73.78	21.99	1.45	12.84	11.39	4.30
2	1	304	3	28.50	57.65	4.87	74.25	21.22	1.39	9.62	8.23	1.80
2	2	81	2	8.50	79.92	1.80	61.22	47.02	0.59	3.14	2.55	0.30
3	1	266	4	23.50	64.12	4.74	75.50	19.20	1.47	9.00	7.53	1.30
4	1	367	6	28.50	40.45	4.12	40.83	120.45	0.20	10.27	10.07	3.60
4	2	28	5	8.50	57.36	0.45	21.16	325.30	0.04	0.91	0.87	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	600	1	44.50	59.00	9.83	90.34	-0.38	4.79	19.09	14.30	3.00
2	1	372	3	28.50	78.94	6.16	90.85	-0.94	4.25	14.46	10.21	1.30
2	2	99	2	8.50	96.58	2.66	74.82	20.29	1.21	4.34	3.13	0.30
3	1	326	4	23.50	89.08	8.07	92.53	-2.74	4.56	13.91	9.35	1.00
4	1	450	6	28.50	42.22	5.28	50.06	79.79	0.36	12.93	12.57	4.00
4	2	34	5	8.50	58.88	0.56	25.70	250.25	0.06	1.12	1.06	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	600	1	44.50	64.68	10.78	90.34	-0.38	5.22	19.52	14.30	2.90
2	1	372	3	28.50	89.37	9.24	90.85	-0.94	4.80	15.02	10.21	1.30
2	2	99	2	8.50	103.04	2.83	74.82	20.29	1.29	4.42	3.13	0.30
3	1	326	4	23.50	104.14	9.43	92.53	-2.74	5.33	14.68	9.35	1.00
4	1	450	6	28.50	42.24	5.28	50.06	79.79	0.36	12.93	12.57	4.00
4	2	34	5	8.50	58.94	0.56	25.70	250.25	0.06	1.12	1.06	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	490	1	44.50	45.02	6.13	73.78	21.99	1.60	12.99	11.39	4.30
2	1	304	3	28.50	64.68	5.46	74.25	21.22	1.63	9.86	8.23	1.80
2	2	81	2	8.50	86.95	1.96	61.22	47.02	0.71	3.26	2.55	0.30
3	1	266	4	23.50	75.73	5.60	75.50	19.20	1.79	9.32	7.53	1.30
4	1	367	6	28.50	40.47	4.13	40.83	120.45	0.20	10.27	10.07	3.60
4	2	28	5	8.50	57.45	0.45	21.16	325.30	0.04	0.91	0.87	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	410	1	44.50	36.83	4.19	61.73	45.79	0.73	10.10	9.37	4.70
2	1	254	3	28.50	51.15	3.61	62.03	45.08	0.73	7.53	6.80	1.90
2	2	68	2	8.50	74.51	1.41	51.39	75.12	0.38	2.52	2.14	0.30
3	1	223	4	23.50	57.14	3.54	63.30	42.19	0.79	7.04	6.26	1.40
4	1	308	6	28.50	39.36	3.37	34.26	162.68	0.13	8.47	8.34	3.30
4	2	23	5	8.50	56.24	0.36	17.38	417.76	0.02	0.74	0.72	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads.osc
Report generation date: 05/12/2016 16:48:19

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

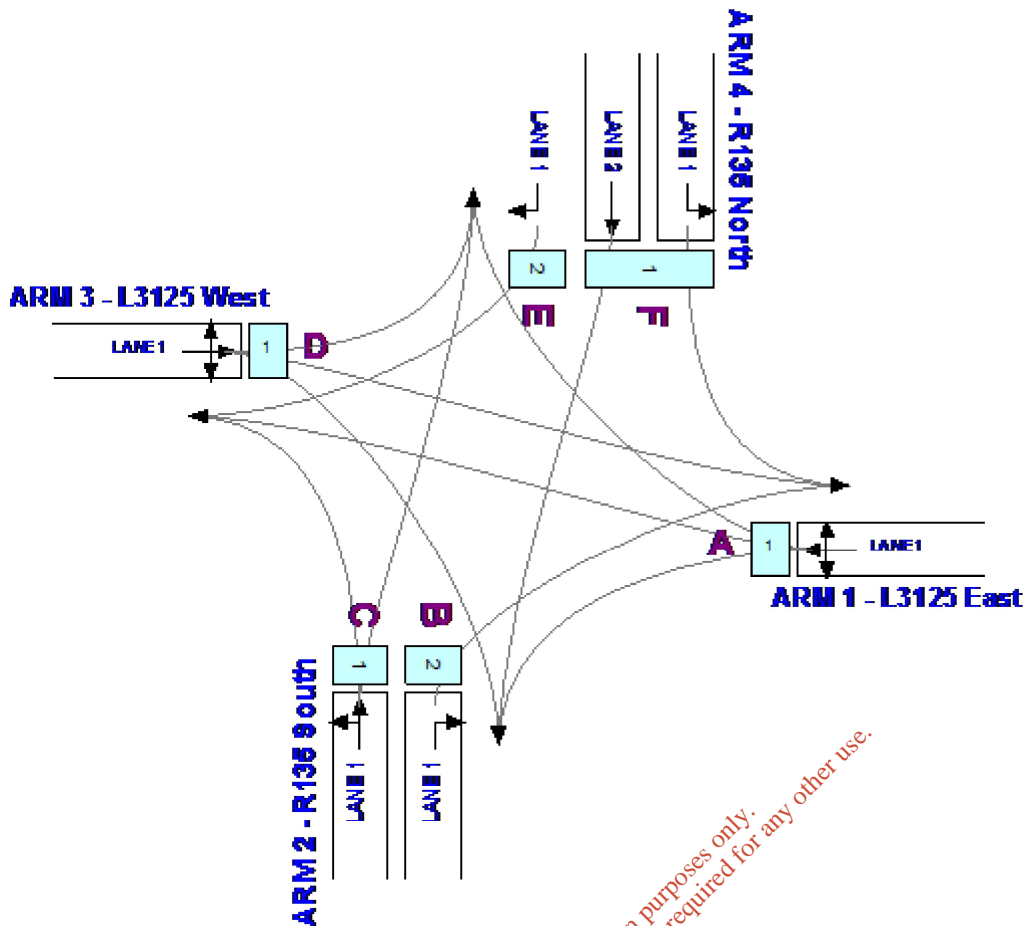
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

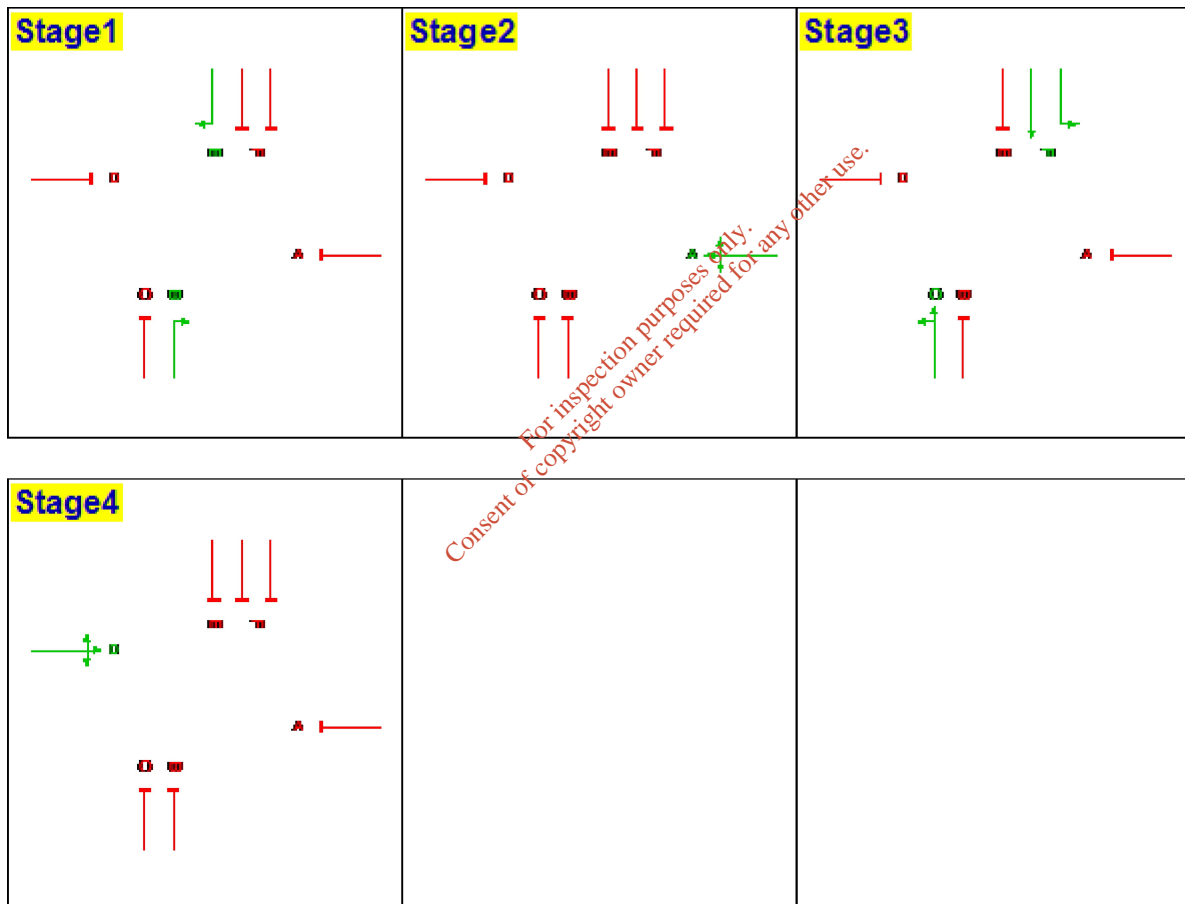
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak + Link	Yes	07:15	08:45	ODTAB	No	D1

Demand Set10 - 2022 AM Peak + Link

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	105	345	48
	Arm 2	92	-	234	81
	Arm 3	170	116	-	11
	Arm 4	122	144	31	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	374	446	546	546	446	374
2 - R135 South	1	C	235	281	344	344	281	235
2 - R135 South	2	B	70	84	103	103	84	70
3 - L3125 West	1	D	223	266	326	326	266	223
4 - R135 North	1	F	198	237	290	290	237	198
4 - R135 North	2	E	22	27	33	33	27	22

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	21	69	10
2 - R135 South	57	20	23
3 - L3125 West	4	57	39
4 - R135 North	41	48	10

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	25.47	19.32	19.32	63.8

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	4.56	20.53	20.53	61.60

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

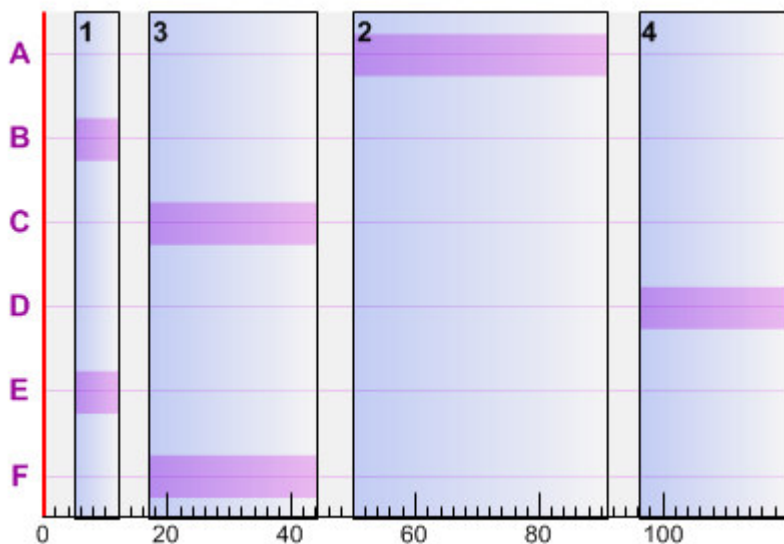
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	27.0	44.0
2	50.0	41.0	91.0
4	96.0	24.0	0.0

Phase Timings

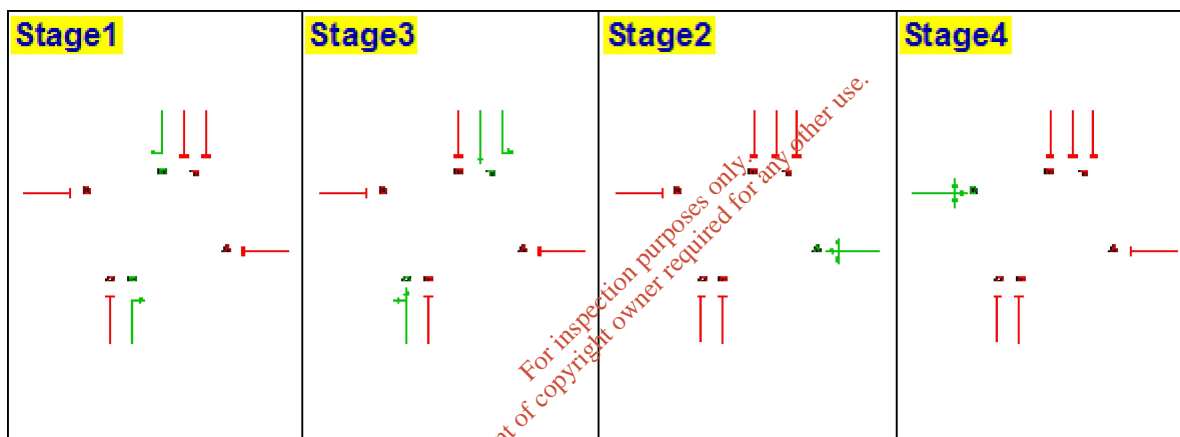
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	50	41.0	91						
B	5	7.0	12						
C	17	27.0	44						
D	96	24.0	0						
E	5	7.0	12						
F	17	27.0	44						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	455	A	42.50	42.91	5.42	71.73	25.47	1.31	12.09	10.78	24.20
2	1	287	C	28.50	55.16	4.40	70.09	28.40	1.14	8.88	7.74	11.00
2	2	86	B	8.50	86.16	2.06	65.00	38.47	0.79	3.50	2.71	1.80
3	1	272	D	25.50	58.79	4.44	71.15	26.49	1.21	8.76	7.55	9.40
4	1	242	F	28.50	38.24	2.57	26.92	234.32	0.07	6.53	6.46	16.60
4	2	27	E	8.50	57.15	0.43	20.41	341.06	0.03	0.88	0.84	0.80

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	374	1	42.50	36.80	3.82	58.96	52.64	0.59	9.29	8.70	4.20
2	1	235	3	28.50	47.79	3.12	57.39	56.81	0.52	6.79	6.27	1.90
2	2	70	2	8.50	71.63	1.39	52.90	70.12	0.37	2.57	2.20	0.30
3	1	223	4	25.50	50.65	3.14	58.33	54.29	0.55	6.67	6.13	1.60
4	1	198	6	28.50	37.55	2.07	22.03	308.61	0.04	5.28	5.24	2.40
4	2	22	5	8.50	55.94	0.34	16.63	441.30	0.02	0.71	0.68	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	446	1	42.50	41.82	5.18	70.31	28.00	1.16	11.70	10.54	4.10
2	1	281	3	28.50	53.66	4.19	68.63	31.14	0.99	8.56	7.57	1.90
2	2	84	2	8.50	82.11	1.92	63.48	41.77	0.67	3.31	2.65	0.30
3	1	266	4	25.50	56.98	4.21	69.58	29.35	1.04	8.42	7.37	1.60
4	1	237	6	28.50	38.16	2.51	26.36	241.37	0.07	6.39	6.32	2.70
4	2	27	5	8.50	57.12	0.43	20.41	341.06	0.03	0.88	0.84	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	546	1	42.50	54.72	8.30	86.08	4.56	3.34	16.54	13.20	3.10
2	1	344	3	28.50	67.74	6.47	84.02	7.12	2.60	11.99	9.39	1.60
2	2	103	2	8.50	101.40	2.90	77.84	15.62	1.42	4.68	3.26	0.30
3	1	326	4	25.50	72.67	6.58	85.28	5.54	2.79	11.94	9.15	1.30
4	1	290	6	28.50	39.03	3.14	32.26	178.98	0.11	7.93	7.82	3.20
4	2	33	5	8.50	58.62	0.54	24.94	260.86	0.05	1.08	1.03	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	546	1	42.50	57.65	8.74	86.08	4.56	3.52	16.71	13.20	3.10
2	1	344	3	28.50	71.71	6.85	84.02	7.12	2.75	12.14	9.39	1.50
2	2	103	2	8.50	110.15	3.15	77.84	15.62	1.54	4.80	3.26	0.30
3	1	326	4	25.50	77.85	7.05	85.28	5.54	2.99	12.14	9.15	1.30
4	1	290	6	28.50	39.04	3.14	32.26	178.98	0.11	7.93	7.82	3.20
4	2	33	5	8.50	58.68	0.54	24.94	260.86	0.05	1.08	1.03	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	446	1	42.50	43.40	5.38	70.31	28.00	1.26	11.80	10.54	4.10
2	1	281	3	28.50	56.33	4.40	68.63	31.14	1.11	8.67	7.57	1.80
2	2	84	2	8.50	91.58	2.14	63.48	41.77	0.82	3.47	2.65	0.30
3	1	266	4	25.50	60.51	4.47	69.58	29.35	1.18	8.55	7.37	1.60
4	1	237	6	28.50	38.16	2.51	26.36	241.37	0.07	6.39	6.32	2.70
4	2	27	5	8.50	57.21	0.43	20.41	341.06	0.04	0.88	0.84	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	374	1	42.50	37.20	3.86	58.96	52.64	0.62	9.32	8.70	4.20
2	1	235	3	28.50	48.59	3.17	57.39	56.81	0.55	6.82	6.27	1.90
2	2	70	2	8.50	76.22	1.48	52.90	70.12	0.42	2.62	2.20	0.30
3	1	223	4	25.50	51.66	3.20	58.33	54.29	0.58	6.71	6.13	1.60
4	1	198	6	28.50	37.56	2.07	22.03	308.61	0.04	5.28	5.24	2.40
4	2	22	5	8.50	56.02	0.34	16.63	441.30	0.02	0.71	0.68	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads AM.osc
Report generation date: 09/12/2016 11:15:46

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

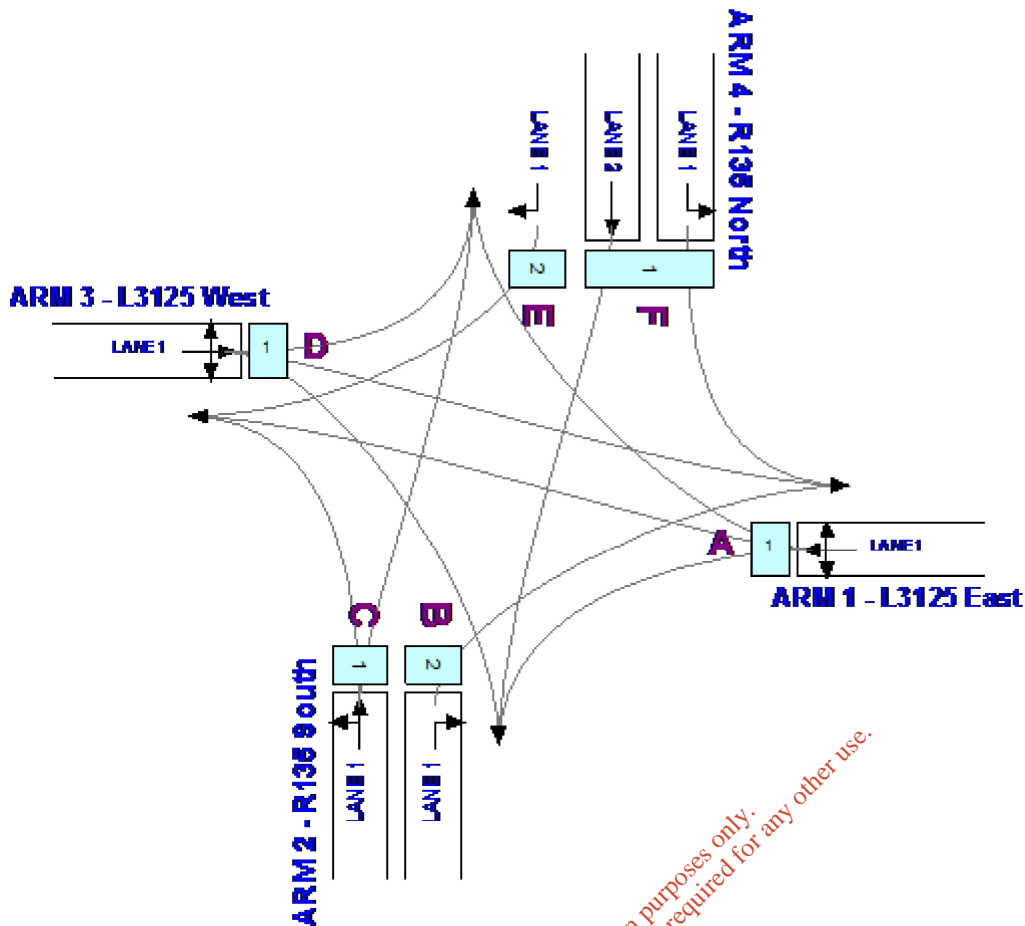
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

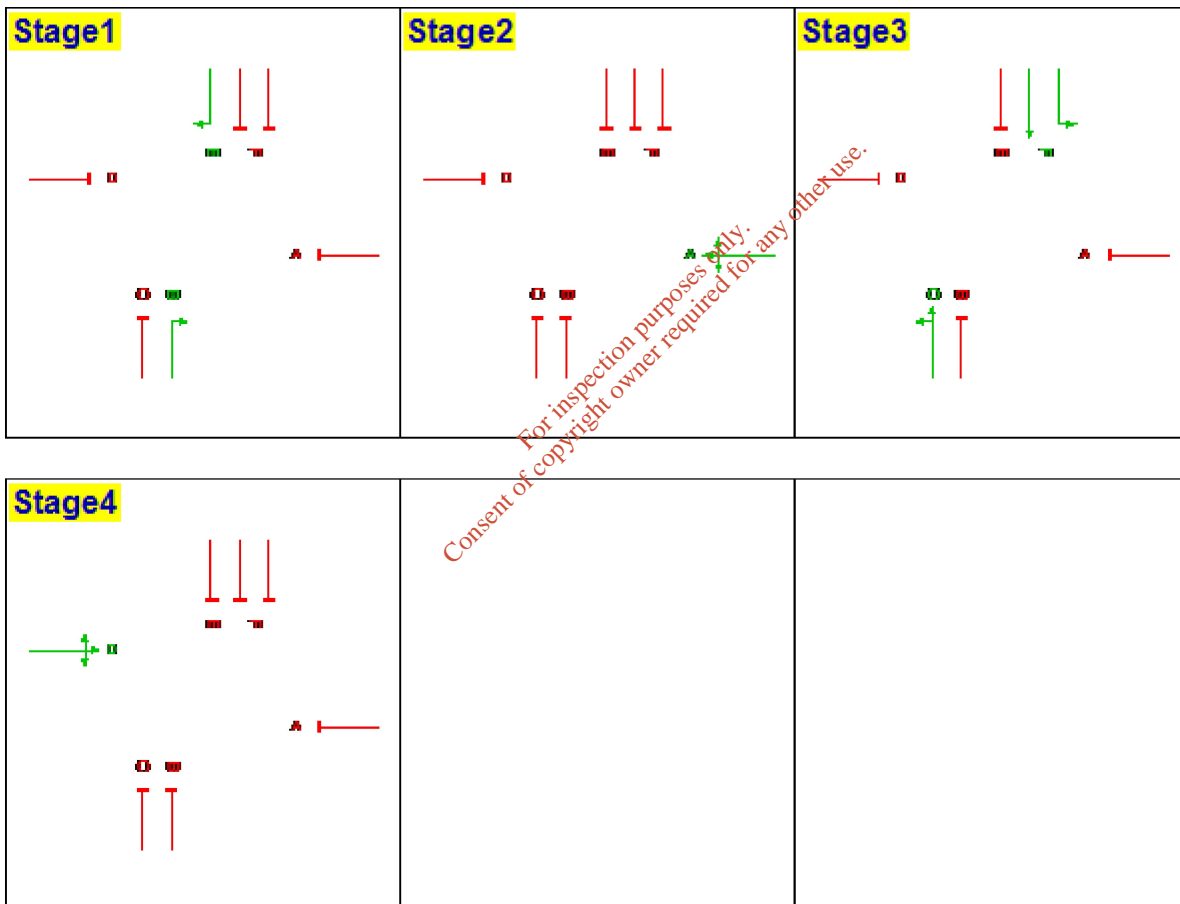
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak No Dev	Yes	07:15	08:45	ODTAB	No	D1
2032 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak + Link	No	07:15	08:45	ODTAB	No	D1

Demand Set7 - 2032 AM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	110	359	101
	Arm 2	96	-	243	84
	Arm 3	177	121	-	12
	Arm 4	254	149	33	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	428	510	625	625	510	428
2 - R135 South	1	C	244	292	357	357	292	244
2 - R135 South	2	B	73	87	107	107	87	73
3 - L3125 West	1	D	233	278	340	340	278	233
4 - R135 North	1	F	301	359	440	440	359	301
4 - R135 North	2	E	26	31	38	38	31	26

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	19	63	18
2 - R135 South	57	20	23
3 - L3125 West	4	57	39
4 - R135 North	58	34	8

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	14.98	24.07	24.07	66.2

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-4.02	26.70	26.70	63.10

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

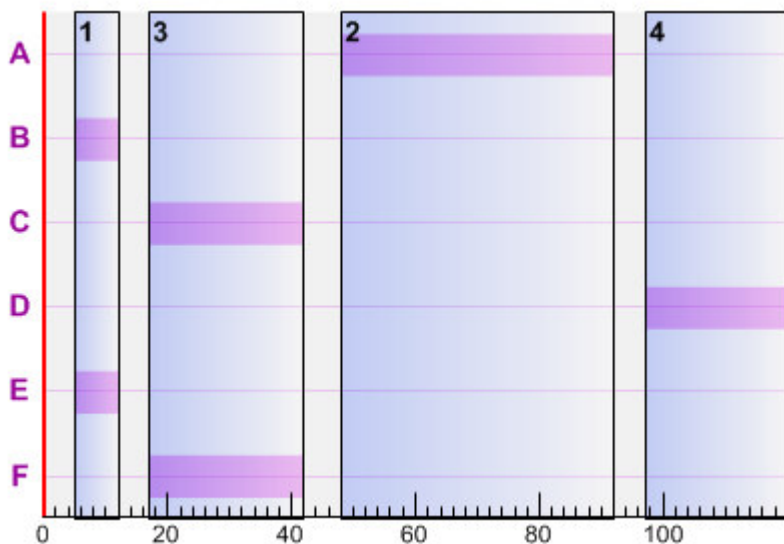
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	25.0	42.0
2	48.0	44.0	92.0
4	97.0	23.0	0.0

Phase Timings

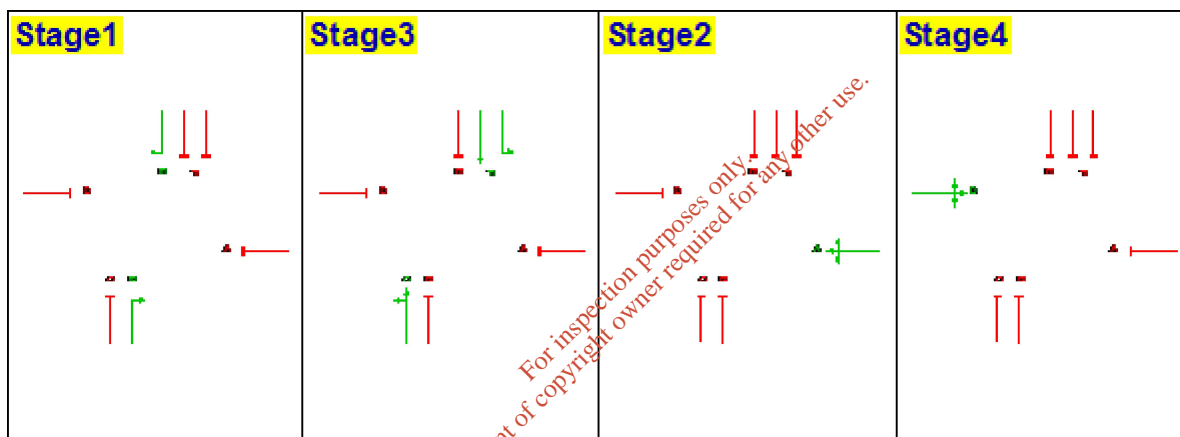
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	48	44.0	92						
B	5	7.0	12						
C	17	25.0	42						
D	97	23.0	0						
E	5	7.0	12						
F	17	25.0	42						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	521	A	45.50	43.89	6.35	76.72	17.31	1.84	13.88	12.04	26.30
2	1	298	C	26.50	65.55	5.43	78.27	14.98	1.93	10.16	8.23	9.00
2	2	89	B	8.50	89.40	2.21	67.26	33.80	0.90	3.71	2.81	1.80
3	1	284	D	24.50	66.27	5.23	77.32	16.40	1.81	9.79	7.99	8.40
4	1	367	F	26.50	42.56	4.34	43.91	104.98	0.24	10.53	10.29	19.80
4	2	32	E	8.50	58.40	0.52	24.18	272.14	0.05	1.05	1.00	0.90

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	428	1	45.50	36.08	4.29	63.03	42.80	0.76	10.44	9.69	4.90
2	1	244	3	26.50	52.65	3.57	64.09	40.43	0.76	7.42	6.66	1.60
2	2	73	2	8.50	73.15	1.48	55.17	63.13	0.42	2.72	2.29	0.30
3	1	233	4	24.50	53.96	3.49	63.44	41.87	0.73	7.21	6.48	1.50
4	1	301	6	26.50	41.16	3.44	36.01	149.92	0.14	8.46	8.32	3.00
4	2	26	5	8.50	56.83	0.41	19.65	358.02	0.03	0.84	0.81	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	510	1	45.50	42.27	5.99	75.10	19.84	1.58	13.33	11.75	4.50
2	1	292	3	26.50	62.13	5.04	76.70	17.34	1.60	9.65	8.05	1.50
2	2	87	2	8.50	84.70	2.05	65.75	36.88	0.75	3.49	2.74	0.30
3	1	278	4	24.50	62.98	4.86	75.69	18.91	1.49	9.30	7.81	1.40
4	1	359	6	26.50	42.37	4.23	42.95	109.55	0.23	10.27	10.05	3.30
4	2	31	5	8.50	58.11	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	625	1	45.50	60.99	10.59	92.04	-2.21	5.60	20.38	14.78	2.90
2	1	357	3	26.50	88.19	6.75	93.77	-4.02	5.16	15.15	9.99	1.10
2	2	107	2	8.50	106.90	3.18	80.87	11.29	1.67	5.06	3.39	0.30
3	1	340	4	24.50	87.51	8.27	92.57	-2.77	4.65	14.33	9.68	1.10
4	1	440	6	26.50	44.34	5.42	52.64	70.97	0.42	12.95	12.53	3.50
4	2	38	5	8.50	59.98	0.63	28.72	213.38	0.08	1.26	1.19	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	625	1	45.50	68.46	11.89	92.04	-2.21	6.24	21.02	14.78	2.80
2	1	357	3	26.50	104.71	10.38	93.77	-4.02	6.12	16.11	9.99	1.10
2	2	107	2	8.50	118.29	3.52	80.87	11.29	1.84	5.22	3.39	0.30
3	1	340	4	24.50	102.06	9.64	92.57	-2.77	5.42	15.10	9.68	1.00
4	1	440	6	26.50	44.37	5.42	52.64	70.97	0.42	12.95	12.53	3.50
4	2	38	5	8.50	60.09	0.63	28.72	213.38	0.08	1.26	1.19	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	510	1	45.50	45.87	6.50	75.10	19.84	1.76	13.51	11.75	4.40
2	1	292	3	26.50	74.91	6.08	76.70	17.34	1.96	10.02	8.05	1.50
2	2	87	2	8.50	97.18	2.35	65.75	36.88	0.95	3.69	2.74	0.30
3	1	278	4	24.50	74.00	5.71	75.69	18.91	1.82	9.62	7.81	1.40
4	1	359	6	26.50	42.40	4.23	42.95	109.55	0.23	10.28	10.05	3.30
4	2	31	5	8.50	58.24	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	428	1	45.50	36.65	4.36	63.03	42.80	0.79	10.48	9.69	4.90
2	1	244	3	26.50	54.80	3.71	64.09	40.43	0.83	7.49	6.66	1.60
2	2	73	2	8.50	78.86	1.60	55.17	63.13	0.49	2.78	2.29	0.30
3	1	233	4	24.50	56.03	3.63	63.44	41.87	0.80	7.28	6.48	1.50
4	1	301	6	26.50	41.18	3.44	36.01	149.92	0.14	8.46	8.32	3.00
4	2	26	5	8.50	56.95	0.41	19.65	358.02	0.03	0.84	0.81	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads.osc
Report generation date: 05/12/2016 16:46:52

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

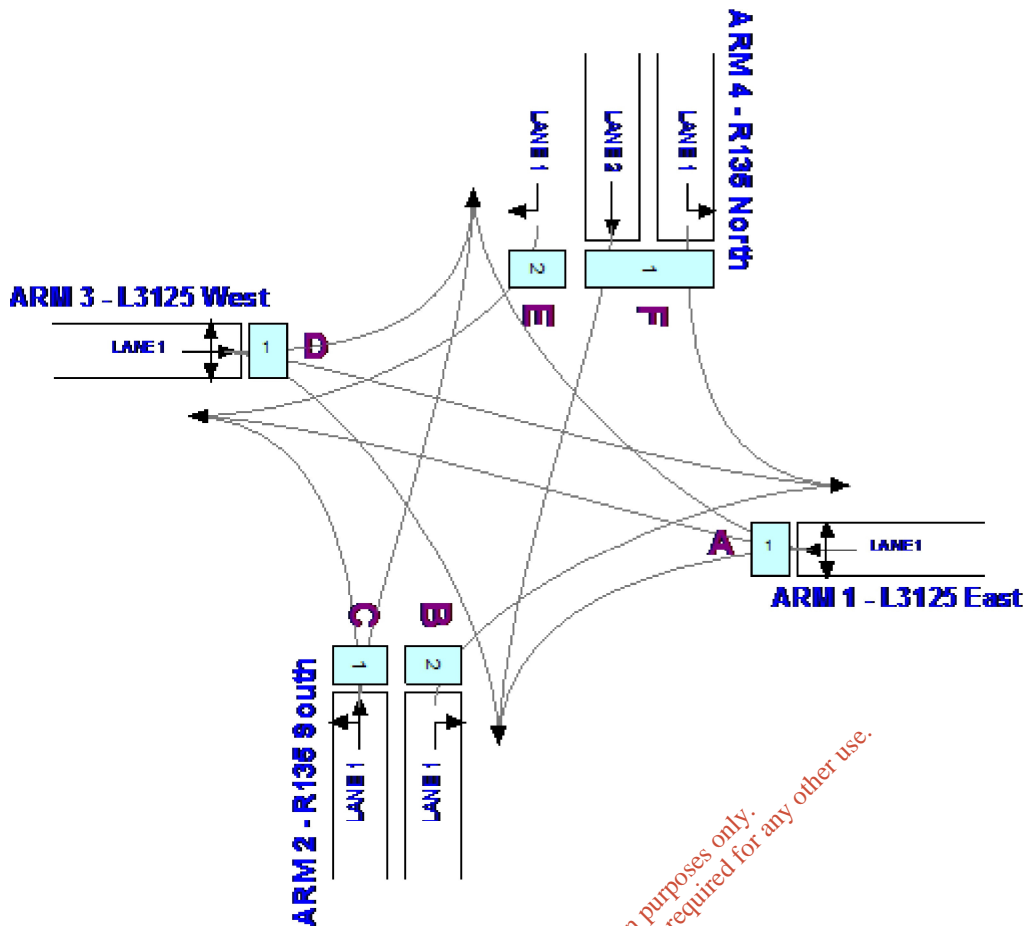
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

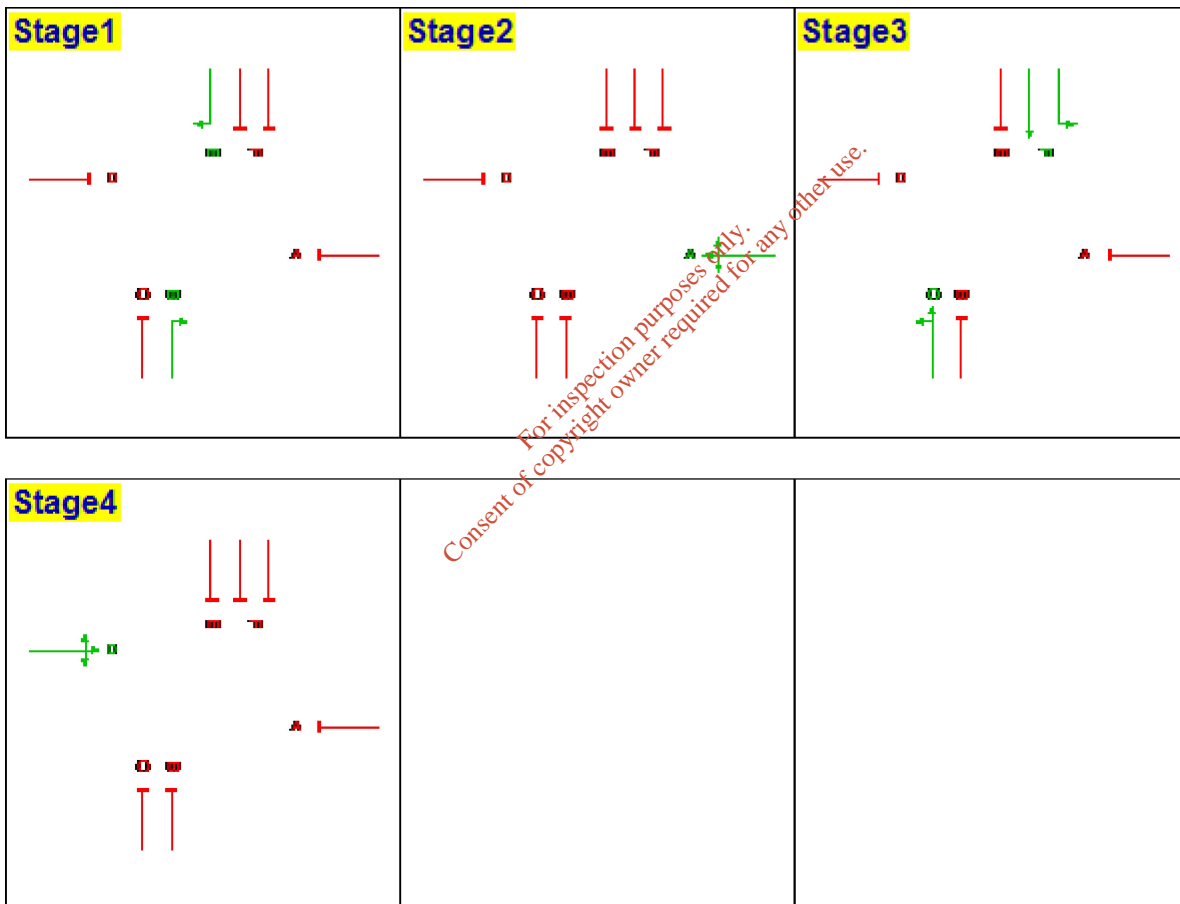
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak With Dev	Yes	07:15	08:45	ODTAB	No	D1
2032 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak + Link	No	07:15	08:45	ODTAB	No	D1

Demand Set8 - 2032 AM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	110	359	101
	Arm 2	96	-	243	85
	Arm 3	177	121	-	12
	Arm 4	254	150	33	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	428	510	625	625	510	428
2 - R135 South	1	C	245	292	358	358	292	245
2 - R135 South	2	B	73	87	107	107	87	73
3 - L3125 West	1	D	233	278	340	340	278	233
4 - R135 North	1	F	302	360	441	441	360	302
4 - R135 North	2	E	26	31	38	38	31	26

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	19	63	18
2 - R135 South	57	20	23
3 - L3125 West	4	57	39
4 - R135 North	58	34	8

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	14.98	24.09	24.09	66.3

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-4.29	26.76	26.76	63.30

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

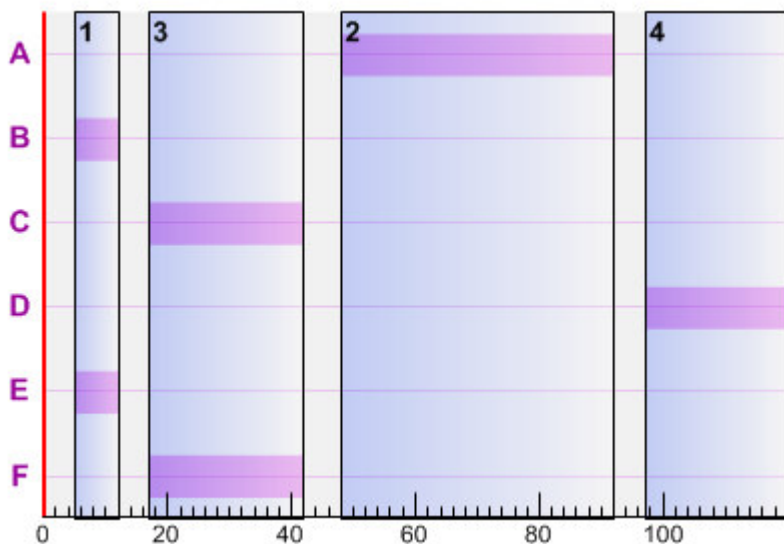
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	25.0	42.0
2	48.0	44.0	92.0
4	97.0	23.0	0.0

Phase Timings

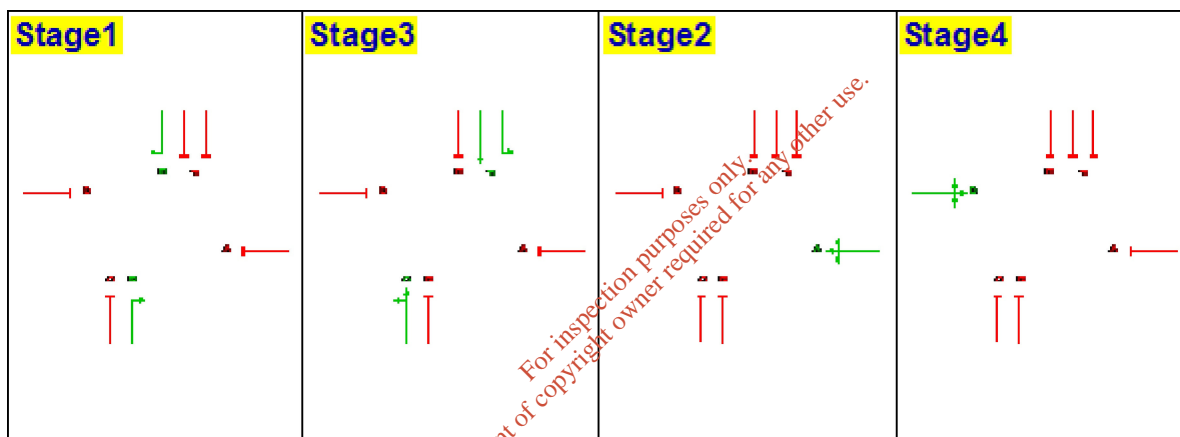
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	48	44.0	92						
B	5	7.0	12						
C	17	25.0	42						
D	97	23.0	0						
E	5	7.0	12						
F	17	25.0	42						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	521	A	45.50	43.89	6.35	76.72	17.31	1.84	13.88	12.04	26.30
2	1	298	C	26.50	65.55	5.43	78.27	14.98	1.93	10.16	8.23	9.00
2	2	89	B	8.50	89.40	2.21	67.26	33.80	0.90	3.71	2.81	1.80
3	1	284	D	24.50	66.27	5.23	77.32	16.40	1.81	9.79	7.99	8.40
4	1	368	F	26.50	42.58	4.35	44.03	104.42	0.25	10.56	10.32	19.90
4	2	32	E	8.50	58.40	0.52	24.18	272.14	0.05	1.05	1.00	0.90

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	428	1	45.50	36.08	4.29	63.03	42.80	0.76	10.44	9.69	4.90
2	1	245	3	26.50	52.79	3.59	64.35	39.86	0.77	7.46	6.69	1.60
2	2	73	2	8.50	73.15	1.48	55.17	63.13	0.42	2.72	2.29	0.30
3	1	233	4	24.50	53.96	3.49	63.44	41.87	0.73	7.21	6.48	1.50
4	1	302	6	26.50	41.18	3.45	36.13	149.10	0.14	8.49	8.35	3.00
4	2	26	5	8.50	56.83	0.41	19.65	358.02	0.03	0.84	0.81	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	510	1	45.50	42.27	5.99	75.10	19.84	1.58	13.33	11.75	4.50
2	1	292	3	26.50	62.15	5.04	76.70	17.34	1.60	9.65	8.05	1.50
2	2	87	2	8.50	84.70	2.05	65.75	36.88	0.75	3.49	2.74	0.30
3	1	278	4	24.50	62.98	4.86	75.69	18.91	1.49	9.30	7.81	1.40
4	1	360	6	26.50	42.39	4.24	43.07	108.96	0.23	10.31	10.08	3.30
4	2	31	5	8.50	58.11	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	625	1	45.50	60.99	10.59	92.04	-2.21	5.60	20.38	14.78	2.90
2	1	358	3	26.50	88.71	6.82	94.03	-4.29	5.26	15.28	10.02	1.10
2	2	107	2	8.50	106.90	3.18	80.87	11.29	1.67	5.06	3.39	0.30
3	1	340	4	24.50	87.51	8.27	92.57	-2.77	4.65	14.33	9.68	1.10
4	1	441	6	26.50	44.37	5.44	52.76	70.58	0.42	12.98	12.56	3.60
4	2	38	5	8.50	59.98	0.63	28.72	213.38	0.08	1.26	1.19	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	625	1	45.50	68.46	11.89	92.04	-2.21	6.24	21.02	14.78	2.80
2	1	358	3	26.50	105.85	10.53	94.03	-4.29	6.27	16.29	10.02	1.10
2	2	107	2	8.50	118.29	3.52	80.87	11.29	1.84	5.22	3.39	0.30
3	1	340	4	24.50	102.06	9.64	92.57	-2.77	5.42	15.10	9.68	1.00
4	1	441	6	26.50	44.40	5.44	52.76	70.58	0.42	12.98	12.56	3.60
4	2	38	5	8.50	60.09	0.63	28.72	213.38	0.08	1.26	1.19	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	510	1	45.50	45.87	6.50	75.10	19.84	1.76	13.51	11.75	4.40
2	1	292	3	26.50	75.39	6.12	76.70	17.34	1.96	10.02	8.05	1.50
2	2	87	2	8.50	97.18	2.35	65.75	36.88	0.95	3.69	2.74	0.30
3	1	278	4	24.50	74.00	5.71	75.69	18.91	1.82	9.62	7.81	1.40
4	1	360	6	26.50	42.42	4.24	43.07	108.96	0.23	10.31	10.08	3.30
4	2	31	5	8.50	58.24	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	428	1	45.50	36.65	4.36	63.03	42.80	0.79	10.48	9.69	4.90
2	1	245	3	26.50	54.96	3.74	64.35	39.86	0.84	7.53	6.69	1.60
2	2	73	2	8.50	78.86	1.60	55.17	63.13	0.49	2.78	2.29	0.30
3	1	233	4	24.50	56.03	3.63	63.44	41.87	0.80	7.28	6.48	1.50
4	1	302	6	26.50	41.20	3.46	36.13	149.10	0.14	8.49	8.35	3.00
4	2	26	5	8.50	56.95	0.41	19.65	358.02	0.03	0.84	0.81	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads.osc
Report generation date: 05/12/2016 16:47:41

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

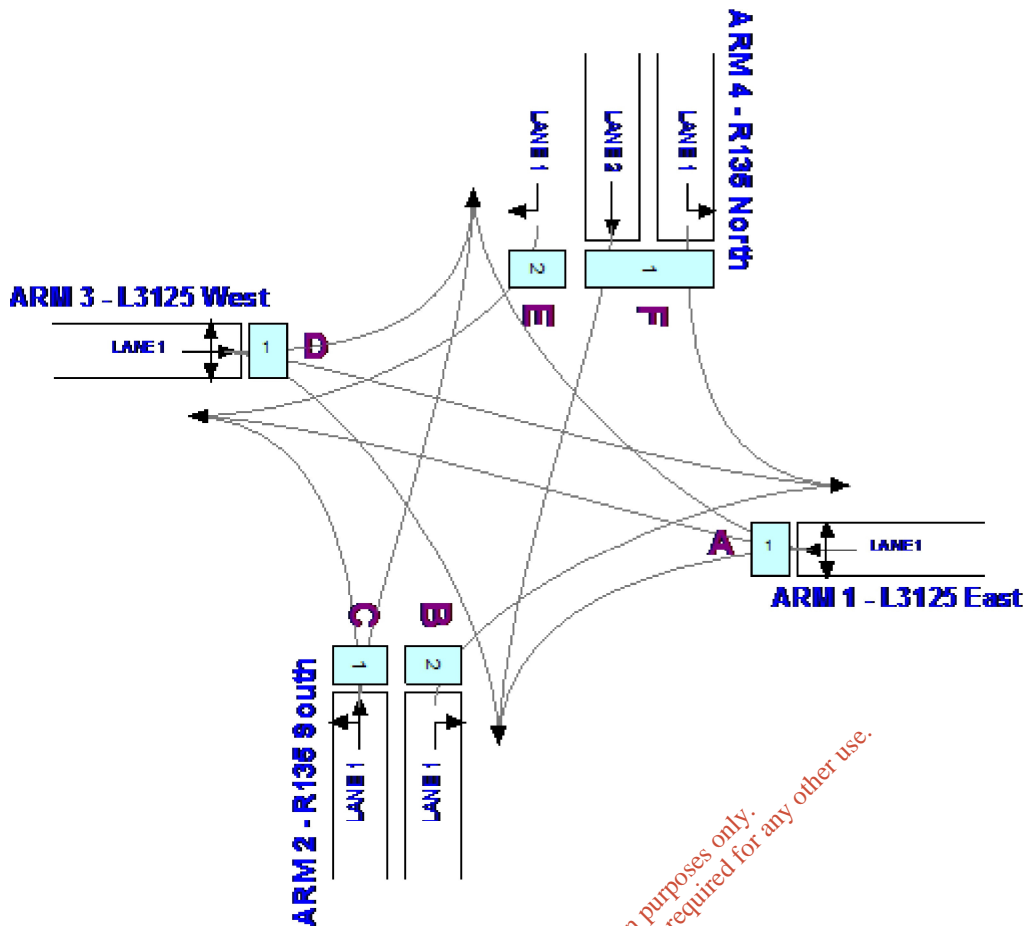
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

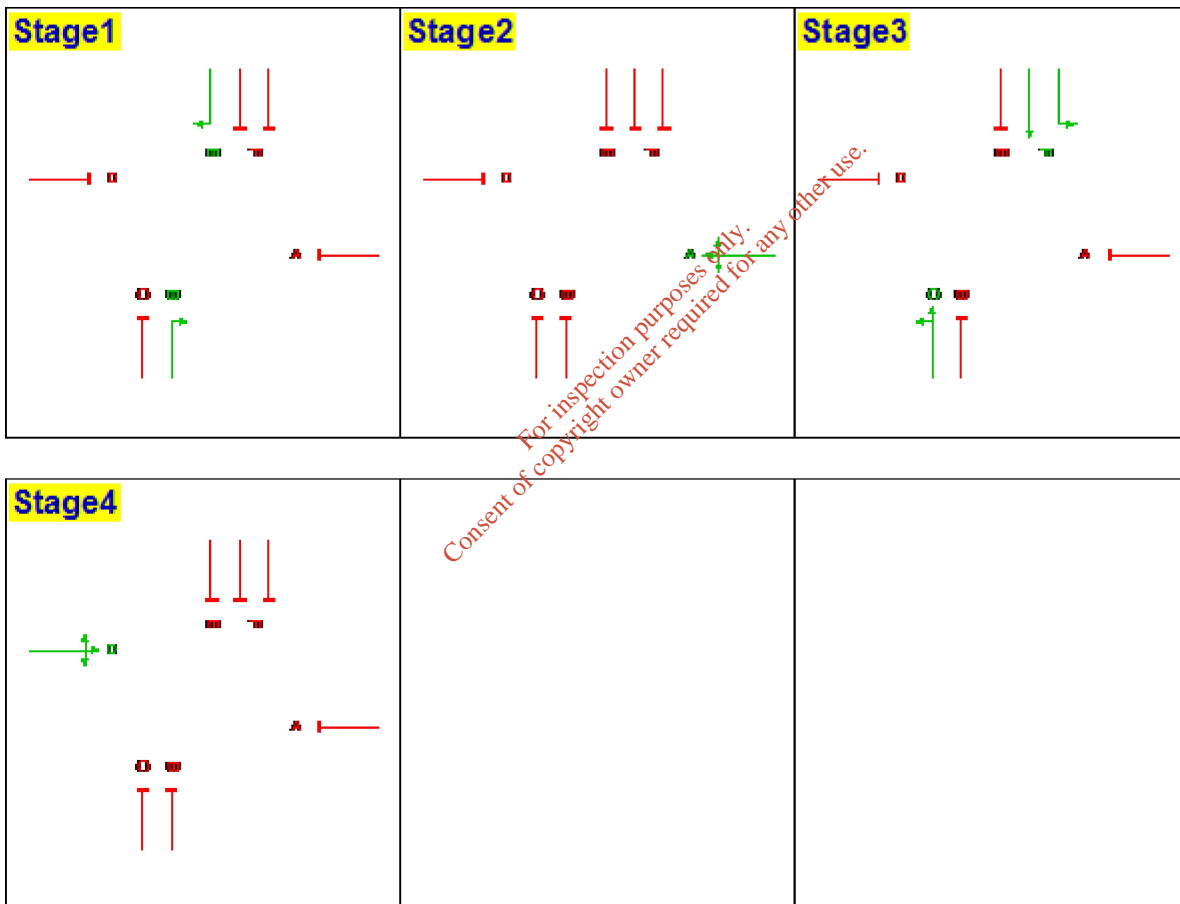
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak Sensitivity	Yes	07:15	08:45	ODTAB	No	D1
2022 AM Peak + Link	No	07:15	08:45	ODTAB	No	D1

Demand Set9 - 2032 AM Peak Sensitivity

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	110	359	101
	Arm 2	96	-	243	101
	Arm 3	177	121	-	12
	Arm 4	254	166	33	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	428	510	625	625	510	428
2 - R135 South	1	C	257	307	376	376	307	257
2 - R135 South	2	B	73	87	106	106	87	73
3 - L3125 West	1	D	233	278	340	340	278	233
4 - R135 North	1	F	316	377	462	462	377	316
4 - R135 North	2	E	24	28	35	35	28	24

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	19	63	18
2 - R135 South	55	23	22
3 - L3125 West	4	57	39
4 - R135 North	56	37	7

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	13.60	24.75	24.75	66.5

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-5.43	27.80	27.80	62.90

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

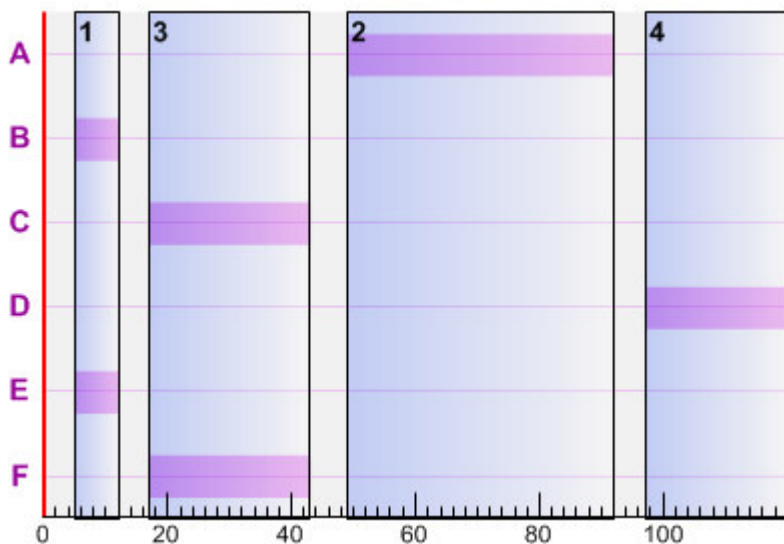
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	26.0	43.0
2	49.0	43.0	92.0
4	97.0	23.0	0.0

Phase Timings

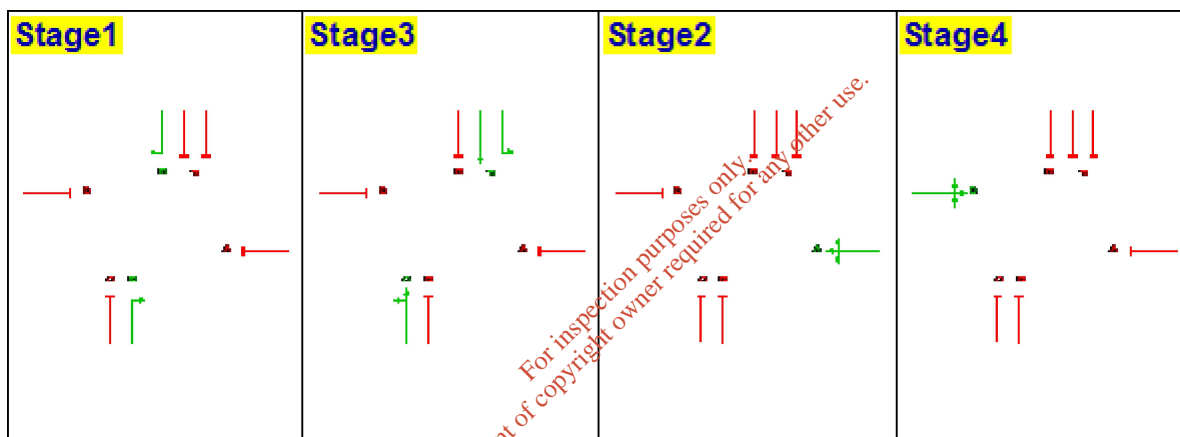
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	49	43.0	92						
B	5	7.0	12						
C	17	26.0	43						
D	97	23.0	0						
E	5	7.0	12						
F	17	26.0	43						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	521	A	44.50	46.17	6.68	78.44	14.73	2.08	14.27	12.20	24.50
2	1	313	C	27.50	65.45	5.69	79.22	13.60	2.08	10.66	8.58	9.50
2	2	89	B	8.50	89.40	2.21	67.26	33.80	0.90	3.71	2.81	1.80
3	1	284	D	24.50	66.27	5.23	77.32	16.40	1.81	9.79	7.99	8.40
4	1	385	F	27.50	41.87	4.48	44.39	102.77	0.25	10.97	10.72	21.40
4	2	29	E	8.50	57.64	0.46	21.92	310.64	0.04	0.94	0.90	0.90

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	428	1	44.50	37.36	4.44	64.44	39.66	0.82	10.64	9.82	4.60
2	1	257	3	27.50	52.15	3.72	65.05	38.36	0.80	7.77	6.96	1.80
2	2	73	2	8.50	73.15	1.48	55.17	63.13	0.42	2.72	2.29	0.30
3	1	233	4	24.50	53.96	3.49	63.44	41.87	0.73	7.21	6.48	1.50
4	1	316	6	27.50	40.45	3.55	36.43	147.04	0.15	8.81	8.67	3.20
4	2	24	5	8.50	56.38	0.38	18.14	396.19	0.03	0.77	0.75	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	510	1	44.50	44.22	6.26	76.79	17.21	1.75	13.66	11.91	4.20
2	1	307	3	27.50	61.88	5.28	77.71	15.82	1.71	10.11	8.40	1.60
2	2	87	2	8.50	84.70	2.05	65.75	36.88	0.75	3.49	2.74	0.30
3	1	278	4	24.50	62.98	4.86	75.69	18.91	1.49	9.30	7.81	1.40
4	1	377	6	27.50	41.69	4.37	43.46	107.07	0.24	10.71	10.48	3.50
4	2	28	5	8.50	57.36	0.45	21.16	325.30	0.04	0.91	0.87	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	625	1	44.50	66.01	11.46	94.10	-4.36	6.69	21.67	14.98	2.50
2	1	376	3	27.50	89.88	9.39	95.17	-5.43	5.83	16.28	10.45	1.10
2	2	106	2	8.50	105.74	3.11	80.11	12.34	1.61	4.96	3.35	0.30
3	1	340	4	24.50	87.51	8.27	92.57	-2.77	4.65	14.33	9.68	1.10
4	1	462	6	27.50	43.70	5.61	53.26	68.97	0.44	13.52	13.08	3.80
4	2	35	5	8.50	59.14	0.57	26.45	240.24	0.06	1.15	1.09	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	625	1	44.50	76.97	13.36	94.10	-4.36	7.75	22.73	14.98	2.40
2	1	376	3	27.50	109.22	11.41	95.17	-5.43	7.10	17.54	10.45	1.10
2	2	106	2	8.50	116.22	3.42	80.11	12.34	1.76	5.11	3.35	0.30
3	1	340	4	24.50	102.06	9.64	92.57	-2.77	5.42	15.10	9.68	1.00
4	1	462	6	27.50	43.72	5.61	53.26	68.97	0.44	13.52	13.08	3.80
4	2	35	5	8.50	59.24	0.58	26.45	240.24	0.06	1.15	1.09	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	510	1	44.50	49.79	7.05	76.79	17.21	1.99	13.90	11.91	4.10
2	1	307	3	27.50	77.31	6.59	77.71	15.82	2.13	10.53	8.40	1.60
2	2	87	2	8.50	96.38	2.33	65.75	36.88	0.95	3.69	2.74	0.30
3	1	278	4	24.50	74.00	5.71	75.69	18.91	1.82	9.62	7.81	1.40
4	1	377	6	27.50	41.72	4.37	43.46	107.07	0.24	10.72	10.48	3.50
4	2	28	5	8.50	57.48	0.45	21.16	325.30	0.04	0.91	0.87	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	428	1	44.50	38.08	4.53	64.44	39.66	0.87	10.68	9.82	4.60
2	1	257	3	27.50	54.40	3.88	65.05	38.36	0.88	7.84	6.96	1.80
2	2	73	2	8.50	78.86	1.60	55.17	63.13	0.49	2.78	2.29	0.30
3	1	233	4	24.50	56.03	3.63	63.44	41.87	0.80	7.28	6.48	1.50
4	1	316	6	27.50	40.47	3.55	36.43	147.04	0.15	8.81	8.67	3.20
4	2	24	5	8.50	56.46	0.38	18.14	396.19	0.03	0.77	0.75	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads.osc
Report generation date: 05/12/2016 16:54:16

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

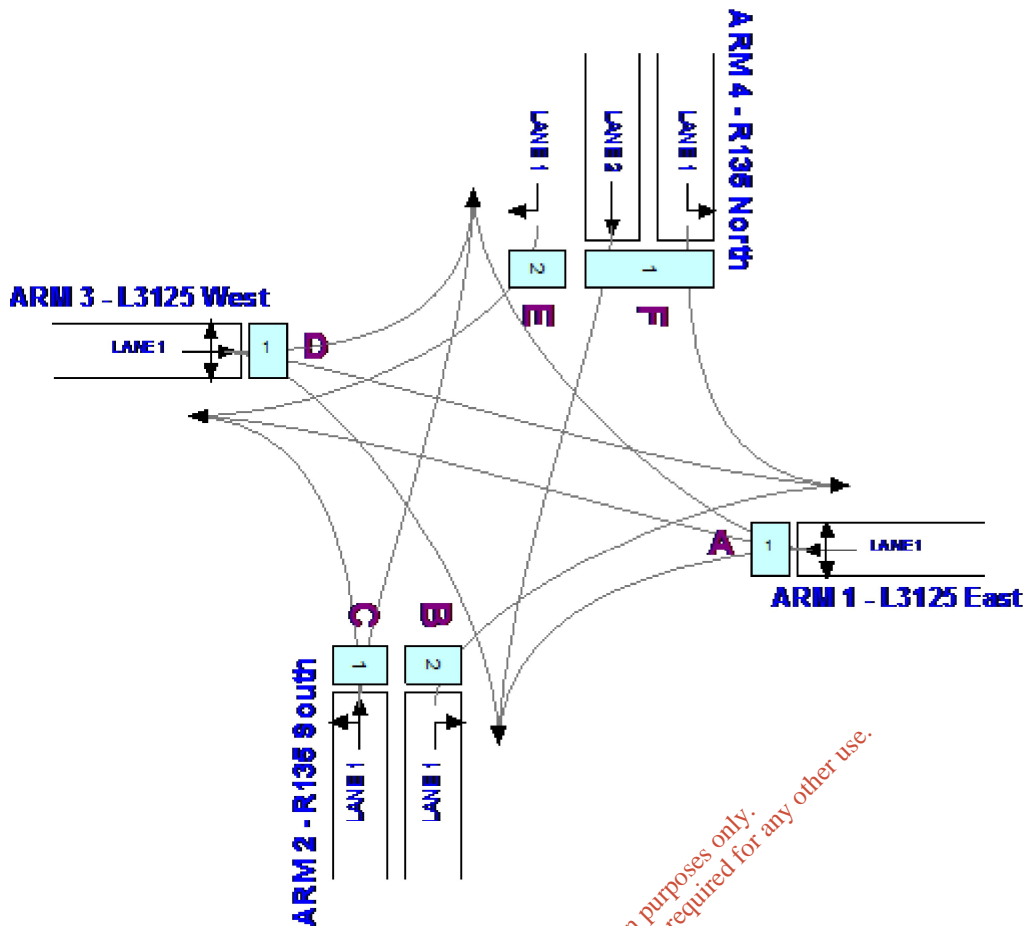
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

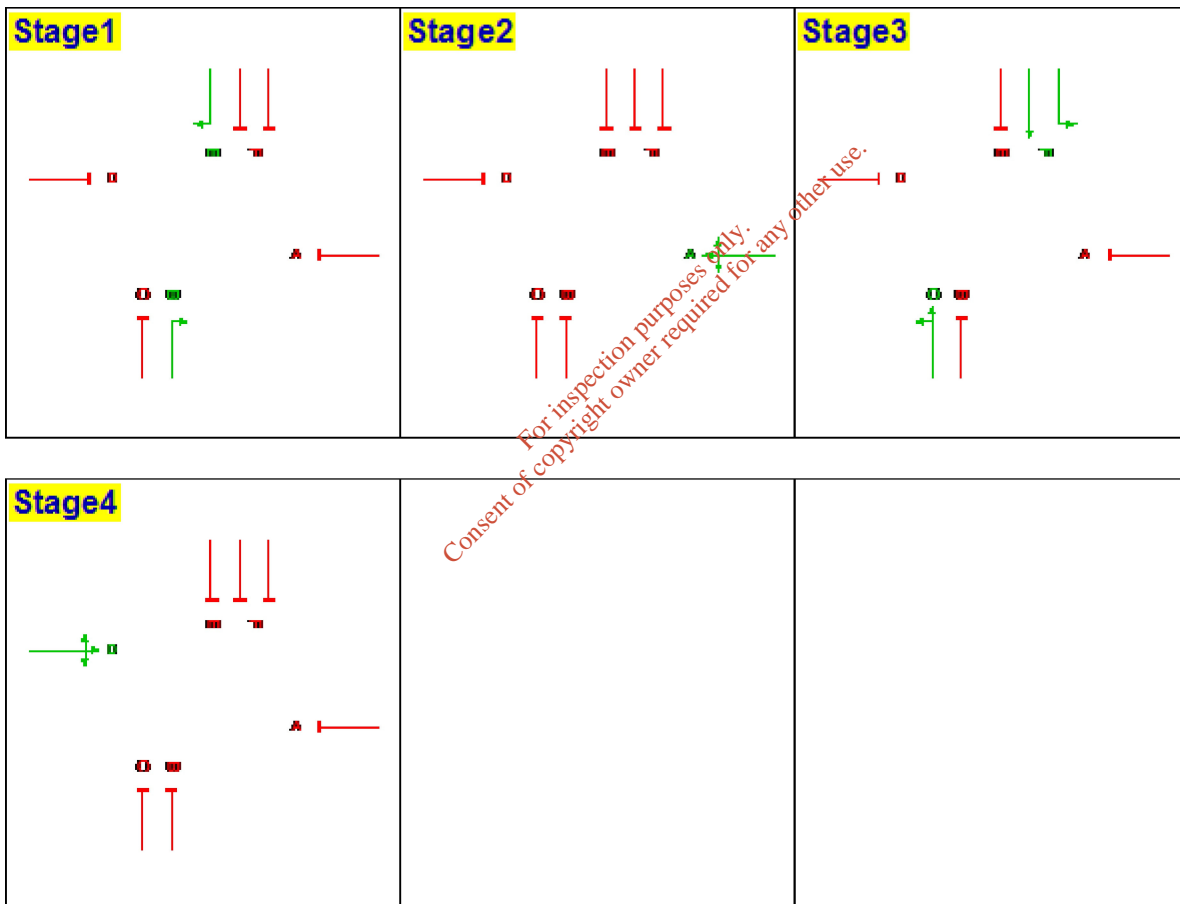
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2022 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak Sensitivity	No	07:15	08:45	ODTAB	No	D1
2032 AM Peak + Link	Yes	07:15	08:45	ODTAB	No	D1

Demand Set10 - 2032 AM Peak + Link

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	110	359	50
	Arm 2	96	-	243	85
	Arm 3	177	121	-	12
	Arm 4	127	150	33	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	389	465	569	569	465	389
2 - R135 South	1	C	245	292	358	358	292	245
2 - R135 South	2	B	73	87	107	107	87	73
3 - L3125 West	1	D	233	278	340	340	278	233
4 - R135 North	1	F	207	247	303	303	247	207
4 - R135 North	2	E	26	31	37	37	31	26

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	21	69	10
2 - R135 South	57	20	23
3 - L3125 West	4	57	39
4 - R135 North	41	48	11

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	20.44	20.91	20.91	63.3

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	0.33	22.71	22.71	60.30

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

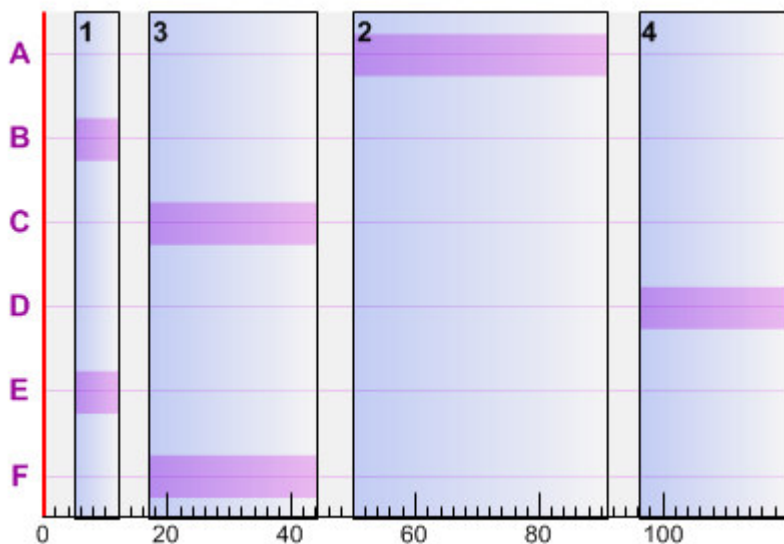
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	27.0	44.0
2	50.0	41.0	91.0
4	96.0	24.0	0.0

Phase Timings

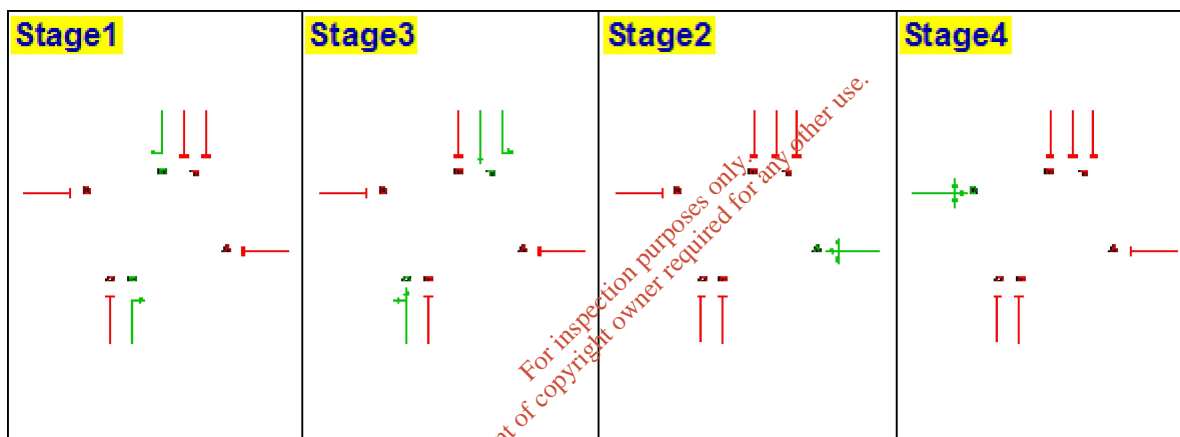
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	50	41.0	91						
B	5	7.0	12						
C	17	27.0	44						
D	96	24.0	0						
E	5	7.0	12						
F	17	27.0	44						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	474	A	42.50	44.89	5.91	74.73	20.44	1.59	12.87	11.27	23.50
2	1	298	C	28.50	57.29	4.74	72.78	23.66	1.35	9.40	8.05	10.80
2	2	89	B	8.50	89.40	2.21	67.26	33.80	0.90	3.71	2.81	1.80
3	1	284	D	25.50	61.59	4.86	74.29	21.15	1.48	9.38	7.90	9.20
4	1	252	F	28.50	38.40	2.69	28.03	221.05	0.08	6.82	6.74	17.10
4	2	31	E	8.50	58.14	0.50	23.43	284.15	0.05	1.01	0.97	0.90

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	389	1	42.50	37.65	4.07	61.33	46.76	0.68	9.76	9.08	4.20
2	1	245	3	28.50	48.78	3.32	59.84	50.41	0.60	7.15	6.55	1.90
2	2	73	2	8.50	73.15	1.48	55.17	63.13	0.42	2.72	2.29	0.30
3	1	233	4	25.50	51.78	3.35	60.95	47.66	0.63	7.05	6.42	1.60
4	1	207	6	28.50	37.69	2.17	23.03	290.84	0.05	5.54	5.49	2.40
4	2	26	5	8.50	56.83	0.41	19.65	358.02	0.03	0.84	0.81	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	465	1	42.50	43.53	5.62	73.31	22.77	1.39	12.43	11.04	4.00
2	1	292	3	28.50	55.45	4.50	71.32	26.20	1.17	9.05	7.88	1.80
2	2	87	2	8.50	84.70	2.05	65.75	36.88	0.75	3.49	2.74	0.30
3	1	278	4	25.50	59.25	4.58	72.72	23.76	1.26	8.98	7.73	1.50
4	1	247	6	28.50	38.32	2.63	27.48	227.55	0.07	6.68	6.60	2.80
4	2	31	5	8.50	58.11	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	569	1	42.50	59.89	9.42	89.70	0.33	4.47	18.29	13.82	2.80
2	1	358	3	28.50	72.79	7.24	87.43	2.93	3.30	13.10	9.80	1.40
2	2	107	2	8.50	106.90	3.18	80.87	11.29	1.67	5.06	3.39	0.30
3	1	340	4	25.50	78.69	7.43	88.94	1.19	3.61	13.19	9.58	1.20
4	1	303	6	28.50	39.26	3.30	33.71	167.01	0.12	8.32	8.20	3.20
4	2	37	5	8.50	59.71	0.61	27.96	221.85	0.07	1.23	1.15	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	569	1	42.50	65.26	10.32	89.70	0.33	4.85	18.67	13.82	2.70
2	1	358	3	28.50	79.24	7.88	87.43	2.93	3.59	13.39	9.80	1.40
2	2	107	2	8.50	118.29	3.52	80.87	11.29	1.84	5.22	3.39	0.30
3	1	340	4	25.50	87.25	8.24	88.94	1.19	4.00	13.57	9.58	1.20
4	1	303	6	28.50	39.26	3.30	33.71	167.01	0.12	8.32	8.20	3.20
4	2	37	5	8.50	59.78	0.61	27.96	221.85	0.07	1.23	1.15	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	465	1	42.50	46.27	5.98	73.31	22.77	1.54	12.58	11.04	3.90
2	1	292	3	28.50	59.64	4.84	71.32	26.20	1.32	9.21	7.88	1.80
2	2	87	2	8.50	97.18	2.35	65.75	36.88	0.95	3.69	2.74	0.30
3	1	278	4	25.50	65.15	5.03	72.72	23.76	1.46	9.19	7.73	1.50
4	1	247	6	28.50	38.32	2.63	27.48	227.55	0.07	6.68	6.60	2.80
4	2	31	5	8.50	58.21	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	389	1	42.50	38.18	4.13	61.33	46.76	0.71	9.79	9.08	4.20
2	1	245	3	28.50	49.82	3.39	59.84	50.41	0.64	7.19	6.55	1.90
2	2	73	2	8.50	78.86	1.60	55.17	63.13	0.49	2.78	2.29	0.30
3	1	233	4	25.50	53.16	3.44	60.95	47.66	0.68	7.10	6.42	1.60
4	1	207	6	28.50	37.69	2.17	23.03	290.84	0.05	5.54	5.49	2.50
4	2	26	5	8.50	56.95	0.41	19.65	358.02	0.03	0.84	0.81	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads PM.osc
Report generation date: 09/12/2016 10:37:35

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

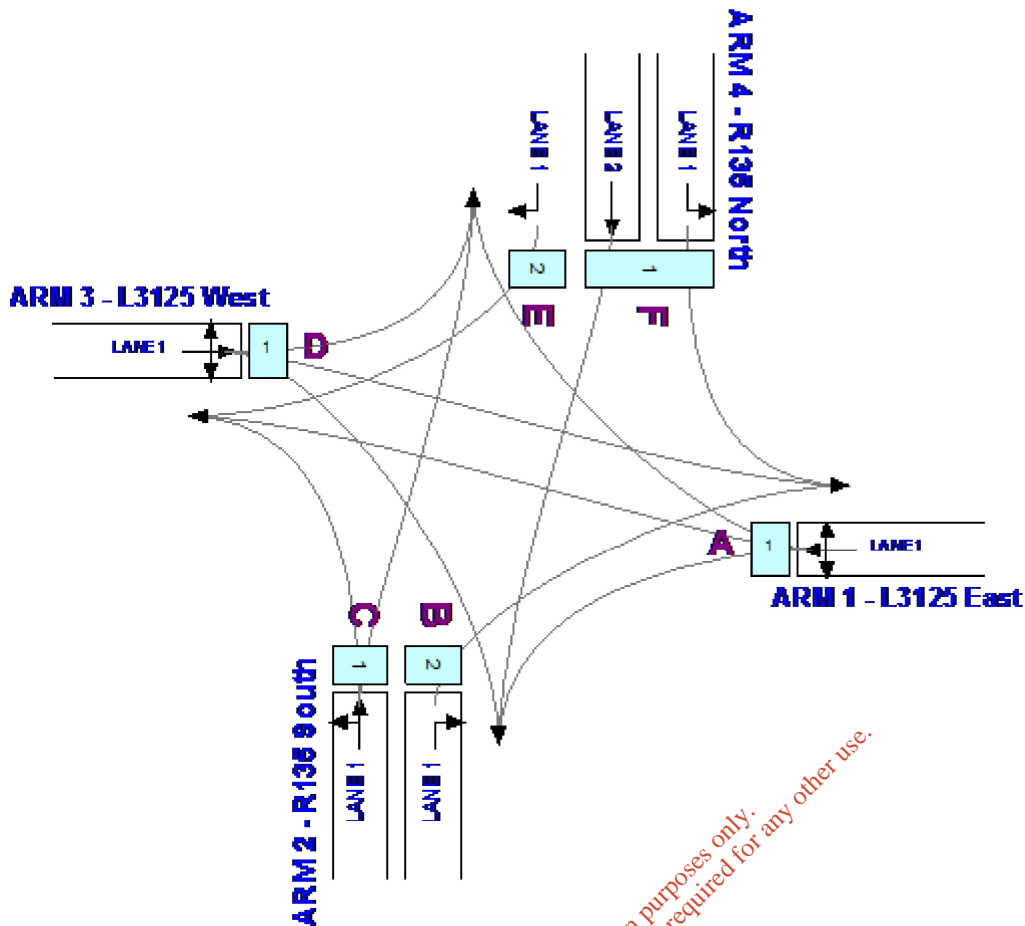
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

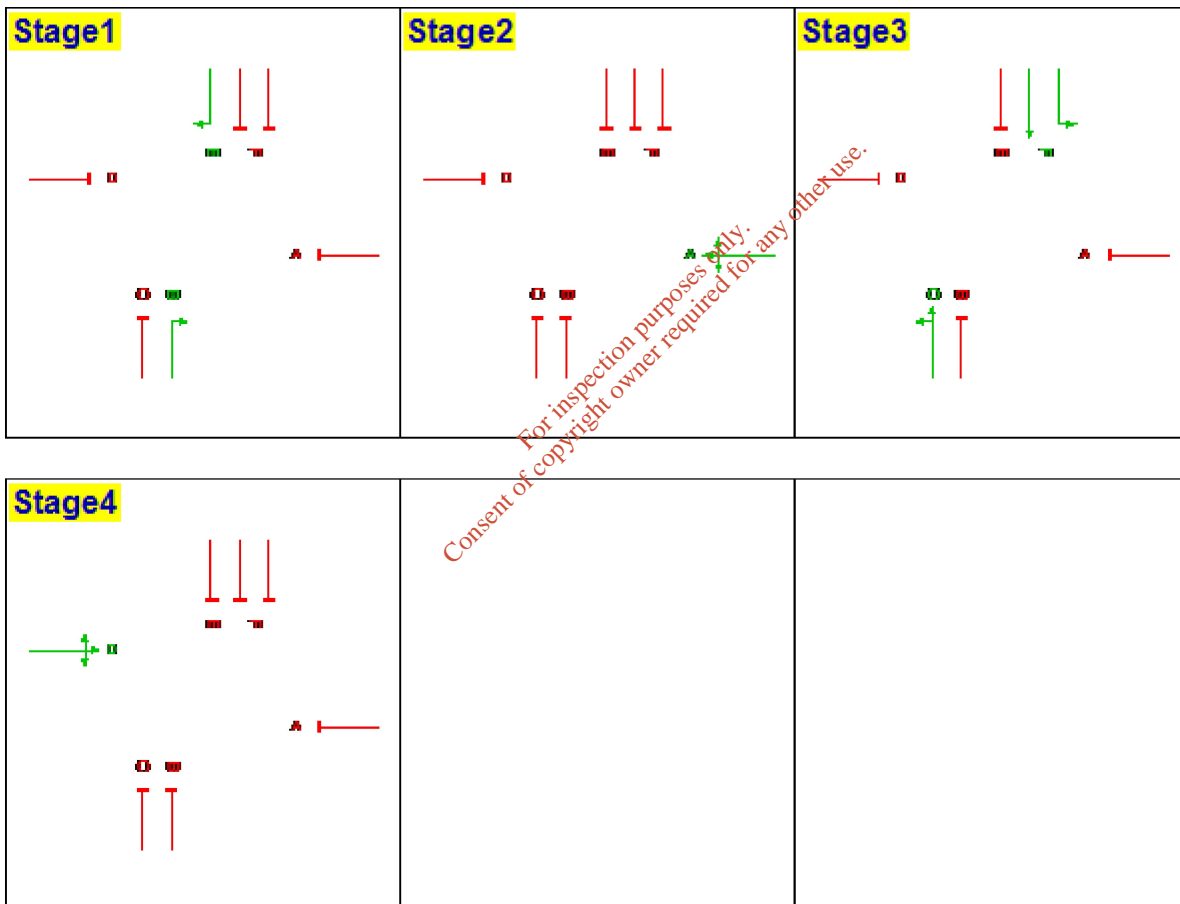
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	Yes	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set6 - 2016 PM Peak Existing

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	91	221	216
	Arm 2	136	-	135	127
	Arm 3	179	175	-	24
	Arm 4	177	97	18	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	396	473	579	579	473	396
2 - R135 South	1	C	197	235	288	288	235	197
2 - R135 South	2	B	101	121	148	148	121	101
3 - L3125 West	1	D	281	335	410	410	335	281
4 - R135 North	1	F	206	246	301	301	246	206
4 - R135 North	2	E	13	16	19	19	16	13

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	34	32	34
3 - L3125 West	6	47	46
4 - R135 North	61	33	6

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	15.83	22.96	22.96	57.3

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-3.47	25.46	25.46	54.40

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

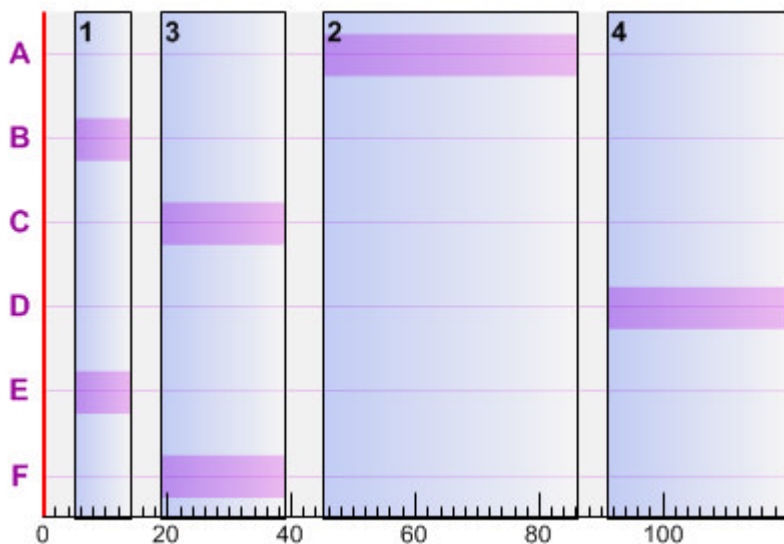
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	9.0	14.0
3	19.0	20.0	39.0
2	45.0	41.0	86.0
4	91.0	29.0	0.0

Phase Timings

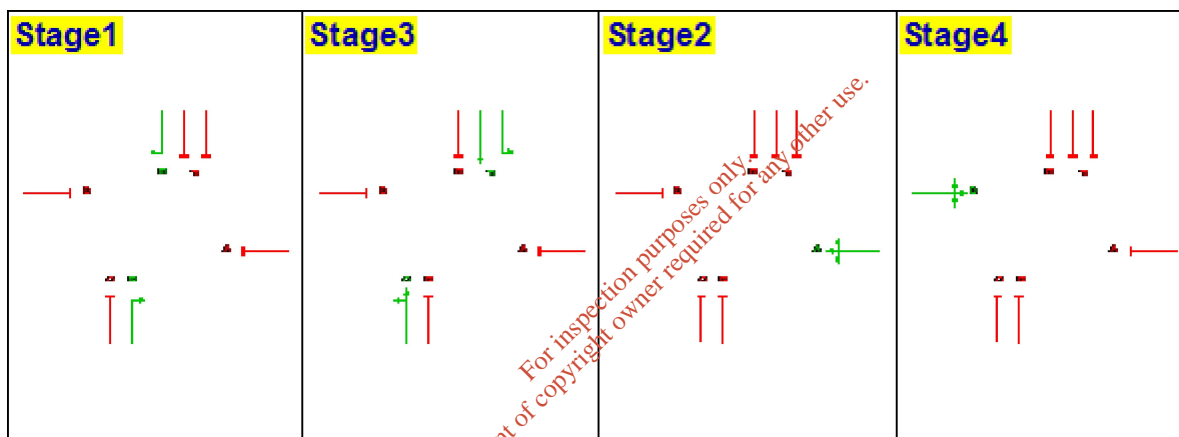
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	45	41.0	86						
B	5	9.0	14						
C	19	20.0	39						
D	91	29.0	0						
E	5	9.0	14						
F	19	20.0	39						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	483	A	42.50	45.96	6.17	76.15	18.19	1.76	13.27	11.51	23.10
2	1	240	C	21.50	72.43	4.83	77.70	15.83	1.83	8.73	6.90	6.40
2	2	123	B	10.50	94.46	3.23	75.25	19.60	1.49	5.32	3.84	2.40
3	1	342	D	30.50	56.21	5.34	74.80	20.33	1.55	10.68	9.13	12.50
4	1	251	F	21.50	45.35	3.16	37.01	143.16	0.15	7.38	7.23	12.30
4	2	16	E	10.50	51.97	0.23	9.79	819.41	0.01	0.50	0.49	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	396	1	42.50	38.06	4.19	62.43	44.16	0.73	9.98	9.26	4.20
2	1	197	3	21.50	57.87	3.17	63.78	41.11	0.73	6.34	5.61	1.10
2	2	101	2	10.50	72.84	2.04	61.79	45.65	0.62	3.75	3.14	0.40
3	1	281	4	30.50	47.34	3.70	61.45	46.45	0.66	8.07	7.40	2.20
4	1	206	6	21.50	44.27	2.53	30.38	196.28	0.09	5.97	5.88	1.80
4	2	13	5	10.50	51.56	0.19	7.95	9999.00	0.00	0.40	0.40	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	473	1	42.50	44.33	5.82	74.57	20.69	1.51	12.76	11.25	3.90
2	1	235	3	21.50	68.26	4.46	76.08	18.30	1.49	8.24	6.75	1.10
2	2	121	2	10.50	87.72	2.95	74.03	21.57	1.20	4.98	3.77	0.40
3	1	335	4	30.50	54.24	5.05	73.26	22.84	1.33	10.25	8.93	2.10
4	1	246	6	21.50	45.22	3.09	36.28	148.10	0.14	7.22	7.08	2.00
4	2	16	5	10.50	51.96	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	579	1	42.50	62.54	10.06	91.28	-1.40	5.11	19.21	14.10	2.60
2	1	288	3	21.50	95.68	7.65	93.24	-3.47	4.56	12.93	8.36	0.80
2	2	148	2	10.50	118.53	4.87	90.55	-0.60	3.04	7.68	4.64	0.40
3	1	410	4	30.50	73.02	8.32	89.67	0.37	4.02	15.13	11.10	1.60
4	1	301	6	21.50	46.69	3.90	44.39	102.77	0.25	9.01	8.76	2.30
4	2	19	5	10.50	52.38	0.28	11.62	674.24	0.01	0.59	0.58	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	579	1	42.50	69.60	11.19	91.28	-1.40	5.66	19.75	14.10	2.60
2	1	288	3	21.50	113.91	9.11	93.24	-3.47	5.43	13.79	8.36	0.80
2	2	148	2	10.50	140.92	5.79	90.55	-0.60	3.59	8.23	4.64	0.40
3	1	410	4	30.50	80.83	9.21	89.67	0.37	4.45	15.55	11.10	1.60
4	1	301	6	21.50	46.71	3.91	44.39	102.77	0.25	9.01	8.76	2.30
4	2	19	5	10.50	52.38	0.28	11.62	674.24	0.01	0.59	0.58	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	473	1	42.50	47.90	6.29	74.57	20.69	1.68	12.93	11.25	3.90
2	1	235	3	21.50	84.06	5.49	76.08	18.30	1.88	8.63	6.75	1.10
2	2	121	2	10.50	114.18	3.84	74.03	21.57	1.67	5.44	3.77	0.40
3	1	335	4	30.50	59.07	5.50	73.26	22.84	1.52	10.45	8.93	2.10
4	1	246	6	21.50	45.24	3.09	36.28	148.10	0.14	7.22	7.08	2.00
4	2	16	5	10.50	51.97	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	396	1	42.50	38.67	4.25	62.43	44.16	0.76	10.02	9.26	4.20
2	1	197	3	21.50	60.94	3.33	63.78	41.11	0.81	6.42	5.61	1.10
2	2	101	2	10.50	81.40	2.28	61.79	45.65	0.73	3.86	3.14	0.40
3	1	281	4	30.50	48.37	3.78	61.45	46.45	0.71	8.11	7.40	2.20
4	1	206	6	21.50	44.29	2.53	30.38	196.28	0.09	5.97	5.88	1.80
4	2	13	5	10.50	51.57	0.19	7.95	9999.00	0.00	0.40	0.40	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads PM.osc
Report generation date: 09/12/2016 10:38:24

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

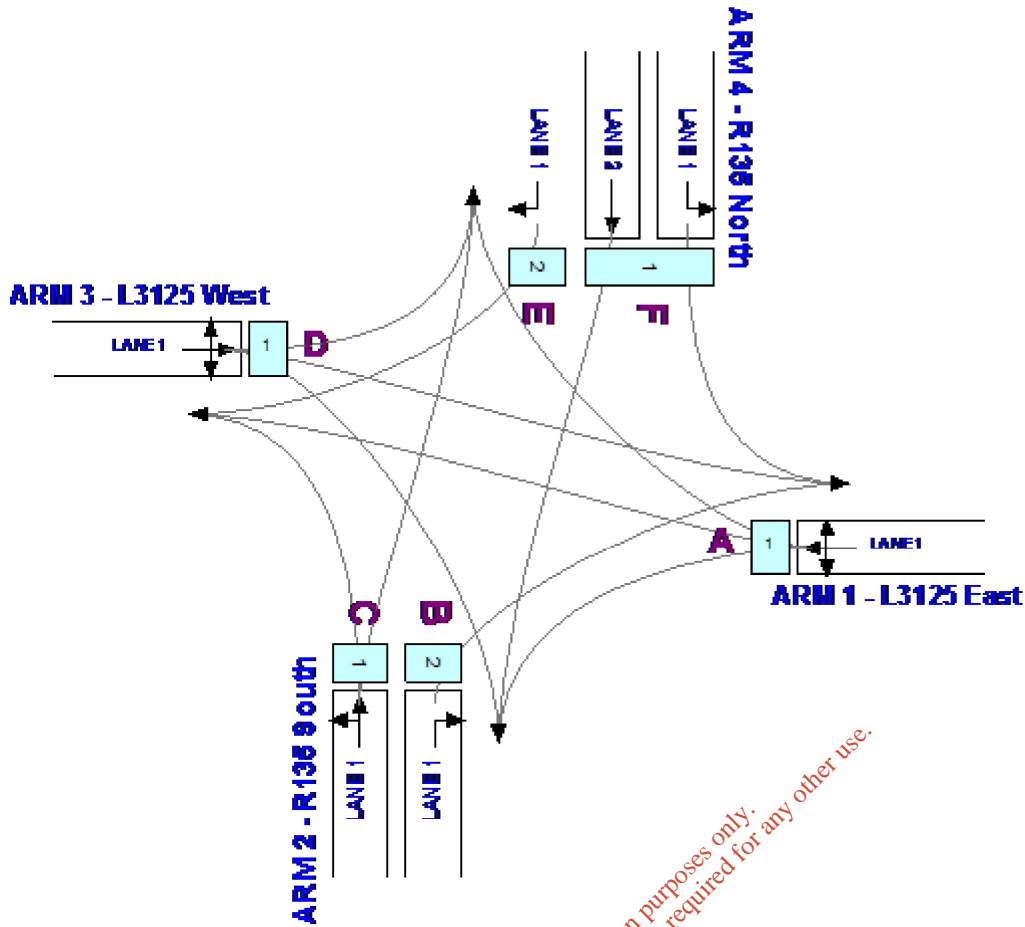
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

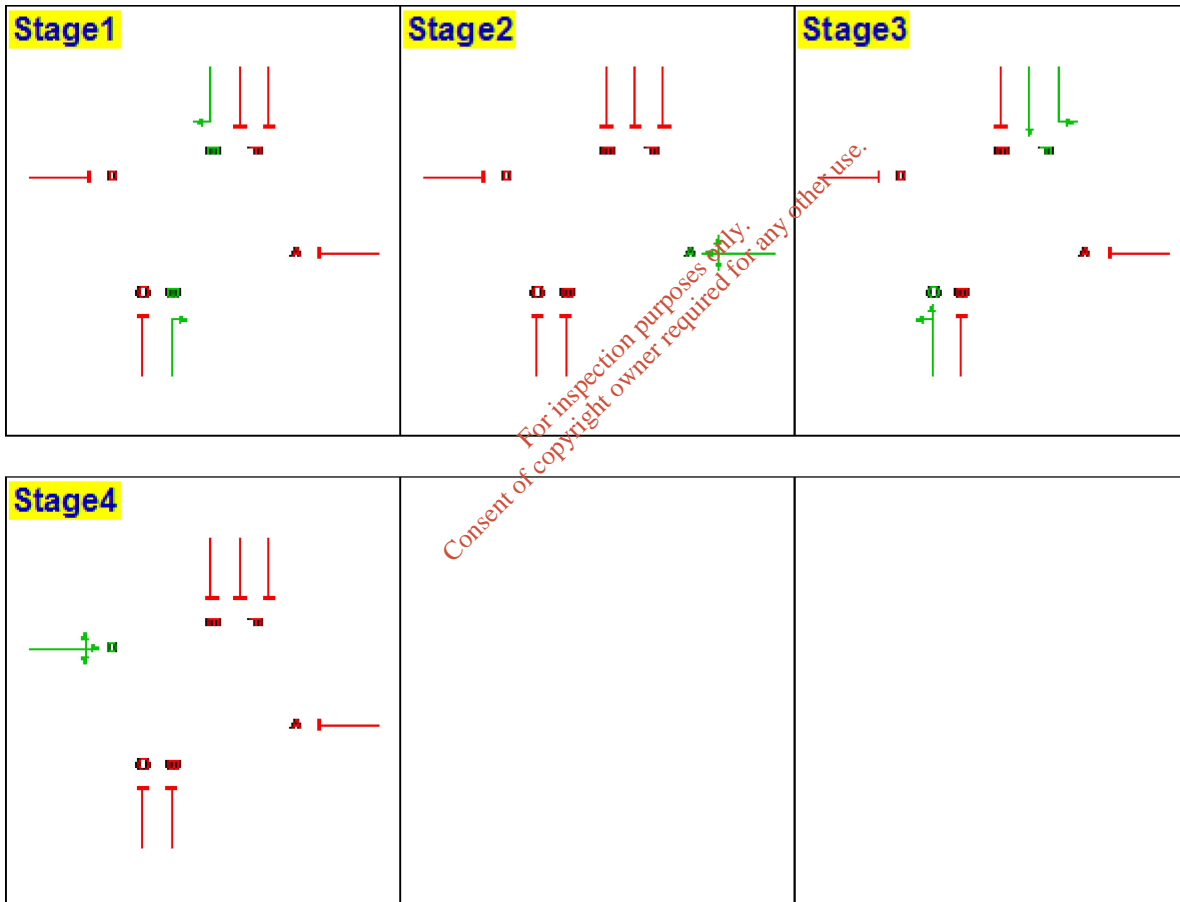
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	Yes	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set7 - 2017 PM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	92	223	218
	Arm 2	137	-	136	128
	Arm 3	180	176	-	24
	Arm 4	178	98	18	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	400	477	585	585	477	400
2 - R135 South	1	C	198	237	290	290	237	198
2 - R135 South	2	B	102	122	150	150	122	102
3 - L3125 West	1	D	282	337	413	413	337	282
4 - R135 North	1	F	207	248	303	303	248	207
4 - R135 North	2	E	13	16	19	19	16	13

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	34	32	34
3 - L3125 West	6	47	46
4 - R135 North	61	33	6

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	14.87	23.42	23.42	57.2

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-4.14	26.12	26.12	54.00

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

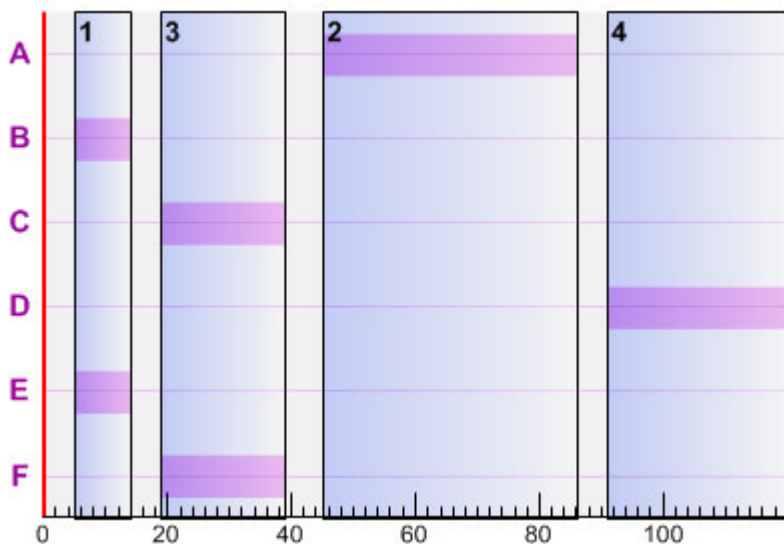
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	9.0	14.0
3	19.0	20.0	39.0
2	45.0	41.0	86.0
4	91.0	29.0	0.0

Phase Timings

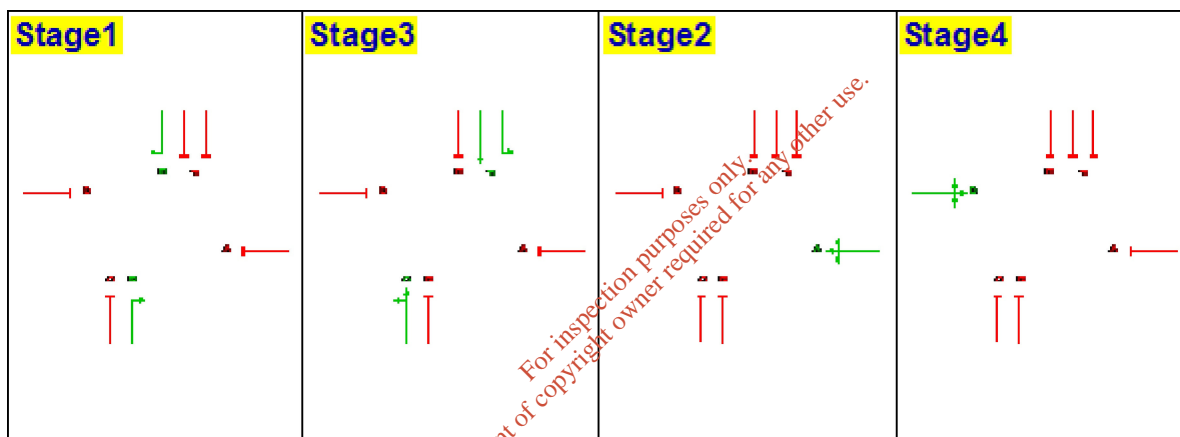
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	45	41.0	86						
B	5	9.0	14						
C	19	20.0	39						
D	91	29.0	0						
E	5	9.0	14						
F	19	20.0	39						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	487	A	42.50	46.47	6.29	76.78	17.22	1.83	13.45	11.62	22.90
2	1	242	C	21.50	73.39	4.93	78.35	14.87	1.91	8.87	6.96	6.40
2	2	125	B	10.50	97.02	3.37	76.48	17.68	1.61	5.51	3.90	2.40
3	1	344	D	30.50	56.61	5.41	75.23	19.63	1.60	10.78	9.18	12.50
4	1	253	F	21.50	45.40	3.19	37.31	141.24	0.15	7.45	7.29	12.40
4	2	16	E	10.50	51.97	0.23	9.79	819.41	0.01	0.50	0.49	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	400	1	42.50	38.31	4.26	63.06	42.72	0.75	10.11	9.36	4.20
2	1	198	3	21.50	58.05	3.19	64.10	40.40	0.74	6.39	5.64	1.10
2	2	102	2	10.50	73.29	2.08	62.40	44.22	0.64	3.81	3.17	0.40
3	1	282	4	30.50	47.43	3.72	61.67	45.93	0.67	8.10	7.43	2.20
4	1	207	6	21.50	44.29	2.55	30.52	194.85	0.09	6.00	5.91	1.80
4	2	13	5	10.50	51.56	0.19	7.95	9999.00	0.00	0.40	0.40	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	477	1	42.50	44.75	5.93	75.20	19.68	1.57	12.92	11.35	3.90
2	1	237	3	21.50	68.91	4.54	76.73	17.30	1.55	8.36	6.81	1.10
2	2	122	2	10.50	88.60	3.00	74.64	20.58	1.25	5.05	3.80	0.40
3	1	337	4	30.50	54.55	5.11	73.70	22.11	1.36	10.35	8.98	2.10
4	1	248	6	21.50	45.27	3.12	36.57	146.10	0.15	7.29	7.14	2.00
4	2	16	5	10.50	51.96	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	585	1	42.50	64.24	10.44	92.23	-2.41	5.55	19.81	14.26	2.50
2	1	290	3	21.50	97.28	7.84	93.89	-4.14	4.78	13.20	8.42	0.80
2	2	150	2	10.50	121.27	5.05	91.77	-1.93	3.25	7.96	4.70	0.40
3	1	413	4	30.50	74.18	8.51	90.32	-0.36	4.23	15.42	11.19	1.60
4	1	303	6	21.50	46.75	3.93	44.68	101.43	0.25	9.08	8.83	2.30
4	2	19	5	10.50	52.38	0.28	11.62	674.24	0.01	0.59	0.58	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	585	1	42.50	72.56	11.79	92.23	-2.41	6.23	20.49	14.26	2.50
2	1	290	3	21.50	116.89	9.42	93.89	-4.14	5.74	14.16	8.42	0.80
2	2	150	2	10.50	146.58	6.11	91.77	-1.93	3.90	8.61	4.70	0.40
3	1	413	4	30.50	82.82	9.50	90.32	-0.36	4.71	15.91	11.19	1.50
4	1	303	6	21.50	46.77	3.94	44.68	101.43	0.25	9.08	8.83	2.30
4	2	19	5	10.50	52.38	0.28	11.62	674.24	0.01	0.59	0.58	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	477	1	42.50	48.94	6.48	75.20	19.68	1.76	13.12	11.35	3.80
2	1	237	3	21.50	86.48	5.69	76.73	17.30	1.98	8.79	6.81	1.10
2	2	122	2	10.50	118.83	4.03	74.64	20.58	1.75	5.56	3.80	0.40
3	1	337	4	30.50	59.89	5.61	73.70	22.11	1.57	10.55	8.98	2.10
4	1	248	6	21.50	45.29	3.12	36.57	146.10	0.15	7.29	7.14	2.00
4	2	16	5	10.50	51.97	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	400	1	42.50	38.96	4.33	63.06	42.72	0.79	10.15	9.36	4.20
2	1	198	3	21.50	61.35	3.37	64.10	40.40	0.83	6.47	5.64	1.10
2	2	102	2	10.50	82.55	2.34	62.40	44.22	0.76	3.93	3.17	0.40
3	1	282	4	30.50	48.50	3.80	61.67	45.93	0.72	8.15	7.43	2.20
4	1	207	6	21.50	44.31	2.55	30.52	194.85	0.09	6.00	5.91	1.80
4	2	13	5	10.50	51.57	0.19	7.95	9999.00	0.00	0.40	0.40	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads PM.osc
Report generation date: 09/12/2016 11:32:51

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

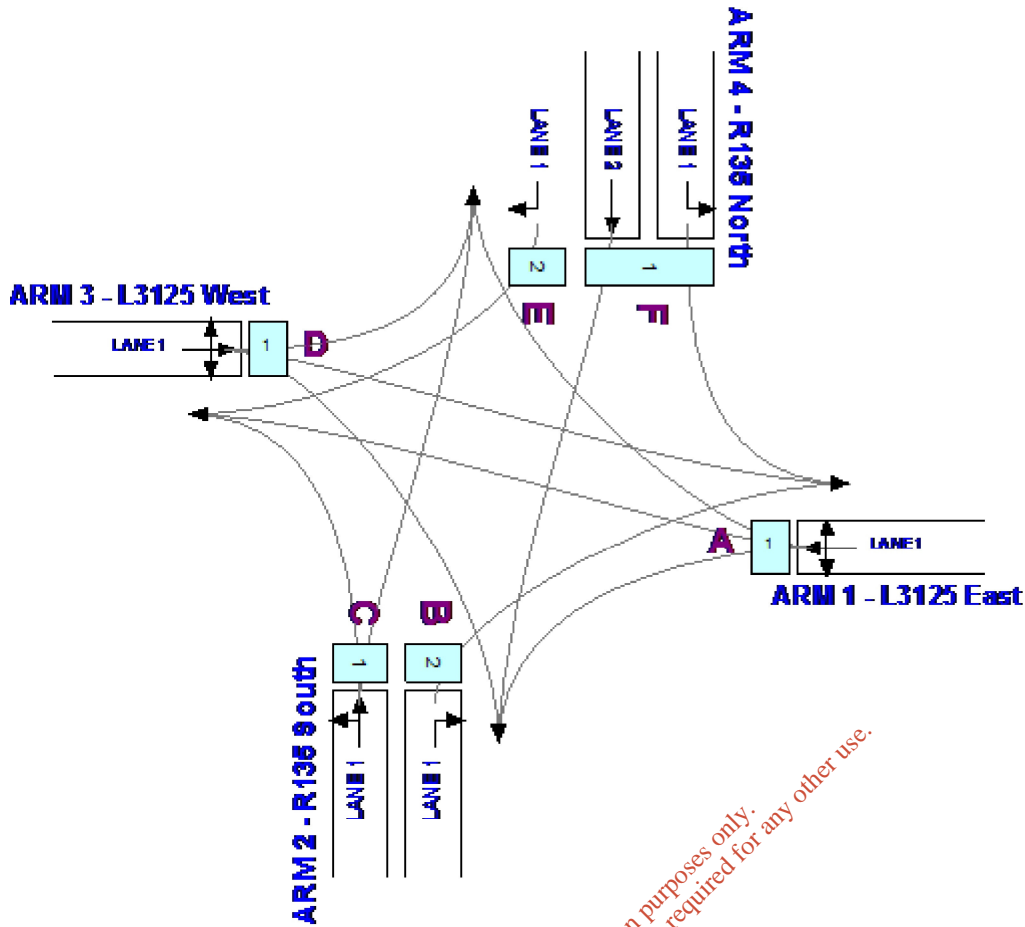
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

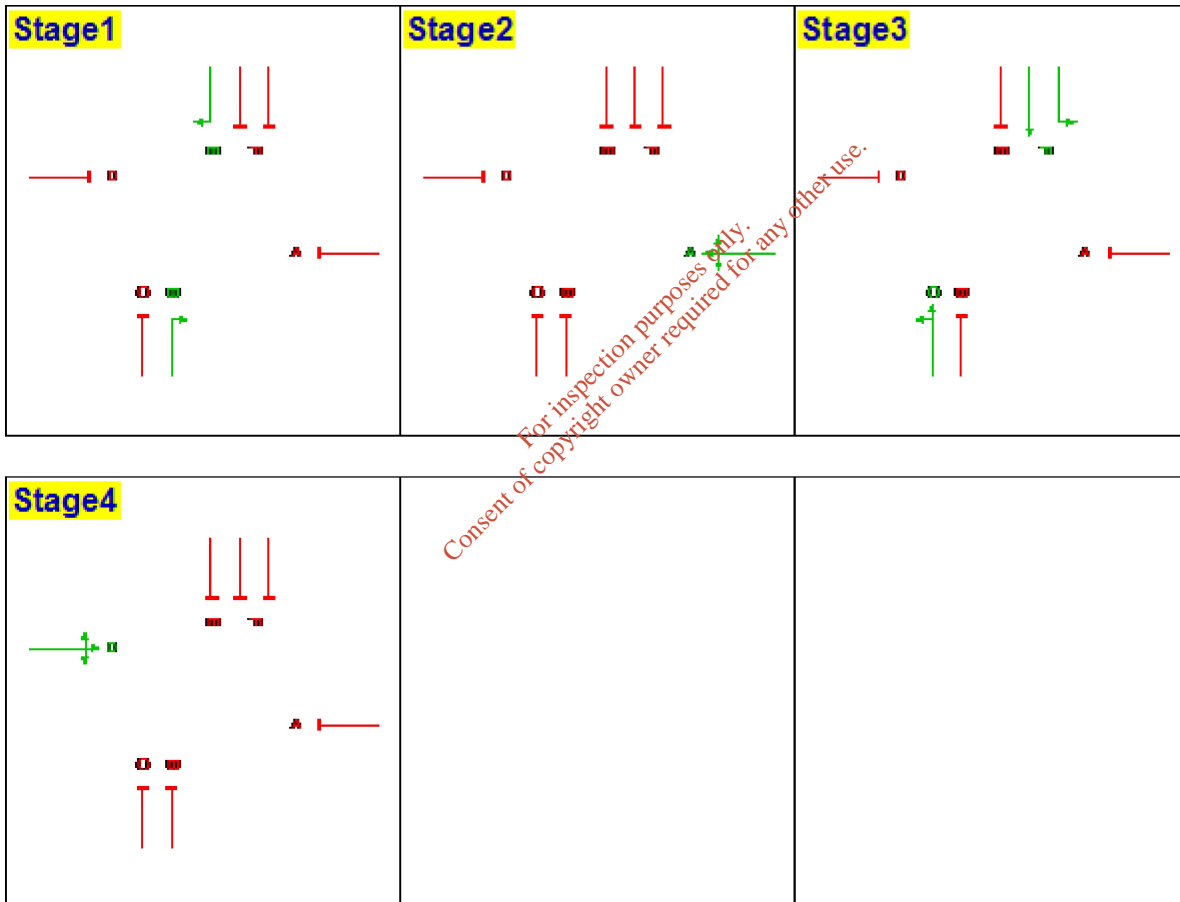
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	Yes	16:30	18:00	ODTAB	No	D1
2022 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2032 PM Peak + Link	No	16:30	18:00	ODTAB	No	D1

Demand Set8 - 2017 PM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	92	223	218
	Arm 2	137	-	136	129
	Arm 3	180	176	-	24
	Arm 4	178	99	18	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	400	477	585	585	477	400
2 - R135 South	1	C	199	238	291	291	238	199
2 - R135 South	2	B	103	122	150	150	122	103
3 - L3125 West	1	D	282	337	413	413	337	282
4 - R135 North	1	F	208	248	304	304	248	208
4 - R135 North	2	E	13	16	19	19	16	13

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	34	32	34
3 - L3125 West	6	47	46
4 - R135 North	60	34	6

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	15.71	23.32	23.32	57.5

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-3.63	26.05	26.05	54.50

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

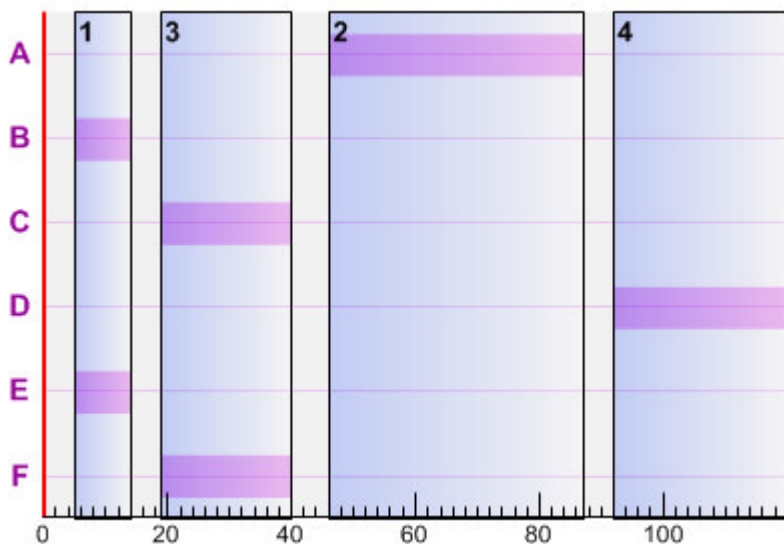
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	9.0	14.0
3	19.0	21.0	40.0
2	46.0	41.0	87.0
4	92.0	28.0	0.0

Phase Timings

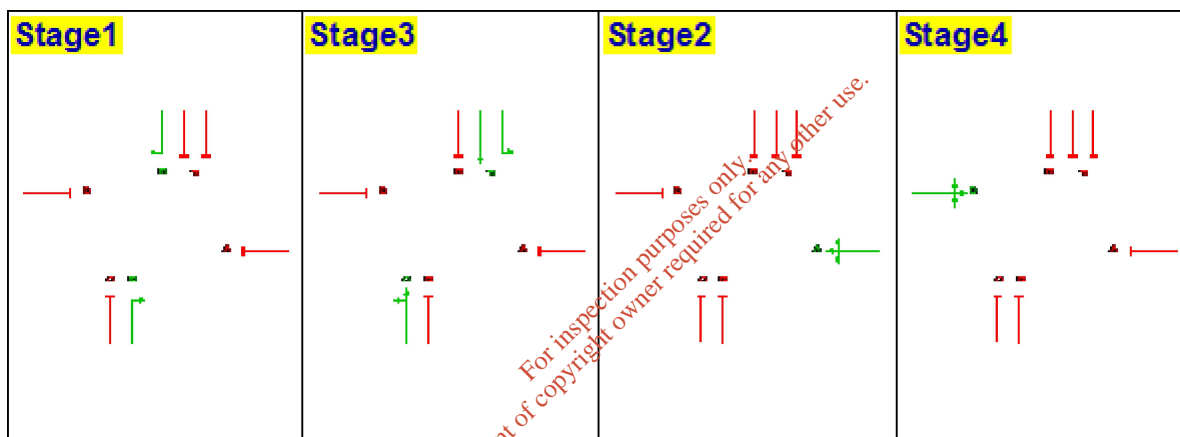
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	46	41.0	87						
B	5	9.0	14						
C	19	21.0	40						
D	92	28.0	0						
E	5	9.0	14						
F	19	21.0	40						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	487	A	42.50	46.47	6.29	76.78	17.22	1.83	13.45	11.62	22.90
2	1	243	C	22.50	67.50	4.56	75.17	19.72	1.55	8.46	6.92	7.10
2	2	125	B	10.50	97.02	3.37	76.48	17.68	1.61	5.51	3.90	2.40
3	1	344	D	29.50	60.33	5.76	77.78	15.71	1.89	11.18	9.29	11.40
4	1	253	F	22.50	44.30	3.11	35.65	152.46	0.14	7.35	7.22	13.10
4	2	16	E	10.50	51.97	0.23	9.79	819.41	0.01	0.50	0.49	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	400	1	42.50	38.31	4.26	63.06	42.72	0.75	10.11	9.36	4.20
2	1	199	3	22.50	55.58	3.07	61.56	46.19	0.65	6.26	5.61	1.20
2	2	103	2	10.50	73.75	2.11	63.02	42.82	0.66	3.86	3.20	0.40
3	1	282	4	29.50	49.26	3.86	63.76	41.14	0.76	8.27	7.51	2.10
4	1	208	6	22.50	43.29	2.50	29.31	207.08	0.08	5.96	5.88	1.90
4	2	13	5	10.50	51.56	0.19	7.95	9999.00	0.00	0.40	0.40	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	477	1	42.50	44.75	5.93	75.20	19.68	1.57	12.92	11.35	3.90
2	1	238	3	22.50	64.45	4.26	73.63	22.24	1.30	8.06	6.77	1.20
2	2	122	2	10.50	88.76	3.01	74.64	20.58	1.25	5.05	3.80	0.40
3	1	337	4	29.50	57.58	5.39	76.20	18.11	1.58	10.67	9.08	1.90
4	1	248	6	22.50	44.18	3.04	34.94	157.55	0.13	7.20	7.07	2.20
4	2	16	5	10.50	51.96	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	585	1	42.50	64.24	10.44	92.23	-2.41	5.55	19.81	14.26	2.50
2	1	291	3	22.50	86.83	7.02	90.02	-0.03	3.70	12.07	8.37	1.00
2	2	150	2	10.50	121.40	5.06	91.77	-1.93	3.25	7.96	4.70	0.40
3	1	413	4	29.50	81.65	9.37	93.39	-3.63	5.32	16.63	11.32	1.40
4	1	304	6	22.50	45.56	3.85	42.84	110.10	0.22	8.99	8.77	2.40
4	2	19	5	10.50	52.38	0.28	11.62	674.24	0.01	0.59	0.58	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	585	1	42.50	72.56	11.79	92.23	-2.41	6.23	20.49	14.26	2.50
2	1	291	3	22.50	98.59	7.97	90.02	-0.03	4.19	12.56	8.37	0.90
2	2	150	2	10.50	146.58	6.11	91.77	-1.93	3.90	8.61	4.70	0.40
3	1	413	4	29.50	95.57	10.96	93.39	-3.63	6.22	17.53	11.32	1.30
4	1	304	6	22.50	45.57	3.85	42.84	110.10	0.22	8.99	8.77	2.40
4	2	19	5	10.50	52.38	0.28	11.62	674.24	0.01	0.59	0.58	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	477	1	42.50	48.94	6.48	75.20	19.68	1.76	13.12	11.35	3.80
2	1	238	3	22.50	73.55	4.86	73.63	22.24	1.56	8.33	6.77	1.20
2	2	122	2	10.50	118.83	4.03	74.64	20.58	1.75	5.56	3.80	0.40
3	1	337	4	29.50	66.99	6.27	76.20	18.11	1.88	10.97	9.08	1.90
4	1	248	6	22.50	44.19	3.04	34.94	157.55	0.13	7.20	7.07	2.20
4	2	16	5	10.50	51.97	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	400	1	42.50	38.96	4.33	63.06	42.72	0.79	10.15	9.36	4.20
2	1	199	3	22.50	57.69	3.19	61.56	46.19	0.71	6.32	5.61	1.20
2	2	103	2	10.50	83.16	2.38	63.02	42.82	0.79	3.99	3.20	0.40
3	1	282	4	29.50	50.76	3.98	63.76	41.14	0.81	8.33	7.51	2.10
4	1	208	6	22.50	43.30	2.50	29.31	207.08	0.08	5.96	5.88	1.90
4	2	13	5	10.50	51.57	0.19	7.95	9999.00	0.00	0.40	0.40	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads PM.osc
Report generation date: 09/12/2016 10:59:37

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

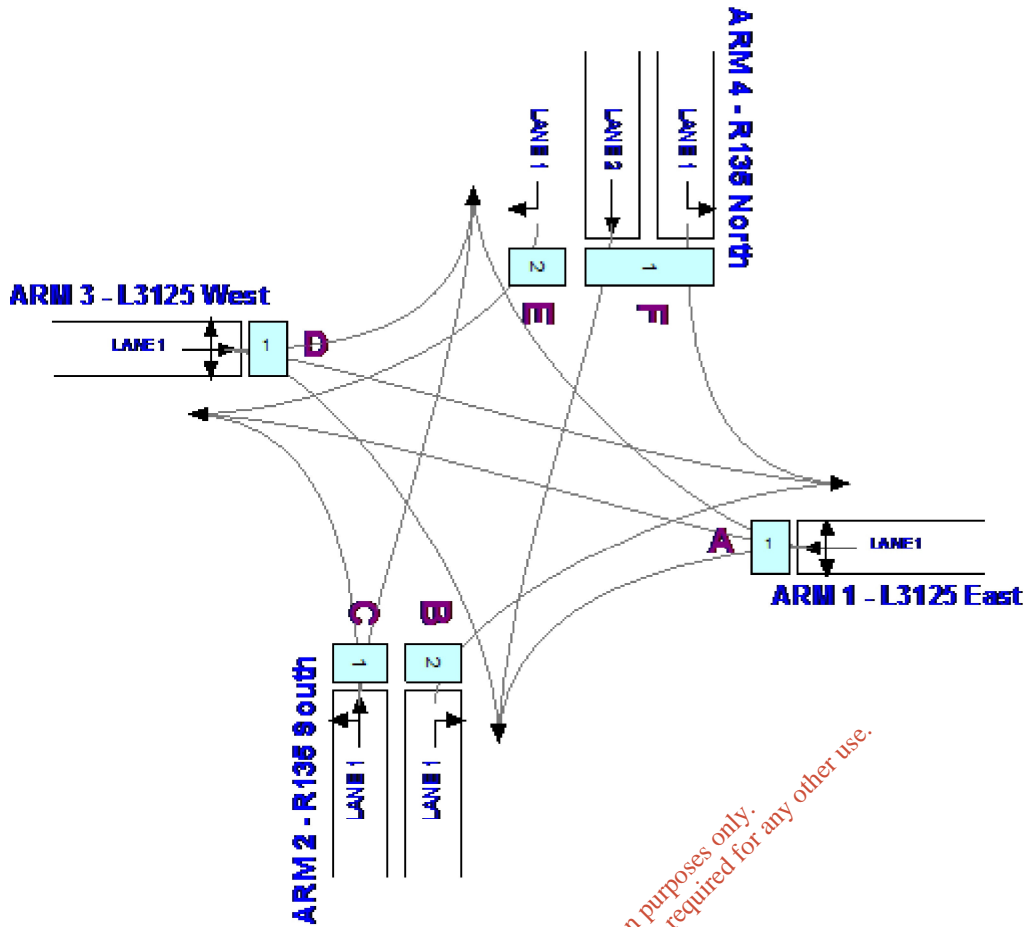
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

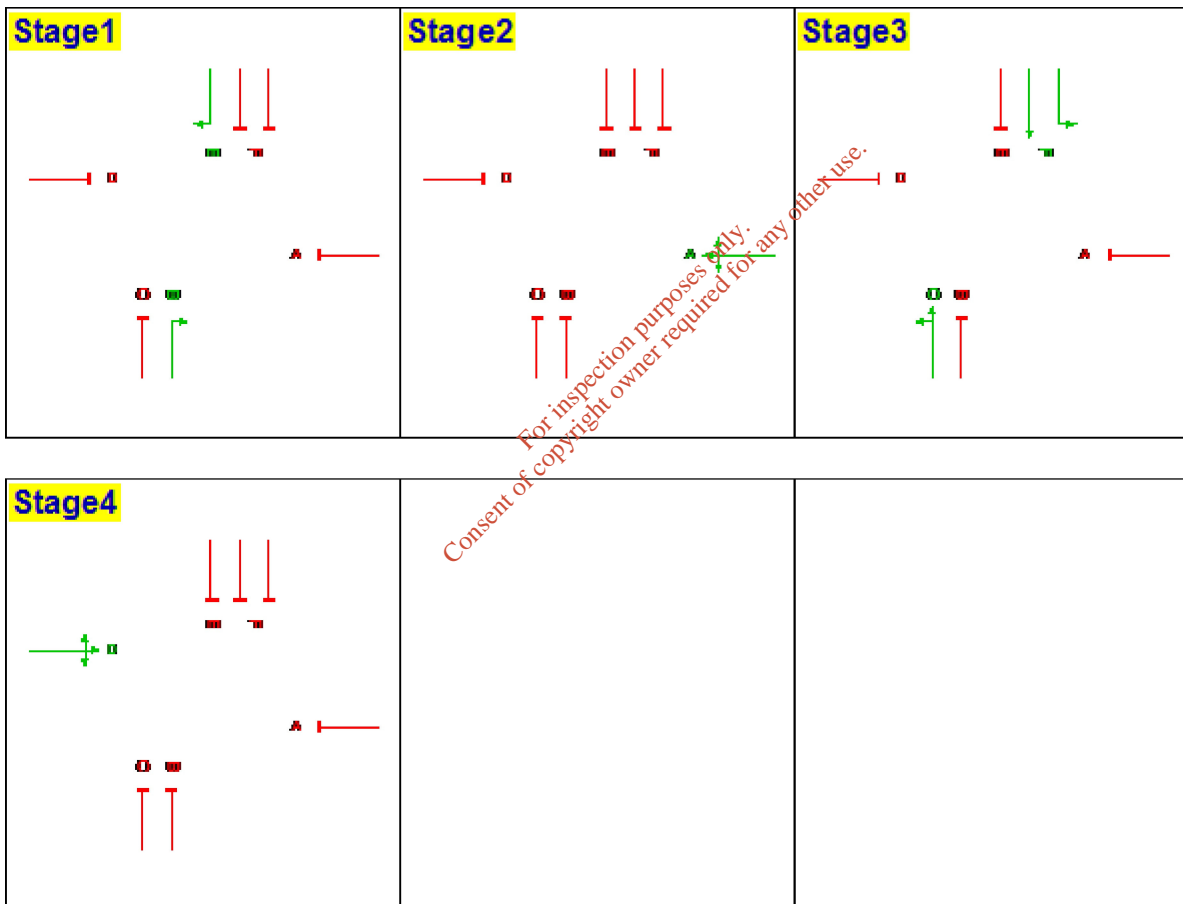
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2022 PM Peak No Dev	Yes	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set9 - 2022 PM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	96	232	227
	Arm 2	143	-	142	133
	Arm 3	188	184	-	25
	Arm 4	186	102	19	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	416	497	609	609	497	416
2 - R135 South	1	C	207	247	303	303	247	207
2 - R135 South	2	B	107	127	156	156	127	107
3 - L3125 West	1	D	295	352	431	431	352	295
4 - R135 North	1	F	216	258	317	317	258	216
4 - R135 North	2	E	14	16	20	20	16	14

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	34	32	34
3 - L3125 West	6	47	46
4 - R135 North	61	33	6

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	10.32	25.83	25.83	55.6

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-8.25	30.07	30.07	52.10

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

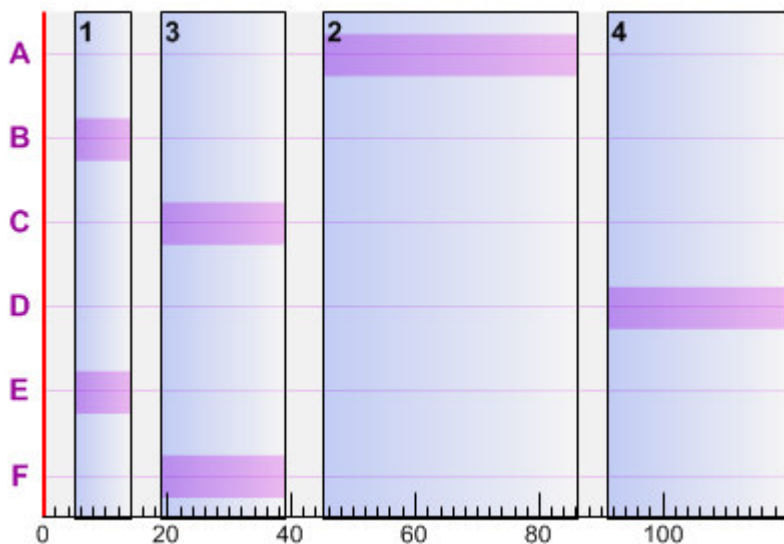
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	9.0	14.0
3	19.0	20.0	39.0
2	45.0	41.0	86.0
4	91.0	29.0	0.0

Phase Timings

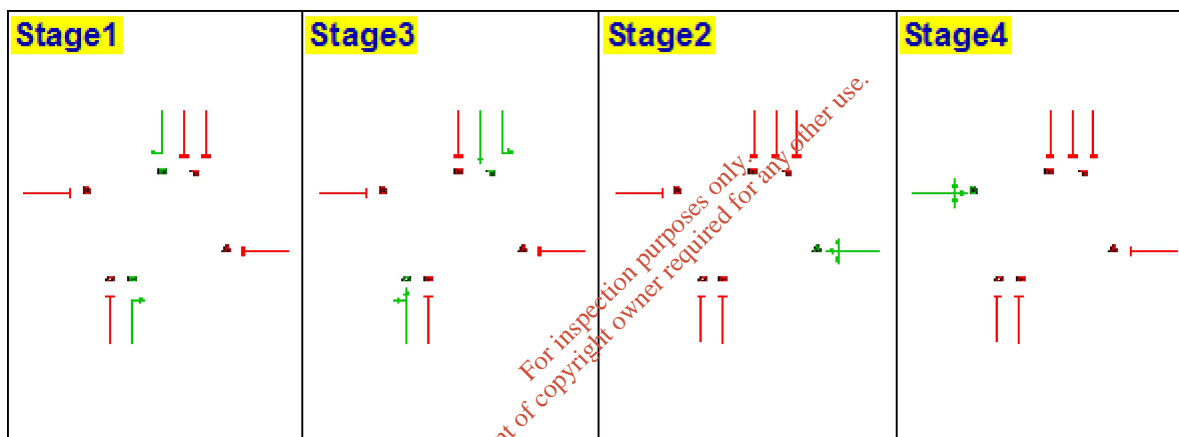
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	45	41.0	86						
B	5	9.0	14						
C	19	20.0	39						
D	91	29.0	0						
E	5	9.0	14						
F	19	20.0	39						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	507	A	42.50	49.36	6.95	79.93	12.60	2.30	14.45	12.15	21.70
2	1	252	C	21.50	78.99	5.53	81.58	10.32	2.42	9.68	7.26	6.20
2	2	130	B	10.50	104.39	3.77	79.54	13.16	1.98	6.04	4.06	2.40
3	1	359	D	30.50	59.98	5.98	78.51	14.63	2.00	11.62	9.62	12.00
4	1	264	F	21.50	45.69	3.35	38.93	131.19	0.17	7.80	7.63	12.70
4	2	17	E	10.50	52.10	0.25	10.40	765.32	0.01	0.53	0.52	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	416	1	42.50	39.33	4.54	65.58	37.23	0.87	10.64	9.77	4.20
2	1	207	3	21.50	59.76	3.44	67.02	34.30	0.88	6.79	5.91	1.10
2	2	107	2	10.50	75.67	2.25	65.46	37.48	0.75	4.08	3.33	0.40
3	1	295	4	30.50	48.69	3.99	64.52	39.50	0.79	8.59	7.79	2.20
4	1	216	6	21.50	44.50	2.67	31.85	182.56	0.10	6.28	6.18	1.90
4	2	14	5	10.50	51.69	0.20	8.57	950.75	0.01	0.43	0.43	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	42.50	47.06	6.50	78.35	14.87	1.93	13.81	11.88	3.70
2	1	247	3	21.50	72.72	4.99	79.97	12.55	1.89	9.00	7.11	1.00
2	2	127	2	10.50	93.32	3.29	77.70	15.83	1.48	5.44	3.96	0.40
3	1	352	4	30.50	57.19	5.59	76.98	16.91	1.67	11.09	9.41	2.00
4	1	258	6	21.50	45.52	3.26	38.04	136.56	0.16	7.60	7.44	2.10
4	2	16	5	10.50	51.96	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	609	1	42.50	72.29	12.23	96.01	-6.26	7.82	22.75	14.93	2.10
2	1	303	3	21.50	108.25	9.11	98.10	-8.25	6.43	15.26	8.83	0.70
2	2	156	2	10.50	131.49	5.70	95.44	-5.70	4.01	8.91	4.90	0.40
3	1	431	4	30.50	82.25	9.85	94.26	-4.52	5.78	17.51	11.73	1.40
4	1	317	6	21.50	47.47	4.15	46.75	92.53	0.29	9.55	9.26	2.30
4	2	20	5	10.50	52.52	0.29	12.24	635.52	0.01	0.62	0.61	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	609	1	42.50	87.90	14.87	96.01	-6.26	9.48	24.41	14.93	2.00
2	1	303	3	21.50	139.59	11.75	98.10	-8.25	8.35	17.17	8.83	0.70
2	2	156	2	10.50	165.93	7.19	95.44	-5.70	5.05	9.95	4.90	0.40
3	1	431	4	30.50	97.56	11.68	94.26	-4.52	6.86	18.58	11.73	1.30
4	1	317	6	21.50	47.19	4.16	46.75	92.53	0.29	9.55	9.26	2.30
4	2	20	5	10.50	52.52	0.29	12.24	635.52	0.01	0.62	0.61	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	42.50	56.04	7.74	78.35	14.87	2.25	14.13	11.88	3.60
2	1	247	3	21.50	107.47	7.37	79.97	12.55	2.62	9.73	7.11	1.00
2	2	127	2	10.50	140.11	4.94	77.70	15.83	2.25	6.21	3.96	0.40
3	1	352	4	30.50	67.69	6.62	76.98	16.91	2.00	11.42	9.41	2.00
4	1	258	6	21.50	45.55	3.26	38.04	136.56	0.16	7.61	7.44	2.10
4	2	16	5	10.50	51.97	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	416	1	42.50	40.27	4.65	65.58	37.23	0.93	10.69	9.77	4.20
2	1	207	3	21.50	64.77	3.72	67.02	34.30	1.00	6.91	5.91	1.10
2	2	107	2	10.50	89.14	2.65	65.46	37.48	0.92	4.25	3.33	0.40
3	1	295	4	30.50	50.24	4.12	64.52	39.50	0.85	8.65	7.79	2.20
4	1	216	6	21.50	44.52	2.67	31.85	182.56	0.10	6.28	6.18	1.90
4	2	14	5	10.50	51.71	0.20	8.57	950.75	0.01	0.43	0.43	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads PM.osc
Report generation date: 09/12/2016 11:35:27

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

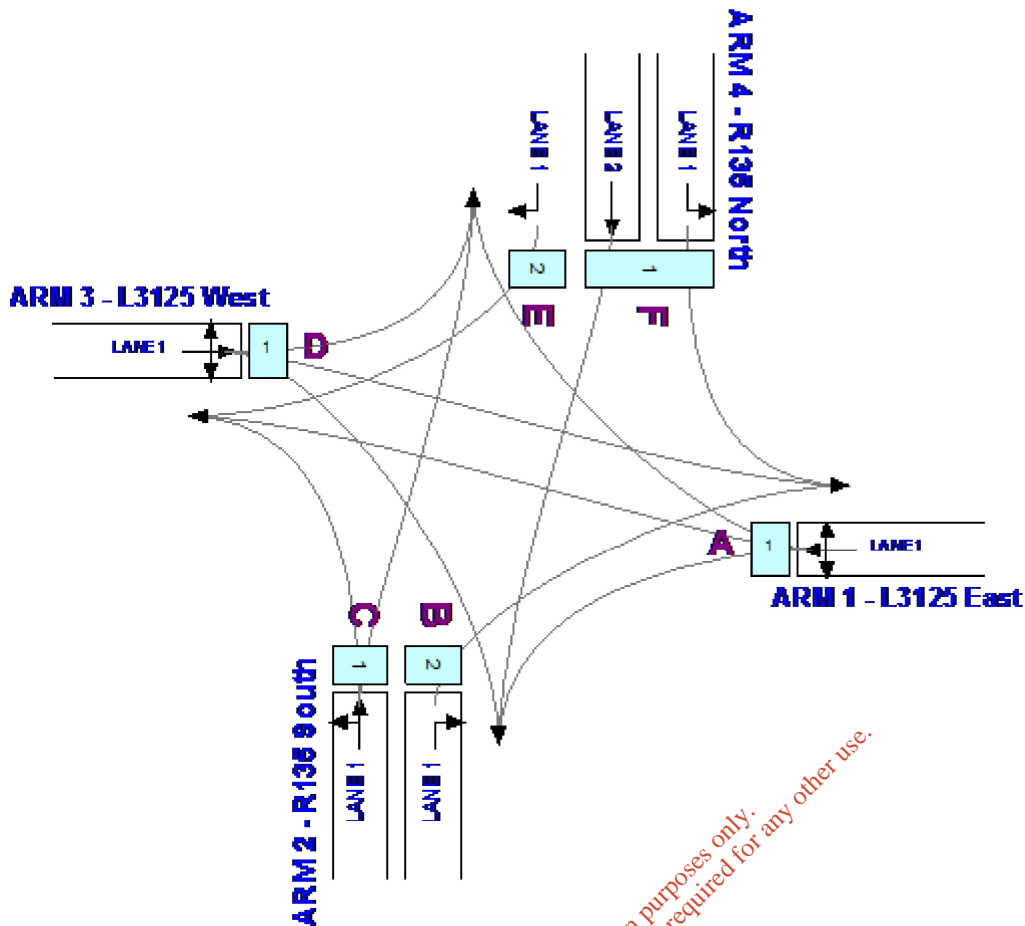
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

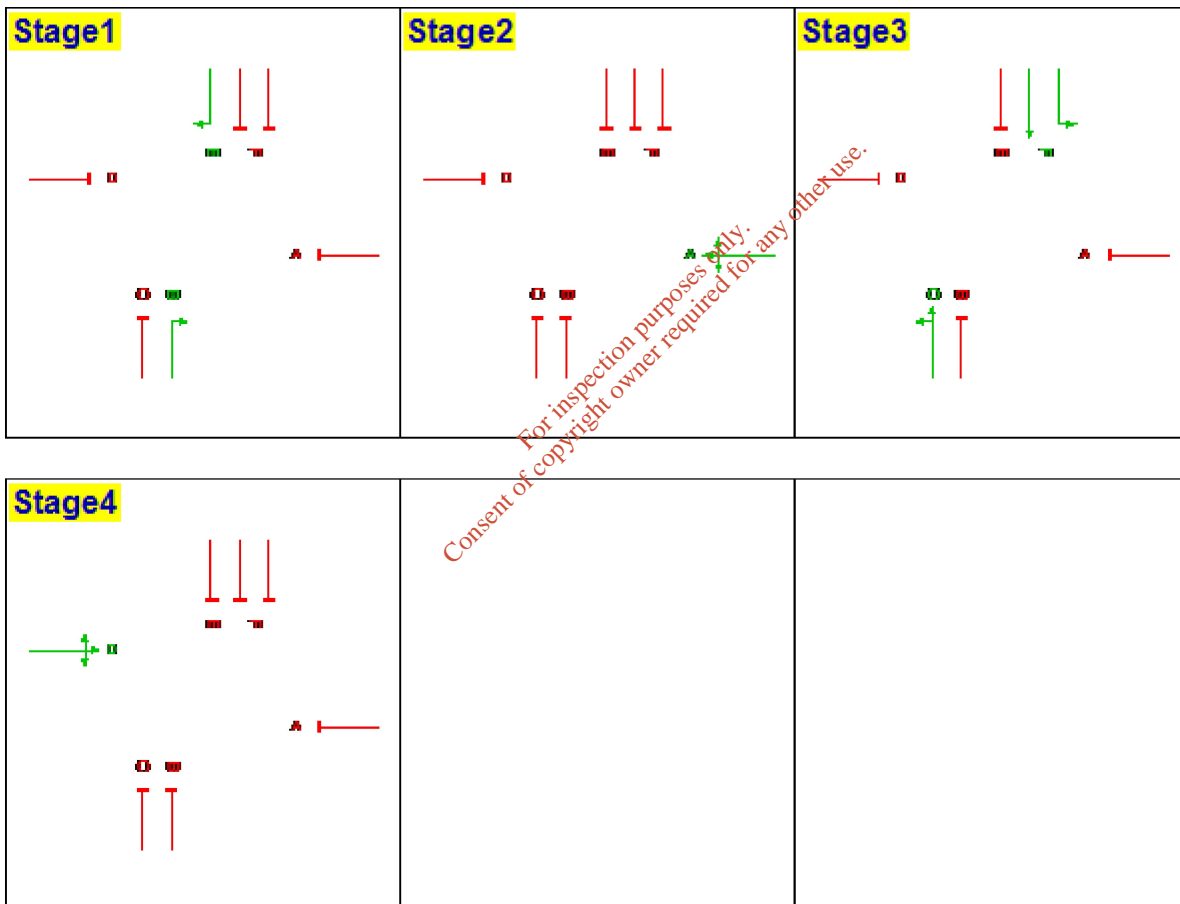
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2022 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2022 PM Peak With Dev	Yes	16:30	18:00	ODTAB	No	D1

Demand Set10 - 2022 PM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	96	232	227
	Arm 2	143	-	142	135
	Arm 3	188	184	-	25
	Arm 4	186	103	19	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	416	497	609	609	497	416
2 - R135 South	1	C	208	248	304	304	248	208
2 - R135 South	2	B	107	128	157	157	128	107
3 - L3125 West	1	D	295	352	431	431	352	295
4 - R135 North	1	F	215	257	314	314	257	215
4 - R135 North	2	E	14	17	20	20	17	14

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	34	32	34
3 - L3125 West	6	47	46
4 - R135 North	60	33	6

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	10.87	25.77	25.77	55.9

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-7.65	30.05	30.05	52.90

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

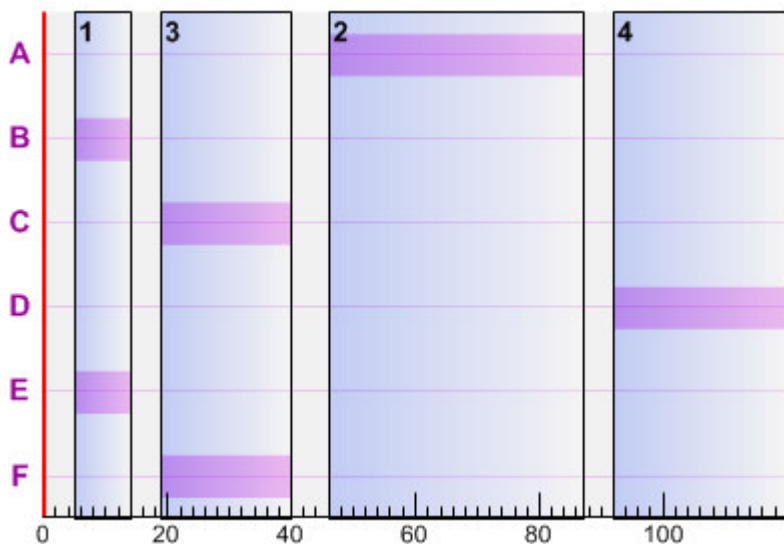
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	9.0	14.0
3	19.0	21.0	40.0
2	46.0	41.0	87.0
4	92.0	28.0	0.0

Phase Timings

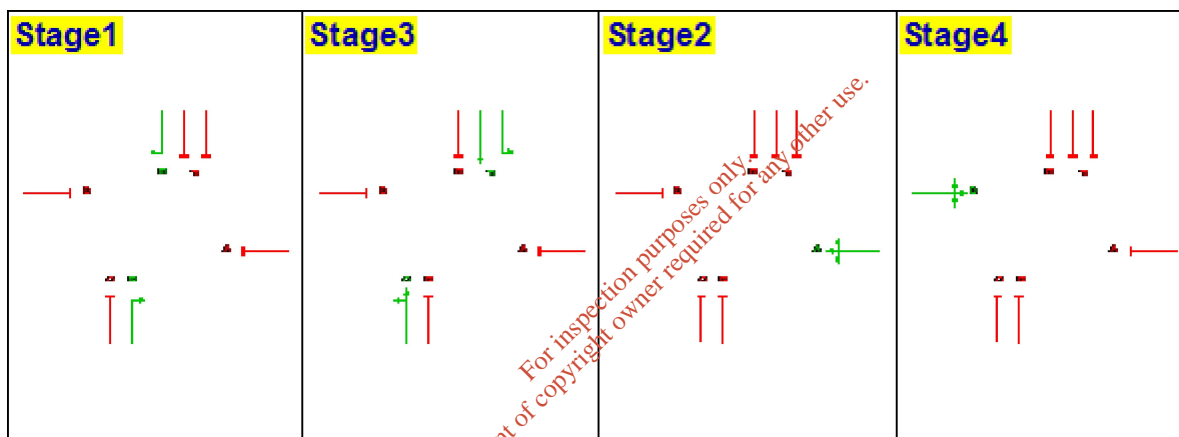
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	46	41.0	87						
B	5	9.0	14						
C	19	21.0	40						
D	92	28.0	0						
E	5	9.0	14						
F	19	21.0	40						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	507	A	42.50	49.36	6.95	79.93	12.60	2.30	14.45	12.15	21.70
2	1	253	C	22.50	71.56	5.03	78.27	14.99	1.91	9.13	7.22	6.90
2	2	131	B	10.50	106.06	3.86	80.15	12.29	2.06	6.15	4.09	2.40
3	1	359	D	29.50	64.65	6.45	81.18	10.87	2.42	12.15	9.72	10.90
4	1	262	F	22.50	44.51	3.24	36.92	143.79	0.15	7.64	7.49	13.40
4	2	17	E	10.50	52.10	0.25	10.40	765.32	0.01	0.53	0.52	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	416	1	42.50	39.33	4.54	65.58	37.23	0.87	10.64	9.77	4.20
2	1	208	3	22.50	57.04	3.30	64.35	39.87	0.76	6.64	5.88	1.20
2	2	107	2	10.50	75.67	2.25	65.46	37.48	0.75	4.08	3.33	0.40
3	1	295	4	29.50	50.68	4.15	66.70	34.92	0.90	8.78	7.88	2.10
4	1	215	6	22.50	43.44	2.59	30.30	197.08	0.09	6.18	6.08	2.00
4	2	14	5	10.50	51.69	0.20	8.57	950.75	0.01	0.43	0.43	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	42.50	47.06	6.50	78.35	14.87	1.93	13.81	11.88	3.70
2	1	248	3	22.50	67.45	4.65	76.72	17.31	1.56	8.63	7.07	1.20
2	2	128	2	10.50	94.15	3.35	78.31	14.93	1.52	5.52	4.00	0.40
3	1	352	4	29.50	60.76	5.94	79.59	13.08	1.97	11.48	9.52	1.90
4	1	257	6	22.50	44.39	3.17	36.21	148.53	0.14	7.48	7.34	2.20
4	2	17	5	10.50	52.09	0.25	10.40	765.32	0.01	0.53	0.52	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	609	1	42.50	72.29	12.23	96.01	-6.26	7.82	22.75	14.93	2.10
2	1	304	3	22.50	95.67	6.08	94.04	-4.30	4.92	13.69	8.77	0.90
2	2	157	2	10.50	133.33	5.81	96.05	-6.30	4.16	9.09	4.93	0.40
3	1	431	4	29.50	91.53	10.96	97.46	-7.65	7.38	19.24	11.86	1.20
4	1	314	6	22.50	45.83	4.00	44.24	103.41	0.25	9.32	9.07	2.50
4	2	20	5	10.50	52.52	0.29	12.24	635.52	0.01	0.62	0.61	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	609	1	42.50	87.90	14.87	96.01	-6.26	9.48	24.41	14.93	2.00
2	1	304	3	22.50	115.03	9.71	94.04	-4.30	5.92	14.68	8.77	0.90
2	2	157	2	10.50	169.38	7.39	96.05	-6.30	5.27	10.20	4.93	0.40
3	1	431	4	29.50	115.61	13.84	97.46	-7.65	9.39	21.24	11.86	1.10
4	1	314	6	22.50	45.85	4.00	44.24	103.41	0.25	9.32	9.07	2.50
4	2	20	5	10.50	52.52	0.29	12.24	635.52	0.01	0.62	0.61	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	42.50	56.04	7.74	78.35	14.87	2.25	14.13	11.88	3.60
2	1	248	3	22.50	84.27	5.80	76.72	17.31	1.98	9.04	7.07	1.10
2	2	128	2	10.50	144.60	5.14	78.31	14.93	2.36	6.36	4.00	0.40
3	1	352	4	29.50	81.15	7.94	79.59	13.08	2.49	12.01	9.52	1.80
4	1	257	6	22.50	44.41	3.17	36.21	148.53	0.14	7.48	7.34	2.20
4	2	17	5	10.50	52.11	0.25	10.40	765.32	0.01	0.53	0.52	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	416	1	42.50	40.27	4.65	65.58	37.23	0.93	10.69	9.77	4.20
2	1	208	3	22.50	60.06	3.47	64.35	39.87	0.84	6.72	5.88	1.20
2	2	107	2	10.50	89.98	2.67	65.46	37.48	0.92	4.25	3.33	0.40
3	1	295	4	29.50	52.97	4.34	66.70	34.92	0.98	8.86	7.88	2.10
4	1	215	6	22.50	43.45	2.60	30.30	197.08	0.09	6.18	6.08	2.00
4	2	14	5	10.50	51.71	0.20	8.57	950.75	0.01	0.43	0.43	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads PM.osc
Report generation date: 09/12/2016 11:04:29

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

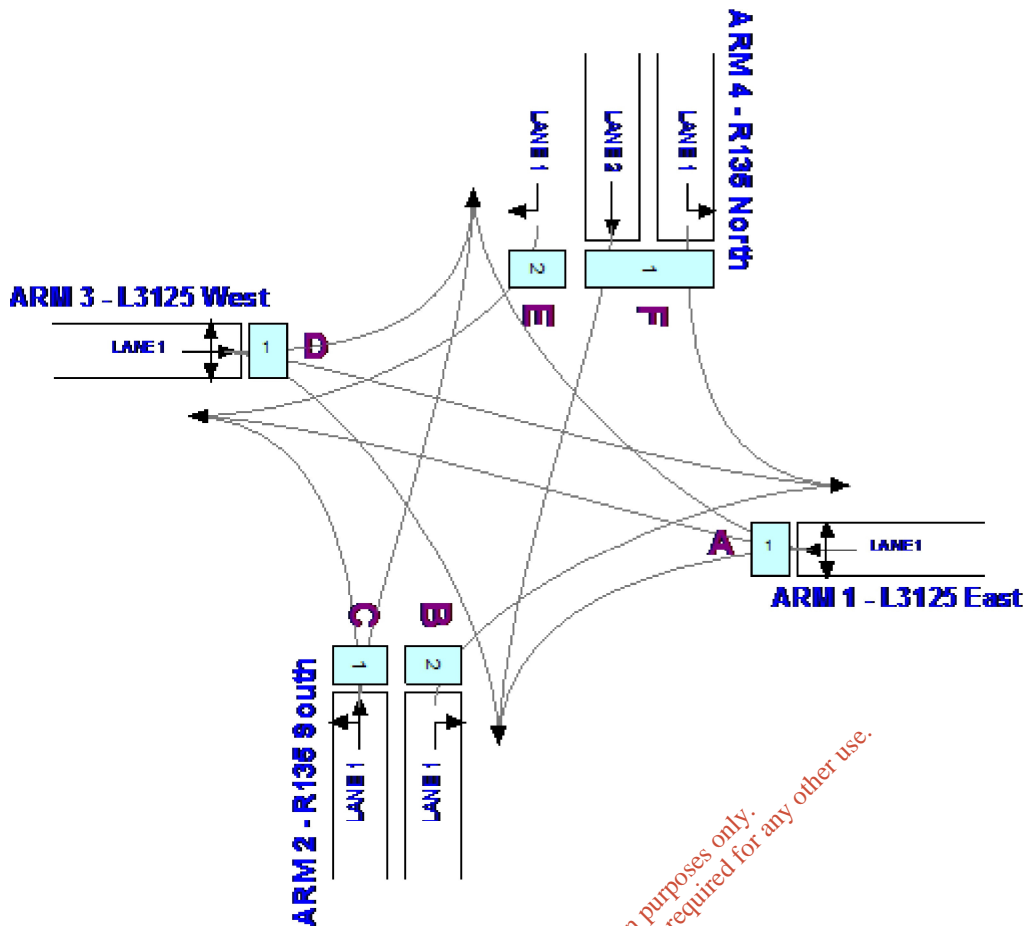
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

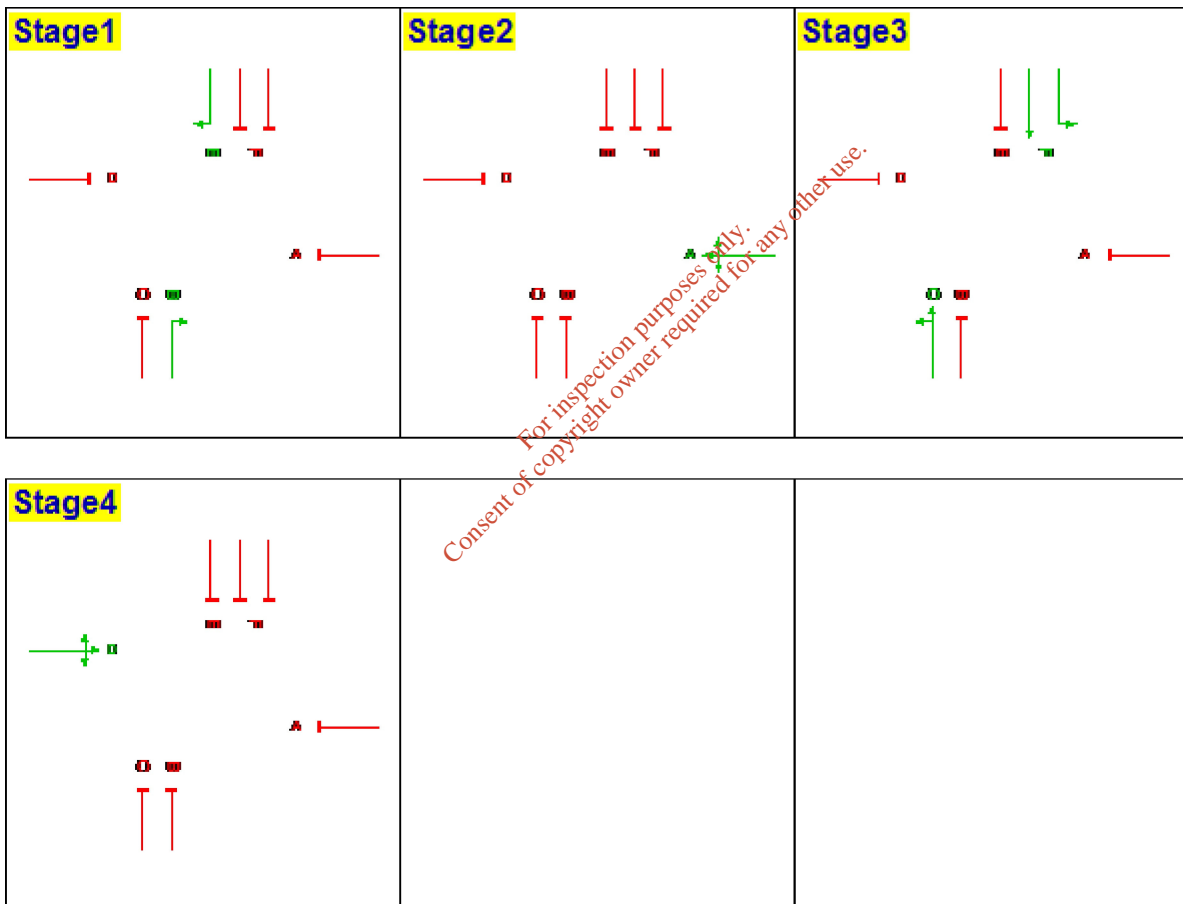
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2022 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2022 PM Peak Sen	Yes	16:30	18:00	ODTAB	No	D1

Demand Set10 - 2022 PM Peak Sen

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	96	232	227
	Arm 2	143	-	142	156
	Arm 3	188	184	-	25
	Arm 4	186	125	19	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	416	497	609	609	497	416
2 - R135 South	1	C	222	265	324	324	265	222
2 - R135 South	2	B	106	126	155	155	126	106
3 - L3125 West	1	D	295	352	431	431	352	295
4 - R135 North	1	F	233	278	340	340	278	233
4 - R135 North	2	E	15	18	22	22	18	15

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	32	35	32
3 - L3125 West	6	47	46
4 - R135 North	56	38	6

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	3.17	27.54	27.54	56.7

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-14.13	32.53	32.53	53.10

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

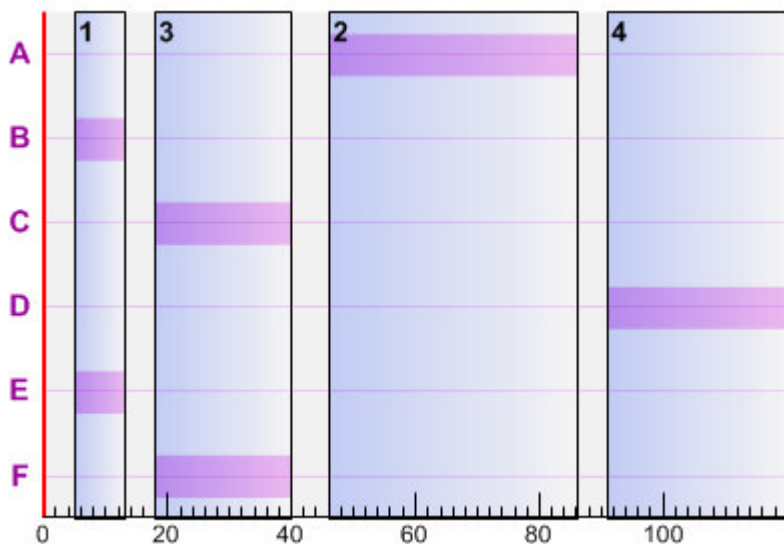
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	8.0	13.0
3	18.0	22.0	40.0
2	46.0	40.0	86.0
4	91.0	29.0	0.0

Phase Timings

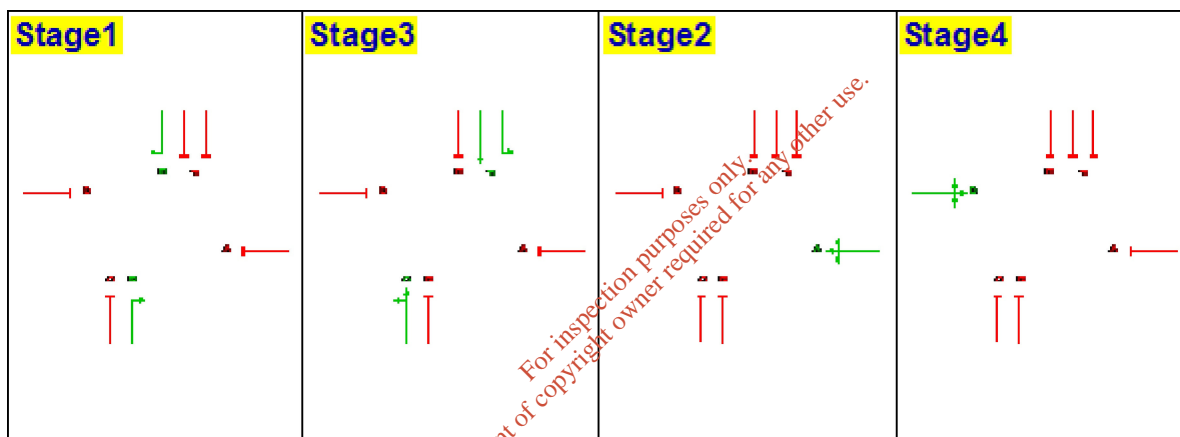
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	46	40.0	86						
B	5	8.0	13						
C	18	22.0	40						
D	91	29.0	0						
E	5	8.0	13						
F	18	22.0	40						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	507	A	41.50	52.44	7.39	81.86	9.95	2.66	14.96	12.30	20.00
2	1	270	C	23.50	72.52	5.44	79.97	12.54	2.16	9.81	7.65	7.20
2	2	129	B	9.50	139.44	5.00	87.23	3.17	3.48	7.54	4.06	2.10
3	1	359	D	30.50	59.98	5.98	78.51	14.63	2.00	11.62	9.62	12.00
4	1	284	F	23.50	43.94	3.47	38.31	134.90	0.17	8.24	8.07	14.80
4	2	18	E	9.50	53.59	0.27	12.17	639.42	0.01	0.57	0.55	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	416	1	41.50	40.79	4.71	67.16	34.00	0.96	10.85	9.89	4.00
2	1	222	3	23.50	56.70	3.50	65.76	36.87	0.82	7.05	6.23	1.30
2	2	106	2	9.50	84.18	2.48	71.68	25.56	1.03	4.36	3.32	0.40
3	1	295	4	30.50	48.69	3.99	64.52	39.50	0.79	8.59	7.79	2.20
4	1	233	6	23.50	42.80	2.77	31.43	186.31	0.10	6.65	6.55	2.20
4	2	15	5	9.50	53.09	0.22	10.14	787.30	0.01	0.47	0.46	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	41.50	49.52	6.84	80.24	12.16	2.18	14.21	12.03	3.40
2	1	265	3	23.50	67.94	5.00	78.49	14.66	1.75	9.25	7.50	1.20
2	2	126	2	9.50	111.92	3.92	85.20	5.63	2.19	6.16	3.97	0.30
3	1	352	4	30.50	57.19	5.59	76.98	16.91	1.67	11.09	9.41	2.00
4	1	278	6	23.50	43.79	3.38	37.51	139.97	0.16	8.05	7.89	2.40
4	2	18	5	9.50	53.60	0.27	12.17	639.42	0.01	0.57	0.55	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	609	1	41.50	79.19	13.40	98.32	-8.46	9.57	24.69	15.12	1.70
2	1	324	3	23.50	98.73	6.89	95.97	-6.22	5.77	15.06	9.29	0.90
2	2	155	2	9.50	165.01	7.10	104.81	-14.13	6.31	11.00	4.68	0.30
3	1	431	4	30.50	82.25	9.85	94.26	-4.52	5.78	17.51	11.73	1.40
4	1	340	6	23.50	45.32	4.28	45.87	96.21	0.27	10.05	9.78	2.70
4	2	22	5	9.50	54.27	0.33	14.88	504.98	0.02	0.70	0.68	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	609	1	41.50	101.75	17.21	98.32	-8.46	12.35	27.47	15.12	1.60
2	1	324	3	23.50	122.26	11.00	95.97	-6.22	7.17	16.46	9.29	0.90
2	2	155	2	9.50	233.76	10.06	104.81	-14.13	8.92	13.60	4.68	0.30
3	1	431	4	30.50	97.56	11.68	94.26	-4.52	6.86	18.58	11.73	1.30
4	1	340	6	23.50	45.34	4.28	45.87	96.21	0.27	10.05	9.78	2.70
4	2	22	5	9.50	54.30	0.33	14.88	504.98	0.02	0.70	0.68	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	41.50	65.32	9.02	80.24	12.16	2.64	14.67	12.03	3.30
2	1	265	3	23.50	90.00	6.63	78.49	14.66	2.28	9.78	7.50	1.20
2	2	126	2	9.50	244.70	8.56	85.20	5.63	4.71	8.68	3.97	0.30
3	1	352	4	30.50	67.69	6.62	76.98	16.91	2.00	11.42	9.41	2.00
4	1	278	6	23.50	43.81	3.38	37.51	139.97	0.16	8.05	7.89	2.40
4	2	18	5	9.50	53.62	0.27	12.17	639.42	0.01	0.57	0.55	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	416	1	41.50	42.04	4.86	67.16	34.00	1.02	10.92	9.89	4.00
2	1	222	3	23.50	60.12	3.71	65.76	36.87	0.92	7.15	6.23	1.30
2	2	106	2	9.50	134.33	3.96	71.68	25.56	1.44	4.77	3.32	0.40
3	1	295	4	30.50	50.24	4.12	64.52	39.50	0.85	8.65	7.79	2.20
4	1	233	6	23.50	42.82	2.77	31.43	186.31	0.10	6.65	6.55	2.20
4	2	15	5	9.50	53.11	0.22	10.14	787.30	0.01	0.47	0.46	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads PM.osc
Report generation date: 09/12/2016 11:12:12

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

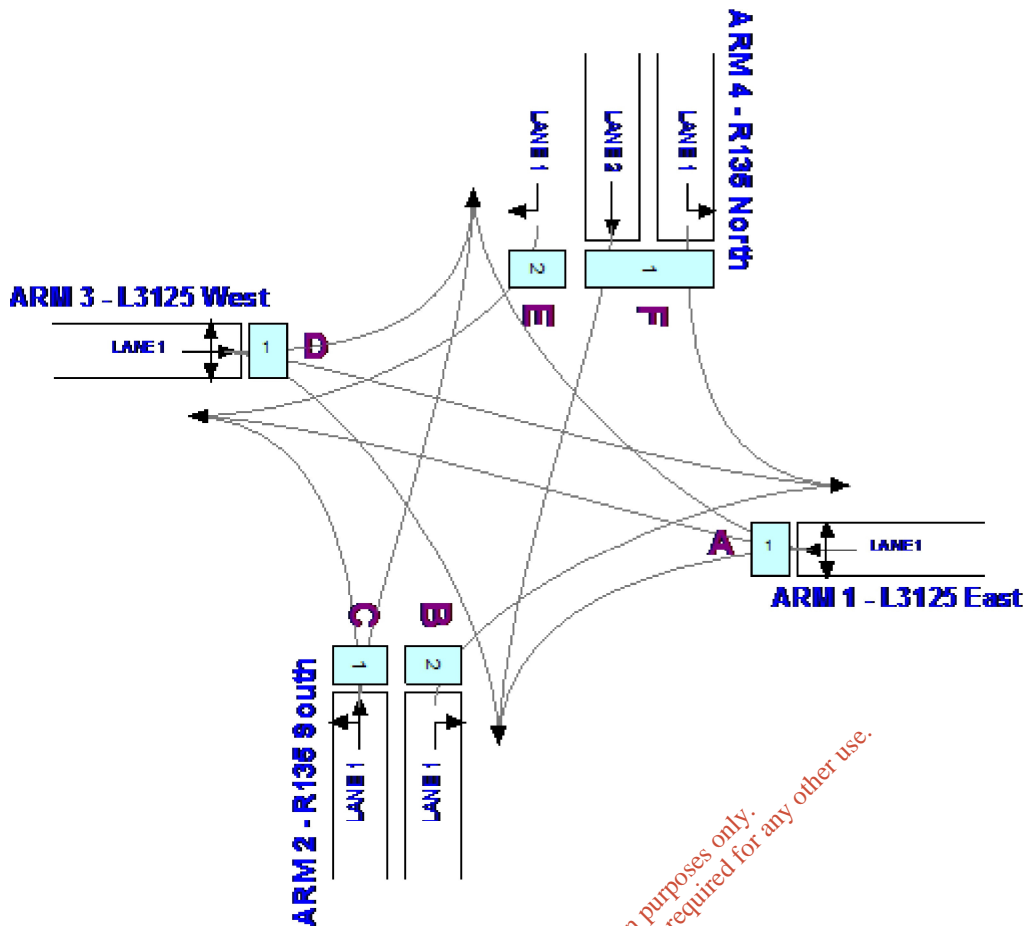
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

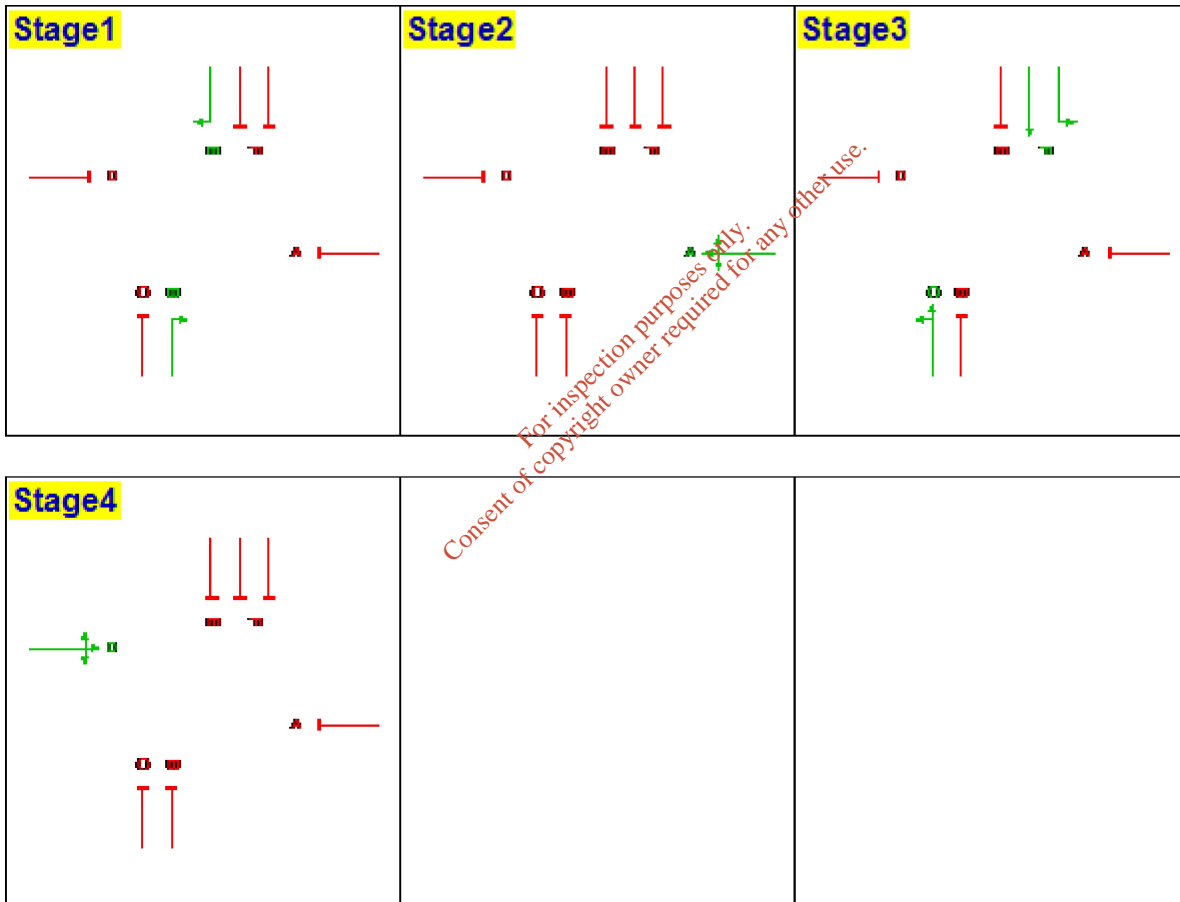
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2022 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2022 PM Peak + Link	Yes	16:30	18:00	ODTAB	No	D1

Demand Set10 - 2022 PM Peak + Link

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	99	232	114
	Arm 2	143	-	142	135
	Arm 3	188	184	-	25
	Arm 4	92	103	19	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	334	399	488	488	399	334
2 - R135 South	1	C	208	248	304	304	248	208
2 - R135 South	2	B	107	128	157	157	128	107
3 - L3125 West	1	D	295	352	431	431	352	295
4 - R135 North	1	F	146	174	214	214	174	146
4 - R135 North	2	E	14	17	21	21	17	14

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	22	52	26
2 - R135 South	34	32	34
3 - L3125 West	6	47	46
4 - R135 North	43	48	9

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	20.10	20.79	20.79	54.7

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-0.05	22.66	22.66	51.90

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

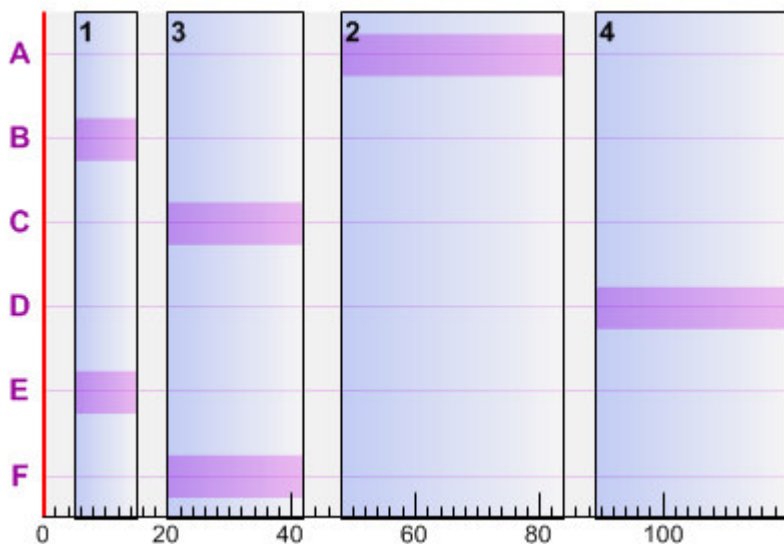
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	10.0	15.0
3	20.0	22.0	42.0
2	48.0	36.0	84.0
4	89.0	31.0	0.0

Phase Timings

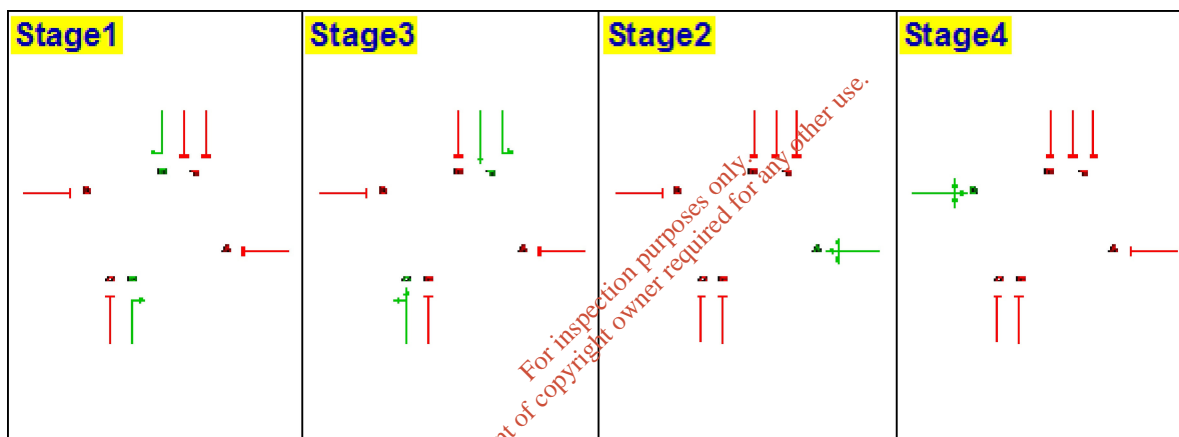
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	48	36.0	84						
B	5	10.0	15						
C	20	22.0	42						
D	89	31.0	0						
E	5	10.0	15						
F	20	22.0	42						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	407	A	37.50	47.80	5.40	72.72	23.76	1.38	11.53	10.15	18.80
2	1	253	C	23.50	65.75	4.62	74.94	20.10	1.53	8.67	7.14	7.60
2	2	131	B	11.50	86.87	3.16	73.18	22.99	1.31	5.37	4.05	2.80
3	1	359	D	32.50	53.19	5.30	73.68	22.15	1.45	10.85	9.40	14.20
4	1	178	F	23.50	41.73	2.06	24.01	274.78	0.05	5.00	4.95	10.60
4	2	17	E	11.50	50.89	0.24	9.50	847.74	0.01	0.52	0.51	0.70

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	334	1	37.50	40.86	3.79	59.68	50.81	0.61	8.81	8.20	3.30
2	1	208	3	23.50	54.52	3.15	61.61	46.08	0.65	6.47	5.82	1.30
2	2	107	2	11.50	69.29	2.06	59.77	50.57	0.56	3.85	3.30	0.40
3	1	295	4	32.50	45.24	3.71	60.55	48.65	0.63	8.25	7.62	2.50
4	1	146	6	23.50	41.14	1.67	19.70	356.92	0.03	4.06	4.03	1.50
4	2	14	5	11.50	50.54	0.20	7.82	9999.00	0.00	0.43	0.42	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	399	1	37.50	46.49	5.15	71.29	26.25	1.21	11.15	9.94	3.20
2	1	248	3	23.50	62.97	4.34	73.46	22.52	1.29	8.29	6.99	1.30
2	2	128	2	11.50	81.45	2.90	71.50	25.87	1.06	5.02	3.96	0.50
3	1	352	4	32.50	51.56	5.04	72.25	24.58	1.26	10.46	9.20	2.40
4	1	174	6	23.50	41.65	2.01	23.47	283.39	0.05	4.88	4.83	1.70
4	2	17	5	11.50	50.89	0.24	9.50	847.74	0.01	0.52	0.51	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	488	1	37.50	61.11	8.28	87.19	3.22	3.52	15.92	12.39	2.40
2	1	304	3	23.50	85.09	7.19	90.04	-0.05	3.76	12.43	8.68	1.00
2	2	157	2	11.50	107.45	4.69	87.70	2.62	2.65	7.54	4.89	0.40
3	1	431	4	32.50	68.47	8.20	88.46	1.74	3.74	15.20	11.46	1.80
4	1	214	6	23.50	42.42	2.52	28.87	211.73	0.08	6.07	5.99	2.00
4	2	21	5	11.50	51.37	0.30	11.73	667.21	0.01	0.65	0.64	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	488	1	37.50	65.27	8.85	87.19	3.22	3.76	16.15	12.39	2.40
2	1	304	3	23.50	96.41	8.14	90.04	-0.05	4.25	12.92	8.68	1.00
2	2	157	2	11.50	123.47	5.38	87.70	2.62	3.03	7.92	4.89	0.40
3	1	431	4	32.50	74.52	8.92	88.46	1.74	4.06	15.52	11.46	1.80
4	1	214	6	23.50	42.43	2.52	28.87	211.73	0.08	6.07	5.99	2.00
4	2	21	5	11.50	51.37	0.30	11.73	667.21	0.01	0.65	0.64	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	399	1	37.50	48.82	5.41	71.29	26.25	1.33	11.27	9.94	3.10
2	1	248	3	23.50	71.43	4.92	73.46	22.52	1.54	8.54	6.99	1.30
2	2	128	2	11.50	98.02	3.49	71.50	25.87	1.38	5.34	3.96	0.50
3	1	352	4	32.50	55.15	5.39	72.25	24.58	1.42	10.62	9.20	2.40
4	1	174	6	23.50	41.66	2.01	23.47	283.39	0.05	4.88	4.83	1.70
4	2	17	5	11.50	50.90	0.24	9.50	847.74	0.01	0.52	0.51	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	334	1	37.50	41.41	3.84	59.68	50.81	0.64	8.84	8.20	3.30
2	1	208	3	23.50	56.43	3.26	61.61	46.08	0.71	6.53	5.82	1.30
2	2	107	2	11.50	74.76	2.22	59.77	50.57	0.64	3.94	3.30	0.40
3	1	295	4	32.50	46.05	3.77	60.55	48.65	0.67	8.29	7.62	2.50
4	1	146	6	23.50	41.14	1.67	19.70	356.92	0.03	4.06	4.03	1.50
4	2	14	5	11.50	50.55	0.20	7.82	9999.00	0.00	0.43	0.42	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads PM.osc
Report generation date: 09/12/2016 11:06:24

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

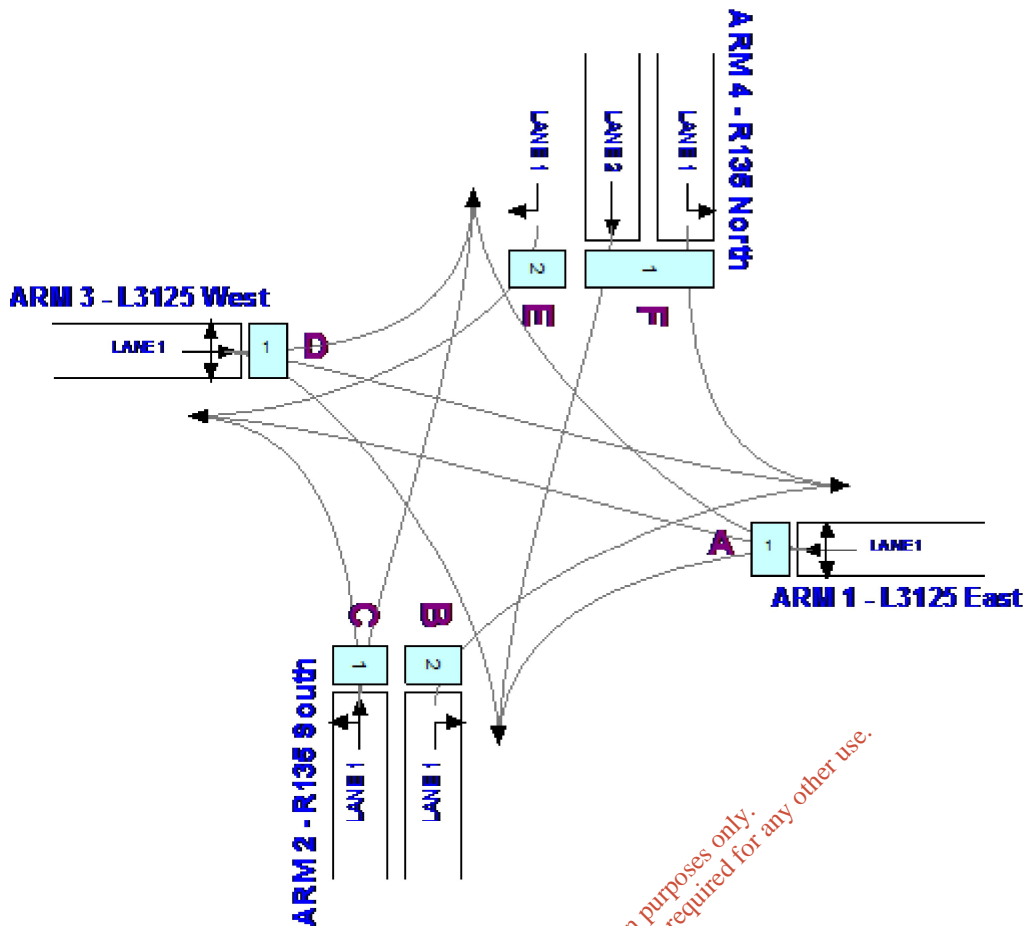
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

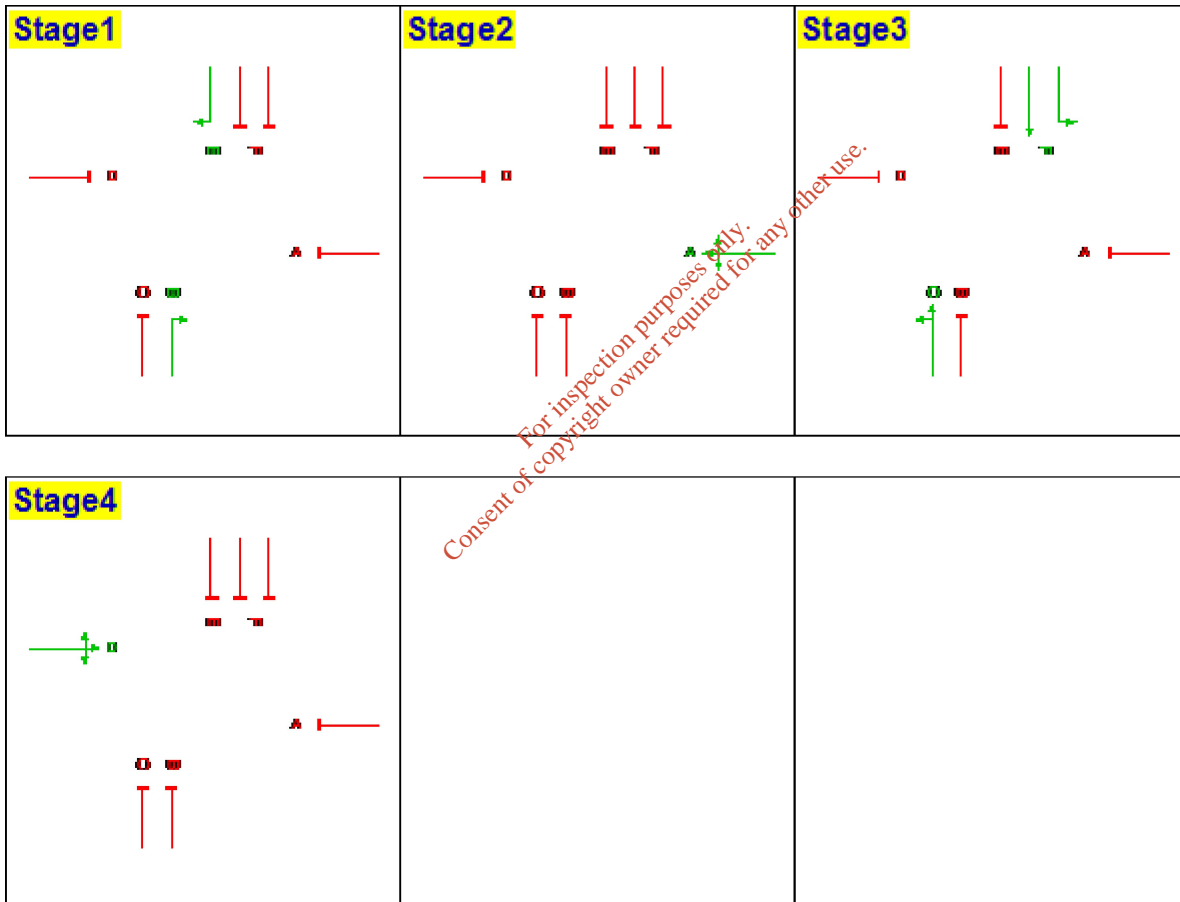
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2022 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2032 PM Peak No Dev	Yes	16:30	18:00	ODTAB	No	D1

Demand Set10 - 2032 PM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	99	240	235
	Arm 2	148	-	147	138
	Arm 3	194	190	-	26
	Arm 4	192	105	20	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	431	514	630	630	514	431
2 - R135 South	1	C	214	256	313	313	256	214
2 - R135 South	2	B	110	132	161	161	132	110
3 - L3125 West	1	D	304	364	445	445	364	304
4 - R135 North	1	F	223	267	327	327	267	223
4 - R135 North	2	E	14	17	21	21	17	14

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	34	32	34
3 - L3125 West	6	47	46
4 - R135 North	61	33	6

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	-0.68	29.24	29.24	55.1

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-17.33	35.40	35.40	51.50

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

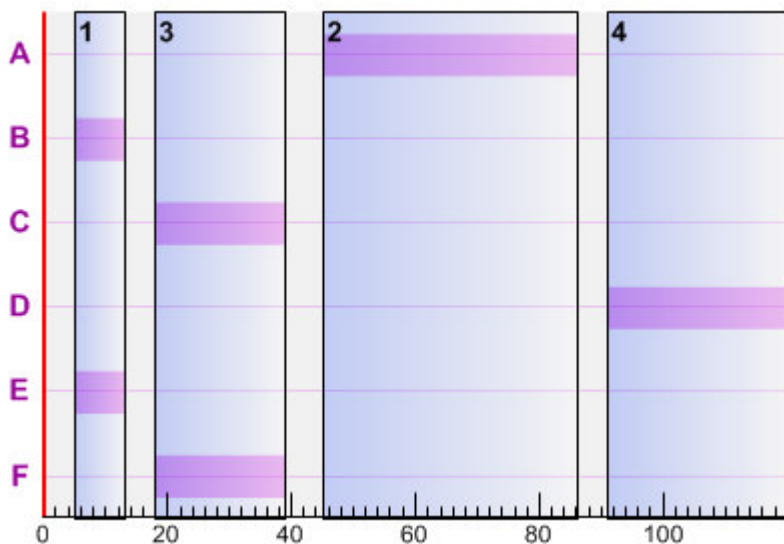
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	8.0	13.0
3	18.0	21.0	39.0
2	45.0	41.0	86.0
4	91.0	29.0	0.0

Phase Timings

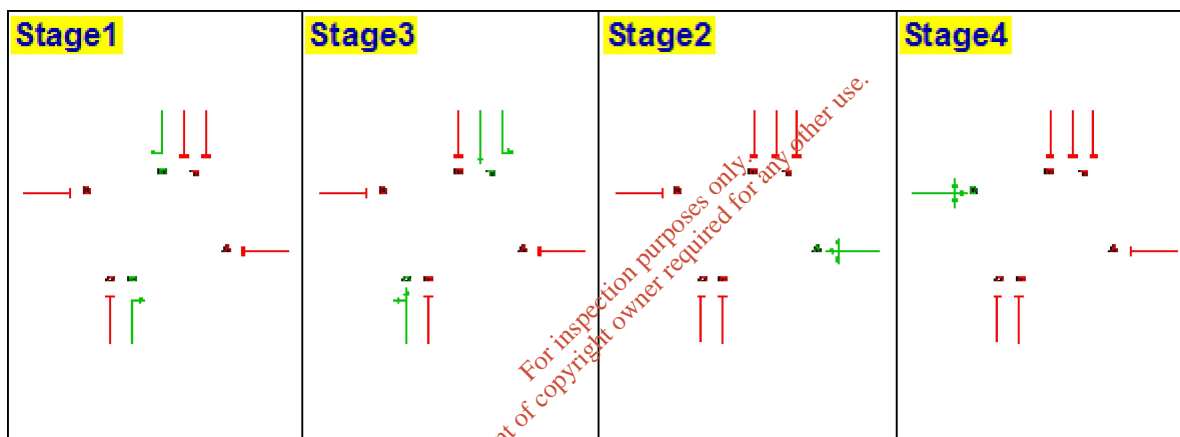
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	45	41.0	86						
B	5	8.0	13						
C	18	21.0	39						
D	91	29.0	0						
E	5	8.0	13						
F	18	21.0	39						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	525	A	42.50	52.64	7.68	82.77	8.74	2.87	15.50	12.63	20.50
2	1	261	C	22.50	75.54	5.48	80.74	11.47	2.28	9.74	7.46	6.70
2	2	134	B	9.50	159.18	5.93	90.61	-0.68	4.65	8.88	4.23	2.00
3	1	371	D	30.50	63.33	6.53	81.14	10.92	2.43	12.39	9.96	11.60
4	1	272	F	22.50	44.75	3.38	38.33	134.82	0.17	7.96	7.79	13.70
4	2	17	E	9.50	53.43	0.25	11.50	682.91	0.01	0.53	0.52	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	431	1	42.50	40.38	4.83	67.95	32.46	1.00	11.16	10.15	4.20
2	1	214	3	22.50	58.09	3.45	66.20	35.95	0.84	6.90	6.05	1.20
2	2	110	2	9.50	87.08	2.66	74.38	21.00	1.19	4.64	3.45	0.40
3	1	304	4	30.50	49.64	4.19	66.49	35.37	0.89	8.93	8.05	2.20
4	1	223	6	22.50	43.61	2.70	31.42	186.42	0.10	6.42	6.32	2.00
4	2	14	5	9.50	52.93	0.21	9.47	850.68	0.01	0.44	0.43	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	514	1	42.50	49.36	7.05	81.03	11.07	2.31	14.65	12.33	3.50
2	1	256	3	22.50	70.18	4.99	79.20	13.64	1.82	9.12	7.31	1.10
2	2	132	2	9.50	121.09	4.44	89.26	0.83	2.74	6.90	4.16	0.30
3	1	364	4	30.50	59.63	6.03	79.61	13.06	1.98	11.74	9.76	2.00
4	1	267	6	22.50	44.63	3.31	37.62	139.22	0.16	7.80	7.64	2.30
4	2	17	5	9.50	53.43	0.25	11.50	682.91	0.01	0.53	0.52	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	630	1	42.50	80.85	14.15	99.32	-9.38	10.65	26.17	15.52	1.70
2	1	313	3	22.50	102.93	6.95	96.83	-7.05	6.02	15.06	9.04	0.80
2	2	161	2	9.50	180.22	8.06	108.87	-17.33	7.77	12.46	4.69	0.30
3	1	445	4	30.50	89.83	11.10	97.32	-7.52	7.43	19.58	12.14	1.20
4	1	327	6	22.50	46.49	4.20	46.08	95.33	0.28	9.75	9.48	2.50
4	2	21	5	9.50	54.10	0.32	14.20	533.79	0.02	0.66	0.65	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	630	1	42.50	106.56	18.65	99.32	-9.38	14.14	29.66	15.52	1.50
2	1	313	3	22.50	129.52	11.26	96.83	-7.05	7.61	16.65	9.04	0.80
2	2	161	2	9.50	265.22	11.86	108.87	-17.33	11.35	16.04	4.69	0.30
3	1	445	4	30.50	113.04	13.97	97.32	-7.52	9.42	21.56	12.14	1.20
4	1	327	6	22.50	46.21	4.20	46.08	95.33	0.28	9.75	9.48	2.50
4	2	21	5	9.50	54.10	0.32	14.20	533.79	0.02	0.66	0.65	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	514	1	42.50	68.84	9.83	81.03	11.07	2.84	15.18	12.33	3.40
2	1	256	3	22.50	97.18	6.91	79.20	13.64	2.43	9.74	7.31	1.10
2	2	132	2	9.50	304.89	11.18	89.26	0.83	7.49	11.66	4.16	0.30
3	1	364	4	30.50	78.80	7.97	79.61	13.06	2.49	12.25	9.76	1.90
4	1	267	6	22.50	44.65	3.31	37.62	139.22	0.16	7.80	7.64	2.30
4	2	17	5	9.50	53.43	0.25	11.50	682.91	0.01	0.53	0.52	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	431	1	42.50	41.72	4.99	67.95	32.46	1.08	11.23	10.15	4.10
2	1	214	3	22.50	62.18	3.70	66.20	35.95	0.95	7.00	6.05	1.20
2	2	110	2	9.50	190.07	5.81	74.38	21.00	1.84	5.29	3.45	0.40
3	1	304	4	30.50	51.77	4.37	66.49	35.37	0.97	9.01	8.05	2.20
4	1	223	6	22.50	43.63	2.70	31.42	186.42	0.10	6.42	6.32	2.00
4	2	14	5	9.50	52.95	0.21	9.47	850.68	0.01	0.44	0.43	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads PM.osc
Report generation date: 09/12/2016 11:08:11

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

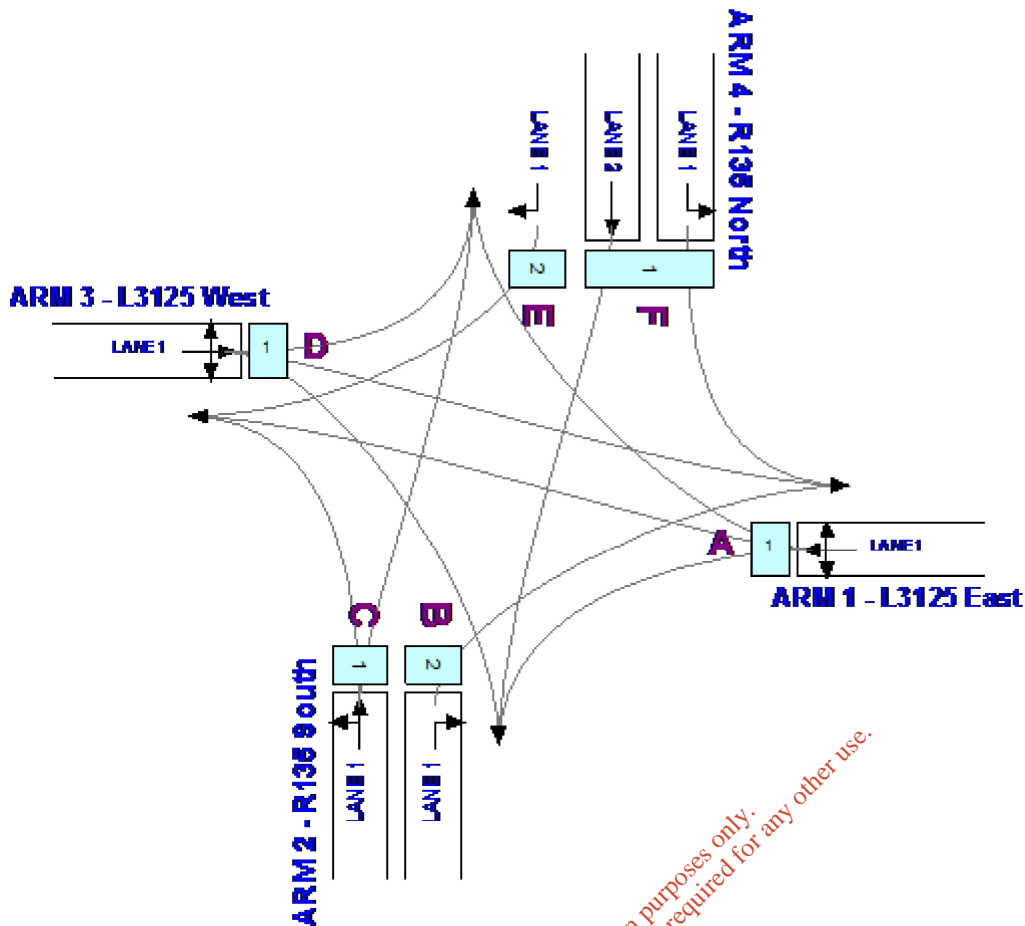
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

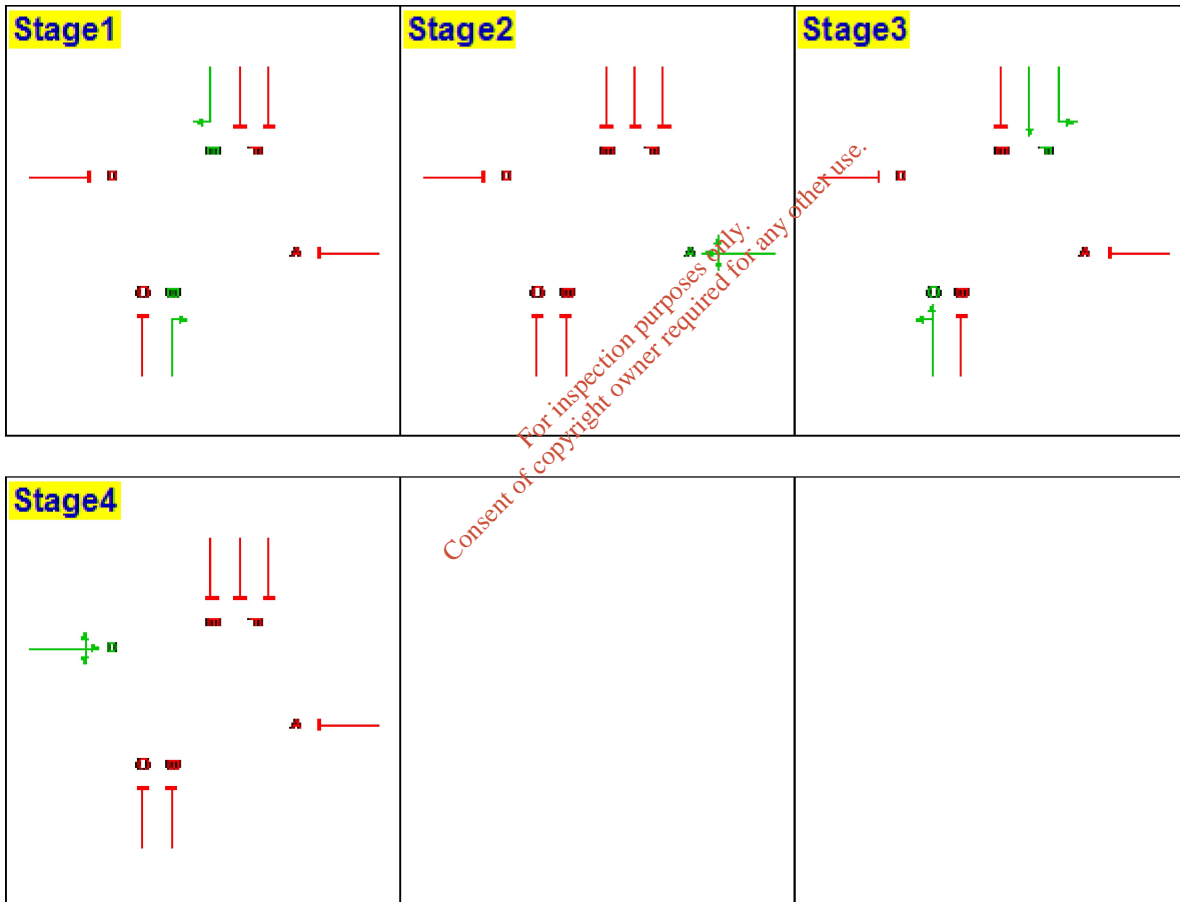
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2022 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2032 PM Peak With Dev	Yes	16:30	18:00	ODTAB	No	D1

Demand Set10 - 2032 PM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	99	240	235
	Arm 2	148	-	147	139
	Arm 3	194	190	-	26
	Arm 4	192	106	20	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	431	514	630	630	514	431
2 - R135 South	1	C	215	257	314	314	257	215
2 - R135 South	2	B	111	132	162	162	132	111
3 - L3125 West	1	D	304	364	445	445	364	304
4 - R135 North	1	F	222	265	324	324	265	222
4 - R135 North	2	E	14	17	21	21	17	14

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	34	32	34
3 - L3125 West	6	47	46
4 - R135 North	60	33	6

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	7.29	28.19	28.19	54.2

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-10.56	34.36	34.36	50.50

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

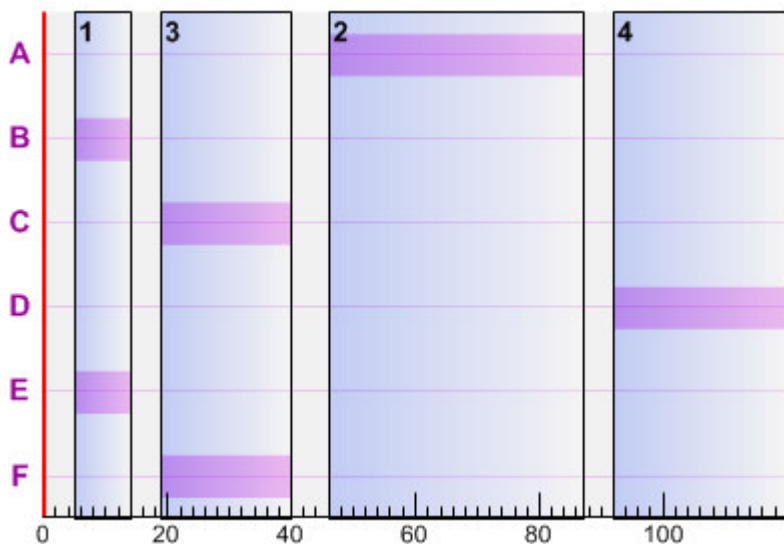
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	9.0	14.0
3	19.0	21.0	40.0
2	46.0	41.0	87.0
4	92.0	28.0	0.0

Phase Timings

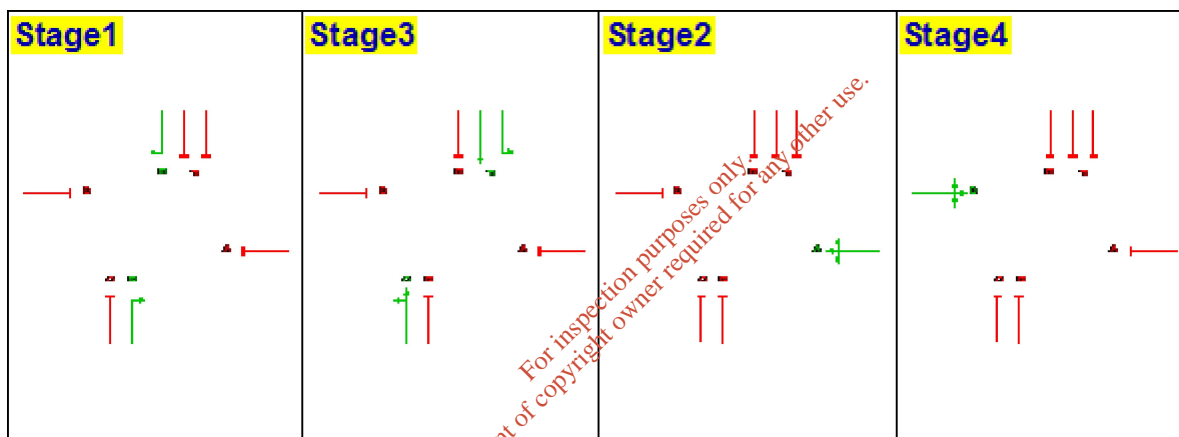
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	46	41.0	87						
B	5	9.0	14						
C	19	21.0	40						
D	92	28.0	0						
E	5	9.0	14						
F	19	21.0	40						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	525	A	42.50	52.64	7.68	82.77	8.74	2.87	15.50	12.63	20.50
2	1	262	C	22.50	76.10	5.54	81.05	11.04	2.33	9.82	7.49	6.70
2	2	135	B	10.50	113.55	4.26	82.59	8.97	2.46	6.68	4.22	2.40
3	1	371	D	29.50	69.11	7.12	83.89	7.29	3.00	13.08	10.07	10.40
4	1	270	F	22.50	44.70	3.35	38.04	136.56	0.16	7.89	7.73	13.60
4	2	17	E	10.50	52.10	0.25	10.40	765.32	0.01	0.53	0.52	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	431	1	42.50	40.38	4.83	67.95	32.46	1.00	11.16	10.15	4.20
2	1	215	3	22.50	58.27	3.48	66.51	35.31	0.86	6.94	6.08	1.20
2	2	111	2	10.50	77.75	2.40	67.91	32.53	0.86	4.31	3.45	0.40
3	1	304	4	29.50	51.76	4.37	68.74	30.93	1.01	9.15	8.14	2.10
4	1	222	6	22.50	43.59	2.69	31.28	187.71	0.10	6.39	6.29	2.00
4	2	14	5	10.50	51.69	0.20	8.57	950.75	0.01	0.43	0.43	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	514	1	42.50	49.36	7.05	81.03	11.07	2.31	14.65	12.33	3.50
2	1	257	3	22.50	70.57	5.04	79.51	13.20	1.85	9.19	7.34	1.10
2	2	132	2	10.50	98.45	3.61	80.76	11.44	1.75	5.87	4.12	0.40
3	1	364	4	29.50	63.72	6.44	82.31	9.35	2.35	12.22	9.87	1.80
4	1	265	6	22.50	44.58	3.28	37.34	141.03	0.15	7.73	7.58	2.20
4	2	17	5	10.50	52.09	0.25	10.40	765.32	0.01	0.53	0.52	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	630	1	42.50	80.85	14.15	99.32	-9.38	10.65	26.17	15.52	1.70
2	1	314	3	22.50	103.81	9.05	97.14	-7.35	6.16	15.23	9.07	0.80
2	2	162	2	10.50	142.76	6.42	99.11	-9.19	4.94	10.03	5.09	0.40
3	1	445	4	29.50	100.22	12.39	100.62	-10.56	9.53	21.73	12.20	1.00
4	1	324	6	22.50	46.41	4.15	45.65	97.14	0.27	9.65	9.38	2.50
4	2	21	5	10.50	52.67	0.31	12.85	600.50	0.01	0.65	0.64	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	630	1	42.50	106.56	18.65	99.32	-9.38	14.14	29.66	15.52	1.50
2	1	314	3	22.50	131.34	11.46	97.14	-7.35	7.83	16.91	9.07	0.80
2	2	162	2	10.50	188.32	8.47	99.11	-9.19	6.52	11.61	5.09	0.40
3	1	445	4	29.50	135.29	16.72	100.62	-10.56	13.06	25.26	12.20	0.90
4	1	324	6	22.50	46.13	4.15	45.65	97.14	0.27	9.65	9.38	2.50
4	2	21	5	10.50	52.67	0.31	12.85	600.50	0.01	0.65	0.64	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	514	1	42.50	68.84	9.83	81.03	11.07	2.84	15.18	12.33	3.40
2	1	257	3	22.50	99.02	7.07	79.51	13.20	2.50	9.83	7.34	1.10
2	2	132	2	10.50	169.75	6.22	80.76	11.44	2.95	7.08	4.12	0.40
3	1	364	4	29.50	102.44	10.36	82.31	9.35	3.23	13.10	9.87	1.70
4	1	265	6	22.50	44.60	3.28	37.34	141.03	0.16	7.73	7.58	2.20
4	2	17	5	10.50	52.11	0.25	10.40	765.32	0.01	0.53	0.52	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	431	1	42.50	41.72	4.99	67.95	32.46	1.08	11.23	10.15	4.10
2	1	215	3	22.50	62.52	3.73	66.51	35.31	0.97	7.05	6.08	1.20
2	2	111	2	10.50	97.96	3.02	67.91	32.53	1.09	4.54	3.45	0.40
3	1	304	4	29.50	55.10	4.65	68.74	30.93	1.12	9.25	8.14	2.00
4	1	222	6	22.50	43.61	2.69	31.28	187.71	0.10	6.39	6.29	2.00
4	2	14	5	10.50	51.71	0.20	8.57	950.75	0.01	0.43	0.43	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads PM.osc
Report generation date: 09/12/2016 11:09:38

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

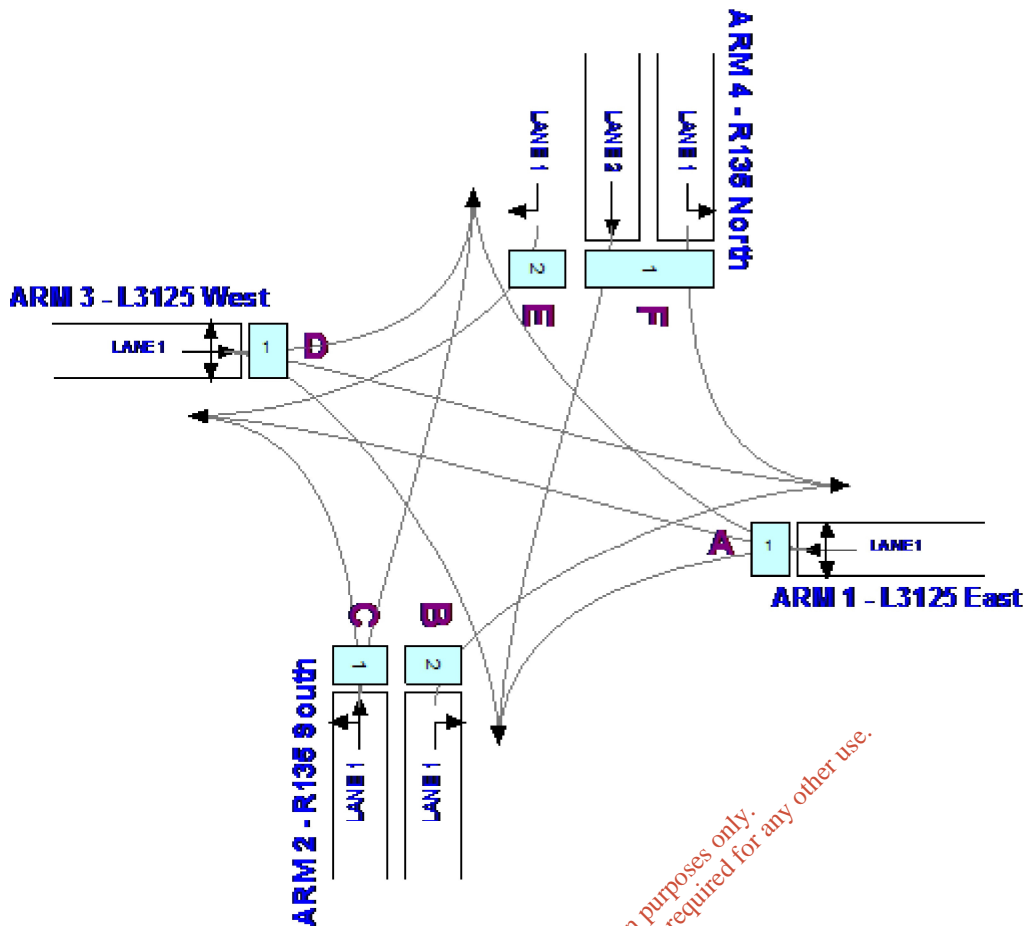
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

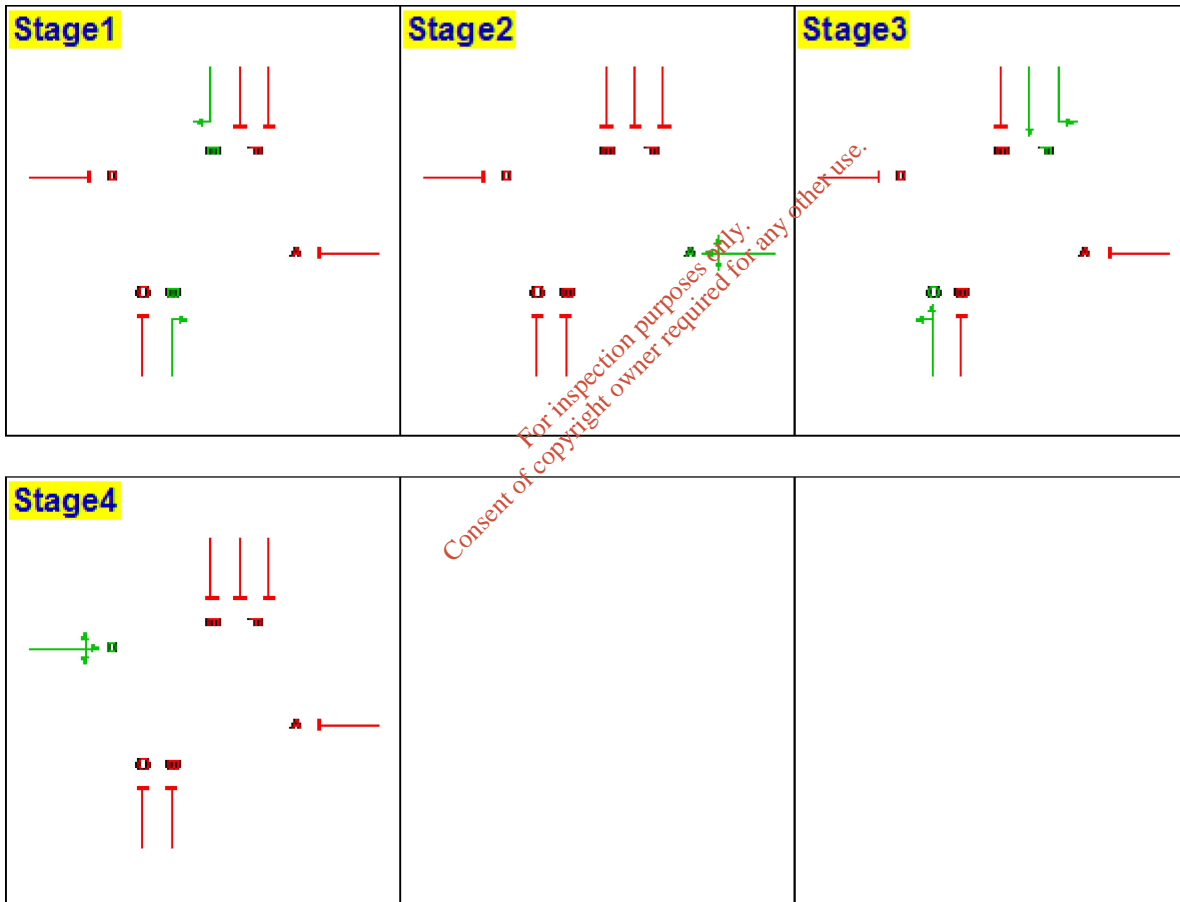
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2022 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2032 PM Peak Sen	Yes	16:30	18:00	ODTAB	No	D1

Demand Set10 - 2032 PM Peak Sen

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	99	240	235
	Arm 2	148	-	147	155
	Arm 3	194	190	-	26
	Arm 4	192	122	20	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	431	514	630	630	514	431
2 - R135 South	1	C	226	270	331	331	270	226
2 - R135 South	2	B	111	133	163	163	133	111
3 - L3125 West	1	D	304	364	445	445	364	304
4 - R135 North	1	F	235	281	344	344	281	235
4 - R135 North	2	E	15	18	22	22	18	15

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	33	34	33
3 - L3125 West	6	47	46
4 - R135 North	57	37	6

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	5.41	29.59	29.59	54.4

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-12.11	36.51	36.51	50.60

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

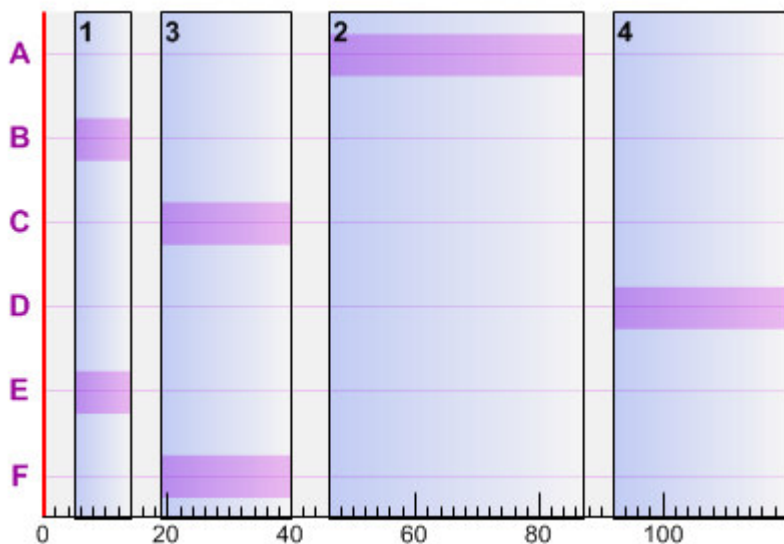
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	9.0	14.0
3	19.0	21.0	40.0
2	46.0	41.0	87.0
4	92.0	28.0	0.0

Phase Timings

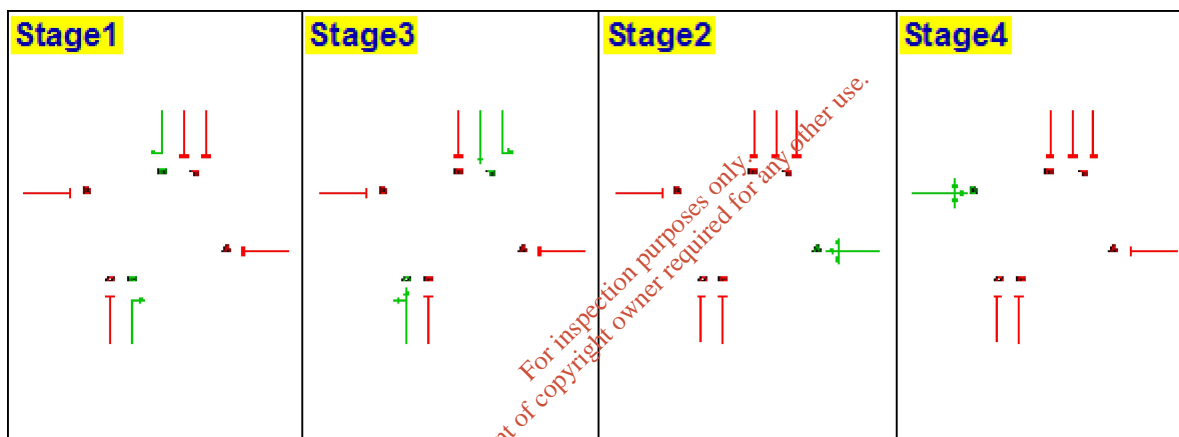
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	46	41.0	87						
B	5	9.0	14						
C	19	21.0	40						
D	92	28.0	0						
E	5	9.0	14						
F	19	21.0	40						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	525	A	42.50	52.64	7.68	82.77	8.74	2.87	15.50	12.63	20.50
2	1	276	C	22.50	85.67	6.57	85.38	5.41	3.29	11.20	7.91	6.30
2	2	136	B	10.50	115.65	4.37	83.21	8.17	2.57	6.82	4.25	2.40
3	1	371	D	29.50	69.11	7.12	83.89	7.29	3.00	13.08	10.07	10.40
4	1	287	F	22.50	45.13	3.60	40.44	122.55	0.19	8.44	8.25	14.10
4	2	18	E	10.50	52.24	0.26	11.01	717.25	0.01	0.56	0.55	0.70

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	431	1	42.50	40.38	4.83	67.95	32.46	1.00	11.16	10.15	4.20
2	1	226	3	22.50	60.42	3.79	69.91	28.73	1.04	7.45	6.41	1.20
2	2	111	2	10.50	77.75	2.40	67.91	32.53	0.86	4.31	3.45	0.40
3	1	304	4	29.50	51.76	4.37	68.74	30.93	1.01	9.15	8.14	2.10
4	1	235	6	22.50	43.87	2.86	33.11	171.80	0.11	6.79	6.68	2.10
4	2	15	5	10.50	51.82	0.22	9.18	880.70	0.01	0.46	0.46	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	514	1	42.50	49.36	7.05	81.03	11.07	2.31	14.65	12.33	3.50
2	1	270	3	22.50	75.94	5.70	83.53	7.75	2.39	10.12	7.73	1.10
2	2	133	2	10.50	99.36	3.67	81.37	10.61	1.81	5.96	4.16	0.40
3	1	364	4	29.50	63.72	6.44	82.31	9.35	2.35	12.22	9.87	1.80
4	1	281	6	22.50	44.97	3.51	39.59	127.30	0.18	8.24	8.06	2.30
4	2	18	5	10.50	52.25	0.26	11.01	717.25	0.01	0.56	0.55	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	630	1	42.50	80.85	14.15	99.32	-9.38	10.65	26.17	15.52	1.70
2	1	331	3	22.50	118.03	10.85	102.40	-12.11	8.92	18.30	9.38	0.70
2	2	163	2	10.50	144.84	6.56	99.72	-9.75	5.11	10.24	5.12	0.30
3	1	445	4	29.50	100.22	12.39	100.62	-10.56	9.53	21.73	12.20	1.00
4	1	344	6	22.50	46.69	4.46	48.47	85.67	0.32	10.32	10.01	2.60
4	2	22	5	10.50	52.81	0.32	13.46	568.66	0.01	0.69	0.67	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	630	1	42.50	106.56	18.65	99.32	-9.38	14.14	29.66	15.52	1.50
2	1	331	3	22.50	164.57	15.13	102.40	-12.11	12.59	21.96	9.38	0.70
2	2	163	2	10.50	192.57	8.72	99.72	-9.75	6.81	11.93	5.12	0.30
3	1	445	4	29.50	135.29	16.72	100.62	-10.56	13.06	25.26	12.20	0.90
4	1	344	6	22.50	46.71	4.46	48.47	85.67	0.32	10.33	10.01	2.60
4	2	22	5	10.50	52.81	0.32	13.46	568.66	0.01	0.69	0.67	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	514	1	42.50	68.84	9.83	81.03	11.07	2.84	15.18	12.33	3.40
2	1	270	3	22.50	142.70	10.70	83.53	7.75	3.76	11.49	7.73	1.00
2	2	133	2	10.50	176.19	6.51	81.37	10.61	3.13	7.29	4.16	0.40
3	1	364	4	29.50	102.44	10.36	82.31	9.35	3.23	13.10	9.87	1.70
4	1	281	6	22.50	45.00	3.51	39.59	127.30	0.18	8.25	8.06	2.30
4	2	18	5	10.50	52.25	0.26	11.01	717.25	0.01	0.56	0.55	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	431	1	42.50	41.72	4.99	67.95	32.46	1.08	11.23	10.15	4.10
2	1	226	3	22.50	68.32	4.29	69.91	28.73	1.21	7.62	6.41	1.20
2	2	111	2	10.50	99.57	3.07	67.91	32.53	1.09	4.54	3.45	0.40
3	1	304	4	29.50	55.10	4.65	68.74	30.93	1.12	9.25	8.14	2.00
4	1	235	6	22.50	43.90	2.87	33.11	171.80	0.11	6.79	6.68	2.10
4	2	15	5	10.50	51.84	0.22	9.18	880.70	0.01	0.46	0.46	0.10

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OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

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File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-04 Huntstown C&D Waste Recovery Facility
TIA\Reports\Appendices\OSCADY PRO\Crossroads PM.osc
Report generation date: 09/12/2016 11:13:52

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

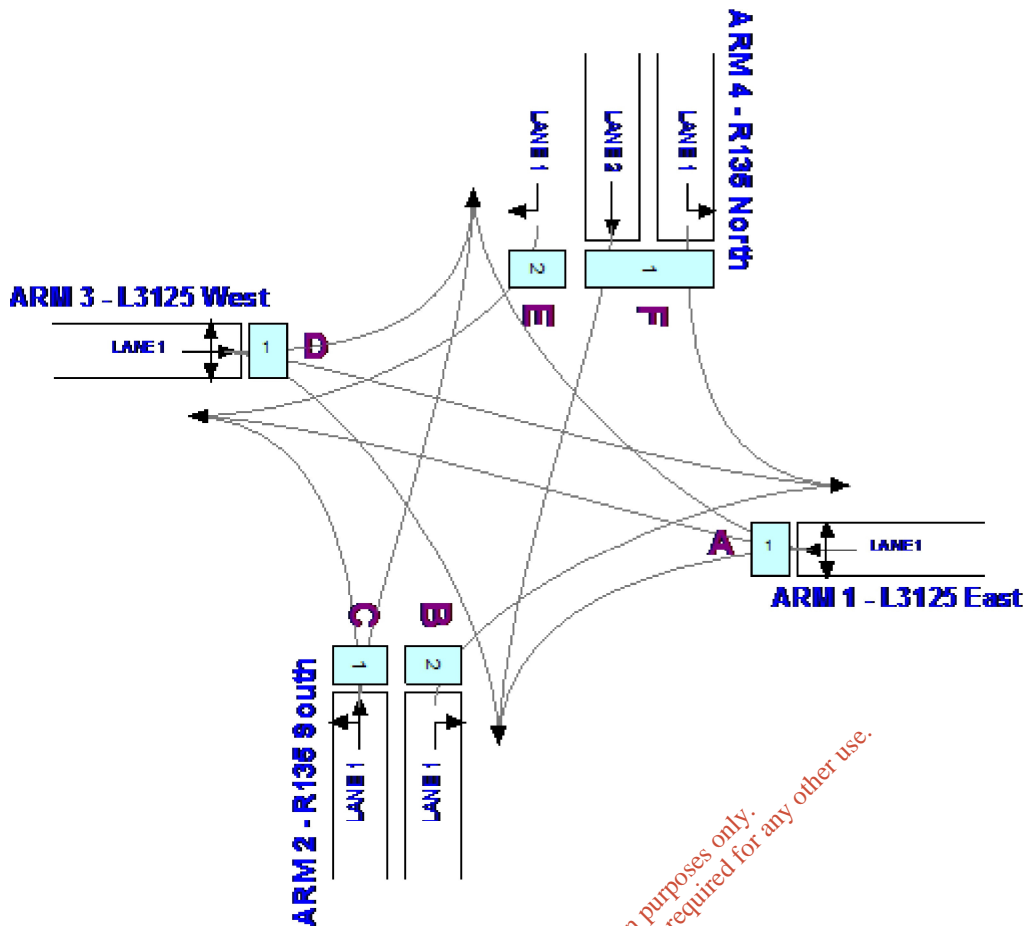
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

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Junction Diagram



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Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

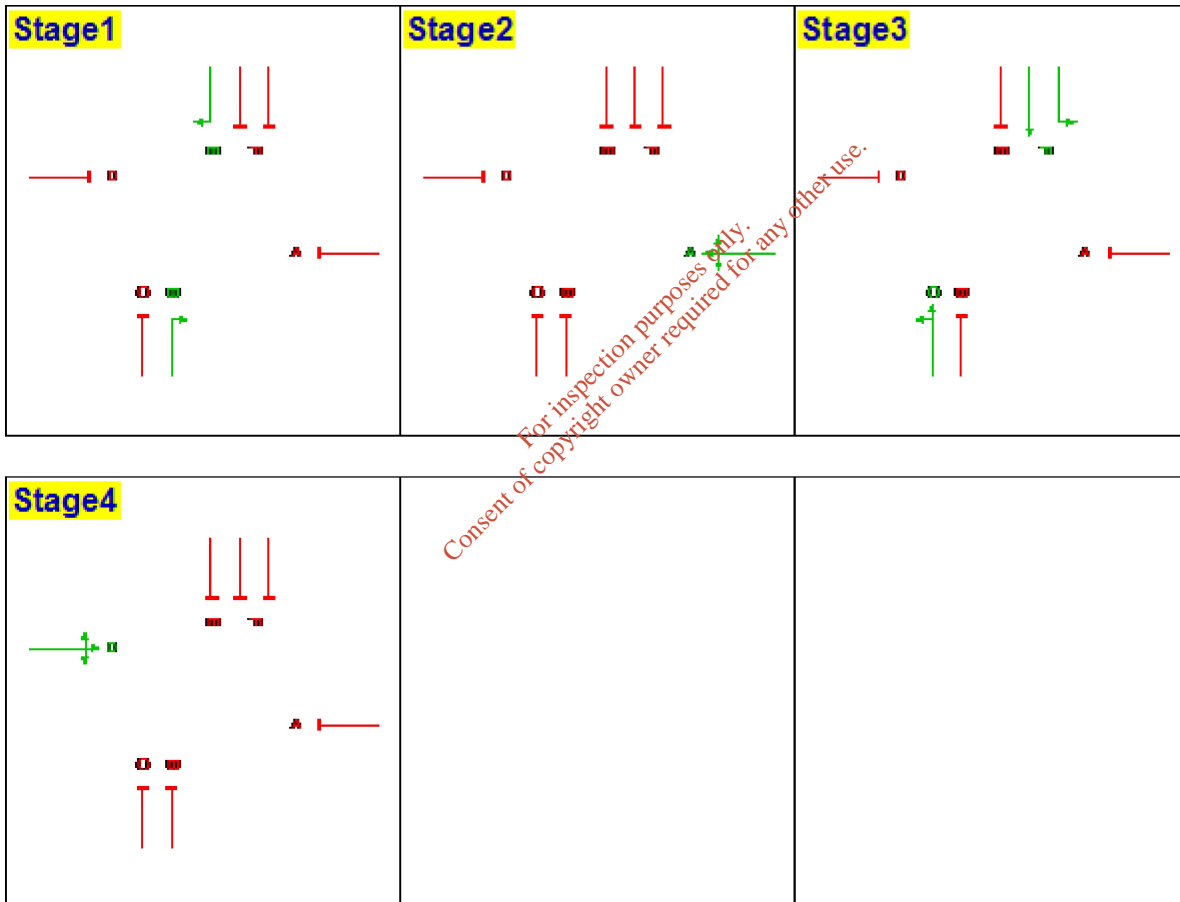
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2022 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2032 PM Peak + Link	Yes	16:30	18:00	ODTAB	No	D1

Demand Set10 - 2032 PM Peak + Link

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	99	240	114
	Arm 2	148	-	147	139
	Arm 3	194	190	-	26
	Arm 4	96	106	20	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	340	406	497	497	406	340
2 - R135 South	1	C	215	257	314	314	257	215
2 - R135 South	2	B	111	132	162	162	132	111
3 - L3125 West	1	D	304	364	445	445	364	304
4 - R135 North	1	F	152	181	222	222	181	152
4 - R135 North	2	E	15	18	22	22	18	15

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	22	53	25
2 - R135 South	34	32	34
3 - L3125 West	6	47	46
4 - R135 North	43	48	9

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Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	18.20	22.00	22.00	54.2

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-1.46	24.39	24.39	51.50

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

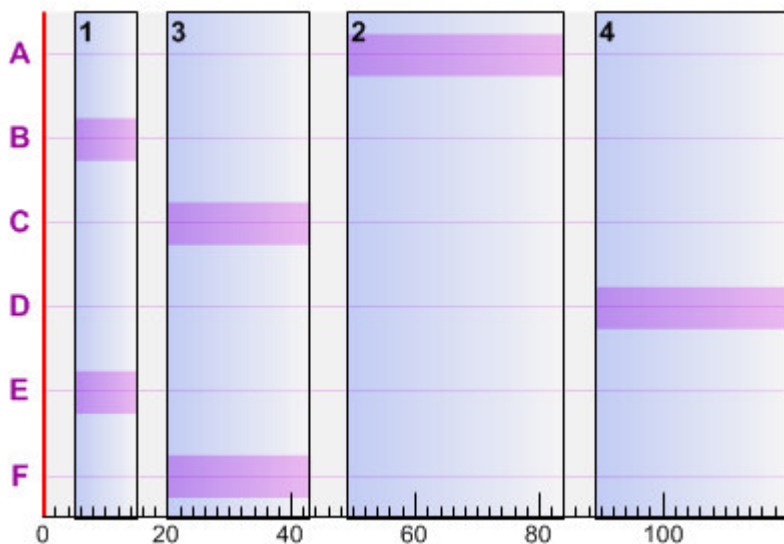
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	10.0	15.0
3	20.0	23.0	43.0
2	49.0	35.0	84.0
4	89.0	31.0	0.0

Phase Timings

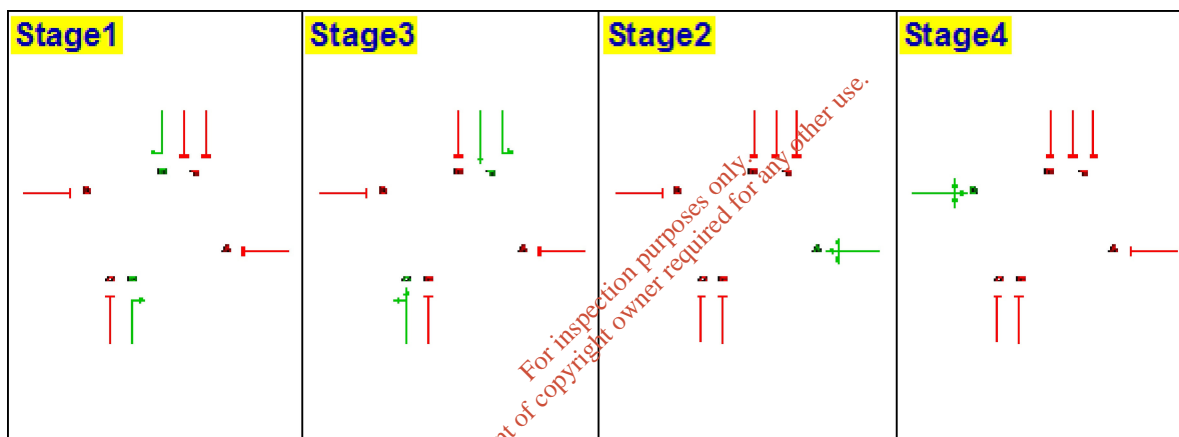
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	49	35.0	84						
B	5	10.0	15						
C	20	23.0	43						
D	89	31.0	0						
E	5	10.0	15						
F	20	23.0	43						

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Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	414	A	36.50	51.24	5.89	76.00	18.43	1.71	12.18	10.47	17.20
2	1	262	C	24.50	63.80	4.64	74.44	20.91	1.48	8.82	7.33	8.20
2	2	135	B	11.50	90.81	3.41	75.41	19.34	1.51	5.69	4.18	2.70
3	1	371	D	32.50	55.31	5.70	76.14	18.20	1.71	11.45	9.74	13.90
4	1	185	F	24.50	40.92	2.10	23.94	275.94	0.05	5.15	5.10	11.50
4	2	18	E	11.50	51.01	0.26	10.05	795.08	0.01	0.55	0.54	0.70

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	340	1	36.50	42.71	4.03	62.41	44.20	0.71	9.17	8.46	3.10
2	1	215	3	24.50	53.22	3.18	61.08	47.34	0.64	6.59	5.96	1.40
2	2	111	2	11.50	70.79	2.18	62.01	45.15	0.63	4.05	3.42	0.50
3	1	304	4	32.50	45.99	3.88	62.39	44.25	0.70	8.57	7.87	2.50
4	1	152	6	24.50	40.34	1.70	19.67	357.56	0.03	4.19	4.16	1.60
4	2	15	5	11.50	50.66	0.21	8.38	974.10	0.01	0.46	0.45	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	406	1	36.50	49.45	5.58	74.53	20.76	1.47	11.72	10.25	2.90
2	1	257	3	24.50	61.31	4.38	73.01	23.26	1.26	8.45	7.19	1.40
2	2	132	2	11.50	84.24	3.09	73.74	22.06	1.20	5.29	4.09	0.50
3	1	364	4	32.50	53.29	5.39	74.71	20.47	1.46	11.01	9.54	2.30
4	1	181	6	24.50	40.85	2.05	23.42	284.25	0.05	5.03	4.98	1.90
4	2	18	5	11.50	51.00	0.26	10.05	795.08	0.01	0.55	0.54	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	36.50	68.76	9.49	91.23	-1.35	4.82	17.62	12.80	2.00
2	1	314	3	24.50	81.85	7.14	89.21	0.89	3.59	12.48	8.89	1.10
2	2	162	2	11.50	113.84	5.12	90.49	-0.55	3.12	8.17	5.05	0.40
3	1	445	4	32.50	73.55	9.09	91.33	-1.46	4.68	16.55	11.87	1.70
4	1	222	6	24.50	41.60	2.57	28.73	213.29	0.08	6.24	6.16	2.20
4	2	22	5	11.50	51.49	0.31	12.29	632.34	0.01	0.68	0.67	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	36.50	77.00	10.63	91.23	-1.35	5.38	18.18	12.80	2.00
2	1	314	3	24.50	91.54	7.98	89.21	0.89	4.01	12.90	8.89	1.10
2	2	162	2	11.50	134.80	6.07	90.49	-0.55	3.68	8.72	5.05	0.40
3	1	445	4	32.50	82.96	10.26	91.33	-1.46	5.27	17.14	11.87	1.60
4	1	222	6	24.50	41.61	2.57	28.73	213.29	0.08	6.24	6.16	2.20
4	2	22	5	11.50	51.49	0.31	12.29	632.34	0.01	0.68	0.67	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	406	1	36.50	54.07	6.10	74.53	20.76	1.67	11.92	10.25	2.90
2	1	257	3	24.50	68.36	4.88	73.01	23.26	1.49	8.68	7.19	1.40
2	2	132	2	11.50	107.35	3.94	73.74	22.06	1.62	5.71	4.09	0.50
3	1	364	4	32.50	58.99	5.96	74.71	20.47	1.69	11.23	9.54	2.30
4	1	181	6	24.50	40.85	2.05	23.42	284.25	0.05	5.03	4.98	1.90
4	2	18	5	11.50	51.02	0.26	10.05	795.08	0.01	0.55	0.54	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	340	1	36.50	43.53	4.11	62.41	44.20	0.75	9.22	8.46	3.10
2	1	215	3	24.50	54.90	3.28	61.08	47.34	0.69	6.65	5.96	1.40
2	2	111	2	11.50	77.84	2.40	62.01	45.15	0.74	4.16	3.42	0.50
3	1	304	4	32.50	47.04	3.97	62.39	44.25	0.75	8.62	7.87	2.50
4	1	152	6	24.50	40.34	1.70	19.67	357.56	0.03	4.19	4.16	1.60
4	2	15	5	11.50	50.67	0.21	8.38	974.10	0.01	0.46	0.45	0.10

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