

TRAFFIC AND TRANSPORTATION 13

CONTENTS

INTRODUCTION	1
Background	1
Existing Development	1
Proposed Development	2
Existing Site Access	2
Future Site Access	2
Existing Road Network	3
Methodology	3
Information Reviewed	4
THE EXISTING ENVIRONMENT	4
Existing Traffic Flows	4
Non-Car Access / Transport	10
Site Access Sightlines	11
Accident History	11
Proposed Road Network Improvements	13
IMPACT OF THE PROPOSED DEVELOPMENT	13
Future Trip Generation	13
Distribution and Assignment	13
Other Planned Developments	14
Traffic Increase	14
Junction Capacity Assessment	15
Road Capacity Assessment	22
CUMULATIVE TRAFFIC IMPACTS	24
PROPOSED MITIGATION MEASURES	25
CONCLUSIONS	25

TABLES

Table 13-1 2016 AM Peak Existing (07:30 – 08:30)	5
Table 13-2 2016 PM Peak Existing (16:45 – 17:45) –	5
Table 13-3 2016 AM Peak Existing (07:30 – 08:30)	7
Table 13-4 2016 PM Peak Existing (16:45 – 17:45)	7
Table 13-5 2016 AM Peak Existing (07:30 – 08:30)	8
Table 13-6 2016 PM Peak Existing (16:45 – 17:45)	8
Table 13-7 2016 AM Peak Existing (07:30 – 08:30)	9
Table 13-8 2016 PM Peak Existing (16:45 – 17:45)	10
Table 13-9 Road Collision History	12
Table 13-10 Future Trip Generation	13
Table 13-11 Peak hour Flows	13
Table 13-12 Future Year Traffic Growth	14
Table 13-13 R135 / N2 Slip Road Priority Junction : AM Peak	15
Table 13-14 R135 / N2 Slip Road Priority Junction : PM Peak	16
Table 13-15 R135 / N2 Link Road Roundabout Junction : AM Peak	17
Table 13-16 R135 / N2 Link Road Roundabout Junction : PM Peak	17
Table 13-17 R135 / Elm Road Signalised Junction : AM Peak	19
Table 13-18 R135 / Elm Road Signalised Junction : PM Peak	19
Table 13-19 R135 / L3125 Signalised Junction : AM Peak	20
Table 13-20 R135 / L3125 Signalised Junction : PM Peak	21
Table 13-21 2016 Existing Annual Average Daily Traffic	22
Table 13-22 2017 Proposed Annual Average Daily Traffic with Development Flows	23
Table 13-23 2023 Proposed Annual Average Daily Traffic with Development Flows	23
Table 13-24 Outputs from Huntstown Quarry on 22 June 2016	24



TRAFFIC AND TRANSPORTATION 13

FIGURES

Figure 13-1 Site Location Map	2
Figure 13-2 R135 / N2 Slip Road Priority Junction Traffic Profile.....	5
Figure 13-3 R135 / N2 Slip Road Priority Junction Vehicle Percentages	5
Figure 13-4 R135 / Elm Road Signalised Junction	6
Figure 13-5 R135 / Elm Road Signalised Junction Vehicle Percentages	6
Figure 13-6 R135 / L3125 Signalised Junction Traffic Profile	7
Figure 13-7 R135 / L3125 Signalised Junction Vehicle Percentages.....	8

APPENDICES

- Appendix 13-A – Traffic Count Data
- Appendix 13-B – Traffic Flow Sheets
- Appendix 13-C – PICADY Results
- Appendix 13-D – ARCADY Results
- Appendix 13-E – OSCADY PRO Results

*For inspection purposes only.
Consent of copyright owner required for any other use.*



TRAFFIC AND TRANSPORTATION 13

INTRODUCTION

Background

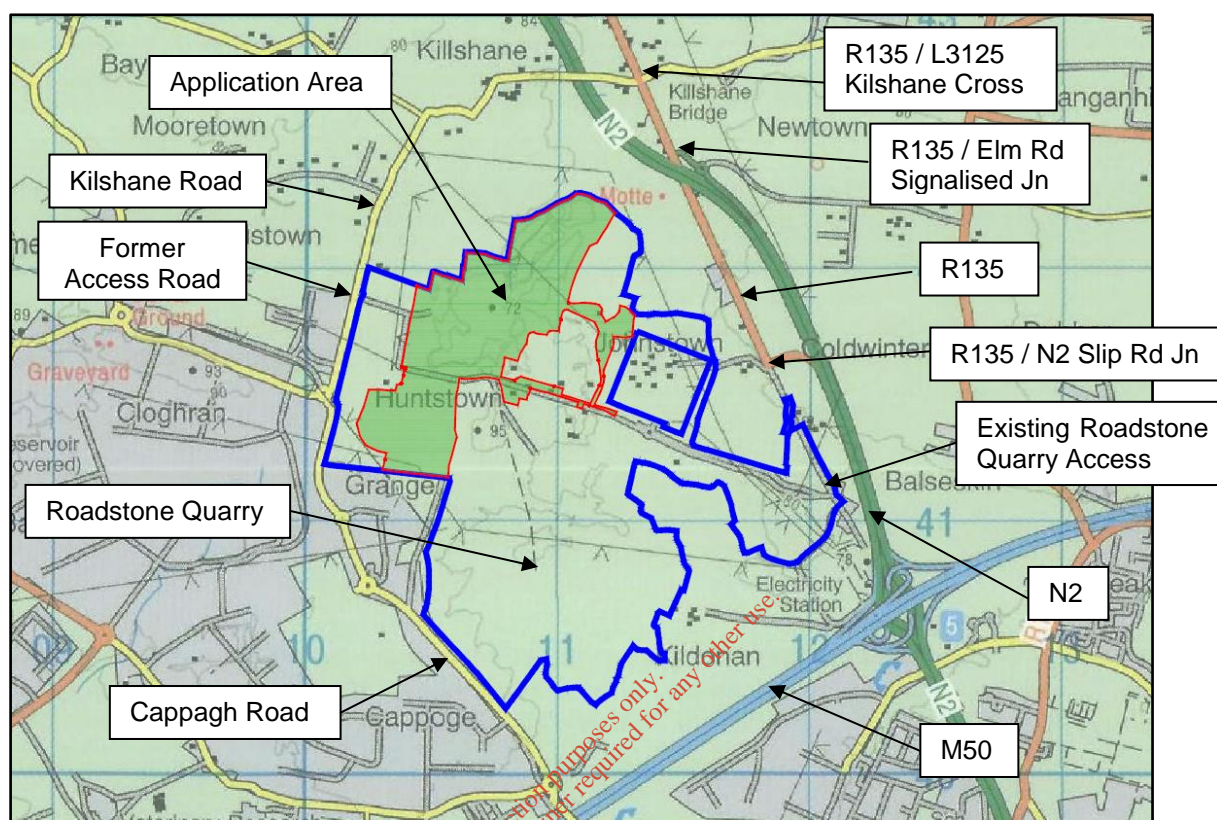
- 13.1 Roadplan Consulting was appointed by SLR Consulting Ireland to identify and assess the traffic and transportation impacts of a proposed planning application to increase the rate of inert soil and stone waste intake at Roadstone's established waste recovery facility at Huntstown Quarry, Finglas, Dublin 11, from a maximum of 750,000 tonnes per annum at present to a maximum of 1,500,000 tonnes per annum in future years.
- 13.2 This chapter predicts the trips expected to be generated by the proposed development, and the impacts of those trips on the operational performance of the local road network and junctions, in particular the following:
- R135 / N2 Slip Road - Priority Junction
 - R135 / Elm Road - Signalised Junction
 - R135 / L3125 - Signalised Junction
 - R135 / N2 - Roundabout Junction

Existing Development

- 13.3 Existing development at Huntstown Quarry is primarily tied to ongoing rock extraction and processing. In addition to established quarrying activities, several associated value-added activities are undertaken at the quarry, including production of concrete blocks and readymix concrete.
- 13.4 The restoration of the entire quarry complex at Huntstown comprising backfilling of 4 separate quarry voids using imported soil and stone waste was previously granted planning permission by Fingal County Council and An Bord Pleanála (Ref. FW12-0022 and ABP Ref. 241693). Backfilling of the North Quarry commenced in October 2015 at permitted rate of 750,000 tonnes per annum. As this activity is technically classified as a waste recovery activity, it is also subject to control by way of an existing EPA waste licence issued in February 2015 (Ref. W0277-01).
- 13.5 The quarry and application site are located within the townlands of Huntstown and Kilshane, Co. Dublin, approximately 2.5km north-west of the Dublin suburb of Finglas and 2km north-west of the interchange between the N2 Dual Carriageway and the M50 Motorway. Positioned north of the M50 motorway, the quarry has two access points: the primary entrance located on the eastern boundary of the Roadstone property holding, located off the R135 Regional Road, also known as the North Road, and a secondary access (which has been closed in recent years and is no longer in use), located on the western boundary of the property, along the Kilshane Road. The location of the application site, and both existing accesses, are shown on Figure 13-1.
- 13.6 The existing site infrastructure at the quarry complex includes internal haul roads, offices and staff welfare facilities, plant storage and maintenance sheds, refuelling facilities and crushing, grading and processing plant used to process blasted rock.

TRAFFIC AND TRANSPORTATION 13

Figure 13-1
Site Location Map



Proposed Development

13.7 This application relates to a proposed increase in the current maximum permitted rate of waste intake at the licensed soil recovery facility at Huntstown Quarry, from a maximum of 750,000 tonnes per annum at present to a maximum of 1,500,000 tonnes per annum in future years. If backfilling progresses at the maximum envisaged rate, the proposed duration of soil recovery and restoration activities at the North Quarry and West Quarry would be approximately 6 years.

Existing Site Access

13.8 Huntstown Quarry is currently accessed via the North Road (R135). The access road leading from North Road to the quarry complex is shared by quarry traffic and traffic going to and from Huntstown Power Station. The access road is approximately 7.3m wide at the site entrance and divides as it runs towards the principal quarries and waste recovery facility. The widths of the inbound and outbound lanes are approximately 3.7m.

Future Site Access

13.9 As this application is tied to a previous grant of planning permission and an existing waste licence, all of the proposed increased waste intake will be imported and delivered via the existing entrance / access road leading off the R135 North Road, located on the eastern side of the Huntstown Quarry complex. The former access road from the Kilshane Road to the west of the quarry complex will not be used.

TRAFFIC AND TRANSPORTATION 13

Existing Road Network

- 13.10 The existing road network within the vicinity of the application site is illustrated in Figure 13-1 and is described further below.
- 13.11 The existing road network around the recovery facility and application site is defined by:
- The R135 regional road to the east, which previously served as the N2 National Primary Road (up to May 2006). This road is known locally as the North Road. It intersects with the N2 Dual carriageway at the Cherryhound Interchange to the north and forms a cul-de-sac to the south (severed by the re-aligned N2);
 - a local road, known as the Kilshane Road (or Cappagh Road) to the west and north of the Huntstown quarry complex; and
 - The M50 Motorway which lies south of the existing quarry.
- 13.12 The N2 Dual Carriageway between the M50 Motorway and Cherryhound Interchange runs immediately east of the R135 Regional Road. It continues northwards from the Cherryhound Interchange as the M2 Motorway to the north of Ashbourne Co. Meath. From there, it becomes the N2 National Primary Road and continues northwards as a single carriageway road through the counties of Meath, Louth and Monaghan to the border with Northern Ireland.
- 13.13 In relation to the local road network, the application site and the Huntstown Quarry complex in general, is located to the north of the M50 motorway, west of the R135 Regional Road (North Road) and the N2 Dual Carriageway and east and south of the Kilshane Road.
- 13.14 Much of the road network around the application site has been upgraded in recent years. The N2 dual carriageway / M2 motorway opened in May 2006 and led to a large and immediate reduction in traffic levels along the former N2 National Primary Road (now the R135 Regional Road) immediately east of Huntstown Quarry. Upgrading of the M50 to provide three lanes of traffic in both directions was also completed in 2010, as was the upgrading of its interchange with the N2 dual carriageway at Finglas to provide for a free-flow interchange.
- 13.15 The existing R135 (North Road) comprises of a single carriageway road generally of about 7.5m width with hard shoulders of varying width. The alignment essentially runs straight from the existing quarry entrance northwards up to the N2 / M2 motorway at the Cherryhound interchange and southwards to the point at which it is severed by the M50 motorway at Finglas.
- 13.16 A speed limit of 50kph applies along the existing R135 regional road. This speed limit applies to traffic which travels between the R135 / N2 roundabout junction and the existing entrance to the quarry complex at Huntstown.

Methodology

- 13.17 HGV traffic travelling to the waste recovery facility via Dublin City and the N2 National Primary Road and/or M50 motorway to the south will access it via the R135 / N2 Slip Road priority junction at Coldwinters. HGV traffic travelling to the facility from the N2 or N3 to the north and north-west will travel via the R135 / N2 Roundabout junction at the Cherryhound Interchange.

TRAFFIC AND TRANSPORTATION 13

- 13.18 HGV traffic departing the recovery facility and travelling toward the city and M50 motorway will initially travel via the R135 / Elm Road signalised junction and turn back onto the southbound carriageway of the N2 dual carriageway. HGV traffic departing the facility and travelling towards the N2 Dual Carriageway northbound or the N3 will travel via the R135 / N2 Roundabout junction.
- 13.19 Having regard to the established / future pattern of traffic movements, the methodology adopted for this assessment is summarised as follows:
- A 12-hour Manual Classified Traffic Counts was undertaken by Tracsis Traffic and Data Services on the 22nd of June 2016. Count information was obtained at the following junctions (shown on Figure 13-1):
 1. R135 / N2 Slip Road Priority Junction at Coldwinters
 2. R135 / Elm Road Signalised Junction at Newtown
 3. R135 / L3125 Kilshane Cross Signalised Junction
 4. R135 / N2 Roundabout Junction (link to Cherryhound Interchange)
 - Existing Traffic Assessment – A spreadsheet model was created which contains the base year do-nothing traffic count data described above. The traffic count data was used to develop a PICADY model of the R135 / N2 Slip Road priority junction, an ARCADY model of the R135 / N2 roundabout junction and an OSCADY PRO model of the R135 / Elm Road signalised junction and the R135 / L3125 signalised junction.
 - Future Year Assessment – The estimated future year traffic volumes on the study area road network, as a result of the increase in background traffic and the additional development related traffic was used to assess the future operational performance of the junctions both at the year of opening of the development and six years after (2023).

Information Reviewed

- 13.20 In preparing this assessment, Roadplan Consulting has made reference to:
- the *Fingal Development Plan 2011 – 2017*,
 - The Institute of Highways and Transportation *Guidelines on the Preparation of Traffic Impact Assessments*,
 - the *TII Transport Assessment Guidelines*,
 - the *TII National Traffic Model*.
 - opening of the development (2017) and six years after (2023).

THE EXISTING ENVIRONMENT

Existing Traffic Flows

- 13.21 The 12-hour traffic flows for each junction are provided in Appendix 13-A – Traffic Count Data. Traffic flows along the R135 were recorded for the four junctions identified previously. The daily profile of traffic flow at each junction along the R135 is shown in the figures below, as is the percentage of each vehicle class. The traffic flows during the busiest AM and PM peak hours were also abstracted from the surveyed data and are shown in the tables below:

TRAFFIC AND TRANSPORTATION 13

Figure 13-2
R135 / N2 Slip Road Priority Junction Traffic Profile

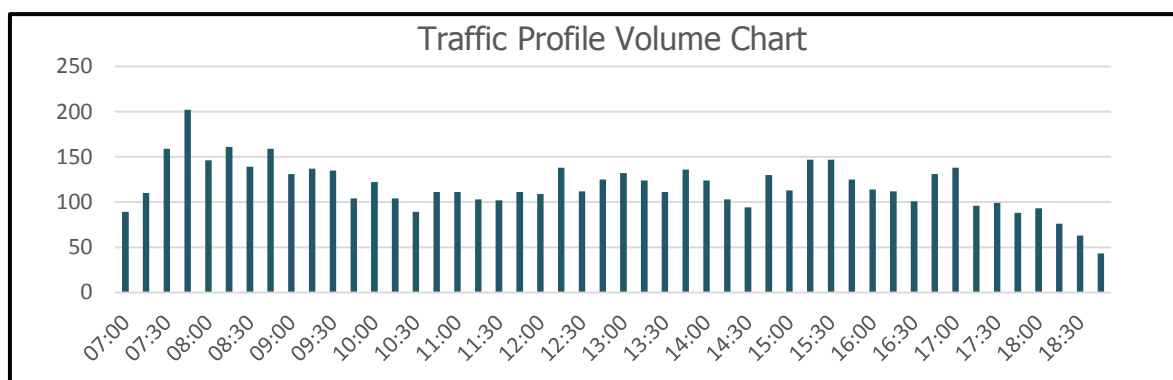


Figure 13-3
R135 / N2 Slip Road Priority Junction Vehicle Percentages

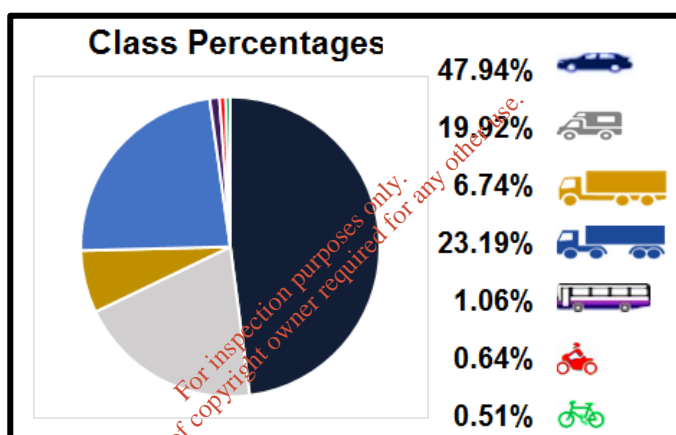


Table 13-1
2016 AM Peak Existing (07:30 – 08:30)

From / To	R135 (North)	N2 Slip Rd	R135 (South)	Totals
R135 (North)		0	58	58
N2 Slip Rd	396		128	524
R135 (South)	86	0		86
Totals	482	0	186	668

Table 13-2
2016 PM Peak Existing (16:45 – 17:45) –

From / To	R135 (North)	N2 Slip Rd	R135 (South)	Totals
R135 (North)		0	20	20
N2 Slip Rd	308		20	328
R135 (South)	116	0		116
Totals	424	0	40	464

TRAFFIC AND TRANSPORTATION 13

Principal features of the existing traffic flows at the R135 / N2 Slip Road priority junction are as follows:

- Overall flows are higher in the am peak compared to the pm peak, with the main traffic flows travelling along the N2 Slip Road;
- The N2 slip is one-way only with turning from the R135 Regional Road prohibited;
- The percentage of HGV movement is high with HGVs comprising approximately 30% of traffic travelling through the junction.

13.22 The daily profile of traffic flow at for the R135 / Elm Road signalised junction is shown in the figures below, as is the percentage of each vehicle class. The traffic flows during the busiest AM and PM peak hours were abstracted from the surveyed data and are shown in the tables below:

Figure 13-4
R135 / Elm Road Signalised Junction

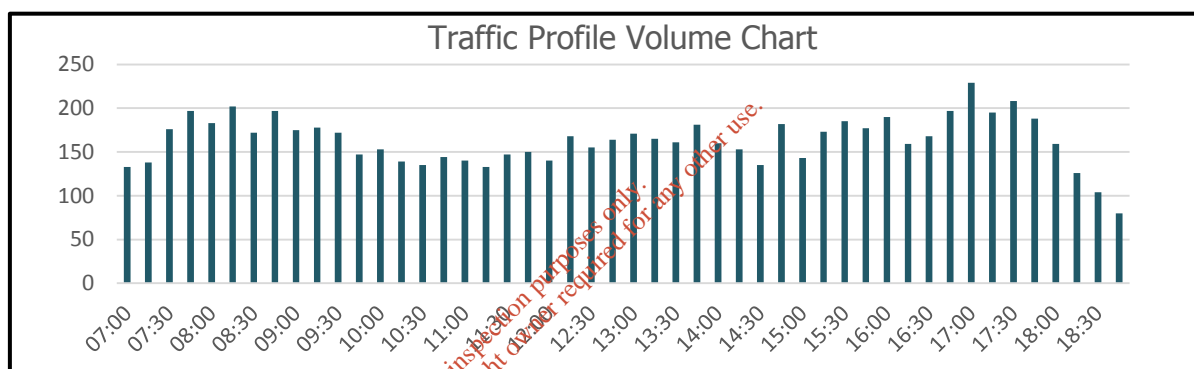
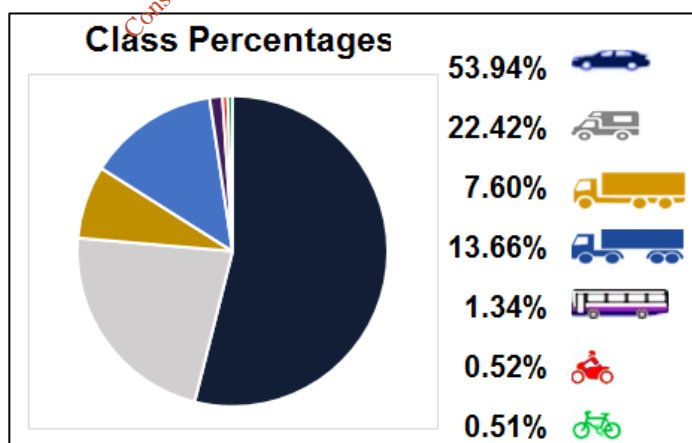


Figure 13-5
R135 / Elm Road Signalised Junction Vehicle Percentages



TRAFFIC AND TRANSPORTATION 13

Table 13-3
2016 AM Peak Existing (07:30 – 08:30)

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)		230	63	293
Elm Road	21		3	24
R135 (South)	319	122		441
Totals	340	352	66	758

Table 13-4
2016 PM Peak Existing (16:45 – 17:45)

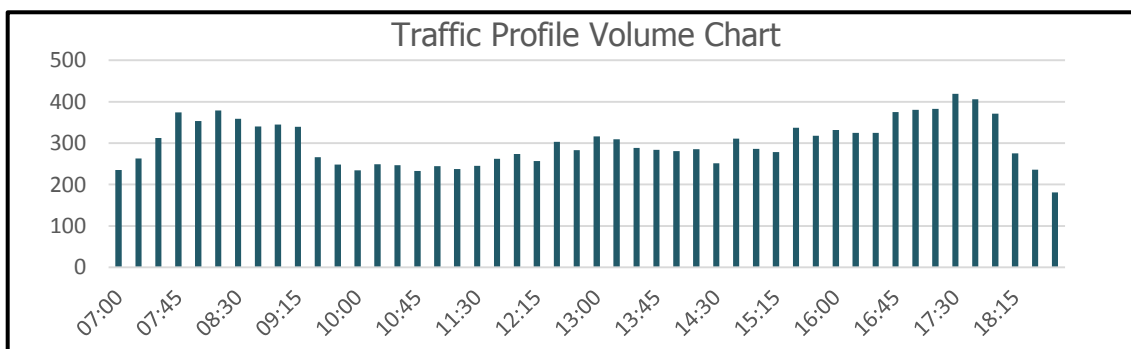
From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)		313	21	334
Elm Road	29		2	31
R135 (South)	320	144		464
Totals	349	457	23	829

Principal features of the existing traffic flows at the R135 / Elm Road Signalised junction are as follows:

- Overall flows are slightly higher in the pm peak compared to the am peak;
- Traffic volumes turning from Elm Road are low in both the am and pm peak;
- The R135 / Elm Road junction provides access to the N2 for vehicles travelling southbound only;
- The percentage of HGV movement is high with HGVs comprising approximately 21% of traffic travelling through the junction.

13.23 The daily profile of traffic flow at for the R135 / L3125 Kilshane crossroads signalised junction is shown in the figures below, as is the percentage of each vehicle class. The traffic flows during the busiest AM and PM peak hours were abstracted from the surveyed data and are shown in the tables below:

Figure 13-6
R135 / L3125 Signalised Junction Traffic Profile



TRAFFIC AND TRANSPORTATION 13

Figure 13-7
R135 / L3125 Signalised Junction Vehicle Percentages

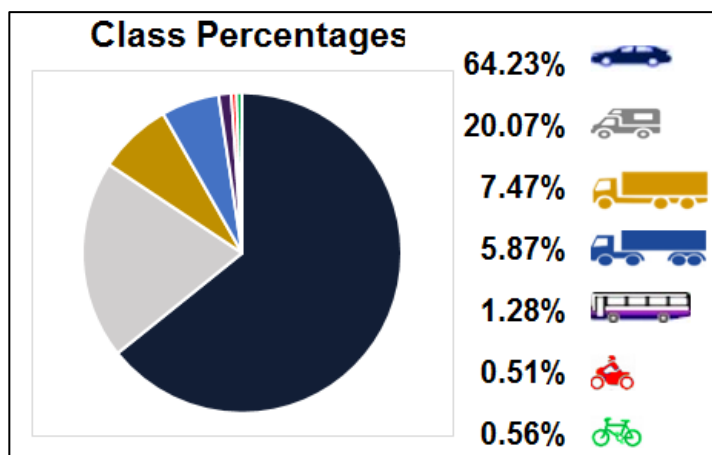


Table 13-5
2016 AM Peak Existing (07:30 – 08:30)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)		219	117	29	365
L3125 (East)	82		90	320	492
R135 (South)	59	73		205	337
L3125 (West)	9	134	81		224
Totals	150	426	288	554	1418

Table 13-6
2016 PM Peak Existing (16:45 – 17:45)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)		168	86	16	270
L3125 (East)	204		85	199	488
R135 (South)	111	120		110	341
L3125 (West)	22	273	163		458
Totals	337	561	334	325	1557

Principal features of the existing traffic flows at the existing R135 / L3125 Signalised junction are as follows;

- Overall flows are slightly higher in the pm peak compared to the am peak;
- The traffic flow travelling along the L3125 local road is higher than that along the R135 Regional Road;
- The L3125 provides access to Ballycoolin Industrial estate to the west and Dublin Airport to the east.

TRAFFIC AND TRANSPORTATION 13

13.24 The daily profile of traffic flow at for the R135 / N2 roundabout junction is shown in the figures below, as is the percentage of each vehicle class. The traffic flows during the busiest AM and PM peak hours were abstracted from the surveyed data and are shown in the tables below:

Figure 13-8
R135 / N2 Roundabout Junction Traffic Profile

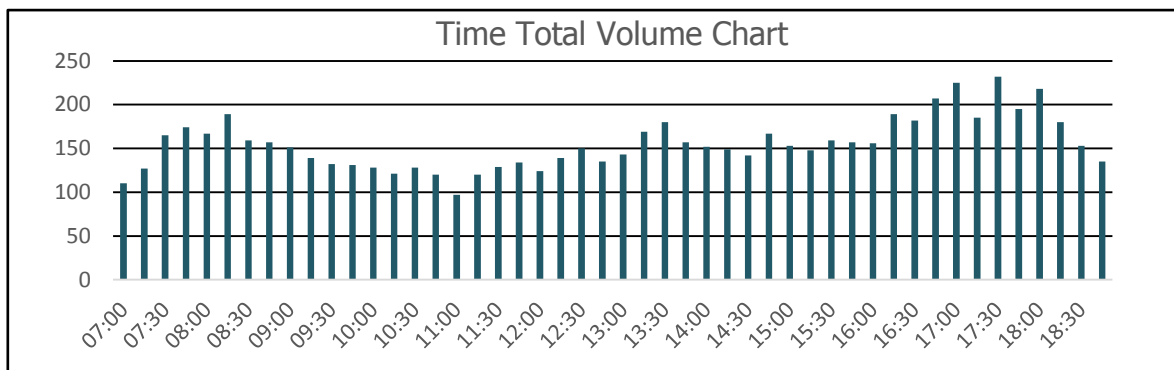


Figure 13-9
R135 / L3125 Signalised Junction Vehicle Percentages

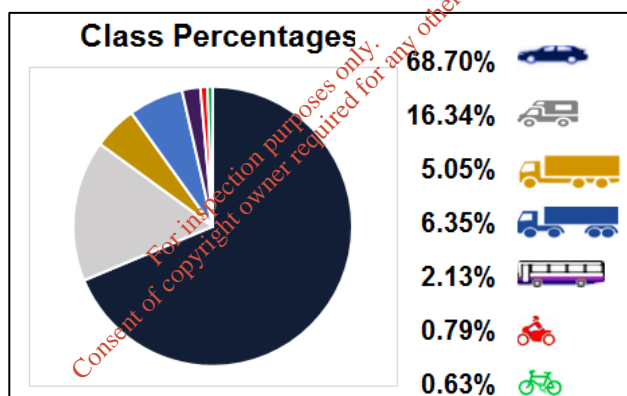


Table 13-7
2016 AM Peak Existing (07:30 – 08:30)

From / To	R135 (North)	R135 (South)	N2 Slip Rd	Totals
R135 (North)		170	139	309
R135 (South)	48		88	136
N2 Slip Rd	63	186		249
Totals	111	356	229	694

TRAFFIC AND TRANSPORTATION 13

Table 13-8
2016 PM Peak Existing (16:45 – 17:45)

From / To	R135 (North)	R135 (South)	N2 Slip Rd	Totals
R135 (North)		111	86	197
R135 (South)	185		175	360
N2 Slip Rd	127	160		278
Totals	312	271	261	844

Principal features of the existing traffic flows at the R135 / N2 Roundabout junction are as follows:

- Overall flows are slightly higher in the pm peak compared to the am peak;
- The flows along the N2 Slip Road are low.

Non-Car Access / Transport

- 13.25 To ascertain opportunities for sustainability, consideration has been given to non-car accessibility for the application site.
- 13.26 Although operation of the facility will be very dependent on road based haulage, staff will require transport means to get to and from work. Non-car accessibility will help reduce traffic impact and also promote social inclusion by providing means of transport to the application site to those who do not have access to a private car.
- 13.27 Access to the application site on foot is by way of the existing pedestrian footway situated to the eastern side of North Road. At approximately 1.5m in width, this footway extends along North Road northwards until reaching the northbound N2 Slip Road / North Road priority junction where it terminates. Pedestrians can then utilise the footpath situated to the western side of the R135 which extends for a short distance in the direction of Cherryhound interchange before terminating at a bus stop. For employees wishing to access this bus stop on foot it is an approximate distance of 1.8 kilometres away or a 25 minute walk (based upon an average walking distance of 1.2m per second.)
- 13.28 A large number of residential dwellings can be reached within a 5 kilometre cycle from the site which, assuming an average speed of 15kmh, equates to a 20 minute cycle ride. This includes the northern suburbs of Santry, Finglas, Charlestown, Dunsink and Castlenock and settlements to the north of the M50 Blanchardstown, Mulhuddart and Tyrellstown.
- 13.29 Adequate provision has already been made for pedestrians and cyclists within the visitor area with internal pedestrian walkways segregated from vehicles and cycle parking provided for those members of staff (or visitors) wishing to travel via bicycle. Whilst the Applicant is limited by what can realistically be undertaken to further improve non-car accessibility within the quarry complex, should the development proposal give rise to an increase in members of staff / visitors wishing to travel via bicycle then additional bicycle parking can be provided.

TRAFFIC AND TRANSPORTATION 13

- 13.30 The closest bus stop to the application site is situated on the North Road (R135), to the north of the N2 off-slip. The bus stop is a simple flag-post stop which is served by the Number 103 and 107 services operated by Bus Eireann. Both of the routes that service the bus stop run from Dublin to surrounding towns and villages.
- 13.31 The Number 103 service runs between Dublin City and Ashbourne / Ratoath. The service operates 7 days a week and begins at 06:30 in the morning and runs at approximately 20 minute intervals throughout the day until approximately midnight.
- 13.32 The Number 107 service runs between Dublin City and Navan, Nobber and Kells. The service operates 7 days a week and begins at 09:00 in the morning and runs at least four services in either direction throughout the day.
- 13.33 As identified above the local bus service is limited; however there are regular 20 minute services available from central Dublin throughout the day, seven days per week. Individual travel by bus to the application site is therefore available as an alternative to the car.

Site Access Sightlines

- 13.34 The requirements for sightlines are stated in the TII Design Manual for Urban Roads and Streets (DMURS). Sightlines are measured along the mainline from a driver's viewing position on the access road, 3.0m back from the mainline edge.
- 13.35 Access to the development is from the existing Roadstone Quarry Access onto the R135. The posted speed limit of the R135 at the existing access to the quarry is 50kph. The existing access provides a sightline in excess of 200m in each direction when assessed in accordance with the DMURS.

Accident History

- 13.36 Accident data has been obtained from the Road Safety Authority (RSA) website at www.rsa.ie. The website provides an interactive online mapping tool which has been navigated to the area of interest in order to determine the local accident history.
- 13.37 Accident data covering a nine year period 2005 to 2013 is provided. Over some of this period, the North Road / R135 Regional Road was designated a National Primary Road and carried significantly higher traffic volumes.
- 13.38 The information provided by the RSA is rudimentary and only provides the number of collisions and the seriousness of the collisions, along with basic details of each incident. The definitions of the severities are given below:
- Fatal (a crash resulting in death);
 - Serious (detention in hospital : includes paralysis, fractures and severe lacerations);
 - Minor (includes whiplash, strains and minor lacerations).
- 13.39 No accidents were recorded at the existing access to the Huntstown quarry complex. Two minor accidents were recorded south of the existing access and one minor accident was recorded north of the existing access. At the existing R135 / Elm Road signalised junction one minor accident was recorded.

TRAFFIC AND TRANSPORTATION 13

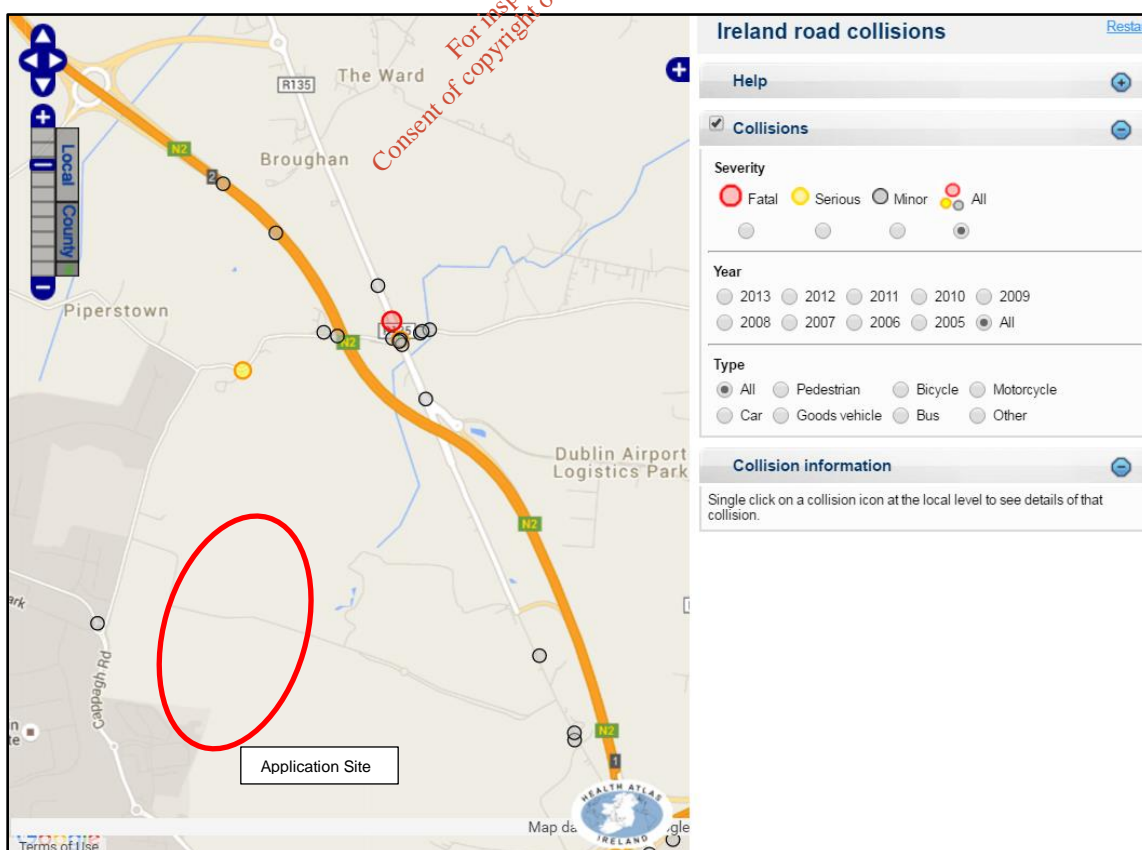
13.40 At the existing R135 / L3125 signalised junction (Kilshane Cross), one fatal accident, one serious accident and eight minor accidents were recorded. A summary of the collision history broken down into vehicle type is provided in Table 13.9 below:

**Table 13-9
Road Collision History**

Vehicle Type	Fatal Collision	Serious Collision	Minor Collision	All Collision
Bus	0	0	0	0
Goods Vehicle	0	1	2	3
Car	0	0	6	6
Motorcycle	1	0	4	5
Bicycle	0	0	0	0
Pedestrian	0	0	0	0
Other	0	0	0	0
Total	1	1	12	14

13.41 The location of each collision is shown in Figure 13.10 below

**Figure 13-10
Road Collisions**



TRAFFIC AND TRANSPORTATION 13

Proposed Road Network Improvements

- 13.42 Fingal County Council has published proposals to provide a future link road from the existing R135 / N2 roundabout to Dublin Airport. This road scheme is identified as a development objective in the Airport Local Area Plan.
- 13.43 It is envisaged that existing traffic flows at the R135 / L3125 Kilshane Crossroad signalised junction will significantly reduce following construction of the airport link road scheme as a result of re-distribution of traffic flows.

IMPACT OF THE PROPOSED DEVELOPMENT

Future Trip Generation

- 13.44 The proposed development will generate an increase in the rate of soil and stone waste intake from 750,000 tonnes per annum to 1,500,000 tonnes per annum (an increase of 750,000 tonnes per annum).
- 13.45 The existing quarry and recovery facility operate between 07:00 to 18:00 on Monday to Saturday and approximately 300 days annually (based on 6 days a week for 50 weeks).
- 13.46 It is anticipated that the waste material will arrive in 20 tonne consignments. Table 13.10 below provides a forecast of the average weekday traffic that is anticipated to be generated by the proposed increase of waste material intake.

Table 13-10
Future Trip Generation

	Volume (Tonnes)	Annual Loads (HGVs)	Average Daily Loads (HGVs)	Average Hourly Loads (HGVs)
OPERATION Waste Material Intake (20t)	750,000	37,500	130	12

Distribution and Assignment

- 13.47 Table 13-10 above indicates that the average increase in daily trip generation of HGV traffic as a result of the increase in waste material intake is 130 HGV loads per day. This equates to a total of 260 HGV movements per day (two-way trips).
- 13.48 The proposed development will be operational for approximately 11 hours per day with a total of 130 trips per day. Therefore for a 1 hour period there will be approximately 12 trips per hour (130 trips / 11 hours) arriving and departing from the proposed development. The table below shows the expected AM and PM peak flows to and from the development.

Table 13-11
Peak hour Flows

	Trips to Development	Trips from Development
AM Peak	12	12
PM Peak	12	12

TRAFFIC AND TRANSPORTATION 13

- 13.49 It is envisaged that all HGV's importing waste material to the recovery facility / quarry complex will approach the site along the M50 Motorway and / or the N2 Dual Carriageway and enter the site using the existing North Road entrance.
- 13.50 As such, all the proposed development traffic within the assessment have been distributed in line with the existing traffic patterns as surveyed.

Other Planned Developments

- 13.51 Planning has previously been granted to Roadstone to for a total aggregate and concrete output of up to 2 million tonnes per annum from the established quarry operations over a 20 year period from 2014 (Fingal County Council Ref. No FW12A-0022 and An Bord Pleanala Ref. No. 06F.241693). The proposed development will generate approximately 330 trips per day (based on working 6 days a week for 50 weeks, arriving and departing in 20 tonne consignments).
- 13.52 Planning permission has also been granted for a proposed Anaerobic Digestion (AD) Facility at Huntstown Quarry. (Planning Ref. FW13A/0089). The proposed development will generate approximately 60 trips per day. Access to the proposed AD facility will be via the existing R135 / Quarry Access priority junction.
- 13.53 Although it is currently uncertain when the AD facility will be developed or become operational, it has been assumed for the purposes of this assessment that the development will be fully operational by 2023.
- 13.54 Sensitivity testing of the above developments has been carried out to show the impact these developments will have on the existing road network with the proposed increased intake to the recovery facility in place in 2023.

Traffic Increase

- 13.55 The TII issues a range of traffic growth factors to be applied to existing traffic flows which are broken down into three groups; low growth, medium growth and high growth. Due to the close proximity of the M50 motorway and the N2 dual carriageway to the quarry complex and application site at Huntstown, it is assumed that medium growth is most likely for the R135 Regional Road.
- 13.56 The zone in which the quarry complex is located is numbered 821 in the TII National Traffic Model. The medium growth factors for each operational phase are as follows:

**Table 13-12
Future Year Traffic Growth**

Road	2016 Existing	2017 Development Operational	2023 Development Operational
All Roads	1.00	+ 0.71%	+ 4.36%

- 13.57 The above percentages have been used to predict the increase in background traffic along the R135 that will occur in future years. It should be noted that these growth factors do not apply to predicted trips generated by the proposed development under assessment. Full summary tables and predicted future traffic flows for 2017 and 2023 future years are included in Appendix 13-B – Traffic Flow Sheets.

TRAFFIC AND TRANSPORTATION 13

Junction Capacity Assessment

13.58 Capacity assessments using the computer programme PICADY for the existing R135 / N2 Slip Road priority junction and ARCADY for the existing R135 / N2 roundabout junction have been carried out, with and without the proposed development. Full details and results of capacity assessments are presented in Appendix 13-C – PICADY Results and in Appendix 13-D – ARCADY Results

13.59 The parameters shown in the tables are defined as follows:

- **Ratio of Flow to Capacity (RFC)** is a factor indicating the flow on a junction arm relative to its capacity. An RFC of 1.0 means the junction has reached its ultimate capacity and an RFC of 0.85 means that the junction has reached its reserve capacity.
- **Avg. Queue** is the average number of vehicles queued over the time period on the junction approach.
- **Queue delay** is the average number of seconds delay to each vehicle in the time period.
- **Total Delay** is the total number of vehicle hours of delay to all vehicles at the junction over the time period.

R135 / N2 Slip Road Priority Junction

13.60 The following tables show the predicted RFC values (Ratio of Flow to Capacity), average queue lengths, average vehicle delay and total delays for the existing R135 / N2 Slip Road priority junction at Coldwinters.

Table 13-13
R135 / N2 Slip Road Priority Junction : AM Peak

AM Peak		2016	2017 No Dev	2017 With Dev	2023 No Dev	2023 With Dev	2023 Sensitivity Flows	
R135 / N2 Slip Road Priority Junction	R135 North	RFC value	-	-	-	-	-	
		Average Queue (Vehicles)	-	-	-	-	-	
		Average delay (sec / veh)	-	-	-	-	-	
		Total Delay (veh / min)	-	-	-	-	-	
	N2 Slip Road	RFC value	0.87	0.88	0.89	0.92	0.93	0.96
		Average Queue (Vehicles)	6	6	6	8	9	11
		Average delay (sec / veh)	28	29	31	34	36	47
		Total Delay (veh / min)	5.20	5.38	5.78	6.79	7.60	9.86
	R135 South	RFC value	-	-	-	-	-	-
		Average Queue (Vehicles)	-	-	-	-	-	-
		Average delay (sec / veh)	-	-	-	-	-	-
		Total Delay (veh / min)	-	-	-	-	-	-

TRAFFIC AND TRANSPORTATION 13

Table 13-14
R135 / N2 Slip Road Priority Junction : PM Peak

PM Peak		2016	2017 No Dev	2017 With Dev	2023 No Dev	2023 With Dev	2023 Sensitivity Flows	
R135 / N2 Slip Road Priority Junction	R135 North	RFC value	-	-	-	-	-	
		Average Queue (Vehicles)	-	-	-	-	-	
		Average delay (sec / veh)	-	-	-	-	-	
		Total Delay (veh / min)	-	-	-	-	-	
	N2 Slip Road	RFC value	0.68	0.69	0.70	0.72	0.73	0.75
		Average Queue (Vehicles)	2	2	2	2	3	3
		Average delay (sec / veh)	17	18	18	19	19	20
		Total Delay (veh / min)	2.13	2.16	2.24	2.44	2.51	2.74
	R135 South	RFC value	-	-	-	-	-	-
		Average Queue (Vehicles)	-	-	-	-	-	-
		Average delay (sec / veh)	-	-	-	-	-	-
		Total Delay (veh / min)	-	-	-	-	-	-

13.61 The tables above indicate that at present the R135 / N2 Slip Road priority junction operates at capacity with queues and delays during the AM peak hour. During the PM peak hour, the junction will operate within capacity with small queues and delays.

13.62 Queues were noted at the junction and a maximum queue of 8 vehicles was noted at the junction during the traffic count, which correlates with the junction assessment.

13.63 With the increased soil waste intake rate in place in 2017 and 2023, the junction will operate at capacity with queues and delays during the AM peak hour. During the PM peak hour, the junction will operate within capacity with small queues and delays.

13.64 In 2023 during the AM peak hour, the junction has a maximum queue of 9 vehicles. The slip road from the N2 is approximately 200m in length and it is unlikely that vehicles will queue back to the N2 dual carriageway.

R135 / N2 Link Road Roundabout Junction

13.65 The following tables show the predicted RFC values (Ratio of Flow to Capacity), average queue lengths, average vehicle delay and total delays for the existing R135 / N2 Link Road priority junction.

TRAFFIC AND TRANSPORTATION 13

Table 13-15
R135 / N2 Link Road Roundabout Junction : AM Peak

AM Peak		2016	2017 No Dev	2017 With Dev	2023 No Dev	2023 With Dev	
R135 / N2 Link Road Roundabout Junction	R135 North	RFC value	0.22	0.22	0.23	0.23	0.24
		Average Queue (Vehicles)	0	0	0	0	0
		Average delay (sec / veh)	3	3	3	3	3
		Total Delay (veh / min)	0.33	0.34	0.35	0.36	0.37
	R135 South	RFC value	0.09	0.10	0.10	0.10	0.11
		Average Queue (Vehicles)	0	0	0	0	0
		Average delay (sec / veh)	2	2	2	2	2
		Total Delay (veh / min)	0.12	0.13	0.14	0.14	0.14
	N2 Link Road	RFC value	0.18	0.18	0.18	0.19	0.19
		Average Queue (Vehicles)	0	0	0	0	0
		Average delay (sec / veh)	3	3	3	3	3
		Total Delay (veh / min)	0.26	0.26	0.26	0.27	0.28

Table 13-16
R135 / N2 Link Road Roundabout Junction : PM Peak

PM Peak		2016	2017 No Dev	2017 With Dev	2023 No Dev	2023 With Dev	
R135 / N2 Link Road Roundabout Junction	R135 North	RFC value	0.13	0.14	0.14	0.14	0.15
		Average Queue (Vehicles)	0	0	0	0	0
		Average delay (sec / veh)	2	2	2	2	2
		Total Delay (veh / min)	0.18	0.19	0.20	0.20	0.20
	R135 South	RFC value	0.22	0.22	0.22	0.23	0.23
		Average Queue (Vehicles)	0	0	0	0	0
		Average delay (sec / veh)	2	2	2	2	2
		Total Delay (veh / min)	0.33	0.34	0.34	0.36	0.37
	N2 Link Road	RFC value	0.22	0.22	0.22	0.23	0.23
		Average Queue (Vehicles)	0	0	0	0	0
		Average delay (sec / veh)	3	3	3	3	3
		Total Delay (veh / min)	0.33	0.34	0.34	0.36	0.37

TRAFFIC AND TRANSPORTATION 13

- 13.66 The tables above indicate that at present the R135 / N2 Link Road roundabout junction will operate within capacity, with no queues and minimal delays during the AM and PM peak hours.
- 13.67 With the proposed development in place in 2017 and 2023 the roundabout junction will operate within capacity with no queues and minimal delays during the AM and PM peak hours.

Signalised Junctions

- 13.68 Capacity assessments using the computer programme OSCADY PRO have also been carried out, with and without the proposed development, on the following junctions:
- R135 / Elm Road Signalised Junction
 - R135 / L3125 Signalised Junction
- 13.69 Full details capacity assessments are contained in Appendix 13-E – OSCADY PRO Results.
- 13.70 The parameters shown in the tables are defined as follows:
- **Max Degree of Saturation (%)** is a ratio of demand to capacity on each approach to the junction, with a value of 100% meaning that demand and capacity are equal and no further traffic is able to progress through the junction. Values over 90% are typically regarded as suffering from traffic congestion, with queues of vehicles beginning to form.
 - **Queue at end of Red** is the number of vehicles queued on the approach arm at the end of red.
 - **Average Delay** is the average number of seconds delay to each vehicle in the time period.
 - **Practical Reserve Capacity** is the capacity available relative to a capacity of 90%. A positive PRC indicates that a junction has spare capacity and may be able to accept more traffic. A negative PRC indicates that the junction is over capacity and is suffering from traffic congestion.

R135 / Elm Road Signalised Junction

- 13.71 The following tables show the predicted degree of saturation, average queue lengths, average vehicle delay, total junction delays and practical reserve capacity for the existing R135 / Elm Road signalised junction.

TRAFFIC AND TRANSPORTATION 13

Table 13-17
R135 / Elm Road Signalised Junction : AM Peak

AM Peak		2016	2017 No Dev	2017 With Dev	2023 No Dev	2023 With Dev	2023 With Sensitivity Flows	
R135 / Elm Road Signalised Junction	R135 North	Max DoS %	28%	29%	29%	30%	30%	31%
		Q red (pcu's)	5	5	5	5	5	5
		Average delay (s)	24	25	25	25	25	28
		PRC %	219.81	213.13	211.14	200.64	198.81	189.06
	N2 Slip Road	Max DoS %	18%	18%	18%	19%	19%	19%
		Q red (pcu's)	1	1	1	1	1	1
		Average delay (s)	56	56	57	57	57	57
		PRC %	396.98	396.98	396.98	377.11	377.11	377.11
	R135 South	Max DoS %	29%	29%	29%	30%	30%	31%
		Q red (pcu's)	4	4	4	5	5	5
		Average delay (s)	29	28	28	28	28	28
		PRC %	212.59	211.11	210.03	203.86	197.93	192.35

Table 13-18
R135 / Elm Road Signalised Junction : PM Peak

PM Peak		2016	2017 No Dev	2017 With Dev	2023 No Dev	2023 With Dev	2023 With Sensitivity Flows	
R135 / Elm Road Signalised Junction	R135 North	Max DoS %	31%	31%	32%	32%	33%	34%
		Q red (pcu's)	5	5	5	5	6	6
		Average delay (s)	20	20	21	20	22	23
		PRC %	193.95	192.36	182.98	180.25	171.31	165.20
	N2 Slip Road	Max DoS %	25%	25%	25%	26%	26%	26%
		Q red (pcu's)	1	1	1	1	1	1
		Average delay (s)	59	59	59	59	59	59
		PRC %	261.44	261.44	261.44	240.79	240.79	240.79
	R135 South	Max DoS %	31%	32%	32%	33%	33%	34%
		Q red (pcu's)	4	4	4	5	5	5
		Average delay (s)	32	32	32	33	33	33
		PRC %	189.15	185.94	185.34	175.23	173.81	164.92

13.72 The tables above indicate that at present the R135 / Elm Road signalised junction operates within capacity with minimal queues and delays during the AM and PM peak hour.

13.73 With the development in place in 2017 and 2023 the junction will continue to operate within capacity with minimal queues and delays during the AM and PM peak hour.

TRAFFIC AND TRANSPORTATION 13

R135 / L3125 Kilshane Crossroads Signalised Junction

13.74 The following tables show the predicted degree of saturation, average queue lengths, average vehicle delay, total junction delays and practical reserve capacity for the existing R135 / Elm Road signalised junction.

Table 13-19
R135 / L3125 Signalised Junction : AM Peak

AM Peak		2016	2017 No Dev	2017 With Dev	2023 No Dev	2023 With Dev	2023 With Link Rd	2023 With Sensitivity Flows	
R135 / L3125 Signalised junction	R135 North	Max DoS %	47%	47%	48%	49%	50%	33%	34%
		Q red (pcu's)	12	12	12	12	12	8	9
		Average delay (s)	42	42	43	43	43	39	39
		PRC %	92.75	91.34	86.76	83.25	80.71	171.49	164.17
	L3125 East	Max DoS %	84%	85%	87%	89%	91%	87%	89%
		Q red (pcu's)	17	17	18	19	20	17	18
		Average delay (s)	51	51	55	56	60	56	64
		PRC %	6.11	5.38	3.06	1.02	-1.20	3.61	1.17
	R135 South	Max DoS %	84%	85%	86%	88%	89%	87%	88%
		Q red (pcu's)	12	12	12	13	14	13	14
		Average delay (s)	95	97	93	101	98	97	79
		PRC %	6.78	5.83	4.24	1.88	0.44	4.10	2.26
	L3125 West	Max DoS %	88%	89%	85%	93%	89%	86%	86%
		Q red (pcu's)	12	13	12	14	14	12	12
		Average delay (s)	92	83	75	91	98	74	80
		PRC %	1.30	0.66	4.94	-3.62	0.48	4.58	4.58

13.75 The table above indicate that, at present, the R135 / L3125 signalised junction is operating at capacity, with queues and delays during the AM peak hour.

13.76 With the proposed development and increased intake in place in 2017, the junction will continue to operate at capacity, with queues and delays during the AM peak hour.

13.77 With the proposed development in place in 2023, the junction will reach its maximum capacity, with queues and delays forming during the AM peak hour.

13.78 It should be noted that with no development in place in 2023, the junction will have reached its maximum capacity, with queues and delays forming during the AM peak hour.

13.79 With the opening of the Western (Airport) Link Road from the R135 / N2 roundabout to Dublin Airport, the impact on the R135 / L3125 signalised junction will be reduced. For the purposed of this assessment, it was assumed that 50% of the traffic flows travelling from the R135 North direction to the L3125 East and from the L3125 East direction to the R135 North would now travel via the Western Link Road. As a result, the signalised junction will operate within capacity in 2023, with queues and delays during the AM peak hour.

TRAFFIC AND TRANSPORTATION 13

13.80 Sensitivity testing of the other approved planning applications within the area (identified previously) indicates that the junction will operate within capacity in 2023, with queues and delays with the opening of the Western Link Road.

Table 13-20
R135 / L3125 Signalised Junction : PM Peak

PM Peak			2016	2017 No Dev	2017 With Dev	2023 No Dev	2023 With Dev	2023 With Link Rd	2023 With Sensitivity Flows
R135 / L3125 Signalised junction	R135 North	Max DoS %	44%	44%	43%	46%	45%	28%	30%
		Q red (pcu's)	9	9	9	10	10	6	7
		Average delay (s)	47	47	46	53	53	42	42
		PRC %	102.77	101.43	106.71	92.53	97.75	217.58	205.44
	L3125 East	Max DoS %	91%	92%	92%	96%	96%	89%	89%
		Q red (pcu's)	19	20	20	23	23	17	17
		Average delay (s)	63	64	64	72	72	65	71
		PRC %	-1.40	-2.41	-2.41	-6.26	-6.29	1.09	1.09
	R135 South	Max DoS %	93%	93%	92%	98%	96%	88%	89%
		Q red (pcu's)	13	13	13	15	14	12	13
		Average delay (s)	119	121	125	131	135	78	86
		PRC %	-3.47	-4.14	-3.22	-8.25	-6.90	1.97	1.33
	L3125 West	Max DoS %	89%	90%	93%	94%	97%	88%	91%
		Q red (pcu's)	15	15	17	18	19	15	17
		Average delay (s)	73	74	82	82	92	68	84
		PRC %	0.37	-0.36	-3.63	-4.52	-7.65	1.74	-1.39

13.81 The table above indicates that at present the R135 / L3125 signalised junction operates outside capacity, with queues and delays during the PM peak hour.

13.82 In 2017 and 2023, if the proposed development is not in place, the R135 / L3125 signalised operates outside capacity, with queues and delays during the PM peak hour.

13.83 With the development in place in 2017 and 2023, the junction will continue to operate outside capacity, with queues and delays during the PM peak hour.

13.84 With the opening of the Western (Airport) Link Road from the R135 / N2 roundabout to Dublin Airport, the impact on the R135 / L3125 signalised junction will reduce. For the purposes of this assessment, it was assumed that 50% of the traffic flows travelling from the R135 North direction to the L3125 East and from the L3125 East direction to the R135 North would now travel via the Western Link Road. As a result, the signalised junction will operate within capacity in 2023, with queues and delays during the AM peak hour.

13.85 Sensitivity testing of granted planning applications within the area indicates that the junction will operate within capacity in 2023 with queues and delays with the opening of the Western Link Road.

TRAFFIC AND TRANSPORTATION 13

Road Capacity Assessment

- 13.86 A capacity assessment of the R135 has been undertaken to determine the impact the proposed development flows will have on the R135 Regional Road. The AM and PM peak hour traffic counts have been converted to AADT (Annual Average Daily Traffic) using the methodology in TII Project Appraisal Guidelines 'Unit 16.2 Expansion Factors for Short Period Traffic Counts'.
- 13.87 The vehicle flows (Annual Average Daily Traffic) given in Table 6/1 of TD 9/12 of the Design Manual for Roads and Bridges represent the approximate two-way flows which correspond to Level of Service D in reasonably level terrain. This is the level of service at which passing becomes extremely difficult and begins to affect the overall flow of the road.
- 13.88 Table 6/1 of the TII TD 9/12 indicates that the R135 would be considered as a Type 2 Single Carriageway, with a capacity of 8,600 AADT for a Level of Service D.
- 13.89 Tables 13-21, 13-22 and 13-23 below calculate the existing AADT for the R135 and the future AADT for the R135 when the development is operational in the years 2017 and 2023.

Table 13-21
2016 Existing Annual Average Daily Traffic

STEP 1: Conversion of short period traffic counts to average daily traffic				
	Flows	PAG* factor	Daily flow (Flows/PAG*factor)	Average Daily traffic for 22 nd June 2016
AM peak (07:30 – 08:30)	633	0.07	9,042	8,315
PM peak (16:45 – 17:45)	683	0.09	7,588	
* Project Appraisal Guidelines				
STEP 2: Conversion of average daily traffic to weekly average daily traffic				
Average Daily traffic for 22 nd June 2016		PAG* factor	WADT Weekly Average Daily Traffic (Avg. Daily traffic * PAG factor)	
8,315		0.97	8,065	
STEP 3: Conversion of weekly average daily traffic to annual average daily traffic				
WADT		PAG* factor	AADT Annually Average Daily Traffic (WADT * PAG factor)	
8,065		0.96	7,742	

- 13.90 From Table 13-21 above the existing AADT for the R135 is 7,742 AADT which is below the recommended 8,600 AADT for a Level of Service D for a Type 2 Single Carriageway.

TRAFFIC AND TRANSPORTATION 13

Table 13-22
2017 Proposed Annual Average Daily Traffic with Development Flows

STEP 1: Conversion of short period traffic counts to average daily traffic				
	Flows	PAG* factor	Daily flow (Flows/PAG*factor)	Average Daily traffic for 22 nd June 2017
AM peak (07:30 – 08:30)	649	0.07	9,271	8,519
PM peak (16:45 – 17:45)	699	0.09	7,767	
<i>* Project Appraisal Guidelines</i>				
STEP 2: Conversion of average daily traffic to weekly average daily traffic				
Average Daily traffic for 22 nd June 2017	PAG* factor	WADT <i>Weekly Average Daily Traffic</i> (Avg. Daily traffic * PAG factor)		
8,519	0.97	8,263		
STEP 3: Conversion of weekly average daily traffic to annual average daily traffic)				
WADT	PAG* factor	AADT <i>Annually Average Daily Traffic</i> (WADT * PAG factor)		
8,263	0.96	7,932		

13.91 From Table 13-22 above, the AADT for the R135 in 2017 with the development fully operational is 7,932 AADT which is below the recommended 8,600 AADT for a Level of Service D for a Type 2 Single Carriageway.

Table 13-23
2023 Proposed Annual Average Daily Traffic with Development Flows

STEP 1: Conversion of short period traffic counts to average daily traffic				
	Flows	PAG* factor	Daily flow (Flows/PAG*factor)	Average Daily traffic for 22 nd June 2023
AM peak (07:30 – 08:30)	677	0.07	9,671	8,891
PM peak (16:45 – 17:45)	730	0.09	8,111	
<i>* Project Appraisal Guidelines</i>				
STEP 2: Conversion of average daily traffic to weekly average daily traffic				
Average Daily traffic for 22 nd June 2016	PAG* factor	WADT <i>Weekly Average Daily Traffic</i> (Avg. Daily traffic * PAG factor)		
8,891	0.97	8,624		
STEP 3: Conversion of weekly average daily traffic to annual average daily traffic)				
WADT	PAG* factor	AADT <i>Annually Average Daily Traffic</i> (WADT * PAG factor)		
8,624	0.98	8,451		

13.92 From Table 13-23 above, the AADT for the R135 in 2023 with the development fully operational is 8,451 AADT which is below the recommended 8,600 AADT for a Level of Service D for a Type 2 Single Carriageway.

TRAFFIC AND TRANSPORTATION 13

CUMULATIVE TRAFFIC IMPACTS

13.93 During the 12 hour traffic count undertaken on 22 June 2016, the HGV traffic movement to and from the quarry was recorded by Roadstone Ltd. The level of quarry outputs and activities that occurred on the day of the traffic survey are summarised in Table 13-24 below.

Table 13-24
Outputs from Huntstown Quarry on 22 June 2016

Material	Volume (Tonnes)	Traffic Movements
Concrete Volume	946	158
Cement	283	10
Sand	1229	49
Clay (Intake)	2488	138
Stone	4489	204
Masonry		6
Total		565

13.94 From Table 13-24 above, it can be seen that the waste intake on the day of the survey was 2488 tonnes (138 trips), which is close to the average daily level of 130 trips associated with the maximum permitted waste intake rate of 750,000 tonnes per annum.

13.95 The cumulative traffic impacts arising from quarry activities on the day of the survey was however in excess of the average level which was indicated by the EIS assessment undertaken in support of the 2012 planning application for continued operation of the quarry.

13.96 The planning application postulated a maximum output level from quarry operations of 2 million tonnes per annum (an average of 6,660 tonnes per day or an average of 330 trips per day). In the event, the total recorded number of trips to and from the quarry on the day of the traffic survey was 427, which is in excess of the average projected traffic levels (at maximum output) envisaged by the 2012 assessment.

13.97 The main reasons for the high level of activity at the quarry on the day of the traffic survey, which is unlikely to be repeated on a continual or ongoing basis, are understood to be as follows:

- activity in the construction industry is seasonal in nature – longer, drier days in summer months generate more activity and greater demand for construction materials;
- there was a particular increase / spike in activities at the quarry, in response to increased demand from on that particular day / week;
- the traffic levels travelling to and from the quarry (particularly concrete) are short-term only and are expected to reduce planning permission has been secured for a new concrete production facility at Feltrim Quarry.

13.98 As the traffic levels recorded on the date of the traffic survey were in excess of the average cumulative levels which will arise if all planned and permitted development around the Huntstown quarry complex does arise in the future

TRAFFIC AND TRANSPORTATION 13

(,an average of 390 trips per day), the junction and road capacity analysis and assessments presented in this impact assessment in respect of the increased waste intake to the existing recovery facility are deemed to be robust and to indicate that additional road / junction capacity constraints (other than those already likely to occur) will not occur in the future if all planned / proposed future development materialises.

PROPOSED MITIGATION MEASURES

13.99 It has been demonstrated in this chapter that the development proposal would generate an increase in HGV movements on the surrounding local network when compared to existing levels. HGV traffic can be of particular concern to both local residents and highway users, and the mitigation measures outlined below should be adopted to alleviate these concerns:

- Roadstone Ltd should adhere to a routing policy to ensure all movements are made via the strategic road network to avoid HGV's passing through residential areas as far as is practical and
- Roadstone Ltd should employ a policy of safety and environmental awareness for all HGV drivers accessing the recovery facility.

CONCLUSIONS

13.100 This chapter assesses the traffic and transport implications of the proposed increase in the current rate of waste intake at its licensed inert soil recovery facility at Huntstown Quarry from a maximum of 750,000 tonnes per annum at present to a maximum of 1,500,000 tonnes per annum. The intake of waste material will have a life span of 6 years.

13.101 The principal objective of this assessment is to provide a detailed consideration of the proposed development in terms of highways and transportation planning. This includes details of all the traffic and movement activity associated with the proposed development, and any resulting traffic and transport related impacts.

13.102 The application site is well located in terms of access to the strategic highway network and all HGV traffic can be routed on roads considered suitable to accommodate frequent HGV movement.

13.103 Junction capacity assessment was carried out to determine the impact the additional development trips would have on the existing junctions within the vicinity of the proposed development. The analysis showed that the existing R135 / Elm Road signalised junction and the R135 / N2 Slip Road roundabout junction will operate when the development is operational in 2017 and 2023.

13.104 The R135 / N2 Slip Road priority junction and the R135 / L3125 signalised junction are currently operating at capacity. With the development operational in 2017 and 2023, both junctions will continue to operate at capacity, with queues and delays during the AM and PM peak hours. It should be noted the development flows will have an insignificant impact on the operational performance of both junctions, as the junctions are operating at capacity with no development in place in 2017 and 2023 during the AM and PM peak hours.

13.105 With the opening of the Western Link Road traffic flows travelling through the R135 / L3125 signalised junction to and from Dublin Airport will re-distribute onto the Western Link Road. As a result the R135 / L3125 signalised junction

TRAFFIC AND TRANSPORTATION 13

will operate within capacity in 2017 and 2023 during the AM and PM peak hours with the proposed development in place.

- 13.106 A road capacity assessment of the R135 (North Road) was carried out to determine the impact the additional development flows would have on the R135 Regional Road. The AM and PM peak hour flows were converted to AADT using the methodology in TII Project Appraisal Guidelines. Table 6/1 of the TII TD 9/12 indicated that the R135 would be considered as a Type 2 Single Carriageway with a capacity of 8,600 AADT for a level of Service D.
- 13.107 The assessment showed that in 2016 the R135 operates within capacity for a level of service D, with an existing AADT level 7742 vehicles.
- 13.108 In 2017 and 2023 with the additional development trips and an increase in the background flows, the R135 will have a proposed AADT of 7,932 in 2017 and a proposed AADT of 8,451 in 2023, which is below the recommended AADT capacity for a Level of Service D for a Type 2 Single Carriageway.
- 13.109 A review of accident records on the surrounding highway network covering the period from 2006 to 2013 showed that no fatal or serious incidents were recorded at the North Road access to the Huntstown Quarry Complex. The one fatal incident and one serious incident recorded both occurred at Kilshane Cross.
- 13.110 Three minor incidents occurred in close proximity to the North Road quarry access. However it appears these took place prior to the N2 road realignment and upgrading. None are therefore specifically relevant to the development proposal, both in terms of location and incident detail. As such, it is considered that the proposed development would not have a significant impact on road safety.
- 13.111 Overall it is considered that the development proposal would have a minimal impact in terms of highways and transportation. For the above reasons the proposed development of the site accords with the national, regional and county planning policies and is considered to be acceptable in traffic and transport terms.

APPENDICES

*For inspection purposes only.
Consent of copyright owner required for any other use.*

APPENDIX 13-A TRAFFIC COUNT DATA

*For inspection purposes only.
Consent of copyright owner required for any other use.*

Entry: Am A - R135 (N)										Destination: Am B - R135 (S)										Destination: Am C - N2 On/Off Slip Road										Arm Totals
Destination: Am A - R135 (N)										Destination: Am B - R135 (S)										Destination: Am C - N2 On/Off Slip Road										Arm Totals
CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	Arm Totals						
07:00	0	0	0	0	0	0	0	15	5	1	0	0	0	1	22	18	8	2	1	0	0	0	29	51						
07:15	0	0	0	0	0	0	0	17	6	3	3	2	0	0	31	15	6	4	1	1	0	0	27	58						
07:30	0	0	0	0	0	0	0	24	10	0	1	2	1	0	38	17	6	6	1	1	0	0	31	69						
07:45	0	0	0	0	0	0	0	34	6	1	1	3	0	2	47	22	3	1	1	1	0	0	29	76						
1 HR	0	0	0	0	0	0	0	90	27	5	5	7	1	3	138	73	23	13	4	2	1	0	116	254						
08:00	0	0	0	0	0	0	0	37	8	0	0	2	0	1	48	22	7	2	3	0	0	0	34	82						
08:15	0	0	0	0	0	0	0	31	4	1	0	0	1	0	37	28	8	3	1	3	2	0	45	82						
08:30	0	0	0	0	0	0	0	19	4	1	0	1	0	0	25	20	5	3	1	0	0	0	29	54						
08:45	0	0	0	0	0	0	0	19	3	1	1	2	0	0	26	23	5	1	1	2	1	0	33	59						
1 HR	0	0	0	0	0	0	0	106	19	3	1	5	1	1	136	93	25	9	6	5	3	0	141	277						
09:00	0	0	0	0	0	0	0	20	3	1	1	1	0	1	27	20	3	1	0	0	0	0	24	51						
09:15	0	0	0	0	0	0	0	25	4	1	0	2	0	0	32	15	2	0	3	0	0	0	20	52						
09:30	0	0	0	0	0	0	0	28	6	0	0	0	0	0	34	14	4	2	1	0	0	0	21	55						
09:45	0	0	0	0	0	0	0	30	5	0	0	1	0	0	36	11	8	2	3	0	0	0	24	60						
1 HR	0	0	0	0	0	0	0	103	18	2	1	4	0	1	129	60	17	5	7	0	0	0	89	218						
10:00	0	0	0	0	0	0	0	11	4	1	1	2	0	1	20	13	10	3	3	0	0	0	29	49						
10:15	0	0	0	0	0	0	0	13	4	1	3	0	0	0	21	13	5	1	0	0	0	0	19	40						
10:30	0	0	0	0	0	0	0	22	5	1	0	1	0	0	29	9	1	1	0	0	2	0	13	42						
10:45	0	0	0	0	0	0	0	9	2	1	2	1	1	0	16	15	3	1	1	0	0	0	22	38						
1 HR	0	0	0	0	0	0	0	55	15	4	6	4	1	1	86	50	19	7	5	0	2	0	83	169						
11:00	0	0	0	0	0	0	0	16	7	0	1	2	0	1	27	3	1	3	0	0	0	0	7	34						
11:15	0	0	0	0	0	0	0	17	3	2	1	0	0	0	23	11	1	1	0	0	0	0	13	36						
11:30	0	0	0	0	0	0	0	22	2	4	1	1	0	0	30	7	3	3	2	0	0	0	15	45						
11:45	0	0	0	0	0	0	0	23	5	0	1	2	0	0	31	10	2	1	1	0	0	0	14	45						
1 HR	0	0	0	0	0	0	0	78	17	6	4	5	0	1	111	31	7	8	3	0	0	0	49	160						
12:00	0	0	0	0	0	0	0	12	5	1	1	0	0	0	19	10	0	3	1	0	0	0	14	33						
12:15	0	0	0	0	0	0	0	15	5	2	0	1	1	1	25	15	3	0	1	0	1	0	20	45						
12:30	0	1	1	0	0	0	2	19	5	0	1	0	0	0	25	13	6	2	0	0	1	0	22	49						
12:45	1	1	0	0	0	0	1	16	4	2	0	1	0	0	23	9	9	1	1	0	0	0	11	35						
1 HR	0	2	1	0	0	0	3	62	19	5	2	2	1	1	92	47	10	6	2	0	2	0	67	162						
13:00	0	0	0	0	0	0	0	14	6	0	0	2	0	0	22	17	4	1	1	0	0	0	23	45						
13:15	1	0	0	0	0	2	3	29	7	2	1	0	0	0	39	10	1	1	0	0	0	0	12	54						
13:30	0	0	0	0	0	0	0	27	5	1	1	1	0	1	37	20	3	0	1	0	0	0	24	61						
13:45	0	0	0	0	0	0	0	19	3	0	2	1	1	0	26	22	2	2	0	0	0	0	22	52						
1 HR	1	0	0	0	0	2	3	89	21	3	4	4	2	1	124	69	10	4	2	0	0	0	85	212						
14:00	1	0	0	0	0	0	1	23	3	1	0	1	0	0	28	19	4	2	1	0	0	0	26	55						
14:15	0	0	0	0	0	0	0	19	9	1	0	1	0	0	30	9	4	4	1	0	0	0	18	48						
14:30	0	0	0	0	0	0	0	26	7	0	0	1	0	0	34	11	5	1	1	0	0	0	18	52						
14:45	0	0	0	0	0	0	0	28	7	0	0	1	1	2	37	16	6	2	0	0	0	0	24	61						
1 HR	1	0	0	0	0	0	1	94	26	2	0	4	1	2	129	55	19	9	3	0	0	0	86	216						
15:00	0	0	0	0	0	0	0	16	3	0	0	2	0	0	21	23	2	2	0	0	0	0	27	48						
15:15	0	0	0	0	0	0	0	20	1	0	0	0	0	0	21	18	3	3	0	0	0	0	24	45						
15:30	0	0	0	0	0	0	0	19	6	0	1	0	0	0	26	12	5	1	0	0	0	0	18	46						
15:45	0	0	0	0	0	0	0	18	3	1	1	1	0	1	25	11	3	2	1	0	1	0	18	43						
1 HR	0	0	0	0	0	0	0	73	13	1	3	4	0	1	95	64	13	8	1	0	1	0	87	182						
16:00	0	0	0	0	0	0	0	17	4	3	0	2	0	0	26	12	5	0	0	0	0	0	17	43						
16:15	0	0	0	0	0	0	0	18	2	0	0	0	4	0	24	19	5	0	0	1	1	0	26	50						
16:30	0	0	0	0	0	0	0	15	1	0	0	1	0	0	18	16	4	1	1	0	0	0	21	39						
16:45	0	0	0	0	0	0	0	28	3	1	0	1	0	0	33	16	7	0	0	0	0	0	23	56						
1 HR	0	0	0	0	0	0	0	79	10	4	0	4	4	0	101	63	21	1	1	1	0	0	87	188						
17:00	0	0	0	0	0	0	0	23	4	0	1	1	0	3	32	19	6	1	0	0	0	0	26	58						
17:15	0	0	0	0	0	0	0	13	6	0	1	2	0	1	23	15	1	2	0	0	0	0	18	41						
17:30	0	0	0	0	0	0	0	14	6	0	1	0	0	0	23	15	1	1	0	0	0	0	18	42						
17:45	0	0	0	0	0	0	0	18	1	0	0	0	1	0	20	13	2	0	0	0	0	0	16	36						
1 HR	0	0	0	0	0	0	0	68	17	1	3	3	2	4	98	62	16	2	2	3	0	0	79	177						
18:00	0	1	0	0	0	0	1	17	1	1	1	3	0	1	24	14	3	0	0	0	0	0	24	49						
18:15	0	0	0	0	0	0	0	19	1	0	0	0	0	0	20	14	3	2	0	0	0	0	19	39						
18:30	0	0	0	0	0	0	0	17	1	0	0	0	0	0	18	15	3	0	0	0	0	0	16	34						
18:45	0	0	0	0	0	0	0	17	1	0	0	3	0	0	21	15	3	1	1	0	0	0	16	37						
1 HR	0	1	0	0	0	0	1	70	4	1	1	6	0	1	83	60	11	3	1	0	0	0	75	159						
12 Hrs	2	3	1	0	0	2	8	967	206	37	30	52	13	17	1322	727	187	75	37	8	10	0	1044	2374						
Check							8	967	206	37	30	52	13	17	1322	727	187	75	37	8	10	0	1044	2374						
Total	2	3	1	0	0	2	8	967	206	37	30	52	13	17	1322	727	187	75	37	8	10	0	1044	2374						
Check							8	967	206	37	30	52	13	17	1322	727	187	75	37	8	10	0	1044	2374						

Entry: Am B - R135 (S)										Destination: Am A - R135 (N)										Destination: Am C - N2 On/Off Slip Road										Arm Totals
Destination: Am B - R135 (S)										Destination: Am A - R135 (N)										Destination: Am C - N2 On/Off Slip Road										Arm Totals
CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	Arm Totals						
07:00	9	2	0	0	0	0	11	3	0	0	0	0	0	0	3	0	0	2	1	0	0	0	6	17						
07:15	6	6	0	1	0	0	13	0	0	0	0	0	0	0	0	8	1	2	3	0	0	0	14	27						
07:30	1	3	0	0	3	0	7	0	0	0	0	0	0	0	0	12	4	0	3	1	0	0	20	27						
07:45	16	4	4	1	0	0	15	0	0	0	0	0	0	0	0	11	1	0	0	0	0	0	21	36						
1 HR	26	15	0	2	3	0	46	0	0	0	0	0	0	0	0	34	8	4	14	1	0	0	61	107						
08:00	7	2	0	1	1	0	12	0	0	0	0	0</																		

Entry: Arm C - N2 On Off Slip Road

	Destination: Arm A - R135 (N)								Destination: Arm B - R135 (S)								Destination: Arm C - N2 On Off Slip Road								Arm Totals		
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total			
07:00	3	2	3	2	0	0	0	10	23	6	2	1	0	0	0	32	0	0	0	0	0	0	0	0	42		
07:15	2	4	1	0	0	0	0	7	25	5	0	2	1	0	1	34	0	1	0	0	0	0	0	0	42		
07:30	8	4	0	0	0	0	0	12	39	9	1	5	2	1	0	57	0	0	0	0	0	0	0	0	69		
07:45	57	6	5	0	0	0	0	65	30	13	0	2	0	0	0	45	0	0	0	0	0	0	0	0	62		
1 Hr	22	16	5	2	0	0	0	45	117	33	3	10	3	1	1	168	0	2	0	0	0	0	0	0	215		
08:00	10	4	0	0	0	0	0	14	27	8	2	2	0	0	0	39	0	0	0	0	0	0	0	0	53		
08:15	16	4	0	0	0	0	1	21	34	6	1	4	0	0	0	45	0	0	0	0	0	0	0	0	66		
08:30	19	6	0	1	0	0	0	26	37	7	1	3	0	0	0	48	0	0	0	0	0	0	0	0	74		
08:45	13	4	1	1	0	0	0	19	34	6	2	3	0	0	0	45	0	0	0	0	0	0	0	0	64		
1 Hr	58	18	1	2	0	1	0	80	132	27	6	12	0	0	0	177	0	0	0	0	0	0	0	0	257		
09:00	11	4	3	1	0	0	0	19	25	8	2	3	3	0	0	41	0	0	0	0	0	0	0	0	60		
09:15	13	3	1	1	0	0	0	18	19	5	1	6	0	0	0	31	0	0	0	0	0	0	0	0	49		
09:30	16	3	2	2	0	0	0	23	11	4	0	6	1	0	0	22	0	0	0	0	0	0	0	0	45		
09:45	16	5	0	0	0	0	0	21	15	3	0	5	1	0	0	24	0	0	0	0	0	0	0	0	42		
1 Hr	56	16	6	4	0	0	0	82	70	20	3	15	5	0	0	113	0	0	1	0	0	0	0	0	196		
10:00	11	6	3	1	0	0	0	22	10	4	1	4	0	0	0	19	1	2	0	0	0	0	0	0	44		
10:15	9	3	2	1	0	0	0	15	17	1	2	1	0	0	0	21	1	0	0	0	0	0	0	0	37		
10:30	9	2	0	0	0	0	0	11	12	4	1	3	0	0	0	21	1	0	0	0	0	0	0	0	33		
10:45	16	3	4	0	0	0	0	23	14	2	0	5	1	0	0	22	0	0	0	0	0	0	0	0	39		
1 Hr	39	14	9	2	0	0	0	65	53	11	4	14	1	0	0	83	3	2	0	0	0	0	0	0	153		
11:00	8	2	1	0	0	0	0	11	10	4	2	2	0	0	0	18	0	0	0	0	0	0	0	0	29		
11:15	12	5	2	2	0	0	0	23	14	3	1	0	0	0	0	18	1	0	0	0	0	0	0	0	42		
11:30	14	3	1	0	0	0	0	19	11	5	1	2	1	0	0	20	1	0	0	1	0	0	0	0	41		
11:45	10	2	2	1	0	0	0	15	11	5	0	4	0	0	0	20	0	0	0	0	0	0	0	0	41		
1 Hr	50	12	6	3	0	0	0	74	46	17	4	8	1	0	0	76	2	0	0	1	0	0	0	0	153		
12:00	12	3	1	1	0	0	0	18	12	3	0	1	0	0	0	16	3	0	0	0	0	0	0	0	37		
12:15	9	2	1	1	0	0	0	13	18	3	1	2	1	0	0	25	0	0	0	0	0	0	0	0	38		
12:30	17	4	4	2	0	0	0	28	19	3	1	0	1	0	0	24	0	0	0	0	0	0	0	0	52		
12:45	17	5	1	1	0	0	0	25	14	2	0	6	0	0	0	25	0	0	0	0	0	0	0	0	48		
1 Hr	52	14	10	4	0	2	0	82	63	11	2	9	5	0	0	90	3	0	0	0	0	0	0	0	3	175	
13:00	15	3	3	0	0	0	0	21	17	3	1	2	0	0	0	23	0	0	0	0	0	0	0	0	44		
13:15	16	4	3	0	0	0	0	23	19	3	2	1	0	0	0	25	0	0	0	0	0	0	0	0	48		
13:30	27	5	2	0	0	0	0	34	25	3	1	1	0	0	0	30	2	0	0	0	0	0	0	0	66		
13:45	17	5	1	1	0	0	0	24	18	4	4	2	0	0	0	31	0	0	0	0	0	0	0	0	48		
1 Hr	75	17	9	1	0	0	0	102	79	13	4	6	0	0	0	102	2	0	0	0	0	0	0	0	0	206	
14:00	15	5	0	1	1	0	0	22	10	2	2	4	1	0	1	20	0	0	0	0	0	0	0	0	42		
14:15	16	5	2	2	0	0	0	25	15	1	2	5	1	0	0	24	0	0	0	0	0	0	0	0	49		
14:30	11	4	2	0	0	0	0	17	18	6	3	2	1	0	0	30	0	0	0	0	0	0	0	0	47		
14:45	11	2	3	0	0	0	0	16	12	4	2	5	0	0	0	23	0	0	0	0	0	0	0	0	39		
1 Hr	53	16	7	3	1	0	0	80	55	13	9	16	3	0	1	97	0	0	0	0	0	0	0	0	177		
15:00	20	4	2	0	1	0	0	27	13	3	0	4	1	0	0	21	0	1	0	0	0	0	0	0	49		
15:15	21	4	2	0	0	0	0	27	15	5	1	3	0	0	0	24	0	0	0	0	0	0	0	0	51		
15:30	11	1	2	2	0	0	0	16	22	3	2	3	0	0	0	30	0	0	0	0	0	0	0	0	46		
15:45	13	6	0	0	0	0	0	22	13	3	3	5	0	0	0	24	0	0	0	0	0	0	0	0	46		
1 Hr	65	15	6	5	1	0	0	92	63	14	6	15	1	0	0	99	0	1	0	0	0	0	0	0	1	192	
16:00	17	9	3	1	0	0	0	31	12	2	0	1	0	0	0	15	1	0	0	0	0	0	0	0	47		
16:15	19	7	2	3	0	0	0	31	10	3	2	7	1	0	0	23	0	0	0	0	0	0	0	0	54		
16:30	30	7	0	1	1	0	0	39	29	7	1	1	0	0	0	38	1	1	0	0	0	0	0	0	68		
16:45	18	7	2	2	0	0	0	29	23	3	1	3	1	0	0	31	0	0	0	0	0	0	0	0	58		
1 Hr	84	30	7	7	1	0	0	130	74	15	4	12	2	0	0	107	2	0	0	0	0	0	0	0	2	239	
17:00	26	5	3	0	0	0	0	35	30	5	0	2	0	0	0	37	0	0	1	0	0	0	0	0	73		
17:15	18	8	2	0	0	0	0	28	35	4	2	1	0	0	0	42	0	0	0	0	0	0	0	0	70		
17:30	27	5	1	0	1	0	0	35	44	0	1	5	0	0	0	50	1	1	0	0	0	0	0	0	86		
17:45	26	3	2	1	1	0	0	35	22	3	2	2	0	0	0	29	1	0	0	0	0	0	0	0	83		
1 Hr	97	21	8	2	1	2	0	131	131	12	5	10	0	0	0	158	2	0	1	0	0	0	0	0	3	292	
18:00	26	3	3	1	0	0	0	34	20	4	2	4	0	1	0	31	0	0	0	0	0	0	0	0	65		
18:15	22	4	1	3	0	0	0	31	11	3	0	2	0	1	1	18	0	0	0	0	0	0	0	0	49		
18:30	25	2	2	1	0	0	0	30	17	2	0	4	1	0	0	24	0	0	0	0	0	0	0	0	54		
18:45	22	3	1	1	0	0	0	27	9	0	0	2	1	0	0	12	0	0	0	0	0	0	0	0	39		
1 Hr	95	12	6	5	0	4	0	122	57	9	2	12	2	2	1	85	0	0	0	0	0	0	0	0	0	207	
12 Hrs	746	201	80	40	4	12	2	1085	940	195	52	139	23	3	3	1355	14	5	2	1	0	0	0	0	22	2462	
Check								1085								1355										2462	
Total	746	201	80	40	4	12	2	1085	940	195	52	139	23	3	3	1355	14	5	2	1	0	0	0	0	0	2462	
Check								1085								1355											2462

ORIGIN SUMMARY

	Origin: Arm A - R135 (N)								Origin: Arm B - R135 (S)								Origin: Arm C - N2 On Off Slip Road								Origin Totals
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	Total	
07:00	33	13	3	1	0	0	0	51	12	2	2	1	0	0	0	17	26	8	5	3	0	0	0	0	42
07:15	32	12	7	4	3	0	0	58	14	7	2	4	0	0	0	27	27	10	1	2	1	0	1	4	42
07:30	41	16	6	2	3	1	0	69	13	7	0	3	4	0	0	27	47	13	1	5	2	1	0	0	69
07:45	57	9	2	2	3	1	2	75	21	7	0	8	0	0	0	36	27	7	0	1	0	0	0	0	63
1 Hr	163	50	18	9	2	1	2	254	60	23	4	16	4												



Client: Roadplan Consulting
 Job No: 3082-IRE-Huntstow Quarry
 Survey Date: 22/06/2016
 Survey Method: Video Observation
 Weather AM: Cloudy
 Weather PM: Cloudy
 Site No: Site 2
 Road: R135 / Elm Road Junction

Entry: Arm A - R135 (N)

	Destination: Arm A - R135 (N)							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
07:00	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0
1.Hr	0	0	0	0	0	0	0	0

	Destination: Arm B - Elm Road							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
31	6	5	6	0	2	0	50	
29	10	6	4	2	0	1	52	
25	12	10	7	2	1	1	58	
26	11	6	2	3	0	0	55	
120	39	27	19	7	3	2	217	

	Destination: Arm C - R135 (S)							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
5	2	0	1	0	0	2	10	
2	1	0	2	1	0	0	6	
16	2	0	3	0	0	2	23	
11	7	0	1	0	0	0	19	
34	12	0	7	1	0	4	58	

Arm Totals
60
58
81
75
275

12 Hrs	1614	652	227	181	43	17	7 <th>2741</th> <th>194</th> <th>70</th> <th>10</th> <th>121</th> <th>2</th> <th>0</th> <th>15</th> <th>412</th>	2741	194	70	10	121	2	0	15	412
Check								416								412
Total	1614	652	227	181	43	17	7	2741	194	70	10	121	2	0	15	412
Check								2741								412

Entry: Arm B - Elm Road

	Destination: Arm A - R135 (N)							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
07:00	0	1	0	0	0	0	1	
07:15	2	0	3	0	0	0	5	
07:30	1	2	0	0	0	0	3	
07:45	4	2	0	0	0	0	6	
1.Hr	7	5	3	0	0	0	15	

	Destination: Arm B - Elm Road							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
0	0	0	0	0	0	0	0	
0	0	0	0	0	0	0	0	
0	0	0	0	0	0	0	0	
0	0	0	0	0	0	0	0	
0	0	0	0	0	0	0	0	
0	0	0	0	0	0	0	0	

	Destination: Arm C - R135 (S)							Total
	CAR	LGV	OGV1	OGV2	PSV	MCL	PCL	
0	0	0	0	0	0	0	0	
0	0	0	0	0	0	0	0	
1	1	0	0	0	0	0	2	
0	0	0	0	0	0	0	0	
0	0	0	0	0	0	0	0	
1	1	0	0	0	0	0	2	

Arm Totals
1
5
5
6
17

12 Hrs	101	51	13	7	1	0	5 <th>178</th> <th>8</th> <th>3</th> <th>1</th> <th>0</th> <th>0</th> <th>0</th> <th>0 <th>12</th> </th>	178	8	3	1	0	0	0	0 <th>12</th>	12
Check								178								12
Total	101	51	13	7	1	0	5	178	8	3	1	0	0	0	0	12
Check								178								12

Entry: Arm C - R135 (S)

Table with columns for Origin: Arm A - R135 (N), Destination: Arm B - Elm Road, Destination: Arm C - R135 (S), and Arm Totals. Rows include time intervals from 07:00 to 18:45 and 12 Hrs, with columns for CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, and Total.

Summary row for the first table showing totals for 12 Hrs and a check of the totals.

ORIGIN SUMMARY

Table with columns for Origin: Arm A - R135 (N), Origin: Arm B - Elm Road, and Origin: Arm C - R135 (S), and Origin Totals. Rows include time intervals from 07:00 to 18:45 and 12 Hrs, with columns for CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, and Total.

Summary row for the second table showing totals for 12 Hrs and a check of the totals.

Entry: Arm C - R135 (S)

Table with columns for Destination: Arm A - R135 (N), Destination: Arm B - N2 Offslip, Destination: Arm C - R135 (S), and Arm Totals. Rows include time intervals (07:00-19:00) and 12 Hrs totals.

ORIGIN SUMMARY

Table with columns for Origin: Arm A - R135 (N), Origin: Arm B - N2 Offslip, Origin: Arm C - R135 (S), and Origin Totals. Rows include time intervals (07:00-19:00) and 12 Hrs totals.

Entry: Arm C - R135 (S)

Table with columns: Destination: Arm A - R135 (N), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Destination: Arm B - Kishane Way (E), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Destination: Arm C - R135 (S), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Destination: Arm D - Kishane Way (W), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Arm Totals. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Summary table for Arm C - R135 (S) with columns: 12 Hrs, Total, Check.

Summary table for Arm B - Kishane Way (E) with columns: 12 Hrs, Total, Check.

Summary table for Arm C - R135 (S) with columns: 12 Hrs, Total, Check.

Summary table for Arm D - Kishane Way (W) with columns: 12 Hrs, Total, Check.

Summary table for Arm Totals with columns: 12 Hrs, Total, Check.

Entry: Arm D - Kishane Way (W)

Table with columns: Destination: Arm A - R135 (N), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Destination: Arm B - Kishane Way (E), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Destination: Arm C - R135 (S), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Destination: Arm D - Kishane Way (W), CAR, LGV, OGV1, OGV2, PSV, MCL, PCL, Total. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Table with columns: Arm Totals. Rows include time slots from 07:00 to 18:45 and 12 Hrs summary.

Summary table for Arm A - R135 (N) with columns: 12 Hrs, Total, Check.

Summary table for Arm B - Kishane Way (E) with columns: 12 Hrs, Total, Check.

Summary table for Arm C - R135 (S) with columns: 12 Hrs, Total, Check.

Summary table for Arm D - Kishane Way (W) with columns: 12 Hrs, Total, Check.

Summary table for Arm Totals with columns: 12 Hrs, Total, Check.

Copyrighted material - not to be reproduced without the permission of the copyright owner. For internal use only. Further reproduction is prohibited.

*For inspection purposes only.
Consent of copyright owner required for any other use.*

APPENDIX 13-B TRAFFIC FLOW SHEETS

R135 / N2 Slip Road Priority Junction - AM Peak Hour**2016 Existing AM Peak**

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	58	58
N2 Slip Road	396	0	128	524
R135 (South)	86	0	0	86
Totals	482	0	186	668

2017 AM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	58	58
N2 Slip Road	399	0	129	528
R135 (South)	87	0	0	87
Totals	485	0	187	673

2017 AM Peak Development Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	6	6
N2 Slip Road	0	0	6	6
R135 (South)	12	0	0	12
Totals	12	0	12	24

2017 AM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	64	64
N2 Slip Road	399	0	135	534
R135 (South)	99	0	0	99
Totals	497	0	199	697

2023 AM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	61	61
N2 Slip Road	416	0	135	551
R135 (South)	90	0	0	90
Totals	507	0	195	702

2023 AM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	67	67
N2 Slip Road	416	0	141	557
R135 (South)	102	0	0	102
Totals	519	0	207	726

2023 AM Peak Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	16	16
N2 Slip Road	0	0	16	16
R135 (South)	32	0	0	32
Totals	32	0	32	64

2023 AM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	83	83
N2 Slip Road	416	0	157	573
R135 (South)	134	0	0	134
Totals	551	0	239	790

R135 / N2 Slip Road Priority Junction - PM Peak Hour**2016 Existing PM Peak**

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	20	20
N2 Slip Road	308	0	20	328
R135 (South)	116	0	0	116
Totals	424	0	40	464

2017 PM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	20	20
N2 Slip Road	310	0	20	330
R135 (South)	117	0	0	117
Totals	427	0	40	467

2017 PM Peak Development Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	6	6
N2 Slip Road	0	0	6	6
R135 (South)	12	0	0	12
Totals	12	0	12	24

2017 PM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	26	26
N2 Slip Road	310	0	26	336
R135 (South)	129	0	0	129
Totals	439	0	52	491

2023 PM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	21	21
N2 Slip Road	324	0	21	345
R135 (South)	122	0	0	122
Totals	446	0	42	488

2023 PM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	27	27
N2 Slip Road	324	0	27	351
R135 (South)	134	0	0	134
Totals	458	0	54	512

2023 PM Peak Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	16	16
N2 Slip Road	0	0	16	16
R135 (South)	32	0	0	32
Totals	32	0	32	64

2023 PM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	Totals
R135 (North)	0	0	43	43
N2 Slip Road	324	0	43	367
R135 (South)	166	0	0	166
Totals	490	0	86	576

R135 / Elm Road Signalised Junction - AM Peak Hour**2016 Existing AM Peak**

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	230	63	293
Elm Road	21	0	3	24
R135 (South)	319	122	0	441
Totals	340	352	66	758

2016 AM Peak PCU Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	283	71	354
Elm Road	22	0	4	26
R135 (South)	371	173	0	544
Totals	393	456	75	924

2017 AM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	285	72	357
Elm Road	22	0	4	26
R135 (South)	374	174	0	548
Totals	396	459	76	931

2017 AM Peak Development Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	0	6	6
Elm Road	0	0	0	0
R135 (South)	6	6	0	12
Totals	6	6	6	18

2017 AM Peak With Development

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	285	78	363
Elm Road	22	0	4	26
R135 (South)	380	180	0	560
Totals	402	465	82	949

2023 AM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	297	75	372
Elm Road	23	0	4	27
R135 (South)	390	182	0	572
Totals	413	479	79	971

2023 AM Peak With Development

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	297	81	378
Elm Road	23	0	4	27
R135 (South)	396	188	0	584
Totals	419	485	85	989

2023 AM Peak Sensitivity Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	0	16	16
Elm Road	0	0	0	0
R135 (South)	16	16	0	32
Totals	16	16	16	48

2023 AM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	297	97	394
Elm Road	23	0	4	27
R135 (South)	412	204	0	616
Totals	435	501	101	1037

R135 / Elm Road Signalised Junction - PM Peak Hour**2016 Existing PM Peak**

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	313	21	334
Elm Road	29	0	2	31
R135 (South)	320	144	0	464
Totals	349	457	23	829

2016 PM Peak PCU Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	337	28	365
Elm Road	30	0	2	32
R135 (South)	378	164	0	542
Totals	408	501	30	939

2017 PM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	339	28	368
Elm Road	30	0	2	32
R135 (South)	381	165	0	546
Totals	411	505	30	946

2017 PM Peak Development Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	0	6	6
Elm Road	0	0	0	0
R135 (South)	6	6	0	12
Totals	6	6	6	18

2017 PM Peak With Development

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	339	34	374
Elm Road	30	0	2	32
R135 (South)	387	171	0	558
Totals	417	511	36	964

2023 PM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	354	29	384
Elm Road	32	0	2	34
R135 (South)	397	172	0	570
Totals	429	527	32	987

2023 PM Peak With Development

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	354	35	390
Elm Road	32	0	2	34
R135 (South)	403	178	0	582
Totals	435	533	38	1005

2023 PM Peak Sensitivity Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	0	16	16
Elm Road	0	0	0	0
R135 (South)	16	16	0	32
Totals	16	16	16	48

2023 PM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	Elm Road	R135 (South)	Totals
R135 (North)	0	354	51	406
Elm Road	32	0	2	34
R135 (South)	419	194	0	614
Totals	451	549	54	1053

R135 / L3125 Signalised Junction - AM Peak Hour**2016 Existing AM Peak**

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	219	117	29	365
L3125 (East)	82	0	90	320	492
R135 (South)	59	73	0	205	337
L3125 (West)	9	134	81	0	224
Totals	150	426	288	554	1418

2016 AM Peak PCU Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	234	137	30	401
L3125 (East)	93	0	101	331	525
R135 (South)	77	88	0	224	389
L3125 (West)	11	163	111	0	285
Totals	181	485	349	585	1600

2017 AM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	236	138	30	404
L3125 (East)	94	0	102	333	529
R135 (South)	78	89	0	226	392
L3125 (West)	11	164	112	0	287
Totals	182	488	351	589	1611

2017 AM Peak Development Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	6	0	6
L3125 (East)	0	0	0	0	0
R135 (South)	6	0	0	0	6
L3125 (West)	0	0	0	0	0
Totals	6	0	6	0	12

2017 AM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	236	144	30	410
L3125 (East)	94	0	102	333	529
R135 (South)	84	89	0	226	398
L3125 (West)	11	164	112	0	287
Totals	188	488	357	589	1623

2023 AM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	246	144	32	421
L3125 (East)	98	0	106	348	552
R135 (South)	81	92	0	235	409
L3125 (West)	12	171	117	0	300
Totals	190	510	367	615	1682

2023 AM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	246	150	32	427
L3125 (East)	98	0	106	348	552
R135 (South)	87	92	0	235	415
L3125 (West)	12	171	117	0	300
Totals	196	510	373	615	1694

R135 / L3125 Signalised Junction - PM Peak Hour**2016 Existing PM Peak**

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	168	86	16	270
N2 Slip Road	204	0	85	199	488
R135 (South)	111	120	0	110	341
L3125 (West)	22	273	163	0	458
Totals	337	561	334	325	1557

2016 PM Peak PCU Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	177	97	18	292
N2 Slip Road	216	0	91	221	528
R135 (South)	127	136	0	135	398
L3125 (West)	24	179	175	0	378
Totals	367	492	363	374	1596

2017 PM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	178	98	18	294
N2 Slip Road	218	0	92	223	532
R135 (South)	128	137	0	136	401
L3125 (West)	24	180	176	0	381
Totals	370	495	366	377	1607

2017 PM Peak Development Flows

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	6	0	6
N2 Slip Road	0	0	0	0	0
R135 (South)	6	0	0	0	6
L3125 (West)	0	0	0	0	0
Totals	6	0	6	0	12

2017 PM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	178	104	18	300
N2 Slip Road	218	0	92	223	532
R135 (South)	134	137	0	136	407
L3125 (West)	24	180	176	0	381
Totals	376	495	372	377	1619

2023 PM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	186	102	19	307
N2 Slip Road	227	0	96	232	555
R135 (South)	133	143	0	142	418
L3125 (West)	25	188	184	0	397
Totals	386	517	382	393	1677

2023 PM Peak With Development

From / To	R135 (North)	N2 Slip Road	R135 (South)	L3125 (West)	Totals
R135 (North)	0	186	108	19	313
N2 Slip Road	227	0	96	232	555
R135 (South)	139	143	0	142	424
L3125 (West)	25	188	184	0	397
Totals	392	517	388	393	1689

R135 / L3125 Signalised Junction - AM Peak Hour**2016 Existing AM Peak**

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	219	117	29	365
L3125 (East)	82	0	90	320	492
R135 (South)	59	73	0	205	337
L3125 (West)	9	134	81	0	224
Totals	150	426	288	554	1418

2016 AM Peak PCU Flows + Re-distributed Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	117	137	30	284
L3125 (East)	46	0	101	331	478
R135 (South)	77	88	0	224	389
L3125 (West)	11	163	111	0	285
Totals	134	368	349	585	1436

2017 AM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	118	138	30	286
L3125 (East)	46	0	102	333	481
R135 (South)	78	89	0	226	392
L3125 (West)	11	164	112	0	287
Totals	135	371	351	589	1446

2017 AM Peak Development Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	6	0	6
L3125 (East)	0	0	0	0	0
R135 (South)	6	0	0	0	6
L3125 (West)	0	0	0	0	0
Totals	6	0	6	0	12

2017 AM Peak With Development

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	118	144	30	292
L3125 (East)	46	0	102	333	481
R135 (South)	84	89	0	226	398
L3125 (West)	11	164	112	0	287
Totals	141	371	357	589	1458

2023 AM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	123	144	32	298
L3125 (East)	48	0	106	348	502
R135 (South)	81	92	0	235	409
L3125 (West)	12	171	117	0	300
Totals	141	387	367	615	1509

2023 AM Peak With Development + Opening of Airport Link Road

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	123	150	32	304
L3125 (East)	48	0	106	348	502
R135 (South)	87	92	0	235	415
L3125 (West)	12	171	117	0	300
Totals	147	387	373	615	1521

2023 AM Peak Sensitivity Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	16	0	16
L3125 (East)	0	0	0	0	0
R135 (South)	16	0	0	0	16
L3125 (West)	0	0	0	0	0
Totals	16	0	16	0	32

2023 AM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	123	166	32	320
L3125 (East)	48	0	106	348	502
R135 (South)	103	92	0	235	431
L3125 (West)	12	171	117	0	300
Totals	163	387	389	615	1553

R135 / L3125 Signalised Junction - PM Peak Hour**2016 Existing PM Peak**

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	168	86	16	270
L3125 (East)	204	0	85	199	488
R135 (South)	111	120	0	110	341
L3125 (West)	22	273	163	0	458
Totals	337	561	334	325	1557

2016 PM Peak PCU Flows + Re-distributed Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	88	97	18	203
L3125 (East)	108	0	91	221	420
R135 (South)	127	136	0	135	398
L3125 (West)	24	179	175	0	378
Totals	259	403	363	374	1399

2017 PM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	89	98	18	204
L3125 (East)	109	0	92	223	423
R135 (South)	128	137	0	136	401
L3125 (West)	24	180	176	0	381
Totals	261	406	366	377	1409

2017 PM Peak Development Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	6	0	6
L3125 (East)	0	0	0	0	0
R135 (South)	6	0	0	0	6
L3125 (West)	0	0	0	0	0
Totals	6	0	6	0	12

2017 PM Peak With Development

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	89	104	18	210
L3125 (East)	109	0	92	223	423
R135 (South)	134	137	0	136	407
L3125 (West)	24	180	176	0	381
Totals	267	406	372	377	1421

2023 PM Peak No Development (PCU Flows + TII Growth Factors)

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	92	102	19	213
L3125 (East)	114	0	96	232	441
R135 (South)	133	143	0	142	418
L3125 (West)	25	188	184	0	397
Totals	272	424	382	393	1470

2023 PM Peak With Development + Opening of Airport Link Road

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	92	108	19	219
L3125 (East)	114	0	96	232	441
R135 (South)	139	143	0	142	424
L3125 (West)	25	188	184	0	397
Totals	278	424	388	393	1482

2023 PM Peak Sensitivity Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	0	16	0	16
L3125 (East)	0	0	0	0	0
R135 (South)	16	0	0	0	16
L3125 (West)	0	0	0	0	0
Totals	16	0	16	0	32

2023 PM Peak With Development Flows + Sensitivity Flows

From / To	R135 (North)	L3125 (East)	R135 (South)	L3125 (West)	Totals
R135 (North)	0	92	124	19	235
L3125 (East)	114	0	96	232	441
R135 (South)	155	143	0	142	440
L3125 (West)	25	188	184	0	397
Totals	294	424	404	393	1514

R135 / N2 Roundabout Junction - AM Peak Hour**2016 Existing AM Peak**

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	170	139	309
R135 (South)	48	0	88	136
N2 Slip Road	63	186	0	249
Totals	111	356	227	694

2017 AM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	171	140	311
R135 (South)	48	0	89	137
N2 Slip Road	63	187	0	251
Totals	112	359	229	699

2017 AM Peak Development Flows

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	0	6	6
R135 (South)	0	0	6	6
N2 Slip Road	0	0	0	0
Totals	0	0	12	12

2017 AM Peak With Development

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	171	146	317
R135 (South)	48	0	95	143
N2 Slip Road	63	187	0	251
Totals	112	359	241	711

2023 AM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	179	146	325
R135 (South)	50	0	92	143
N2 Slip Road	66	195	0	262
Totals	117	374	239	729

2023 AM Peak With Development

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	179	152	331
R135 (South)	50	0	98	149
N2 Slip Road	66	195	0	262
Totals	117	374	251	741

R135 / N2 Roundabout Junction - PM Peak Hour**2016 Existing PM Peak**

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	111	86	197
R135 (South)	185	0	175	360
N2 Slip Road	127	160	0	287
Totals	312	271	261	844

2017 PM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	112	87	198
R135 (South)	186	0	176	363
N2 Slip Road	128	161	0	289
Totals	314	273	263	850

2017 PM Peak Development Flows

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	0	6	6
R135 (South)	0	0	6	6
N2 Slip Road	0	0	0	0
Totals	0	0	12	12

2017 PM Peak With Development

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	112	93	204
R135 (South)	186	0	182	369
N2 Slip Road	128	161	0	289
Totals	314	273	275	862

2023 PM Peak No Development (Existing + TII Growth Factors)

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	117	90	207
R135 (South)	194	0	184	378
N2 Slip Road	133	168	0	302
Totals	328	285	274	887

2023 PM Peak With Development

From / To	R135 (North)	R135 (South)	N2 Slip Road	Totals
R135 (North)	0	117	96	213
R135 (South)	194	0	190	384
N2 Slip Road	133	168	0	302
Totals	328	285	286	899

APPENDIX 13-C PICADY RESULTS

*For inspection purposes only.
Consent of copyright owner required for any other use.*

TRL LIMITED

(C) COPYRIGHT 2006

CAPACITIES, QUEUES, AND DELAYS AT 3 OR 4-ARM MAJOR/MINOR PRIORITY JUNCTIONS

PICADY 5.0 ANALYSIS PROGRAM
RELEASE 3.0 (JUNE 2006)

ADAPTED FROM PICADY/3 WHICH IS CROWN COPYRIGHT
BY PERMISSION OF THE CONTROLLER OF HMSO

FOR SALES AND DISTRIBUTION INFORMATION,
PROGRAM ADVICE AND MAINTENANCE CONTACT:
TRL SOFTWARE BUREAU
TEL: CROWTHORNE (01344) 770758, FAX: 770864
EMAIL: SoftwareBureau@trl.co.uk

THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF HIS RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

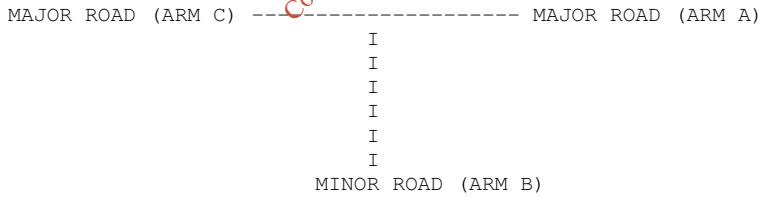
Run with file:-
"C:\PICADY\2016\16047-02\R135 - N2 Slip Road Priority Junction.vpi"
(drive-on-the-left) at 09:30:29 on Thursday, 4 August 2016

.RUN INFORMATION

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan Consulting
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

.MAJOR/MINOR JUNCTION CAPACITY AND DELAY

INPUT DATA



ARM A IS R135 North
ARM B IS N2 Slip Road
ARM C IS R135 South

STREAM LABELLING CONVENTION

STREAM A-B CONTAINS TRAFFIC GOING FROM ARM A TO ARM B
STREAM B-AC CONTAINS TRAFFIC GOING FROM ARM B TO ARM A AND TO ARM C
ETC.

.GEOMETRIC DATA

```

-----
I          DATA ITEM          I  MINOR ROAD B  I
-----
I  TOTAL MAJOR ROAD CARRIAGEWAY WIDTH  I ( W )  7.20 M.  I
I  CENTRAL RESERVE WIDTH                I (WCR )  0.00 M.  I
I
I  MAJOR ROAD RIGHT TURN - WIDTH        I (WC-B)  2.20 M.  I
I          - VISIBILITY                  I (VC-B) 200.0 M.  I
I          - BLOCKS TRAFFIC              I          NO      I
I
I  MINOR ROAD - VISIBILITY TO LEFT      I (VB-C)  50.0 M.  I
I          - VISIBILITY TO RIGHT        I (VB-A)  50.0 M.  I
I          - LANE 1 WIDTH                I (WB-C)  -        I
I          - LANE 2 WIDTH                I (WB-A)  -        I
I          - WIDTH AT 0 M FROM JUNC.    I          6.00 M.  I
I          - WIDTH AT 5 M FROM JUNC.    I          6.00 M.  I
I          - WIDTH AT 10 M FROM JUNC.   I          6.00 M.  I
I          - WIDTH AT 15 M FROM JUNC.   I          6.00 M.  I
I          - WIDTH AT 20 M FROM JUNC.   I          6.00 M.  I
I          - LENGTH OF FLARED SECTION   I          10 VEHS I
-----
    
```

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

```

-----
I Intercept For Slope For Opposing   Slope For Opposing I
I Stream B-C   Stream  A-C           Stream A-B         I
-----
I      602.92           0.22           0.09              I
-----
    
```

```

-----
I Intercept For Slope For Opposing   Slope For Opposing Slope For Opposing Slope For OpposingI
I Stream B-A   Stream  A-C           Stream A-B         Stream  C-A         Stream C-B         I
-----
I      476.98           0.21           0.09              0.13              0.30              I
-----
    
```

```

-----
I Intercept For Slope For Opposing   Slope For Opposing I
I Stream C-B   Stream  A-C           Stream A-B         I
-----
I      689.79           0.25           0.25              I
-----
    
```

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

```

-----
I ARM I FLOW SCALE (%) I
-----
I A   I      100      I
I B   I      100      I
I C   I      100      I
-----
    
```

Demand set: 2016 AM Peak - Existing Flows

Consent of copyright owner required for any other use.

TIME PERIOD BEGINS 07.15 AND ENDS 08.45

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	I	NUMBER OF MINUTES FROM START WHEN			RATE OF FLOW (VEH/MIN)		
		I	I	I	I	I	I
I	ARM	FLOW STARTS	TOP OF PEAK	FLOW STOPS	BEFORE	AT TOP	AFTER
I	I	TO RISE	IS REACHED	FALLING	PEAK	OF PEAK	PEAK
I	ARM A	15.00	45.00	75.00	0.73	1.09	0.73
I	ARM B	15.00	45.00	75.00	6.55	9.83	6.55
I	ARM C	15.00	45.00	75.00	1.08	1.61	1.08

I	I	TURNING PROPORTIONS			
		TURNING COUNTS (VEH/HR)			
I		I (PERCENTAGE OF H.V.S)			
I		I			
I	TIME	FROM/TO	ARM A	ARM B	ARM C
I	07.15 - 08.45				
I		ARM A	0.000	0.000	1.000
I			0.0	0.0	58.0
I			(0.0)	(0.0)	(14.0)
I					
I		ARM B	0.756	0.000	0.244
I			396.0	0.0	128.0
I			(10.0)	(0.0)	(33.0)
I					
I		ARM C	1.000	0.000	0.000
I			86.0	0.0	0.0
I			(60.0)	(0.0)	(0.0)
I					

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
 THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT
 FOR DEMAND SET 2016 AM Peak - Existing Flows
 AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	07.15-07.30									
I	B-C	1.61	5.84	0.275		0.00	0.37	5.2		0.23
I	B-A	4.97	8.61	0.577		0.00	1.31	17.9		0.26
I	C-A	1.08								
I	C-B	0.00	10.26	0.000		0.00	0.00	0.0		0.00
I	A-B	0.00								
I	A-C	0.73								

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	07.30-07.45									
I	B-C	1.92	5.55	0.345		0.37	0.51	7.4		0.27
I	B-A	5.93	8.52	0.696		1.31	2.13	29.3		0.37
I	C-A	1.29								
I	C-B	0.00	10.22	0.000		0.00	0.00	0.0		0.00
I	A-B	0.00								
I	A-C	0.87								

Appendix C – PICADY Results

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-C	2.35	4.70	0.500		0.51	0.95	13.1		0.42	I
I	B-A	7.27	8.40	0.865		2.13	4.88	60.4		0.68	I
I	C-A	1.58									I
I	C-B	0.00	10.17	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	1.06									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-C	2.35	4.47	0.526		0.95	1.06	15.4		0.47	I
I	B-A	7.27	8.40	0.865		4.88	5.44	78.0		0.80	I
I	C-A	1.58									I
I	C-B	0.00	10.17	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	1.06									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-C	1.92	5.47	0.351		1.06	0.56	8.9		0.29	I
I	B-A	5.93	8.52	0.696		5.44	2.47	42.7		0.45	I
I	C-A	1.29									I
I	C-B	0.00	10.22	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	0.87									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-C	1.61	5.80	0.277		0.56	0.39	6.1		0.24	I
I	B-A	4.97	8.61	0.577		2.47	1.42	23.0		0.29	I
I	C-A	1.08									I
I	C-B	0.00	10.26	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	0.73									I

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	0.4	
07.45	0.5	*
08.00	0.9	*
08.15	1.1	*
08.30	0.6	*
08.45	0.4	

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	1.3	*
07.45	2.1	**
08.00	4.9	*****
08.15	5.4	*****
08.30	2.5	**
08.45	1.4	*

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.0
07.45	0.0
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

STREAM	TOTAL DEMAND	* QUEUEING * DELAY	* INCLUSIVE QUEUEING * DELAY	
(VEH)	(VEH/H)	(MIN)	(MIN/VEH)	
B-C	176.2	117.5	56.1	0.32
B-A	545.1	363.4	251.3	0.46
C-A	118.4	78.9		
C-B	0.0	0.0	0.0	0.00
A-B	0.0	0.0		
A-C	79.8	53.2		
ALL	919.5	613.0	307.4	0.33

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

For inspection purposes only. Consent of copyright owner required for any other use.

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing A-C	Slope For Stream A-B	Opposing A-B	I
I	602.92	0.22		0.09		I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing A-C	Slope For Stream A-B	Opposing A-B	Slope For Stream C-A	Opposing C-A	Slope For Stream C-B	Opposing C-B	I
I	476.98	0.21		0.08		0.13		0.30		I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing A-C	Slope For Stream A-B	Opposing A-B	I
I	689.79	0.25		0.25		I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2017 AM Peak - No Development

TIME PERIOD BEGINS 07.15 AND ENDS 08.45

LENGTH OF TIME PERIOD - 90 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK	I
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.73	I	1.09	I	0.73	I
I	ARM B	I	15.00	I	45.00	I	75.00	I	6.60	I	9.90	I	6.60	I
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.09	I	1.63	I	1.09	I

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	07.15 - 08.45	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(14.0)	I
I		I	ARM B	I	0.756	I	0.000	I	0.244	I
I		I		I	399.0	I	0.0	I	129.0	I
I		I		I	(10.0)	I	(0.0)	I	(33.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	87.0	I	0.0	I	0.0	I
I		I		I	(60.0)	I	(0.0)	I	(0.0)	I

For inspection purposes only. Consent of copyright owner required for any other use.

Appendix C – PICADY Results

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2017 AM Peak - No Development
AND FOR TIME PERIOD 1

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.15-07.30									
B-C	1.62	5.83	0.278		0.00	0.38	5.3		0.23
B-A	5.01	8.61	0.582		0.00	1.33	18.2		0.27
C-A	1.09								
C-B	0.00	10.26	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.73								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.30-07.45									
B-C	1.93	5.54	0.349		0.38	0.52	7.5		0.28
B-A	5.98	8.52	0.702		1.33	2.19	29.9		0.38
C-A	1.30								
C-B	0.00	10.22	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.87								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.45-08.00									
B-C	2.37	4.63	0.511		0.52	0.99	13.6		0.43
B-A	7.32	8.39	0.872		2.19	5.08	62.5		0.70
C-A	1.60								
C-B	0.00	10.17	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.06								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
B-C	2.37	4.37	0.542		0.99	1.13	16.2		0.49
B-A	7.32	8.39	0.872		5.08	5.70	81.5		0.84
C-A	1.60								
C-B	0.00	10.17	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.06								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
B-C	1.93	5.45	0.355		1.13	0.57	9.1		0.29
B-A	5.98	8.52	0.702		5.70	2.55	44.3		0.46
C-A	1.30								
C-B	0.00	10.22	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.87								

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-C	1.62	5.79	0.279		0.57	0.40	6.2		0.24	I
I	B-A	5.01	8.61	0.582		2.55	1.45	23.5		0.29	I
I	C-A	1.09									I
I	C-B	0.00	10.26	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	0.73									I

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	0.4	
07.45	0.5	*
08.00	1.0	*
08.15	1.1	*
08.30	0.6	*
08.45	0.4	

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	1.3	*
07.45	2.2	**
08.00	5.1	*****
08.15	5.7	*****
08.30	2.5	***
08.45	1.4	*

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	0.0	
07.45	0.0	
08.00	0.0	
08.15	0.0	
08.30	0.0	
08.45	0.0	

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I		I
I	I	I	I	I	* DELAY *	I	* DELAY *	I		I
I	I	I	(VEH)	I	(MIN)	I	(MIN)	I	(MIN/VEH)	I
I	B-C	I	177.6	I	58.0	I	58.0	I	0.33	I
I	B-A	I	549.2	I	260.0	I	260.1	I	0.47	I
I	C-A	I	119.7	I		I		I		I
I	C-B	I	0.0	I	0.0	I	0.0	I	0.00	I
I	A-B	I	0.0	I		I		I		I
I	A-C	I	79.8	I		I		I		I
I	ALL	I	926.3	I	317.9	I	318.1	I	0.34	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	602.92		0.22		0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
I	476.98		0.21		0.08		0.13		0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	689.79		0.25		0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2017 AM Peak - With Development

TIME PERIOD BEGINS 07.15 AND ENDS 08.45

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK	I
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.80	I	1.20	I	0.80	I
I	ARM B	I	15.00	I	45.00	I	75.00	I	6.68	I	10.01	I	6.68	I
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.24	I	1.86	I	1.24	I

I	TIME	TURNING PROPORTIONS			I	
		FROM/TO	ARM A	ARM B		ARM C
I	07.15 - 08.45	I	ARM A	0.000	0.000	1.000
I			0.0	0.0	64.0	
I			(0.0)	(0.0)	(14.0)	
I						
I		ARM B	0.747	0.000	0.253	
I			399.0	0.0	135.0	
I			(10.0)	(0.0)	(33.0)	
I						
I		ARM C	1.000	0.000	0.000	
I			99.0	0.0	0.0	
I			(60.0)	(0.0)	(0.0)	
I						

Consent of copyright owner required for any other use.
 For inspection purposes only.

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2017 AM Peak - With Development
AND FOR TIME PERIOD 1

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.15-07.30									
B-C	1.69	5.84	0.290		0.00	0.40	5.6		0.24
B-A	5.01	8.51	0.588		0.00	1.37	18.6		0.27
C-A	1.24								
C-B	0.00	10.24	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.80								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.30-07.45									
B-C	2.02	5.54	0.365		0.40	0.56	8.0		0.28
B-A	5.98	8.41	0.711		1.37	2.27	31.0		0.39
C-A	1.48								
C-B	0.00	10.20	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.96								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.45-08.00									
B-C	2.48	4.52	0.548		0.56	1.14	15.4		0.47
B-A	7.32	8.27	0.885		2.27	5.45	66.2		0.75
C-A	1.82								
C-B	0.00	10.14	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.17								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
B-C	2.48	4.19	0.591		1.14	1.35	19.3		0.57
B-A	7.32	8.27	0.885		5.45	6.20	88.2		0.91
C-A	1.82								
C-B	0.00	10.14	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.17								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
B-C	2.02	5.43	0.373		1.35	0.61	9.9		0.30
B-A	5.98	8.41	0.711		6.20	2.67	47.3		0.49
C-A	1.48								
C-B	0.00	10.20	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.96								

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-C	1.69	5.80	0.292		0.61	0.42	6.6		0.25	I
I	B-A	5.01	8.51	0.588		2.67	1.49	24.2		0.30	I
I	C-A	1.24									I
I	C-B	0.00	10.24	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	0.80									I

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	0.4	
07.45	0.6	*
08.00	1.1	*
08.15	1.4	*
08.30	0.6	*
08.45	0.4	

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	1.4	*
07.45	2.3	**
08.00	5.4	*****
08.15	6.2	*****
08.30	2.7	***
08.45	1.5	*

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.0
07.45	0.0
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I
I	I	I	I	I	* DELAY *	I	* DELAY *	I
I	I	(VEH)	(VEH/H)	(MIN)	(MIN/VEH)	(MIN)	(MIN/VEH)	I
I	B-C	I 185.8	I 123.9	I 64.9	I 0.35	I 64.9	I 0.35	I
I	B-A	I 549.2	I 366.1	I 275.5	I 0.50	I 275.6	I 0.50	I
I	C-A	I 136.3	I 90.8	I	I	I	I	I
I	C-B	I 0.0	I 0.0	I 0.0	I 0.00	I 0.0	I 0.00	I
I	A-B	I 0.0	I 0.0	I	I	I	I	I
I	A-C	I 88.1	I 58.7	I	I	I	I	I
I	ALL	I 959.4	I 639.6	I 340.3	I 0.35	I 340.5	I 0.35	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing A-C	Slope For Stream A-B	Opposing A-B	I
I	602.92	0.22		0.09		I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing A-C	Slope For Stream A-B	Opposing A-B	Slope For Stream C-A	Opposing C-A	Slope For Stream C-B	Opposing C-B	I
I	476.98	0.21		0.08		0.13		0.30		I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing A-C	Slope For Stream A-B	Opposing A-B	I
I	689.79	0.25		0.25		I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2023 AM Peak - No Development

TIME PERIOD BEGINS 07.15 AND ENDS 08.45

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK	I
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.76	I	1.14	I	0.76	I
I	ARM B	I	15.00	I	45.00	I	75.00	I	6.89	I	10.33	I	6.89	I
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.13	I	1.69	I	1.13	I

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	07.15 - 08.45	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(14.0)	I
I		I	ARM B	I	0.755	I	0.000	I	0.245	I
I		I		I	416.0	I	0.0	I	135.0	I
I		I		I	(10.0)	I	(0.0)	I	(33.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	90.0	I	0.0	I	0.0	I
I		I		I	(60.0)	I	(0.0)	I	(0.0)	I

Consent of copyright owner required for any other use.
 For inspection purposes only.

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2023 AM Peak - No Development
AND FOR TIME PERIOD 1

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.15-07.30									
B-C	1.69	5.78	0.293		0.00	0.41	5.7		0.24
B-A	5.22	8.58	0.608		0.00	1.48	20.0		0.28
C-A	1.13								
C-B	0.00	10.25	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.77								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.30-07.45									
B-C	2.02	5.45	0.371		0.41	0.57	8.2		0.29
B-A	6.23	8.49	0.734		1.48	2.52	34.1		0.42
C-A	1.35								
C-B	0.00	10.21	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.91								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.45-08.00									
B-C	2.48	4.10	0.605		0.57	1.40	18.5		0.58
B-A	7.63	8.36	0.913		1.48	6.47	76.5		0.84
C-A	1.65								
C-B	0.00	10.16	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.12								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
B-C	2.48	3.53	0.702		1.40	2.04	27.5		0.87
B-A	7.63	8.36	0.913		6.47	7.63	106.8		1.08
C-A	1.65								
C-B	0.00	10.16	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.12								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
B-C	2.02	5.27	0.383		2.04	0.64	10.9		0.33
B-A	6.23	8.49	0.734		7.63	3.05	56.5		0.57
C-A	1.35								
C-B	0.00	10.21	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.91								

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-C	1.69	5.73	0.296		0.64	0.43	6.8		0.25	I
I	B-A	5.22	8.58	0.608		3.05	1.62	26.5		0.31	I
I	C-A	1.13									I
I	C-B	0.00	10.25	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	0.77									I

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	0.4	
07.45	0.6	*
08.00	1.4	*
08.15	2.0	**
08.30	0.6	*
08.45	0.4	

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	1.5	*
07.45	2.5	***
08.00	6.5	*****
08.15	7.6	*****
08.30	3.0	***
08.45	1.6	**

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.0
07.45	0.0
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I
I	I	I	I	I	* DELAY *	I	* DELAY *	I
I	I	(VEH)	(VEH/H)	I	(MIN)	(MIN/VEH)	I	(MIN)
I	I	(VEH)	(VEH/H)	I	(MIN)	(MIN/VEH)	I	(MIN)
I	B-C	I 185.8	I 123.9	I	77.5	I 0.42	I 77.6	I 0.42
I	B-A	I 572.6	I 381.7	I	320.5	I 0.56	I 320.6	I 0.56
I	C-A	I 123.9	I 82.6	I		I	I	I
I	C-B	I 0.0	I 0.0	I	0.0	I 0.00	I 0.0	I 0.00
I	A-B	I 0.0	I 0.0	I		I	I	I
I	A-C	I 84.0	I 56.0	I		I	I	I
I	ALL	I 966.3	I 644.2	I	398.0	I 0.41	I 398.2	I 0.41

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	602.92		0.22		0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
I	476.98		0.21		0.08		0.13		0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
I	689.79		0.25		0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: 2023 AM Peak - With Development

TIME PERIOD BEGINS 07.15 AND ENDS 08.45

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK	I
I	ARM A	I	15.00	I	45.00	I	75.00	I	0.84	I	1.26	I	0.84	I
I	ARM B	I	15.00	I	45.00	I	75.00	I	6.96	I	10.44	I	6.96	I
I	ARM C	I	15.00	I	45.00	I	75.00	I	1.27	I	1.91	I	1.27	I

I	TIME	I	FROM/TO	I	ARM A	I	ARM B	I	ARM C	I
I	07.15 - 08.45	I	ARM A	I	0.000	I	0.000	I	1.000	I
I		I		I	(0.0)	I	(0.0)	I	(14.0)	I
I		I	ARM B	I	0.747	I	0.000	I	0.253	I
I		I		I	416.0	I	0.0	I	141.0	I
I		I		I	(10.0)	I	(0.0)	I	(33.0)	I
I		I	ARM C	I	1.000	I	0.000	I	0.000	I
I		I		I	102.0	I	0.0	I	0.0	I
I		I		I	(60.0)	I	(0.0)	I	(0.0)	I

Consent of copyright owner required for any other use.
 For inspection purposes only.

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2023 AM Peak - With Development
AND FOR TIME PERIOD 1

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.15-07.30									
B-C	1.77	5.79	0.306		0.00	0.43	6.0		0.25
B-A	5.22	8.49	0.615		0.00	1.52	20.5		0.29
C-A	1.28								
C-B	0.00	10.23	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.84								
07.30-07.45									
B-C	2.11	5.44	0.388		0.43	0.62	8.8		0.30
B-A	6.23	8.39	0.743		1.52	2.62	35.3		0.43
C-A	1.53								
C-B	0.00	10.19	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.00								
07.45-08.00									
B-C	2.59	3.92	0.660		0.62	1.72	22.2		0.68
B-A	7.63	8.24	0.926		2.62	6.99	81.4		0.90
C-A	1.87								
C-B	0.00	10.13	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.23								
08.00-08.15									
B-C	2.59	3.19	0.811		1.72	3.11	39.3		1.28
B-A	7.63	8.24	0.926		6.99	8.42	116.8		1.18
C-A	1.87								
C-B	0.00	10.13	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.23								
08.15-08.30									
B-C	2.11	5.23	0.404		3.11	0.70	12.8		0.36
B-A	6.23	8.39	0.743		8.42	3.22	61.6		0.63
C-A	1.53								
C-B	0.00	10.19	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.00								

Consent of copyright owner required for any other use.

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-C	1.77	5.74	0.308		0.70	0.46	7.2		0.25	I
I	B-A	5.22	8.49	0.615		3.22	1.67	27.4		0.33	I
I	C-A	1.28									I
I	C-B	0.00	10.23	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	0.84									I

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	0.4	
07.45	0.6	*
08.00	1.7	**
08.15	3.1	***
08.30	0.7	*
08.45	0.5	

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
07.30	1.5	**
07.45	2.6	***
08.00	7.0	*****
08.15	8.4	*****
08.30	3.2	***
08.45	1.7	**

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.0
07.45	0.0
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I		I
I	I	I	I	I	* DELAY *	I	* DELAY *	I		I
I	I	I	(VEH)	I	(MIN)	I	(MIN)	I	(MIN/VEH)	I
I	I	I	(VEH/H)	I	(MIN/VEH)	I	(MIN/VEH)	I		I
I	B-C	I	194.1	I	129.4	I	96.3	I	0.50	I
I	B-A	I	572.6	I	381.7	I	343.1	I	0.60	I
I	C-A	I	140.4	I	93.6	I		I		I
I	C-B	I	0.0	I	0.0	I	0.0	I	0.00	I
I	A-B	I	0.0	I	0.0	I		I		I
I	A-C	I	92.2	I	61.5	I		I		I
I	ALL	I	999.3	I	666.2	I	439.3	I	0.44	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
602.92	0.22		0.09		I

I Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
476.98	0.21		0.08		0.13		0.30		I

I Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
689.79	0.25		0.25		I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I ARM	I FLOW SCALE (%)	I
I A	I 100	I
I B	I 100	I
I C	I 100	I

Demand set: 2016 PM Peak - Existing Flows

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I ARM	I NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I TOP OF PEAK IS REACHED	I FLOW STOPS FALLING	I RATE OF FLOW (VEH/MIN) BEFORE PEAK	I AT TOP OF PEAK	I AFTER PEAK
I ARM A	I 15.00	I 45.00	I 75.00	I 0.25	I 0.38	I 0.25
I ARM B	I 15.00	I 45.00	I 75.00	I 4.10	I 6.15	I 4.10
I ARM C	I 15.00	I 45.00	I 75.00	I 1.45	I 2.18	I 1.45

I TIME	I FROM/TO	I TURNING PROPORTIONS		
		I ARM A	I ARM B	I ARM C
I 16.30 - 18.00	I ARM A	I 0.000	I 0.000	I 1.000
		I (0.0)	I (0.0)	I (35.0)
		I 308.0	I 0.0	I 20.0
	I ARM B	I 0.939	I 0.000	I 0.061
		I (19.0)	I (0.0)	I (50.0)
		I 116.0	I 0.0	I 0.0
I ARM C	I 1.000	I 0.000	I 0.000	
	I (15.0)	I (0.0)	I (0.0)	
	I 116.0	I 0.0	I 0.0	

For inspection purposes only. Consent of copyright owner required for any other use.

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2016 PM Peak - Existing Flows
AND FOR TIME PERIOD 2

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)

I 16.30-16.45									
I B-C	0.25	5.12	0.049		0.00	0.05	0.7		0.21
I B-A	3.86	8.40	0.460		0.00	0.83	11.6		0.22
I C-A	1.46								
I C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
I A-B	0.00								
I A-C	0.25								

I 16.45-17.00									
I B-C	0.30	4.96	0.060		0.05	0.06	0.9		0.21
I B-A	4.61	8.34	0.554		0.83	1.20	17.0		0.27
I C-A	1.74								
I C-B	0.00	10.36	0.000		0.00	0.00	0.0		0.00
I A-B	0.00								
I A-C	0.30								

I 17.00-17.15									
I B-C	0.37	4.71	0.078		0.00	0.08	1.2		0.23
I B-A	5.65	8.25	0.685		1.20	2.03	27.8		0.37
I C-A	2.13								
I C-B	0.00	10.34	0.000		0.00	0.00	0.0		0.00
I A-B	0.00								
I A-C	0.37								

I 17.15-17.30									
I B-C	0.37	4.69	0.078		0.08	0.08	1.3		0.23
I B-A	5.65	8.25	0.685		2.03	2.09	31.0		0.38
I C-A	2.13								
I C-B	0.00	10.34	0.000		0.00	0.00	0.0		0.00
I A-B	0.00								
I A-C	0.37								

I 17.30-17.45									
I B-C	0.30	4.94	0.061		0.08	0.07	1.0		0.22
I B-A	4.61	8.34	0.554		2.09	1.29	20.7		0.28
I C-A	1.74								
I C-B	0.00	10.36	0.000		0.00	0.00	0.0		0.00
I A-B	0.00								
I A-C	0.30								

Consent of copyright holder required for any other use.

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.45-18.00										I
I	B-C	0.25	5.10	0.049		0.07	0.05	0.8		0.21	I
I	B-A	3.86	8.40	0.460		1.29	0.87	13.8		0.22	I
I	C-A	1.46									I
I	C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	0.25									I

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1
17.45	0.1
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	0.8	*
17.00	1.2	*
17.15	2.0	**
17.30	2.1	**
17.45	1.3	*
18.00	0.9	*

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I	* DELAY *	I
I	I	I	I	I	I	I	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I
I	I	(VEH)	(VEH/H)	I	(MIN)	(MIN/VEH)	I	(MIN)	(MIN/VEH)	I
I	B-C	I 27.5	I 18.4	I	5.9	I 0.22	I	5.9	I 0.22	I
I	B-A	I 423.9	I 282.6	I	121.8	I 0.29	I	121.8	I 0.29	I
I	C-A	I 159.7	I 106.4	I		I	I		I	I
I	C-B	I 0.0	I 0.0	I	0.0	I 0.00	I	0.0	I 0.00	I
I	A-B	I 0.0	I 0.0	I		I	I		I	I
I	A-C	I 27.5	I 18.4	I		I	I		I	I
I	ALL	I 638.7	I 425.8	I	127.7	I 0.20	I	127.8	I 0.20	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I Intercept For Stream B-C	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B				
602.92	0.22	0.09				

I Intercept For Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	
476.98	0.21	0.08	0.13	0.30	I

I Intercept For Stream C-B	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B				
689.79	0.25	0.25				

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I ARM	I FLOW SCALE (%)	I
I A	I 100	I
I B	I 100	I
I C	I 100	I

Demand set: 2017 PM Peak - No Development

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I ARM	I NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I TOP OF PEAK IS REACHED	I FLOW STOPS FALLING	I RATE OF FLOW (VEH/MIN) BEFORE PEAK	I AT TOP OF PEAK	I AFTER PEAK
I ARM A	I 15.00	I 45.00	I 75.00	I 0.25	I 0.38	I 0.25
I ARM B	I 15.00	I 45.00	I 75.00	I 4.13	I 6.19	I 4.13
I ARM C	I 15.00	I 45.00	I 75.00	I 1.46	I 2.19	I 1.46

I TIME	I FROM/TO	I TURNING PROPORTIONS		
		I ARM A	I ARM B	I ARM C
I 16.30 - 18.00	I ARM A	I 0.000	I 0.000	I 1.000
		I (0.0)	I (0.0)	I (35.0)
		I 310.0	I 0.0	I 20.0
	I ARM B	I 0.939	I 0.000	I 0.061
		I (19.0)	I (0.0)	I (50.0)
		I 117.0	I 0.0	I 0.0
	I ARM C	I 1.000	I 0.000	I 0.000
		I (15.0)	I (0.0)	I (0.0)
		I	I	I

For inspection purposes only. Consent of copyright owner required for any other use.

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2017 PM Peak - No Development
AND FOR TIME PERIOD 2

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)

16.30-16.45									
B-C	0.25	5.11	0.049		0.00	0.05	0.7		0.21
B-A	3.89	8.40	0.463		0.00	0.84	11.7		0.22
C-A	1.47								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.25								

16.45-17.00									
B-C	0.30	4.95	0.060		0.05	0.06	0.9		0.21
B-A	4.64	8.33	0.557		0.84	1.21	17.2		0.27
C-A	1.75								
C-B	0.00	10.36	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.30								

17.00-17.15									
B-C	0.37	4.70	0.078		0.00	0.08	1.2		0.23
B-A	5.69	8.25	0.690		1.21	2.07	28.3		0.37
C-A	2.15								
C-B	0.00	10.34	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.37								

17.15-17.30									
B-C	0.37	4.68	0.078		0.08	0.08	1.3		0.23
B-A	5.69	8.25	0.690		2.07	2.14	31.6		0.39
C-A	2.15								
C-B	0.00	10.34	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.37								

17.30-17.45									
B-C	0.30	4.94	0.061		0.08	0.07	1.0		0.22
B-A	4.64	8.33	0.557		2.14	1.31	21.0		0.28
C-A	1.75								
C-B	0.00	10.36	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.30								

Consent of copyright owner required for any other use.

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.45-18.00										I
I	B-C	0.25	5.10	0.049		0.07	0.05	0.8		0.21	I
I	B-A	3.89	8.40	0.463		1.31	0.89	14.0		0.22	I
I	C-A	1.47									I
I	C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	0.25									I

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1
17.45	0.1
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	0.8	*
17.00	1.2	*
17.15	2.1	**
17.30	2.1	**
17.45	1.3	*
18.00	0.9	*

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I	I
I	I	I	I	I	* DELAY *	I	* DELAY *	I	I
I	I	I	(VEH)	I	(MIN)	I	(MIN)	I	I
I	I	I	(VEH/H)	I	(MIN/VEH)	I	(MIN/VEH)	I	I
I	B-C	I	27.5	I	6.0	I	6.0	I	0.22
I	B-A	I	426.7	I	123.9	I	123.9	I	0.29
I	C-A	I	161.0	I		I		I	
I	C-B	I	0.0	I	0.0	I	0.0	I	0.00
I	A-B	I	0.0	I		I		I	
I	A-C	I	27.5	I		I		I	
I	ALL	I	642.8	I	129.8	I	129.9	I	0.20

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
602.92	0.22		0.09		I

I Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
476.98	0.21		0.08		0.13		0.30		I

I Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
689.79	0.25		0.25		I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I ARM	I FLOW SCALE (%)	I
I A	I 100	I
I B	I 100	I
I C	I 100	I

Demand set: 2017 PM Peak - With Development

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I ARM	I NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I TOP OF PEAK IS REACHED	I FLOW STOPS FALLING	I RATE OF FLOW (VEH/MIN) BEFORE PEAK	I AT TOP OF PEAK	I AFTER PEAK
I ARM A	I 15.00	I 45.00	I 75.00	I 0.32	I 0.49	I 0.32
I ARM B	I 15.00	I 45.00	I 75.00	I 4.20	I 6.30	I 4.20
I ARM C	I 15.00	I 45.00	I 75.00	I 1.61	I 2.42	I 1.61

I TIME	I TURNING PROPORTIONS		
	I FROM/TO	I ARM A	I ARM B
I 16.30 - 18.00	I ARM A	I 0.000	I 0.000
	I	I 0.0	I 0.0
	I	I (0.0)	I (0.0)
	I	I	I
	I ARM B	I 0.923	I 0.000
	I	I 310.0	I 0.0
	I	I (19.0)	I (0.0)
	I	I	I
	I ARM C	I 1.000	I 0.000
	I	I 129.0	I 0.0
	I	I (15.0)	I (0.0)
	I	I	I

For inspection purposes only. Consent of copyright owner required for any other use.

Appendix C – PICADY Results

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2017 PM Peak - With Development
AND FOR TIME PERIOD 2

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	0.33	5.10	0.064		0.00	0.07	1.0		0.21
B-A	3.89	8.35	0.466		0.00	0.85	11.8		0.22
C-A	1.62								
C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.33								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	0.39	4.93	0.079		0.07	0.08	1.2		0.22
B-A	4.64	8.28	0.561		0.85	1.23	17.4		0.27
C-A	1.93								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.39								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	0.48	4.67	0.102		0.08	0.11	1.6		0.24
B-A	5.69	8.18	0.696		1.23	2.12	28.9		0.38
C-A	2.37								
C-B	0.00	10.30	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.48								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	0.48	4.65	0.103		0.11	0.11	1.7		0.24
B-A	5.69	8.18	0.696		2.12	2.19	32.5		0.40
C-A	2.37								
C-B	0.00	10.30	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.48								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	0.39	4.92	0.079		0.11	0.09	1.4		0.22
B-A	4.64	8.28	0.561		2.19	1.33	21.4		0.28
C-A	1.93								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.39								

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.45-18.00										I
I	B-C	0.33	5.08	0.064		0.09	0.07	1.1		0.21	I
I	B-A	3.89	8.35	0.466		1.33	0.90	14.2		0.23	I
I	C-A	1.62									I
I	C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	0.33									I

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1
17.45	0.1
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	0.8	*
17.00	1.2	*
17.15	2.1	**
17.30	2.2	**
17.45	1.3	*
18.00	0.9	*

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND (VEH)	I	23.9	I	7.9	I	0.22	I	7.9	I	0.22	I
I		I	(VEH/H)	I		I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-C	I	35.8	I	23.9	I	7.9	I	0.22	I	7.9	I	0.22	I
I	B-A	I	426.7	I	284.5	I	126.2	I	0.30	I	126.3	I	0.30	I
I	C-A	I	177.6	I	118.4	I		I		I		I		I
I	C-B	I	0.0	I	0.0	I	0.0	I	0.00	I	0.0	I	0.00	I
I	A-B	I	0.0	I	0.0	I		I		I		I		I
I	A-C	I	35.8	I	23.9	I		I		I		I		I
I	ALL	I	675.8	I	450.6	I	134.2	I	0.20	I	134.2	I	0.20	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I Intercept For Stream B-C	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
602.92	0.22	0.09	I

I Intercept For Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
476.98	0.21	0.08	0.13	0.30	I

I Intercept For Stream C-B	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
689.79	0.25	0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I ARM	I FLOW SCALE (%)	I
I A	100	I
I B	100	I
I C	100	I

Demand set: 2023 PM Peak - No Development

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I ARM	I NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I TOP OF PEAK IS REACHED	I FLOW STOPS FALLING	I RATE OF FLOW (VEH/MIN) BEFORE PEAK	I AT TOP OF PEAK	I AFTER PEAK
I ARM A	15.00	45.00	75.00	0.26	0.39	0.26
I ARM B	15.00	45.00	75.00	4.31	6.47	4.31
I ARM C	15.00	45.00	75.00	1.52	2.29	1.52

I TIME	I TURNING PROPORTIONS		
	I FROM/TO	I ARM A	I ARM B
I 16.30 - 18.00	I ARM A	I 0.000	I 0.000
	I	I (0.0)	I (0.0)
	I ARM B	I 0.939	I 0.000
	I	I 324.0	I 0.0
	I	I (19.0)	I (0.0)
	I ARM C	I 1.000	I 0.000
I	I 122.0	I 0.0	
I	I (15.0)	I (0.0)	

For inspection purposes only. Consent of copyright owner required for any other use.

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

 QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2023 PM Peak - No Development
 AND FOR TIME PERIOD 2

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	0.26	5.08	0.052		0.00	0.05	0.8		0.21
B-A	4.07	8.38	0.485		0.00	0.91	12.7		0.23
C-A	1.53								
C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.26								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	0.31	4.91	0.064		0.05	0.07	1.0		0.22
B-A	4.85	8.32	0.584		0.91	1.35	19.0		0.28
C-A	1.83								
C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.31								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	0.39	4.60	0.084		0.07	0.09	1.3		0.24
B-A	5.95	8.23	0.723		2.35	2.39	32.2		0.41
C-A	2.24								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.39								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	0.39	4.57	0.084		0.09	0.09	1.4		0.24
B-A	5.95	8.23	0.723		2.39	2.48	36.7		0.43
C-A	2.24								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.39								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	0.31	4.89	0.064		0.09	0.07	1.1		0.22
B-A	4.85	8.32	0.584		2.48	1.46	23.7		0.30
C-A	1.83								
C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.31								

Consent of copyright owner required for any other use.
 For inspection purposes only.

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.45-18.00										I
I	B-C	0.26	5.06	0.052		0.07	0.06	0.9		0.21	I
I	B-A	4.07	8.38	0.485		1.46	0.97	15.4		0.24	I
I	C-A	1.53									I
I	C-B	0.00	10.37	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	0.26									I

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1
17.45	0.1
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	0.9	*
17.00	1.3	*
17.15	2.4	**
17.30	2.5	**
17.45	1.5	*
18.00	1.0	*

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I	* DELAY *	I
I	I	I	I	I	I	I	I	I	I	I
I	I	I	I	I	I	I	I	I	I	I
I	I	(VEH)	(VEH/H)	I	(MIN)	(MIN/VEH)	I	(MIN)	(MIN/VEH)	I
I	B-C	I 28.9	I 19.3	I	6.4	I 0.22	I	6.4	I 0.22	I
I	B-A	I 446.0	I 297.3	I	139.6	I 0.31	I	139.7	I 0.31	I
I	C-A	I 167.9	I 111.9	I		I	I		I	I
I	C-B	I 0.0	I 0.0	I	0.0	I 0.00	I	0.0	I 0.00	I
I	A-B	I 0.0	I 0.0	I		I	I		I	I
I	A-C	I 28.9	I 19.3	I		I	I		I	I
I	ALL	I 671.7	I 447.8	I	146.0	I 0.22	I	146.0	I 0.22	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I Intercept For Stream B-C	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
602.92	0.22	0.09	I

I Intercept For Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
476.98	0.21	0.08	0.13	0.30	I

I Intercept For Stream C-B	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
689.79	0.25	0.25	I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I ARM	I FLOW SCALE (%)	I
A	100	I
B	100	I
C	100	I

Demand set: 2023 PM Peak - With Development

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I ARM	I NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I TOP OF PEAK IS REACHED	I FLOW STOPS FALLING	I RATE OF FLOW (VEH/MIN) BEFORE PEAK	I AT TOP OF PEAK	I AFTER PEAK
ARM A	15.00	45.00	75.00	0.34	0.51	0.34
ARM B	15.00	45.00	75.00	4.39	6.58	4.39
ARM C	15.00	45.00	75.00	1.67	2.51	1.67

I TIME	I TURNING PROPORTIONS		
	I FROM/TO	I ARM A	I ARM B
16.30 - 18.00	ARM A	0.000	0.000
		(0.0)	(0.0)
	ARM B	0.923	0.000
		(19.0)	(0.0)
	ARM C	1.000	0.000
		(15.0)	(0.0)

For inspection purposes only. Consent of copyright owner required for any other use.

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

 QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2023 PM Peak - With Development
 AND FOR TIME PERIOD 2

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	0.34	5.06	0.067		0.00	0.07	1.0		0.21
B-A	4.07	8.33	0.488		0.00	0.92	12.9		0.23
C-A	1.68								
C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.34								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	0.40	4.89	0.083		0.07	0.09	1.3		0.22
B-A	4.85	8.26	0.588		0.92	1.37	19.2		0.29
C-A	2.01								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.40								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	0.50	4.57	0.108		0.00	0.12	1.7		0.25
B-A	5.95	8.16	0.729		1.07	2.45	33.0		0.42
C-A	2.46								
C-B	0.00	10.30	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.50								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	0.50	4.54	0.109		0.12	0.12	1.8		0.25
B-A	5.95	8.16	0.729		2.45	2.56	37.7		0.45
C-A	2.46								
C-B	0.00	10.30	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.50								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	0.40	4.87	0.083		0.12	0.09	1.4		0.22
B-A	4.85	8.26	0.588		2.56	1.49	24.1		0.31
C-A	2.01								
C-B	0.00	10.33	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.40								

Consent of copyright holder required for any other use.

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.45-18.00										I
I	B-C	0.34	5.04	0.067		0.09	0.07	1.1		0.21	I
I	B-A	4.07	8.33	0.488		1.49	0.98	15.5		0.24	I
I	C-A	1.68									I
I	C-B	0.00	10.35	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	0.34									I

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1
17.45	0.1
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	0.9	*
17.00	1.4	*
17.15	2.5	**
17.30	2.6	***
17.45	1.5	*
18.00	1.0	*

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING * * DELAY *	I	* INCLUSIVE QUEUEING * * DELAY *	I	I	
I	I	(VEH)	(VEH/H)	I	(MIN)	(MIN/VEH)	I	(MIN)	(MIN/VEH)	I
I	B-C	I	37.2	I	24.8	I	8.4	I	0.23	I
I	B-A	I	446.0	I	297.3	I	142.5	I	0.32	I
I	C-A	I	184.4	I	123.0	I		I		I
I	C-B	I	0.0	I	0.0	I	0.0	I	0.00	I
I	A-B	I	0.0	I	0.0	I		I		I
I	A-C	I	37.2	I	24.8	I		I		I
I	ALL	I	704.7	I	469.8	I	150.9	I	0.21	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
602.92	0.22		0.09		I

I Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	Slope For Stream C-A	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
476.98	0.21		0.08		0.13		0.30		I

I Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
689.79	0.25		0.25		I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I ARM	I FLOW SCALE (%)	I
I A	I 100	I
I B	I 100	I
I C	I 100	I

Demand set: 2023 AM Peak - Development Flows + Sensitivity Flows

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I ARM	I NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I TOP OF PEAK IS REACHED	I RATE OF FLOW (VEH/MIN) BEFORE PEAK	I AT TOP OF PEAK	I AFTER PEAK
I ARM A	I 15.00	I 45.00	I 1.04	I 1.56	I 1.04
I ARM B	I 15.00	I 45.00	I 7.16	I 10.74	I 7.16
I ARM C	I 15.00	I 45.00	I 1.67	I 2.51	I 1.67

I TIME	I FROM/TO	I TURNING PROPORTIONS		
		I ARM A	I ARM B	I ARM C
I 16.30 - 18.00	I ARM A	I 0.000	I 0.000	I 1.000
		I (0.0)	I (0.0)	I (14.0)
		I 0.0	I 0.0	I 83.0
	I ARM B	I 0.726	I 0.000	I 0.274
		I 416.0	I 0.0	I 157.0
		I (10.0)	I (0.0)	I (33.0)
I ARM C	I 1.000	I 0.000	I 0.000	
	I 134.0	I 0.0	I 0.0	
	I (60.0)	I (0.0)	I (0.0)	

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2023 AM Peak - Development Flows + Sensitivity Flows
AND FOR TIME PERIOD 435

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	1.97	5.81	0.339		0.00	0.50	7.0		0.26
B-A	5.22	8.25	0.633		0.00	1.63	21.8		0.31
C-A	1.68								
C-B	0.00	10.18	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.04								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	2.35	5.41	0.435		0.50	0.74	10.5		0.32
B-A	6.23	8.12	0.768		1.63	2.93	39.0		0.49
C-A	2.01								
C-B	0.00	10.12	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.24								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	2.88	3.28	0.878		0.74	3.99	44.1		1.33
B-A	7.63	7.93	0.962		2.93	8.69	96.8		1.08
C-A	2.46								
C-B	0.00	10.05	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.52								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	2.88	3.04	0.948		3.99	6.25	78.7		2.24
B-A	7.63	7.93	0.963		8.69	11.21	150.8		1.55
C-A	2.46								
C-B	0.00	10.05	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.52								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	2.35	5.04	0.467		6.25	0.92	21.9		0.49
B-A	6.23	8.12	0.768		11.21	3.81	82.1		0.85
C-A	2.01								
C-B	0.00	10.12	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	1.24								

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.45-18.00										I
I	B-C	1.97	5.75	0.343		0.92	0.54	8.5		0.27	I
I	B-A	5.22	8.25	0.633		3.81	1.82	30.4		0.36	I
I	C-A	1.68									I
I	C-B	0.00	10.18	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	1.04									I

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	0.5	*
17.00	0.7	*
17.15	4.0	****
17.30	6.2	*****
17.45	0.9	*
18.00	0.5	*

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	1.6	**
17.00	2.9	***
17.15	8.7	*****
17.30	11.2	*****
17.45	3.8	****
18.00	1.8	**

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I		I
I	I	I	I	I	* DELAY *	I	* DELAY *	I		I
I	I	(VEH)	(VEH/H)	I	(MIN)	(MIN/VEH)	I	(MIN)	(MIN/VEH)	I
I	B-C	I 216.1	I 144.1	I	170.7	I 0.79	I	170.7	I 0.79	I
I	B-A	I 572.6	I 381.7	I	420.8	I 0.73	I	421.0	I 0.74	I
I	C-A	I 184.4	I 123.0	I		I	I	I	I	I
I	C-B	I 0.0	I 0.0	I	0.0	I 0.00	I	0.0	I 0.00	I
I	A-B	I 0.0	I 0.0	I		I	I	I	I	I
I	A-C	I 114.2	I 76.2	I		I	I	I	I	I
I	ALL	I 1087.4	I 724.9	I	591.5	I 0.54	I	591.8	I 0.54	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I Intercept For Stream B-C	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
602.92	0.22		0.09		I

I Intercept For Stream B-A	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream C-A	Slope For Stream C-B	Opposing Stream C-B	I
476.98	0.21		0.08	0.13	0.30		I

I Intercept For Stream C-B	Slope For Stream A-C	Opposing Stream A-C	Slope For Stream A-B	Opposing Stream A-B	I
689.79	0.25		0.25		I

NB These values do not allow for any site specific corrections

.TRAFFIC DEMAND DATA

I ARM	I FLOW SCALE (%)	I
A	100	I
B	100	I
C	100	I

Demand set: 2023 PM Peak - Development Flows + Sensitivity Flows

TIME PERIOD BEGINS 16.30 AND ENDS 18.00

LENGTH OF TIME PERIOD - 90 MINUTES.
 LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I ARM	I NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	I TOP OF PEAK IS REACHED	I RATE OF FLOW (VEH/MIN) BEFORE PEAK	I AT TOP OF PEAK	I AFTER PEAK
ARM A	15.00	45.00	0.54	0.81	0.54
ARM B	15.00	45.00	4.59	6.88	4.59
ARM C	15.00	45.00	2.08	3.11	2.08

I TIME	I TURNING PROPORTIONS		
	I FROM/TO	I ARM A	I ARM B
16.30 - 18.00	ARM A	0.000	0.000
		(0.0)	(0.0)
	ARM B	0.883	0.000
		(19.0)	(0.0)
	ARM C	1.000	0.000
		(15.0)	(0.0)

For inspection purposes only. Consent of copyright owner required for any other use.

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

 QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2023 PM Peak - Development Flows + Sensitivity Flows
 AND FOR TIME PERIOD 990

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
B-C	0.54	5.02	0.107		0.00	0.12	1.7		0.22
B-A	4.07	8.21	0.495		0.00	0.95	13.2		0.23
C-A	2.08								
C-B	0.00	10.28	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.54								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
B-C	0.64	4.84	0.133		0.12	0.15	2.2		0.24
B-A	4.85	8.11	0.599		0.95	1.43	20.0		0.30
C-A	2.49								
C-B	0.00	10.25	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.64								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
B-C	0.79	4.48	0.176		0.15	0.21	3.0		0.27
B-A	5.95	7.97	0.746		1.43	2.64	35.3		0.46
C-A	3.05								
C-B	0.00	10.21	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.79								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
B-C	0.79	4.45	0.177		0.21	0.21	3.2		0.27
B-A	5.95	7.97	0.746		2.64	2.77	40.8		0.49
C-A	3.05								
C-B	0.00	10.21	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.79								

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
B-C	0.64	4.81	0.134		0.21	0.16	2.4		0.24
B-A	4.85	8.11	0.599		2.77	1.56	25.5		0.32
C-A	2.49								
C-B	0.00	10.25	0.000		0.00	0.00	0.0		0.00
A-B	0.00								
A-C	0.64								

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.45-18.00										I
I	B-C	0.54	5.00	0.108		0.16	0.12	1.9		0.22	I
I	B-A	4.07	8.21	0.495		1.56	1.01	16.1		0.25	I
I	C-A	2.08									I
I	C-B	0.00	10.28	0.000		0.00	0.00	0.0		0.00	I
I	A-B	0.00									I
I	A-C	0.54									I

QUEUE FOR STREAM B-C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.2
17.15	0.2
17.30	0.2
17.45	0.2
18.00	0.1

QUEUE FOR STREAM B-A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
16.45	1.0	*
17.00	1.4	*
17.15	2.6	***
17.30	2.8	***
17.45	1.6	**
18.00	1.0	*

QUEUE FOR STREAM C-B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0
17.45	0.0
18.00	0.0

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I	
I	I	I	I	I	* DELAY *	I	* DELAY *	I	
I	I	(VEH)	(VEH/H)	(MIN)	(MIN/VEH)	(MIN)	(MIN/VEH)	I	
I	B-C	I	59.2	I	39.5	I	14.4	I	0.24
I	B-A	I	446.0	I	297.3	I	150.8	I	0.34
I	C-A	I	228.5	I	152.3	I		I	
I	C-B	I	0.0	I	0.0	I	0.0	I	0.00
I	A-B	I	0.0	I	0.0	I		I	
I	A-C	I	59.2	I	39.5	I		I	
I	ALL	I	792.8	I	528.5	I	165.2	I	0.21

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

APPENDIX 13-D ARCADY RESULTS

*For inspection purposes only.
Consent of copyright owner required for any other use.*

ARCADY 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

(c) Copyright TRL Limited, 2004

Adapted from ARCADY/3 which is Crown Copyright
by permission of the controller of HMSO

For sales and distribution information,
program advice and maintenance, contact:

TRL Limited Tel: +44 (0) 1344 770018
Crowthorne House Fax: +44 (0) 1344 770864
Nine Mile Ride Email: softwarebureau@trl.co.uk
Wokingham, Berks. Web: www.trlsoftware.co.uk
RG40 3GA, UK

THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"C:\ARCADY\2016\16047-02\R135 - N2 Roundabout\AM Peak\2016 AM Peak - Existing Flows.vai"
(drive-on-the-left) at 10:23:23 on Wednesday, 20 July 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
For inspection purposes only.

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
.LENGTH OF TIME PERIOD - 90 MINUTES.
.LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2016 AM Peak - Existing Flows

ARM	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	TOP OF PEAK IS REACHED	MINUTES FROM START WHEN FLOW STOPS IF FALLING	RATE OF FLOW (VEH/MIN) BEFORE PEAK	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	RATE OF FLOW (VEH/MIN) AFTER PEAK
ARM A	15.00	45.00	75.00	3.86	5.79	3.86
ARM B	15.00	45.00	75.00	1.70	2.55	1.70
ARM C	15.00	45.00	75.00	3.11	4.67	3.11

DEMAND SET TITLE: 2016 AM Peak - Existing Flows

TIME	TURNING PROPORTIONS			TURNING COUNTS (VEH/HR)			(PERCENTAGE OF H.V.S)			
	FROM/TO	ARM A	ARM B	ARM C	ARM A	ARM B	ARM C	ARM A	ARM B	ARM C
07.15 - 08.45	ARM A	0.000	0.550	0.450	0.0	170.0	139.0	(0.0)	(7.0)	(16.0)
	ARM B	0.353	0.000	0.647	48.0	0.0	88.0	(15.0)	(0.0)	(24.0)
	ARM C	0.253	0.747	0.000	63.0	186.0	0.0	(2.0)	(10.0)	(0.0)

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.15-07.30									
ARM A	3.86	26.28	0.147		0.0	0.2	2.5		0.04
ARM B	1.70	26.95	0.063		0.0	0.1	1.0		0.04
ARM C	3.11	25.66	0.121		0.0	0.1	2.0		0.04

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.30-07.45									
ARM A	4.61	26.03	0.177		0.2	0.2	3.2		0.05
ARM B	2.03	26.76	0.076		0.1	0.1	1.2		0.04
ARM C	3.72	25.59	0.145		0.1	0.2	2.5		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.45-08.00									
ARM A	5.65	25.68	0.220		0.2	0.3	4.2		0.05
ARM B	2.49	26.49	0.094		0.1	0.1	1.5		0.04
ARM C	4.55	25.50	0.179		0.2	0.2	3.2		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
ARM A	5.65	25.68	0.220		0.3	0.3	4.2		0.05
ARM B	2.49	26.49	0.094		0.1	0.1	1.6		0.04
ARM C	4.55	25.50	0.179		0.2	0.2	3.3		0.05

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
ARM A	4.61	26.03	0.177		0.3	0.2	3.3		0.05
ARM B	2.03	26.76	0.076		0.1	0.1	1.2		0.04
ARM C	3.72	25.59	0.145		0.2	0.2	2.6		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
ARM A	3.86	26.28	0.147		0.2	0.2	2.6		0.04
ARM B	1.70	26.95	0.063		0.1	0.1	1.0		0.04
ARM C	3.11	25.66	0.121		0.2	0.1	2.1		0.04

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.3
08.15	0.3
08.30	0.2
08.45	0.2

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.1

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

ARM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * * DELAY * (MIN)	* INCLUSIVE QUEUEING * * DELAY * (MIN)
A	423.7	282.5	20.0	20.0
B	186.5	124.3	7.6	7.6
C	341.4	227.6	15.7	15.7
ALL	951.6	634.4	43.3	43.3

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

(c) Copyright TRL Limited, 2004

Adapted from ARCADY/3 which is Crown Copyright
by permission of the controller of HMSO

For sales and distribution information,
program advice and maintenance, contact:

TRL Limited Tel: +44 (0) 1344 770018
Crowthorne House Fax: +44 (0) 1344 770864
Nine Mile Ride Email: softwarebureau@trl.co.uk
Wokingham, Berks. Web: www.trlsoftware.co.uk
RG40 3GA, UK

THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"C:\ARCADY\2016\16047-02\R135 - N2 Roundabout\AM Peak\2017 AM Peak - No Development.vai"
(drive-on-the-left) at 10:09:18 on Wednesday, 20 July 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
For inspection purposes only.

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
.LENGTH OF TIME PERIOD - 90 MINUTES.
.LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2017 AM Peak - No Development

I	I	NUMBER OF MINUTES FROM START WHEN			RATE OF FLOW (VEH/MIN)		
		I FLOW STARTS	I TOP OF PEAK	I FLOW STOPS	I BEFORE	I AT TOP	I AFTER
I	I	I TO RISE	I IS REACHED	I IF FALLING	I PEAK	I OF PEAK	I PEAK
I	ARM A	I 15.00	I 45.00	I 75.00	I 3.89	I 5.83	I 3.89
I	ARM B	I 15.00	I 45.00	I 75.00	I 1.71	I 2.57	I 1.71
I	ARM C	I 15.00	I 45.00	I 75.00	I 3.13	I 4.69	I 3.13

DEMAND SET TITLE: 2017 AM Peak - No Development

I	I	TURNING PROPORTIONS			
		TURNING COUNTS (VEH/HR)			
I		I (PERCENTAGE OF H.V.S)			
I		I			
I	TIME	I FROM/TO	I ARM A	I ARM B	I ARM C
I	07.15 - 08.45	I	I	I	I
I		I ARM A	I 0.000	I 0.550	I 0.450
I		I	I 0.0	I 171.0	I 140.0
I		I	I (0.0)	I (7.0)	I (16.0)
I		I	I	I	I
I		I ARM B	I 0.350	I 0.000	I 0.650
I		I	I 48.0	I 0.0	I 89.0
I		I	I (15.0)	I (0.0)	I (24.0)
I		I	I	I	I
I		I ARM C	I 0.252	I 0.748	I 0.000
I		I	I 63.0	I 187.0	I 0.0
I		I	I (2.0)	I (10.0)	I (0.0)
I		I	I	I	I

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	07.15-07.30									
I	ARM A	3.89	26.27	0.148		0.0	0.2	2.6		0.04
I	ARM B	1.71	26.94	0.064		0.0	0.1	1.0		0.04
I	ARM C	3.13	25.66	0.122		0.0	0.1	2.0		0.04

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	07.30-07.45									
I	ARM A	4.64	26.02	0.178		0.2	0.2	3.2		0.05
I	ARM B	2.04	26.74	0.076		0.1	0.1	1.2		0.04
I	ARM C	3.73	25.59	0.146		0.1	0.2	2.5		0.05

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	07.45-08.00									
I	ARM A	5.69	25.67	0.221		0.2	0.3	4.2		0.05
I	ARM B	2.50	26.47	0.095		0.1	0.1	1.5		0.04
I	ARM C	4.57	25.49	0.179		0.2	0.2	3.2		0.05

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	08.00-08.15									
I	ARM A	5.69	25.67	0.222		0.3	0.3	4.3		0.05
I	ARM B	2.50	26.47	0.095		0.1	0.1	1.6		0.04
I	ARM C	4.57	25.49	0.179		0.2	0.2	3.3		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
ARM A	4.64	26.02	0.178		0.3	0.2	3.3		0.05
ARM B	2.04	26.74	0.076		0.1	0.1	1.3		0.04
ARM C	3.73	25.59	0.146		0.2	0.2	2.6		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
ARM A	3.89	26.27	0.148		0.2	0.2	2.6		0.04
ARM B	1.71	26.94	0.064		0.1	0.1	1.0		0.04
ARM C	3.13	25.66	0.122		0.2	0.1	2.1		0.04

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.3
08.15	0.3
08.30	0.2
08.45	0.2

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.1

For inspection purposes only.
Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

ARM	TOTAL DEMAND (VEH)	CAPACITY (VEH/H)	* QUEUEING * * DELAY * (MIN)	(MIN/VEH)	* INCLUSIVE QUEUEING * * DELAY * (MIN)	(MIN/VEH)
A	426.4	284.3	20.2	0.05	20.2	0.05
B	187.9	125.2	7.6	0.04	7.6	0.04
C	342.8	228.5	15.8	0.05	15.8	0.05
ALL	957.1	638.1	43.6	0.05	43.6	0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

(c) Copyright TRL Limited, 2004

Adapted from ARCADY/3 which is Crown Copyright
by permission of the controller of HMSO

For sales and distribution information,
program advice and maintenance, contact:

TRL Limited Tel: +44 (0) 1344 770018
Crowthorne House Fax: +44 (0) 1344 770864
Nine Mile Ride Email: softwarebureau@trl.co.uk
Wokingham, Berks. Web: www.trlsoftware.co.uk
RG40 3GA, UK

THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"C:\ARCADY\2016\16047-02\R135 - N2 Roundabout\AM Peak\2017 AM Peak - With Development.vai"
(drive-on-the-left) at 10:10:48 on Wednesday, 20 July 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
For inspection purposes only.

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
.LENGTH OF TIME PERIOD - 90 MINUTES.
.LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2017 AM Peak - With Development

ARM	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	TOP OF PEAK IS REACHED	MINUTES FROM START WHEN FLOW STOPS IF FALLING	RATE OF FLOW (VEH/MIN) BEFORE PEAK	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	RATE OF FLOW (VEH/MIN) AFTER PEAK
ARM A	15.00	45.00	75.00	3.96	5.94	3.96
ARM B	15.00	45.00	75.00	1.79	2.68	1.79
ARM C	15.00	45.00	75.00	3.13	4.69	3.13

DEMAND SET TITLE: 2017 AM Peak - With Development

TIME	TURNING PROPORTIONS		
	ARM A	ARM B	ARM C
07.15 - 08.45	0.000	0.539	0.461
	0.0	171.0	146.0
	(0.0)	(7.0)	(16.0)
	0.336	0.000	0.664
	48.0	0.0	95.0
	(15.0)	(0.0)	(24.0)
	0.252	0.748	0.000
	63.0	187.0	0.0
	(2.0)	(10.0)	(0.0)

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.15-07.30									
ARM A	3.96	26.25	0.151		0.0	0.2	2.6		0.04
ARM B	1.79	26.87	0.067		0.0	0.1	1.1		0.04
ARM C	3.13	25.66	0.122		0.0	0.1	2.0		0.04

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.30-07.45									
ARM A	4.73	26.00	0.182		0.2	0.2	3.3		0.05
ARM B	2.13	26.66	0.080		0.1	0.1	1.3		0.04
ARM C	3.73	25.59	0.146		0.1	0.2	2.5		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
07.45-08.00									
ARM A	5.80	25.65	0.226		0.2	0.3	4.3		0.05
ARM B	2.61	26.38	0.099		0.1	0.1	1.6		0.04
ARM C	4.57	25.49	0.179		0.2	0.2	3.2		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.00-08.15									
ARM A	5.80	25.65	0.226		0.3	0.3	4.4		0.05
ARM B	2.61	26.38	0.099		0.1	0.1	1.6		0.04
ARM C	4.57	25.49	0.179		0.2	0.2	3.3		0.05

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
ARM A	4.73	25.99	0.182		0.3	0.2	3.4		0.05
ARM B	2.13	26.66	0.080		0.1	0.1	1.3		0.04
ARM C	3.73	25.59	0.146		0.2	0.2	2.6		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
ARM A	3.96	26.25	0.151		0.2	0.2	2.7		0.04
ARM B	1.79	26.86	0.067		0.1	0.1	1.1		0.04
ARM C	3.13	25.66	0.122		0.2	0.1	2.1		0.04

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.3
08.15	0.3
08.30	0.2
08.45	0.2

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.1

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

ARM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * (MIN)	* DELAY * (MIN/VEH)	* INCLUSIVE QUEUEING * (MIN)	* DELAY * (MIN/VEH)
A	434.7	289.8	20.7	0.05	20.7	0.05
B	196.1	130.7	8.0	0.04	8.0	0.04
C	342.8	228.5	15.8	0.05	15.8	0.05
ALL	973.6	649.0	44.5	0.05	44.5	0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

(c) Copyright TRL Limited, 2004

Adapted from ARCADY/3 which is Crown Copyright
by permission of the controller of HMSO

For sales and distribution information,
program advice and maintenance, contact:

TRL Limited Tel: +44 (0) 1344 770018
Crowthorne House Fax: +44 (0) 1344 770864
Nine Mile Ride Email: softwarebureau@trl.co.uk
Wokingham, Berks. Web: www.trlsoftware.co.uk
RG40 3GA, UK

THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-

"C:\ARCADY\2016\16047-02\R135 - N2 Roundabout\AM Peak\2023 AM Peak - No Development.vai"
(drive-on-the-left) at 10:12:31 on Wednesday, 20 July 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
For inspection purposes only.

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
.LENGTH OF TIME PERIOD - 90 MINUTES.
.LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2023 AM Peak - No Development

I	I	NUMBER OF MINUTES FROM START WHEN			RATE OF FLOW (VEH/MIN)			I
I	ARM	I FLOW STARTS	I TOP OF PEAK	I FLOW STOPS	I BEFORE	I AT TOP	I AFTER	I
I	I	I TO RISE	I IS REACHED	I FALLING	I PEAK	I OF PEAK	I PEAK	I
I	ARM A	I 15.00	I 45.00	I 75.00	I 4.06	I 6.09	I 4.06	I
I	ARM B	I 15.00	I 45.00	I 75.00	I 1.77	I 2.66	I 1.77	I
I	ARM C	I 15.00	I 45.00	I 75.00	I 3.26	I 4.89	I 3.26	I

DEMAND SET TITLE: 2023 AM Peak - No Development

I	TURNING PROPORTIONS			I		
I	TURNING COUNTS (VEH/HR)			I		
I	(PERCENTAGE OF H.V.S)			I		
I	TIME	I FROM/TO	I ARM A	I ARM B	I ARM C	I
I	07.15 - 08.45	I	I	I	I	I
I		I ARM A	I 0.000	I 0.551	I 0.449	I
I		I	I 0.0	I 179.0	I 146.0	I
I		I	I (0.0)	I (7.0)	I (16.0)	I
I		I	I	I	I	I
I		I ARM B	I 0.352	I 0.000	I 0.648	I
I		I	I 50.0	I 0.0	I 92.0	I
I		I	I (15.0)	I (0.0)	I (24.0)	I
I		I	I	I	I	I
I		I ARM C	I 0.253	I 0.747	I 0.000	I
I		I	I 66.0	I 195.0	I 0.0	I
I		I	I (2.0)	I (10.0)	I (0.0)	I
I		I	I	I	I	I

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I	TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	I
I		(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	I
I				(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	I
I	07.15-07.30										I
I	ARM A	4.06	26.22	0.155		0.0	0.2	2.7		0.05	I
I	ARM B	1.77	26.90	0.066		0.0	0.1	1.0		0.04	I
I	ARM C	3.26	25.64	0.127		0.0	0.1	2.1		0.04	I

I	TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	I
I		(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	I
I				(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	I
I	07.30-07.45										I
I	ARM A	4.85	25.95	0.187		0.2	0.2	3.4		0.05	I
I	ARM B	2.12	26.70	0.079		0.1	0.1	1.3		0.04	I
I	ARM C	3.90	25.57	0.152		0.1	0.2	2.7		0.05	I

I	TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	I
I		(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	I
I				(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	I
I	07.45-08.00										I
I	ARM A	5.94	25.60	0.232		0.2	0.3	4.5		0.05	I
I	ARM B	2.60	26.42	0.098		0.1	0.1	1.6		0.04	I
I	ARM C	4.77	25.48	0.187		0.2	0.2	3.4		0.05	I

I	TIME	DEMAND	CAPACITY	DEMAND/	PEDESTRIAN	START	END	DELAY	GEOMETRIC DELAY	AVERAGE DELAY	I
I		(VEH/MIN)	(VEH/MIN)	CAPACITY	FLOW	QUEUE	QUEUE	(VEH.MIN/	(VEH.MIN/	PER ARRIVING	I
I				(RFC)	(PEDS/MIN)	(VEHS)	(VEHS)	TIME SEGMENT)	TIME SEGMENT)	VEHICLE (MIN)	I
I	08.00-08.15										I
I	ARM A	5.94	25.59	0.232		0.3	0.3	4.5		0.05	I
I	ARM B	2.60	26.42	0.098		0.1	0.1	1.6		0.04	I
I	ARM C	4.77	25.47	0.187		0.2	0.2	3.4		0.05	I

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
ARM A	4.85	25.95	0.187		0.3	0.2	3.5		0.05
ARM B	2.12	26.69	0.079		0.1	0.1	1.3		0.04
ARM C	3.90	25.57	0.152		0.2	0.2	2.7		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
ARM A	4.06	26.22	0.155		0.2	0.2	2.8		0.05
ARM B	1.77	26.90	0.066		0.1	0.1	1.1		0.04
ARM C	3.26	25.64	0.127		0.2	0.1	2.2		0.04

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.3
08.15	0.3
08.30	0.2
08.45	0.2

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.1

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

ARM	TOTAL DEMAND (VEH)	CAPACITY (VEH/H)	* QUEUEING * * DELAY * (MIN)	(MIN/VEH)	* INCLUSIVE QUEUEING * * DELAY * (MIN)	(MIN/VEH)
A	445.6	297.1	21.4	0.05	21.4	0.05
B	194.7	129.8	7.9	0.04	7.9	0.04
C	357.9	238.6	16.6	0.05	16.6	0.05
ALL	998.2	665.5	45.9	0.05	45.9	0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

(c) Copyright TRL Limited, 2004

Adapted from ARCADY/3 which is Crown Copyright
by permission of the controller of HMSO

For sales and distribution information,
program advice and maintenance, contact:

TRL Limited Tel: +44 (0) 1344 770018
Crowthorne House Fax: +44 (0) 1344 770864
Nine Mile Ride Email: softwarebureau@trl.co.uk
Wokingham, Berks. Web: www.trlsoftware.co.uk
RG40 3GA, UK

THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"C:\ARCADY\2016\16047-02\R135 - N2 Roundabout\AM Peak\2023 AM Peak - With Development.vai"
(drive-on-the-left) at 10:13:55 on Wednesday, 20 July 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
For inspection purposes only.

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 07.15 AND ENDS 08.45
.LENGTH OF TIME PERIOD - 90 MINUTES.
.LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2023 AM Peak - With Development

I	I	NUMBER OF MINUTES FROM START WHEN			RATE OF FLOW (VEH/MIN)		
		I	I	I	I	I	I
I	ARM	I	I	I	I	I	I
I		TO RISE	IS REACHED	IF FALLING	BEFORE PEAK	AT TOP OF PEAK	AFTER PEAK
I	ARM A	I 15.00	I 45.00	I 75.00	I 4.14	I 6.21	I 4.14
I	ARM B	I 15.00	I 45.00	I 75.00	I 1.85	I 2.78	I 1.85
I	ARM C	I 15.00	I 45.00	I 75.00	I 3.26	I 4.89	I 3.26

DEMAND SET TITLE: 2023 AM Peak - With Development

I	I	TURNING PROPORTIONS			
		I	I	I	
I		TURNING COUNTS (VEH/HR)			
I		(PERCENTAGE OF H.V.S)			
I	TIME	I	ARM A	ARM B	ARM C
I		FROM/TO			
I	07.15 - 08.45	I	I	I	I
I		ARM A	0.000	0.541	0.459
I			0.0	179.0	152.0
I			(0.0)	(7.0)	(16.0)
I		ARM B	0.338	0.000	0.662
I			50.0	0.0	98.0
I			(15.0)	(0.0)	(24.0)
I		ARM C	0.253	0.747	0.000
I			66.0	195.0	0.0
I			(2.0)	(10.0)	(0.0)
I					

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	07.15-07.30									
I	ARM A	4.14	26.20	0.158		0.0	0.2	2.8		0.05
I	ARM B	1.85	26.83	0.069		0.0	0.1	1.1		0.04
I	ARM C	3.26	25.64	0.127		0.0	0.1	2.1		0.04

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	07.30-07.45									
I	ARM A	4.94	25.93	0.191		0.2	0.2	3.5		0.05
I	ARM B	2.21	26.62	0.083		0.1	0.1	1.3		0.04
I	ARM C	3.90	25.57	0.152		0.1	0.2	2.7		0.05

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	07.45-08.00									
I	ARM A	6.05	25.57	0.237		0.2	0.3	4.6		0.05
I	ARM B	2.71	26.33	0.103		0.1	0.1	1.7		0.04
I	ARM C	4.77	25.48	0.187		0.2	0.2	3.4		0.05

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
I	08.00-08.15									
I	ARM A	6.05	25.57	0.237		0.3	0.3	4.6		0.05
I	ARM B	2.71	26.33	0.103		0.1	0.1	1.7		0.04
I	ARM C	4.77	25.47	0.187		0.2	0.2	3.4		0.05

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.15-08.30									
ARM A	4.94	25.93	0.191		0.3	0.2	3.6		0.05
ARM B	2.21	26.61	0.083		0.1	0.1	1.4		0.04
ARM C	3.90	25.57	0.152		0.2	0.2	2.7		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
08.30-08.45									
ARM A	4.14	26.19	0.158		0.2	0.2	2.9		0.05
ARM B	1.85	26.83	0.069		0.1	0.1	1.1		0.04
ARM C	3.26	25.64	0.127		0.2	0.1	2.2		0.04

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.2
07.45	0.2
08.00	0.3
08.15	0.3
08.30	0.2
08.45	0.2

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.1
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
07.30	0.1
07.45	0.2
08.00	0.2
08.15	0.2
08.30	0.2
08.45	0.1

For inspection purposes only.
Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

ARM	TOTAL DEMAND (VEH)	CAPACITY (VEH/H)	* QUEUEING * (MIN)	* DELAY * (MIN/VEH)	* INCLUSIVE QUEUEING * (MIN)	* DELAY * (MIN/VEH)
A	453.9	302.6	21.9	0.05	21.9	0.05
B	202.9	135.3	8.3	0.04	8.3	0.04
C	357.9	238.6	16.6	0.05	16.6	0.05
ALL	1014.7	676.5	46.8	0.05	46.8	0.05

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

ARCADY 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

(c) Copyright TRL Limited, 2004

Adapted from ARCADY/3 which is Crown Copyright
by permission of the controller of HMSO

For sales and distribution information,
program advice and maintenance, contact:

TRL Limited Tel: +44 (0) 1344 770018
Crowthorne House Fax: +44 (0) 1344 770864
Nine Mile Ride Email: softwarebureau@trl.co.uk
Wokingham, Berks. Web: www.trlsoftware.co.uk
RG40 3GA, UK

THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"C:\ARCADY\2016\16047-02\R135 - N2 Roundabout\PM Peak\2016 PM Peak - Existing Flows.vai"
(drive-on-the-left) at 10:17:51 on Wednesday, 20 July 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
For inspection purposes only.

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT	PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I	
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I	
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I	

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 16.30 AND ENDS 18.00
.LENGTH OF TIME PERIOD - 90 MINUTES.
.LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2016 PM Peak - Existing Flows

ARM	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	TOP OF PEAK IS REACHED	FLOW STOPS IF FALLING	RATE OF FLOW (VEH/MIN) BEFORE PEAK	AT TOP OF PEAK	AFTER PEAK
ARM A	15.00	45.00	75.00	2.46	3.69	2.46
ARM B	15.00	45.00	75.00	4.50	6.75	4.50
ARM C	15.00	45.00	75.00	3.59	5.38	3.59

DEMAND SET TITLE: 2016 PM Peak - Existing Flows

TIME	TURNING PROPORTIONS			TURNING COUNTS (VEH/HR)			(PERCENTAGE OF H.V.S)			
	FROM/TO	ARM A	ARM B	ARM C	ARM A	ARM B	ARM C	ARM A	ARM B	ARM C
16.30 - 18.00	ARM A	0.000	0.563	0.437	0.0	111.0	86.0	(0.0)	(8.0)	(5.0)
	ARM B	0.514	0.000	0.486	185.0	0.0	175.0	(9.0)	(0.0)	(9.0)
	ARM C	0.443	0.557	0.000	127.0	160.0	0.0	(9.0)	(10.0)	(0.0)

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
ARM A	2.46	27.54	0.089		0.0	0.1	1.4		0.04
ARM B	4.50	30.36	0.148		0.0	0.2	2.6		0.04
ARM C	3.59	24.37	0.147		0.0	0.2	2.5		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
ARM A	2.94	27.32	0.108		0.1	0.1	1.8		0.04
ARM B	5.37	30.24	0.178		0.2	0.2	3.2		0.04
ARM C	4.28	24.12	0.178		0.2	0.2	3.2		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
ARM A	3.60	27.00	0.133		0.1	0.2	2.3		0.04
ARM B	6.58	30.07	0.219		0.2	0.3	4.1		0.04
ARM C	5.25	23.78	0.221		0.2	0.3	4.2		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
ARM A	3.60	27.00	0.133		0.2	0.2	2.3		0.04
ARM B	6.58	30.07	0.219		0.3	0.3	4.2		0.04
ARM C	5.25	23.78	0.221		0.3	0.3	4.2		0.05

Appendix D - ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
ARM A	2.94	27.31	0.108		0.2	0.1	1.8		0.04
ARM B	5.37	30.24	0.178		0.3	0.2	3.3		0.04
ARM C	4.28	24.12	0.178		0.3	0.2	3.3		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
ARM A	2.46	27.54	0.089		0.1	0.1	1.5		0.04
ARM B	4.50	30.36	0.148		0.2	0.2	2.6		0.04
ARM C	3.59	24.36	0.147		0.2	0.2	2.6		0.05

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

ARM	TOTAL DEMAND (VEH)	CAPACITY (VEH/H)	* QUEUEING * (MIN)	* INCLUSIVE QUEUEING * (MIN)	* DELAY * (MIN/VEH)	* DELAY * (MIN/VEH)
A	270.1	180.1	11.1	11.1	0.04	0.04
B	493.6	329.1	20.0	20.0	0.04	0.04
C	393.5	262.4	20.1	20.1	0.05	0.05
ALL	1157.3	771.5	51.2	51.2	0.04	0.04

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

ARCADY 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

(c) Copyright TRL Limited, 2004

Adapted from ARCADY/3 which is Crown Copyright
by permission of the controller of HMSO

For sales and distribution information,
program advice and maintenance, contact:

TRL Limited Tel: +44 (0) 1344 770018
Crowthorne House Fax: +44 (0) 1344 770864
Nine Mile Ride Email: softwarebureau@trl.co.uk
Wokingham, Berks. Web: www.trlsoftware.co.uk
RG40 3GA, UK

THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"C:\ARCADY\2016\16047-02\R135 - N2 Roundabout\PM Peak\2017 PM Peak - With Development.vai"
(drive-on-the-left) at 10:20:35 on Wednesday, 20 July 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

Consent of copyright owner required for any other use.
For inspection purposes only.

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 16.30 AND ENDS 18.00
.LENGTH OF TIME PERIOD - 90 MINUTES.
.LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2017 PM Peak - With Development

ARM	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	TOP OF PEAK IS REACHED	MINUTES FROM START WHEN FLOW STOPS IF FALLING	RATE OF FLOW (VEH/MIN) BEFORE PEAK	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	RATE OF FLOW (VEH/MIN) AFTER PEAK
ARM A	15.00	45.00	75.00	2.56	3.84	2.56
ARM B	15.00	45.00	75.00	4.60	6.90	4.60
ARM C	15.00	45.00	75.00	3.61	5.42	3.61

DEMAND SET TITLE: 2017 PM Peak - With Development

TIME	TURNING PROPORTIONS			TURNING COUNTS (VEH/HR)			(PERCENTAGE OF H.V.S)		
	ARM A	ARM B	ARM C	ARM A	ARM B	ARM C	ARM A	ARM B	ARM C
16.30 - 18.00	0.000	0.546	0.454	0.0	112.0	93.0	(0.0)	(8.0)	(5.0)
	0.505	0.000	0.495	186.0	0.0	182.0	(9.0)	(0.0)	(9.0)
	0.443	0.557	0.000	128.0	161.0	0.0	(9.0)	(10.0)	(0.0)

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
ARM A	2.56	27.55	0.093		0.0	0.1	1.5		0.04
ARM B	4.60	30.31	0.152		0.0	0.2	2.6		0.04
ARM C	3.61	24.36	0.148		0.0	0.2	2.6		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
ARM A	3.06	27.32	0.112		0.1	0.1	1.9		0.04
ARM B	5.49	30.18	0.182		0.2	0.2	3.3		0.04
ARM C	4.31	24.11	0.179		0.2	0.2	3.2		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
ARM A	3.75	27.01	0.139		0.1	0.2	2.4		0.04
ARM B	6.73	30.00	0.224		0.2	0.3	4.3		0.04
ARM C	5.28	23.77	0.222		0.2	0.3	4.2		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
ARM A	3.75	27.00	0.139		0.2	0.2	2.4		0.04
ARM B	6.73	30.00	0.224		0.3	0.3	4.3		0.04
ARM C	5.28	23.77	0.222		0.3	0.3	4.3		0.05

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
ARM A	3.06	27.32	0.112		0.2	0.1	1.9		0.04
ARM B	5.49	30.18	0.182		0.3	0.2	3.4		0.04
ARM C	4.31	24.11	0.179		0.3	0.2	3.3		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
ARM A	2.56	27.55	0.093		0.1	0.1	1.6		0.04
ARM B	4.60	30.31	0.152		0.2	0.2	2.7		0.04
ARM C	3.61	24.36	0.148		0.2	0.2	2.7		0.05

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

ARM	TOTAL DEMAND (VEH)	CAPACITY (VEH/H)	* QUEUEING * * DELAY * (MIN)	* INCLUSIVE QUEUEING * * DELAY * (MIN)
A	281.1	187.4	11.7	11.7
B	504.6	336.4	20.6	20.6
C	396.3	264.2	20.2	20.2
ALL	1182.0	788.0	52.5	52.5

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

A R C A D Y 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

(c) Copyright TRL Limited, 2004

Adapted from ARCADY/3 which is Crown Copyright
by permission of the controller of HMSO

For sales and distribution information,
program advice and maintenance, contact:

TRL Limited Tel: +44 (0) 1344 770018
Crowthorne House Fax: +44 (0) 1344 770864
Nine Mile Ride Email: softwarebureau@trl.co.uk
Wokingham, Berks. Web: www.trlsoftware.co.uk
RG40 3GA, UK

THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"C:\ARCADY\2016\16047-02\R135 - N2 Roundabout\PM Peak\2023 PM Peak - No Development.vai"
(drive-on-the-left) at 10:21:42 on Wednesday, 20 July 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

Consent of copyright owner required for any other use.
For inspection purposes only.

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 16.30 AND ENDS 18.00
.LENGTH OF TIME PERIOD - 90 MINUTES.
.LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2023 PM Peak - No Development

	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	MINUTES FROM START WHEN TOP OF PEAK IS REACHED	MINUTES FROM START WHEN FLOW STOPS IF FALLING	RATE OF FLOW (VEH/MIN) BEFORE PEAK	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	RATE OF FLOW (VEH/MIN) AFTER PEAK
ARM A	15.00	45.00	75.00	2.59	3.88	2.59
ARM B	15.00	45.00	75.00	4.72	7.09	4.72
ARM C	15.00	45.00	75.00	3.76	5.64	3.76

DEMAND SET TITLE: 2023 PM Peak - No Development

TIME	TURNING PROPORTIONS		
	ARM A	ARM B	ARM C
16.30 - 18.00	0.000	0.565	0.435
	0.0	117.0	90.0
	(0.0)	(8.0)	(5.0)
	0.513	0.000	0.487
	194.0	0.0	184.0
	(9.0)	(0.0)	(9.0)
	0.442	0.558	0.000
	133.0	168.0	0.0
	(9.0)	(10.0)	(0.0)

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
ARM A	2.59	27.48	0.094		0.0	0.1	1.5		0.04
ARM B	4.72	30.33	0.156		0.0	0.2	2.7		0.04
ARM C	3.76	24.31	0.155		0.0	0.2	2.7		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
ARM A	3.09	27.24	0.113		0.1	0.1	1.9		0.04
ARM B	5.64	30.21	0.187		0.2	0.2	3.4		0.04
ARM C	4.49	24.04	0.187		0.2	0.2	3.4		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
ARM A	3.78	26.92	0.141		0.1	0.2	2.4		0.04
ARM B	6.91	30.03	0.230		0.2	0.3	4.4		0.04
ARM C	5.50	23.69	0.232		0.2	0.3	4.5		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
ARM A	3.78	26.92	0.141		0.2	0.2	2.4		0.04
ARM B	6.91	30.03	0.230		0.3	0.3	4.5		0.04
ARM C	5.50	23.69	0.232		0.3	0.3	4.5		0.05

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
ARM A	3.09	27.24	0.113		0.2	0.1	1.9		0.04
ARM B	5.64	30.20	0.187		0.3	0.2	3.5		0.04
ARM C	4.49	24.04	0.187		0.3	0.2	3.5		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
ARM A	2.59	27.48	0.094		0.1	0.1	1.6		0.04
ARM B	4.72	30.33	0.156		0.2	0.2	2.8		0.04
ARM C	3.76	24.30	0.155		0.2	0.2	2.8		0.05

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

ARM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * * DELAY * (MIN)	(MIN/VEH)	* INCLUSIVE QUEUEING * * DELAY * (MIN)	(MIN/VEH)
A	283.8	189.2	11.8	0.04	11.8	0.04
B	518.3	345.5	21.3	0.04	21.3	0.04
C	412.7	275.2	21.4	0.05	21.4	0.05
ALL	1214.9	809.9	54.5	0.04	54.5	0.04

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

ARCADY 6

ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 4.0 (FEBRUARY 2006)

(c) Copyright TRL Limited, 2004

Adapted from ARCADY/3 which is Crown Copyright
by permission of the controller of HMSO

For sales and distribution information,
program advice and maintenance, contact:

TRL Limited Tel: +44 (0) 1344 770018
Crowthorne House Fax: +44 (0) 1344 770864
Nine Mile Ride Email: softwarebureau@trl.co.uk
Wokingham, Berks. Web: www.trlsoftware.co.uk
RG40 3GA, UK

THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"C:\ARCADY\2016\16047-02\R135 - N2 Roundabout\PM Peak\2023 PM Peak - With Development.vai"
(drive-on-the-left) at 10:22:40 on Wednesday, 20 July 2016

.FILE PROPERTIES

RUN TITLE: Huntstown Quarry
LOCATION: Huntstown
DATE: 17/07/16
CLIENT: Roadstone
ENUMERATOR: Roadplan
JOB NUMBER: 16047-02
STATUS: TIA
DESCRIPTION:

.INPUT DATA

ARM A - R135 North
ARM B - R135 South
ARM C - N2 Slip Road

.GEOMETRIC DATA

I	ARM	I	V (M)	I	E (M)	I	L (M)	I	R (M)	I	D (M)	I	PHI (DEG)	I	SLOPE	I	INTERCEPT (PCU/MIN)	I
I	ARM A	I	3.50	I	8.00	I	30.00	I	20.00	I	60.00	I	51.0	I	0.562	I	30.623	I
I	ARM B	I	3.50	I	9.50	I	30.00	I	20.00	I	60.00	I	49.0	I	0.596	I	33.767	I
I	ARM C	I	7.00	I	9.00	I	30.00	I	3.00	I	13.00	I	53.0	I	0.551	I	28.086	I

V = approach half-width L = effective flare length D = inscribed circle diameter
E = entry width R = entry radius PHI = entry angle

.TRAFFIC DEMAND DATA

(Only sets included in the current run are shown)

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

.TIME PERIOD BEGINS 16.30 AND ENDS 18.00
.LENGTH OF TIME PERIOD - 90 MINUTES.
.LENGTH OF TIME SEGMENT - 15 MINUTES.

.DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: 2023 PM Peak - With Development

ARM	NUMBER OF MINUTES FROM START WHEN FLOW STARTS TO RISE	TOP OF PEAK IS REACHED	MINUTES FROM START WHEN FLOW STOPS IF FALLING	RATE OF FLOW (VEH/MIN) BEFORE PEAK	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	RATE OF FLOW (VEH/MIN) AFTER PEAK
ARM A	15.00	45.00	75.00	2.66	3.99	2.66
ARM B	15.00	45.00	75.00	4.80	7.20	4.80
ARM C	15.00	45.00	75.00	3.76	5.64	3.76

DEMAND SET TITLE: 2023 PM Peak - With Development

TIME	TURNING PROPORTIONS		
	ARM A	ARM B	ARM C
16.30 - 18.00	0.000	0.549	0.451
	0.0	117.0	96.0
	(0.0)	(8.0)	(5.0)
	0.505	0.000	0.495
	194.0	0.0	190.0
	(9.0)	(0.0)	(9.0)
	0.442	0.558	0.000
	133.0	168.0	0.0
	(9.0)	(10.0)	(0.0)

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.30-16.45									
ARM A	2.66	27.50	0.097		0.0	0.1	1.6		0.04
ARM B	4.80	30.29	0.158		0.0	0.2	2.8		0.04
ARM C	3.76	24.31	0.155		0.0	0.2	2.7		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
16.45-17.00									
ARM A	3.18	27.26	0.117		0.1	0.1	2.0		0.04
ARM B	5.73	30.15	0.190		0.2	0.2	3.5		0.04
ARM C	4.49	24.04	0.187		0.2	0.2	3.4		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.00-17.15									
ARM A	3.89	26.94	0.145		0.1	0.2	2.5		0.04
ARM B	7.02	29.97	0.234		0.2	0.3	4.5		0.04
ARM C	5.50	23.69	0.232		0.2	0.3	4.5		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.15-17.30									
ARM A	3.89	26.93	0.145		0.2	0.2	2.5		0.04
ARM B	7.02	29.97	0.234		0.3	0.3	4.6		0.04
ARM C	5.50	23.69	0.232		0.3	0.3	4.5		0.05

Appendix D – ARCADY Results

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.30-17.45									
ARM A	3.18	27.25	0.117		0.2	0.1	2.0		0.04
ARM B	5.73	30.15	0.190		0.3	0.2	3.6		0.04
ARM C	4.49	24.04	0.187		0.3	0.2	3.5		0.05

TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)
17.45-18.00									
ARM A	2.66	27.49	0.097		0.1	0.1	1.6		0.04
ARM B	4.80	30.29	0.158		0.2	0.2	2.9		0.04
ARM C	3.76	24.30	0.155		0.2	0.2	2.8		0.05

QUEUE AT ARM A

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.2
17.30	0.2
17.45	0.1
18.00	0.1

QUEUE AT ARM B

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

QUEUE AT ARM C

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.2
17.00	0.2
17.15	0.3
17.30	0.3
17.45	0.2
18.00	0.2

For inspection purposes only.
Consent of copyright owner required for any other use.

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

ARM	TOTAL DEMAND (VEH)	DEMAND (VEH/H)	* QUEUEING * * DELAY * (MIN)	* INCLUSIVE QUEUEING * * DELAY * (MIN)
A	292.1	194.7	12.2	12.2
B	526.5	351.0	21.8	21.8
C	412.7	275.2	21.4	21.4
ALL	1231.3	820.9	55.3	55.3

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

APPENDIX 13-E OSCADY PRO RESULTS

*For inspection purposes only.
Consent of copyright owner required for any other use.*

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads.osc
Report generation date: 05/08/2016 16:02:32

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

For inspection purposes only.
Consent of copyright owner required for any other use.

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

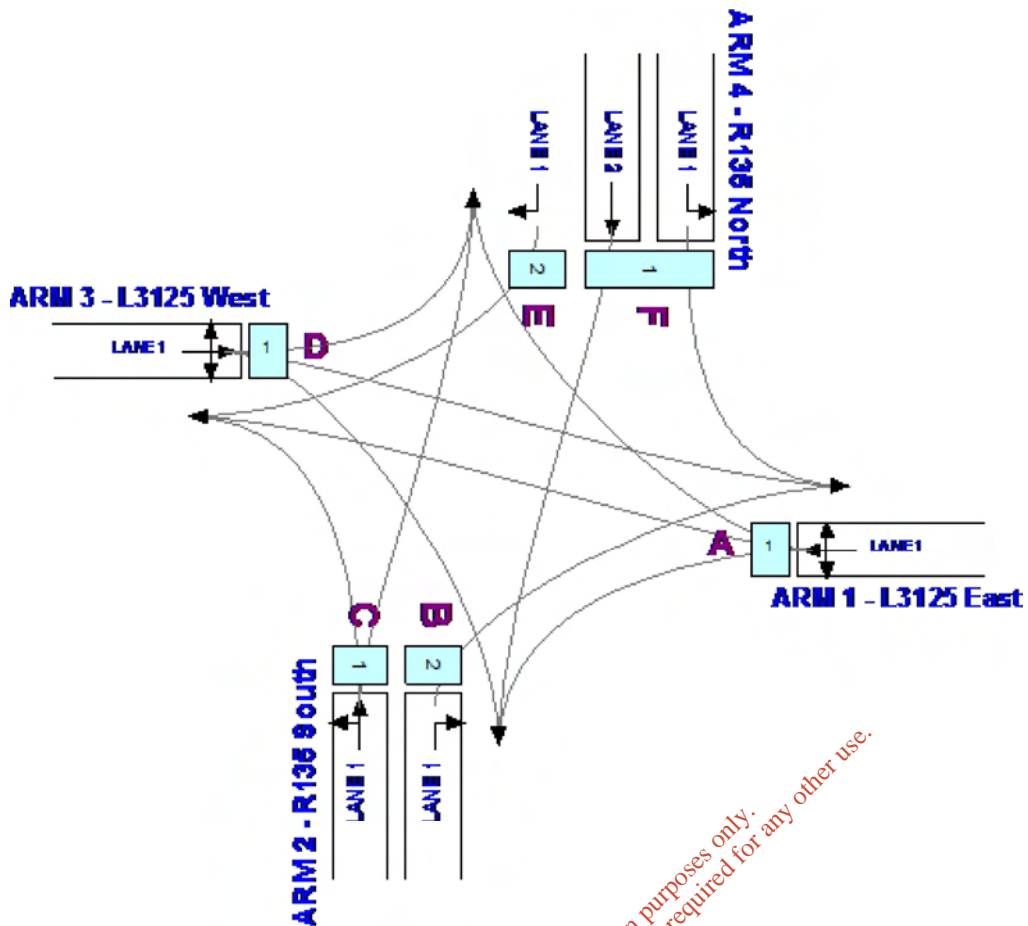
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

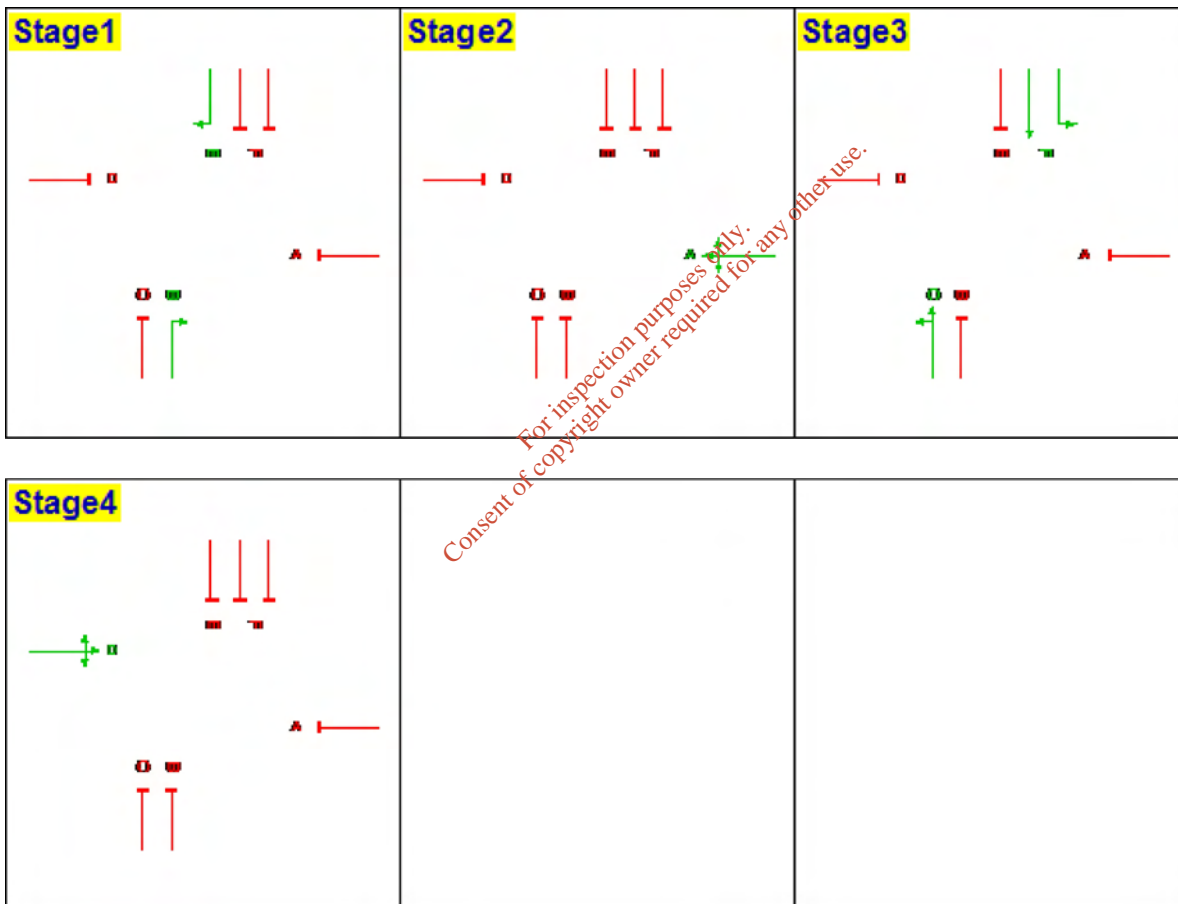
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	Yes	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set1 - 2016 AM Peak Existing

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	101	331	93
	Arm 2	88	-	224	77
	Arm 3	163	111	-	11
	Arm 4	234	137	30	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	394	470	576	576	470	394
2 - R135 South	1	C	228	272	333	333	272	228
2 - R135 South	2	B	67	80	98	98	80	67
3 - L3125 West	1	D	214	255	313	313	255	214
4 - R135 North	1	F	277	330	405	405	330	277
4 - R135 North	2	E	21	25	31	31	25	21

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	19	63	18
2 - R135 South	58	20	23
3 - L3125 West	4	57	39
4 - R135 North	58	34	7

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	21.48	20.44	20.44	68.8

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	1.30	21.71	21.71	66.40

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

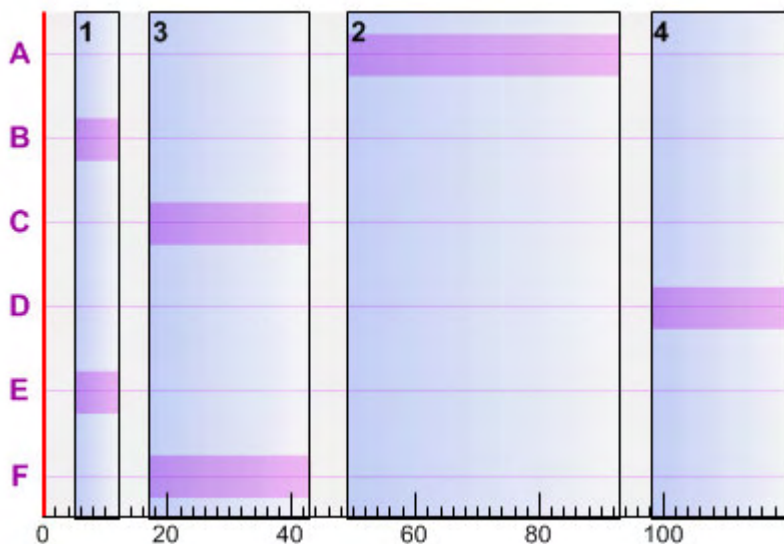
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	26.0	43.0
2	49.0	44.0	93.0
4	98.0	22.0	0.0

Phase Timings

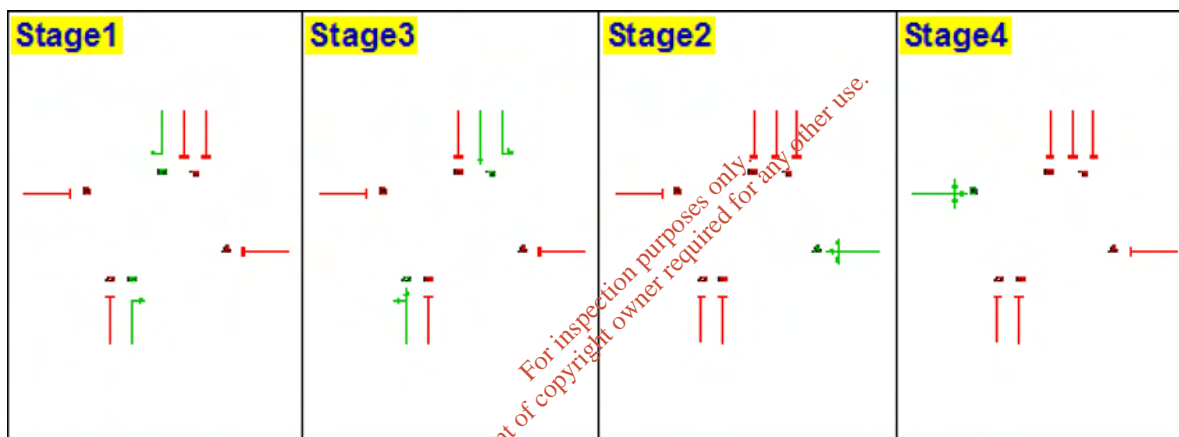
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	49	44.0	93						
B	5	7.0	12						
C	17	26.0	43						
D	98	22.0	0						
E	5	7.0	12						
F	17	26.0	43						

For inspection purposes only. Consent of copyright owner required for any other use.

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	480	A	45.50	39.92	5.32	70.68	27.33	1.24	12.22	10.99	28.10
2	1	278	C	27.50	56.47	4.36	70.36	27.90	1.15	8.72	7.56	10.30
2	2	82	B	8.50	82.36	1.88	61.97	45.23	0.66	3.25	2.58	1.70
3	1	261	D	23.50	64.03	4.64	74.08	21.48	1.45	8.83	7.38	8.00
4	1	337	F	27.50	40.87	3.83	38.85	131.65	0.17	9.46	9.28	19.90
4	2	26	E	8.50	56.91	0.41	19.65	358.02	0.03	0.84	0.81	0.80

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	394	1	45.50	34.32	3.76	58.02	55.12	0.56	9.42	8.85	4.90
2	1	228	3	27.50	48.83	3.09	57.71	55.95	0.53	6.67	6.14	1.80
2	2	67	2	8.50	70.20	1.31	50.64	77.74	0.33	2.43	2.10	0.30
3	1	214	4	23.50	53.71	3.19	60.74	48.17	0.62	6.62	5.99	1.40
4	1	277	6	27.50	39.74	3.06	31.93	181.83	0.10	7.64	7.53	2.90
4	2	21	5	8.50	55.72	0.33	15.87	467.07	0.02	0.67	0.65	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	470	1	45.50	38.93	5.08	69.21	30.04	1.09	11.83	10.74	4.70
2	1	272	3	27.50	54.85	4.14	68.85	30.73	1.00	8.39	7.39	1.70
2	2	80	2	8.50	79.12	1.76	60.46	48.86	0.57	3.08	2.52	0.30
3	1	255	4	23.50	61.41	4.35	72.38	24.34	1.22	8.42	7.20	1.30
4	1	330	6	27.50	40.73	3.73	38.04	136.56	0.16	9.24	9.08	3.30
4	2	25	5	8.50	56.66	0.39	18.89	376.34	0.03	0.81	0.78	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	576	1	45.50	50.51	8.08	84.82	6.11	3.09	16.56	13.47	3.70
2	1	333	3	27.50	69.41	6.42	84.29	6.78	2.62	11.79	9.17	1.50
2	2	98	2	8.50	95.38	2.60	74.06	21.52	1.16	4.26	3.10	0.30
3	1	313	4	23.50	81.61	7.10	88.84	1.30	3.50	12.45	8.95	1.10
4	1	405	6	27.50	42.30	4.76	46.69	92.75	0.29	11.61	11.32	3.60
4	2	31	5	8.50	58.11	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	576	1	45.50	52.66	8.43	84.82	6.11	3.21	16.69	13.47	3.70
2	1	333	3	27.50	73.75	6.82	84.29	6.78	2.79	11.96	9.17	1.50
2	2	98	2	8.50	101.46	2.76	74.06	21.52	1.24	4.33	3.10	0.30
3	1	313	4	23.50	90.90	7.90	88.84	1.30	3.89	12.84	8.95	1.10
4	1	405	6	27.50	42.32	4.76	46.69	92.75	0.29	11.62	11.32	3.60
4	2	31	5	8.50	58.15	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	470	1	45.50	40.09	5.23	69.21	30.04	1.17	11.91	10.74	4.70
2	1	272	3	27.50	57.78	4.37	68.85	30.73	1.12	8.51	7.39	1.70
2	2	80	2	8.50	85.73	1.91	60.46	48.86	0.68	3.20	2.52	0.30
3	1	255	4	23.50	67.99	4.82	72.38	24.34	1.43	8.63	7.20	1.30
4	1	330	6	27.50	40.75	3.74	38.04	136.56	0.17	9.24	9.08	3.30
4	2	25	5	8.50	56.72	0.39	18.89	376.34	0.03	0.81	0.78	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	394	1	45.50	34.63	3.79	58.02	55.12	0.58	9.44	8.85	4.90
2	1	228	3	27.50	49.71	3.15	57.71	55.95	0.56	6.70	6.14	1.80
2	2	67	2	8.50	73.79	1.37	50.64	77.74	0.37	2.47	2.10	0.30
3	1	214	4	23.50	55.28	3.29	60.74	48.17	0.67	6.67	5.99	1.40
4	1	277	6	27.50	39.76	3.06	31.93	181.83	0.11	7.64	7.53	2.90
4	2	21	5	8.50	55.81	0.33	15.87	467.07	0.02	0.67	0.65	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads.osc
Report generation date: 05/08/2016 16:05:05

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

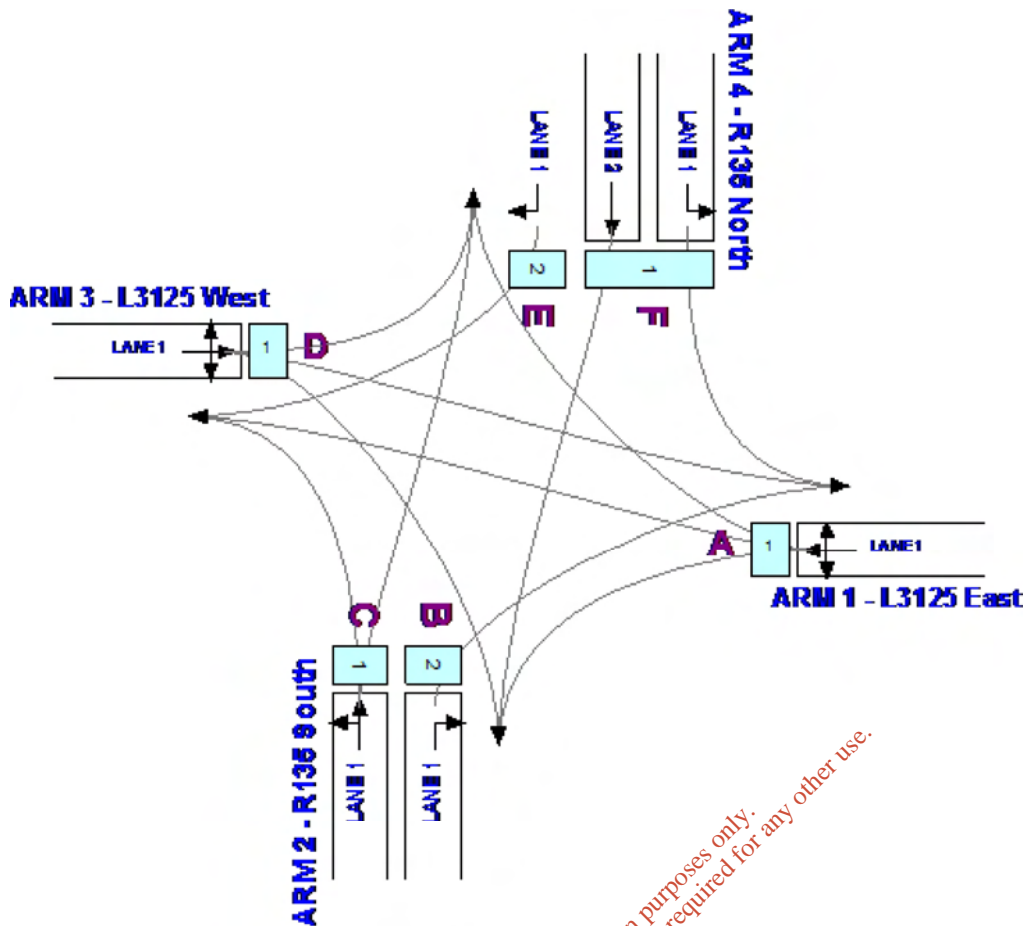
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

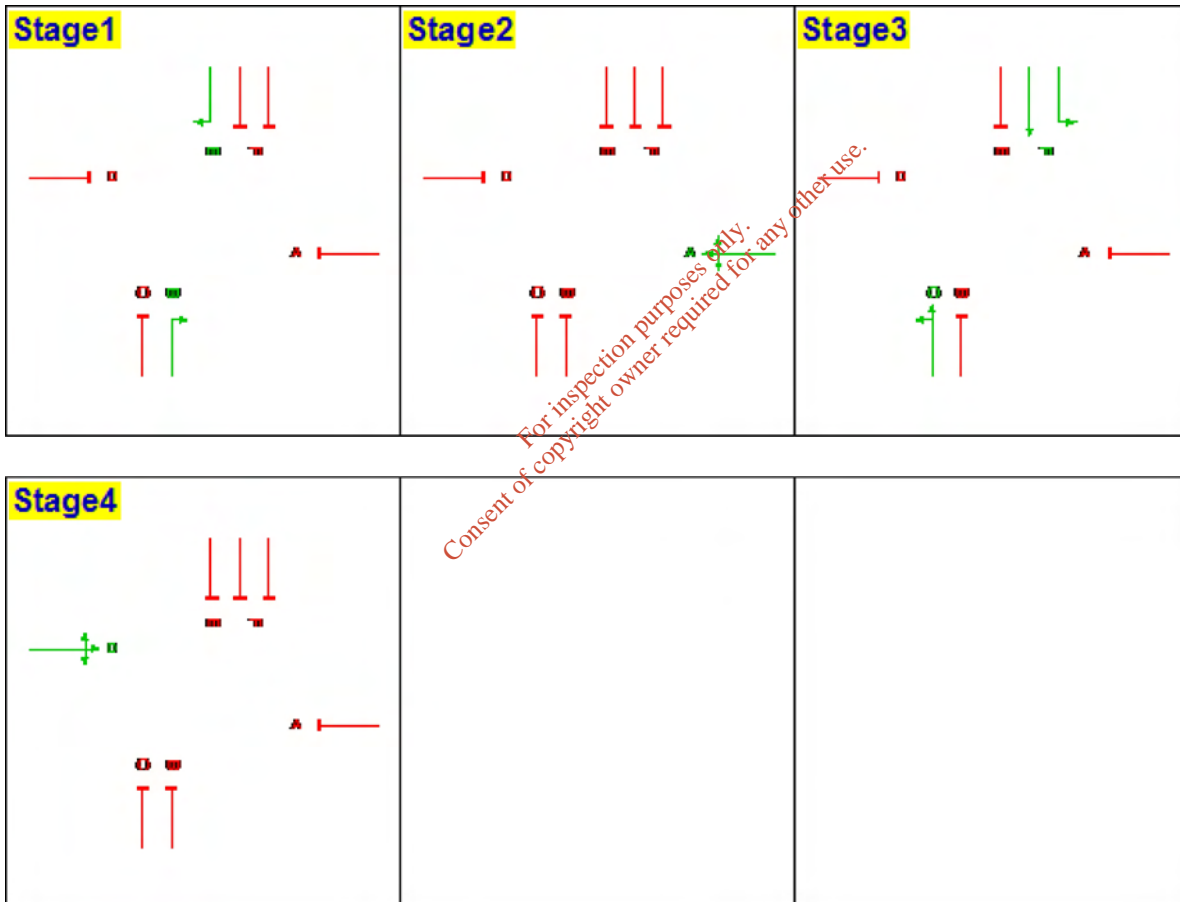
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	Yes	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set2 - 2017 AM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	102	333	94
	Arm 2	89	-	226	78
	Arm 3	164	112	-	11
	Arm 4	236	138	30	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	397	474	580	580	474	397
2 - R135 South	1	C	230	275	336	336	275	230
2 - R135 South	2	B	68	81	99	99	81	68
3 - L3125 West	1	D	215	257	315	315	257	215
4 - R135 North	1	F	279	333	408	408	333	279
4 - R135 North	2	E	21	25	31	31	25	21

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	19	63	18
2 - R135 South	58	20	23
3 - L3125 West	4	57	39
4 - R135 North	58	34	7

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	21.02	20.71	20.71	68.8

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	0.66	22.10	22.10	66.20

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

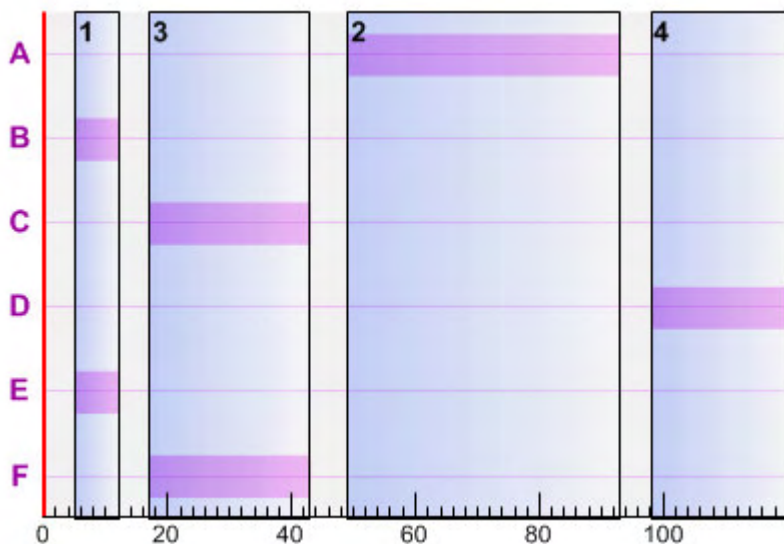
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	26.0	43.0
2	49.0	44.0	93.0
4	98.0	22.0	0.0

Phase Timings

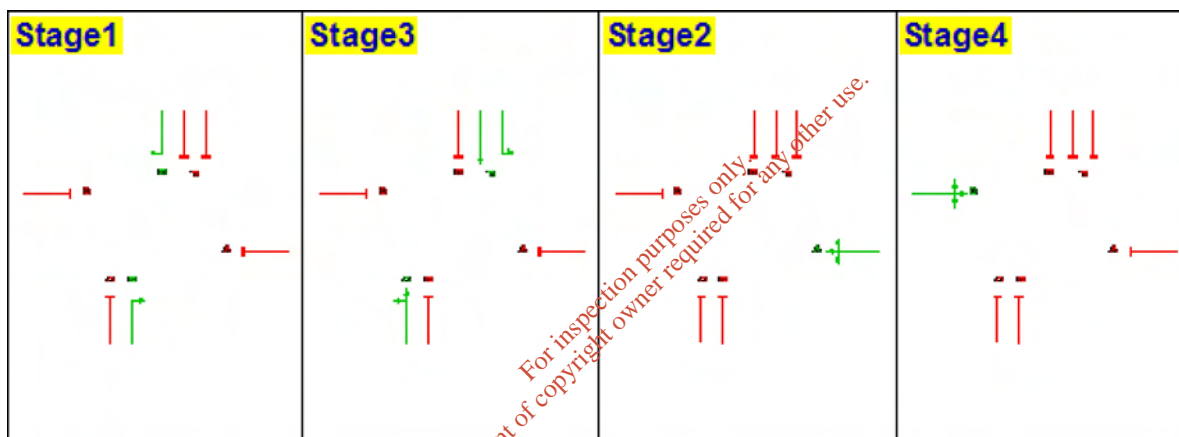
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	49	44.0	93						
B	5	7.0	12						
C	17	26.0	43						
D	98	22.0	0						
E	5	7.0	12						
F	17	26.0	43						

For inspection purposes only. Consent of copyright owner required for any other use.

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	484	A	45.50	40.25	5.41	71.27	26.28	1.28	12.37	11.09	28.00
2	1	280	C	27.50	56.86	4.42	70.87	26.99	1.19	8.81	7.62	10.30
2	2	83	B	8.50	83.26	1.92	62.73	43.48	0.69	3.31	2.61	1.70
3	1	262	D	23.50	64.32	4.68	74.37	21.02	1.48	8.89	7.41	8.00
4	1	340	F	27.50	40.93	3.87	39.20	129.60	0.18	9.55	9.37	20.00
4	2	26	E	8.50	56.91	0.41	19.65	358.02	0.03	0.84	0.81	0.80

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	397	1	45.50	34.46	3.80	58.46	53.95	0.58	9.50	8.92	4.90
2	1	230	3	27.50	49.03	3.13	58.22	54.60	0.54	6.74	6.19	1.80
2	2	68	2	8.50	70.67	1.33	51.39	75.12	0.34	2.48	2.14	0.30
3	1	215	4	23.50	53.84	3.22	61.03	47.48	0.63	6.66	6.02	1.40
4	1	279	6	27.50	39.78	3.08	32.17	179.81	0.11	7.70	7.59	2.90
4	2	21	5	8.50	55.72	0.33	15.87	467.07	0.02	0.67	0.65	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	474	1	45.50	39.22	5.16	69.80	28.94	1.13	11.97	10.84	4.70
2	1	275	3	27.50	55.33	4.23	69.61	29.30	1.05	8.53	7.48	1.70
2	2	81	2	8.50	79.92	1.80	61.22	47.02	0.59	3.14	2.55	0.30
3	1	257	4	23.50	61.86	4.42	72.95	23.37	1.26	8.52	7.26	1.30
4	1	333	6	27.50	40.79	3.77	38.39	134.43	0.17	9.34	9.17	3.30
4	2	25	5	8.50	56.66	0.39	18.89	376.34	0.03	0.81	0.78	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	580	1	45.50	51.19	8.23	85.41	5.38	3.23	16.81	13.58	3.60
2	1	336	3	27.50	70.51	6.58	85.05	5.83	2.77	12.02	9.26	1.40
2	2	99	2	8.50	96.58	2.66	74.82	20.29	1.21	4.34	3.13	0.30
3	1	315	4	23.50	82.71	7.24	89.41	0.66	3.64	12.65	9.01	1.10
4	1	408	6	27.50	42.37	4.80	47.04	91.34	0.30	11.71	11.41	3.70
4	2	31	5	8.50	58.11	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	580	1	45.50	53.57	8.63	85.41	5.38	3.37	16.95	13.58	3.60
2	1	336	3	27.50	75.27	7.03	85.05	5.83	2.95	12.21	9.26	1.40
2	2	99	2	8.50	103.04	2.83	74.82	20.29	1.29	4.42	3.13	0.30
3	1	315	4	23.50	92.68	8.11	89.41	0.66	4.07	13.09	9.01	1.10
4	1	408	6	27.50	42.39	4.80	47.04	91.34	0.30	11.71	11.41	3.70
4	2	31	5	8.50	58.15	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	474	1	45.50	40.49	5.33	69.80	28.94	1.22	12.06	10.84	4.70
2	1	275	3	27.50	58.58	4.48	69.61	29.30	1.18	8.66	7.48	1.70
2	2	81	2	8.50	86.95	1.96	61.22	47.02	0.71	3.26	2.55	0.30
3	1	257	4	23.50	69.05	4.93	72.95	23.37	1.49	8.75	7.26	1.30
4	1	333	6	27.50	40.81	3.77	38.39	134.43	0.17	9.34	9.17	3.30
4	2	25	5	8.50	56.72	0.39	18.89	376.34	0.03	0.81	0.78	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	397	1	45.50	34.79	3.84	58.46	53.95	0.60	9.52	8.92	4.90
2	1	230	3	27.50	49.97	3.19	58.22	54.60	0.58	6.77	6.19	1.80
2	2	68	2	8.50	74.51	1.41	51.39	75.12	0.38	2.52	2.14	0.30
3	1	215	4	23.50	55.50	3.31	61.03	47.48	0.68	6.71	6.02	1.40
4	1	279	6	27.50	39.79	3.08	32.17	179.81	0.11	7.70	7.59	2.90
4	2	21	5	8.50	55.81	0.33	15.87	467.07	0.02	0.67	0.65	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads.osc
Report generation date: 05/08/2016 16:06:20

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

For inspection purposes only.
Consent of copyright owner required for any other use.

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

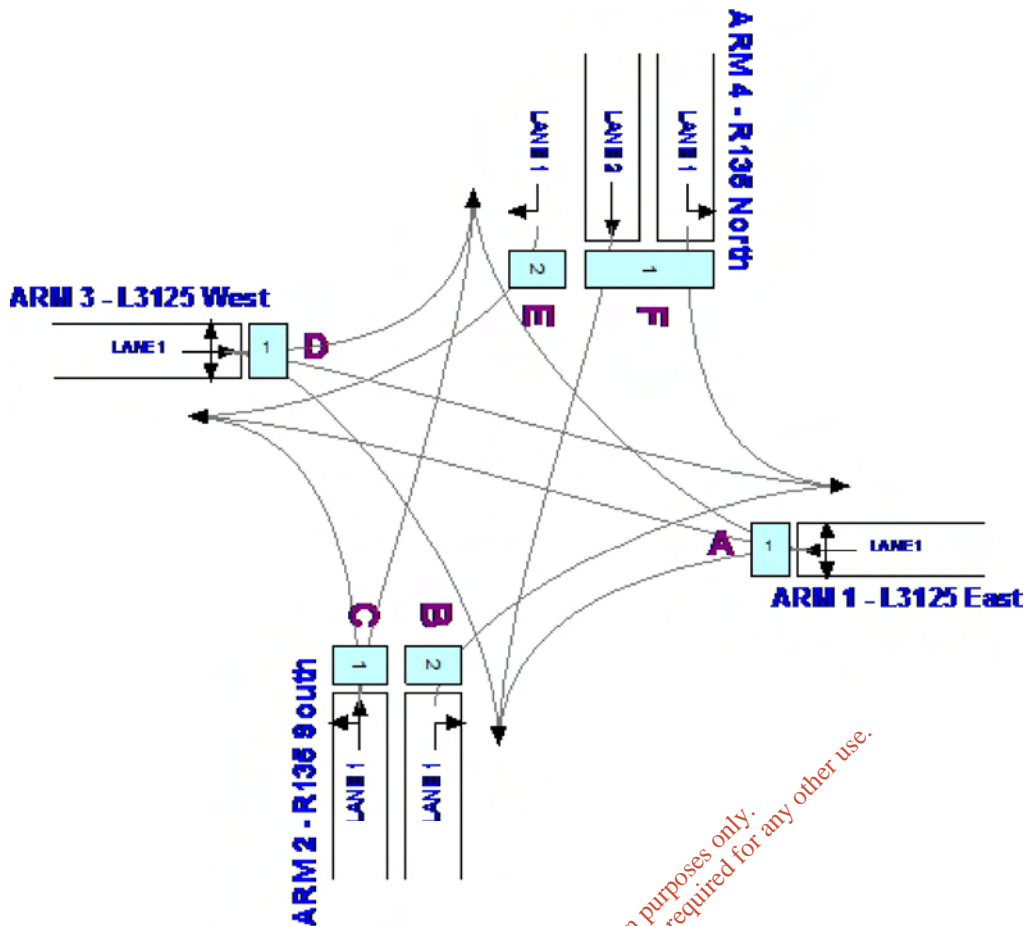
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

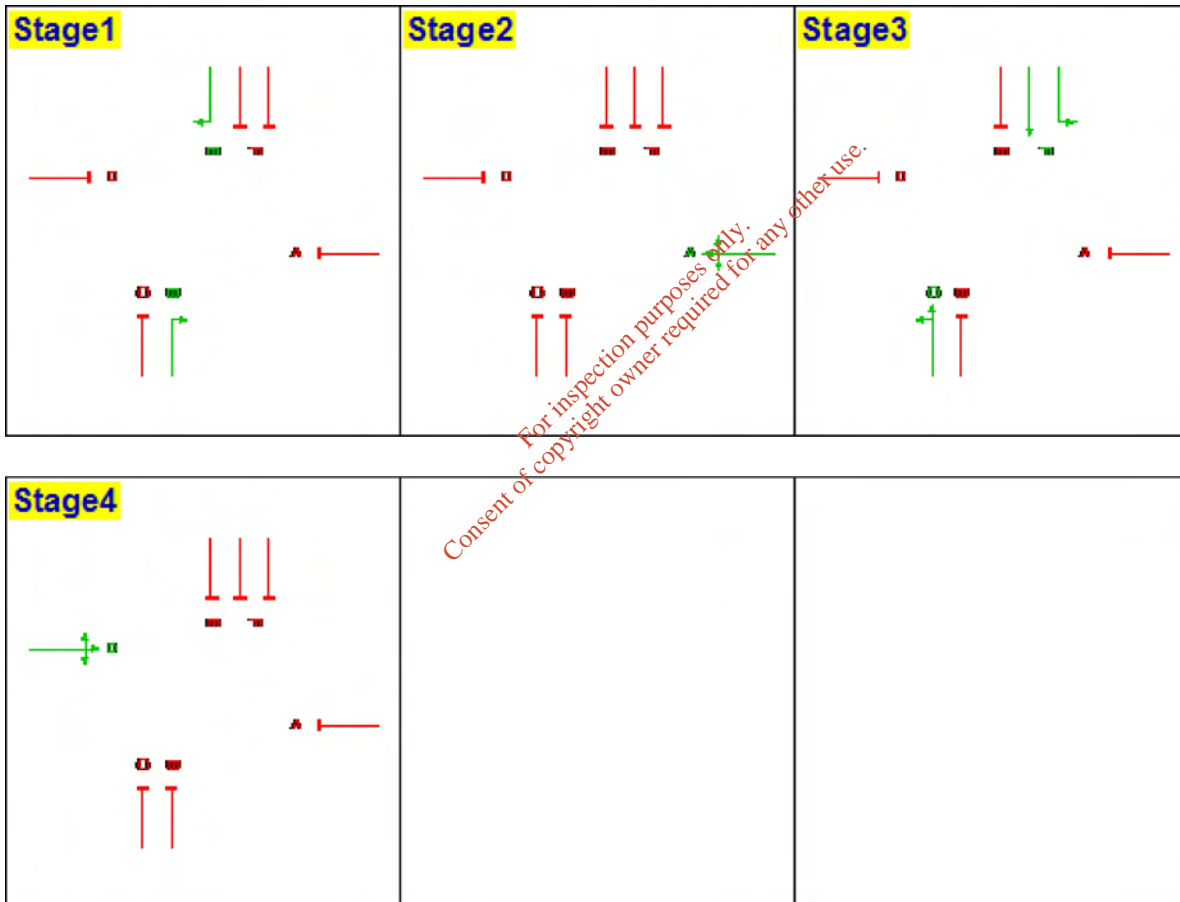
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	Yes	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set3 - 2017 AM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	102	333	94
	Arm 2	89	-	226	84
	Arm 3	164	112	-	11
	Arm 4	236	144	30	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	397	474	580	580	474	397
2 - R135 South	1	C	233	279	341	341	279	233
2 - R135 South	2	B	66	79	96	96	79	66
3 - L3125 West	1	D	215	257	315	315	257	215
4 - R135 North	1	F	286	341	418	418	341	286
4 - R135 North	2	E	22	26	31	31	26	22

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	19	63	18
2 - R135 South	57	21	22
3 - L3125 West	4	57	39
4 - R135 North	58	35	7

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	23.50	20.75	20.75	68.1

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	3.06	22.16	22.16	65.60

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

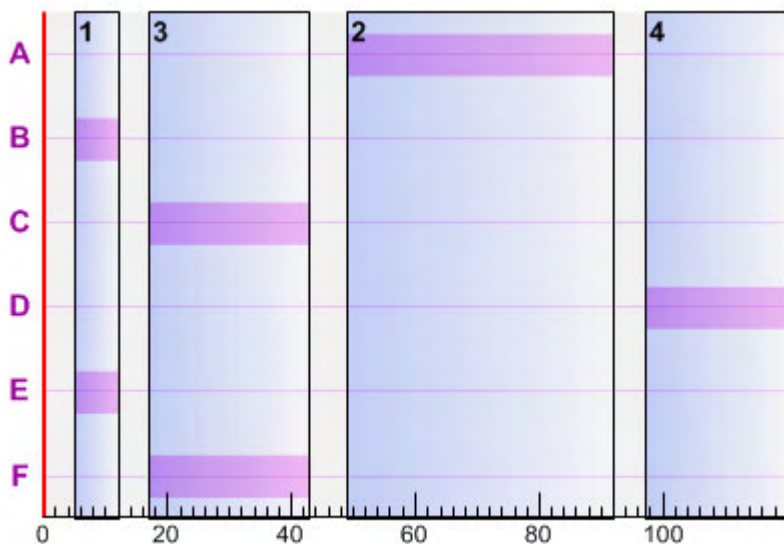
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	26.0	43.0
2	49.0	43.0	92.0
4	97.0	23.0	0.0

Phase Timings

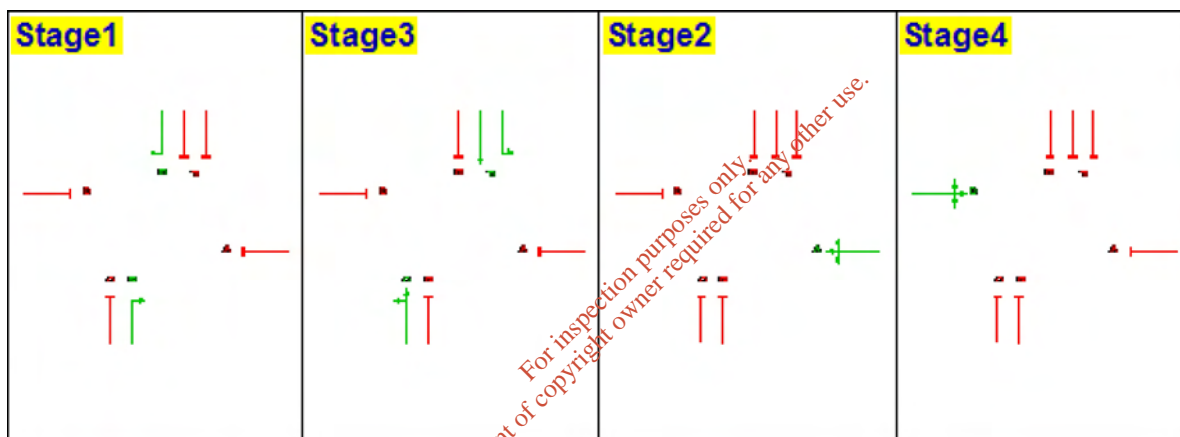
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	49	43.0	92						
B	5	7.0	12						
C	17	26.0	43						
D	97	23.0	0						
E	5	7.0	12						
F	17	26.0	43						

For inspection purposes only. Consent of copyright owner required for any other use.

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	484	A	44.50	42.00	5.65	72.87	23.50	1.42	12.66	11.24	26.30
2	1	284	C	27.50	57.67	4.55	71.88	25.20	1.27	9.01	7.74	10.20
2	2	80	B	8.50	80.64	1.79	60.46	48.86	0.61	3.13	2.52	1.70
3	1	262	D	24.50	60.16	4.38	71.33	26.17	1.22	8.55	7.33	8.80
4	1	348	F	27.50	41.09	3.97	40.12	124.33	0.19	9.80	9.61	20.30
4	2	26	E	8.50	56.91	0.41	19.65	358.02	0.03	0.84	0.81	0.80

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	397	1	44.50	35.62	3.93	59.77	50.57	0.62	9.67	9.04	4.70
2	1	233	3	27.50	49.35	3.19	58.97	52.61	0.57	6.85	6.28	1.80
2	2	66	2	8.50	69.75	1.28	49.88	80.43	0.31	2.39	2.07	0.30
3	1	215	4	24.50	51.72	3.09	58.54	53.75	0.55	6.51	5.96	1.50
4	1	286	6	27.50	39.90	3.17	32.97	172.96	0.11	7.91	7.79	3.00
4	2	22	5	8.50	55.94	0.34	16.63	441.30	0.02	0.71	0.68	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	474	1	44.50	40.82	5.37	71.37	26.11	1.24	12.23	10.98	4.40
2	1	279	3	27.50	56.02	4.34	70.62	27.45	1.11	8.71	7.59	1.70
2	2	79	2	8.50	78.42	1.72	59.71	50.74	0.54	3.03	2.49	0.30
3	1	257	4	24.50	58.39	4.17	69.97	28.62	1.06	8.25	7.19	1.50
4	1	341	6	27.50	40.95	3.88	39.31	128.93	0.18	9.58	9.40	3.30
4	2	26	5	8.50	56.89	0.41	19.65	358.02	0.03	0.84	0.81	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	580	1	44.50	54.50	8.78	87.33	3.06	3.73	17.49	13.76	3.30
2	1	341	3	27.50	72.42	6.86	86.31	4.27	3.02	12.42	9.40	1.40
2	2	96	2	8.50	93.28	2.49	72.55	24.05	1.07	4.11	3.03	0.30
3	1	315	4	24.50	74.87	6.55	85.76	4.94	2.85	11.77	8.92	1.20
4	1	418	6	27.50	42.60	4.95	48.19	86.76	0.32	12.04	11.72	3.70
4	2	31	5	8.50	58.11	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	580	1	44.50	57.89	9.33	87.33	3.06	3.95	17.71	13.76	3.30
2	1	341	3	27.50	78.11	7.40	86.31	4.27	3.26	12.66	9.40	1.40
2	2	96	2	8.50	98.43	2.62	72.55	24.05	1.13	4.17	3.03	0.30
3	1	315	4	24.50	80.68	7.06	85.76	4.94	3.08	11.99	8.92	1.20
4	1	418	6	27.50	42.62	4.95	48.19	86.76	0.32	12.04	11.72	3.70
4	2	31	5	8.50	58.15	0.50	23.43	284.15	0.05	1.01	0.97	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	474	1	44.50	42.56	5.60	71.37	26.11	1.35	12.33	10.98	4.40
2	1	279	3	27.50	59.88	4.64	70.62	27.45	1.26	8.86	7.59	1.70
2	2	79	2	8.50	84.19	1.85	59.71	50.74	0.65	3.13	2.49	0.30
3	1	257	4	24.50	62.42	4.46	69.97	28.62	1.21	8.40	7.19	1.50
4	1	341	6	27.50	40.97	3.88	39.31	128.93	0.18	9.58	9.40	3.30
4	2	26	5	8.50	56.95	0.41	19.65	358.02	0.03	0.84	0.81	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	397	1	44.50	36.02	3.97	59.77	50.57	0.65	9.69	9.04	4.70
2	1	233	3	27.50	50.38	3.26	58.97	52.61	0.61	6.88	6.28	1.80
2	2	66	2	8.50	73.08	1.34	49.88	80.43	0.35	2.42	2.07	0.30
3	1	215	4	24.50	52.84	3.16	58.54	53.75	0.59	6.55	5.96	1.50
4	1	286	6	27.50	39.92	3.17	32.97	172.96	0.11	7.91	7.79	3.00
4	2	22	5	8.50	56.02	0.34	16.63	441.30	0.02	0.71	0.68	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads.osc
Report generation date: 05/08/2016 16:07:00

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

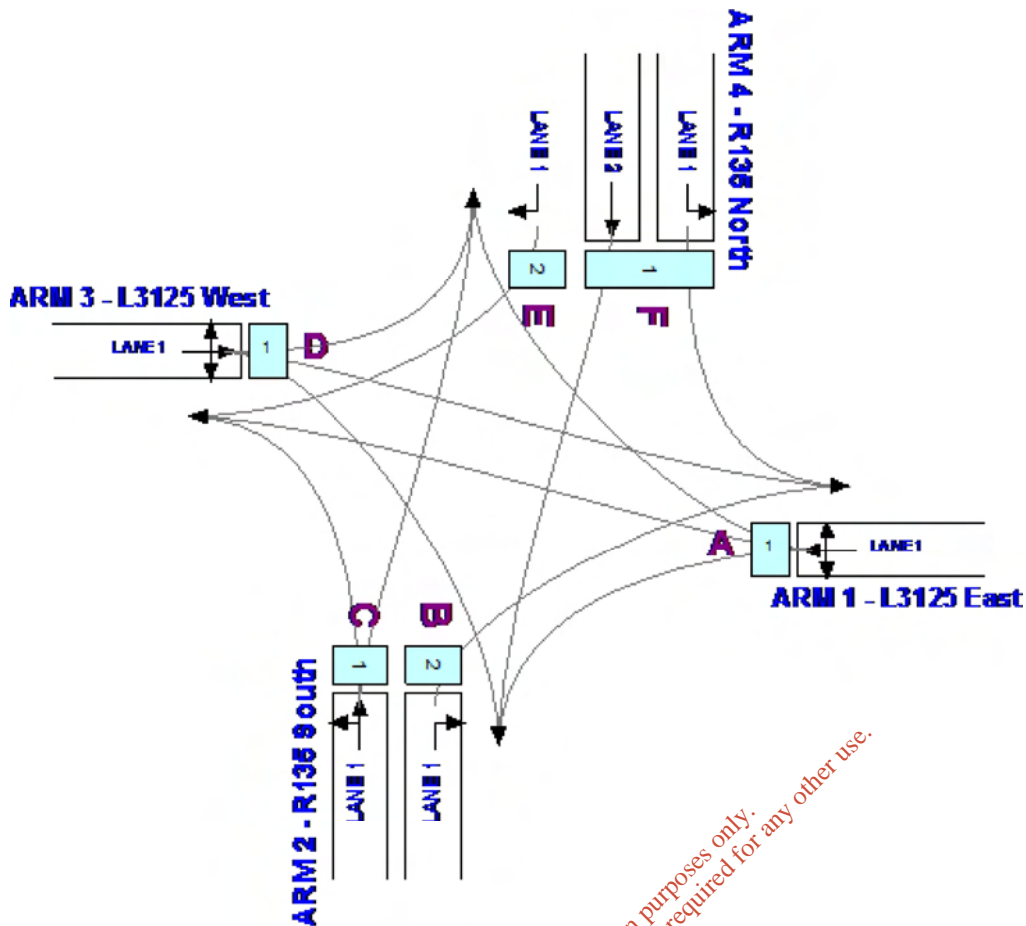
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

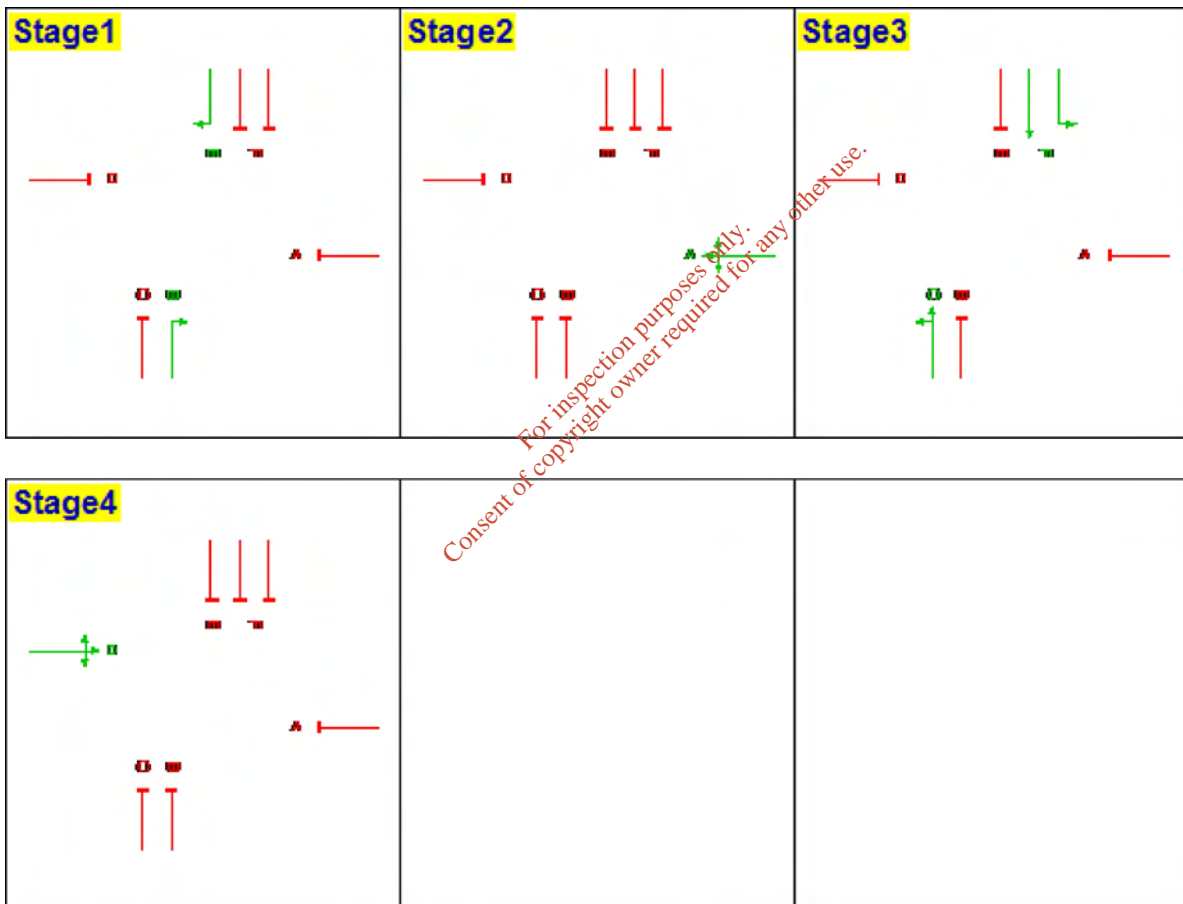
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	Yes	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set4 - 2023 AM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	106	348	98
	Arm 2	92	-	235	81
	Arm 3	171	117	-	12
	Arm 4	246	144	32	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	414	494	605	605	494	414
2 - R135 South	1	C	239	285	349	349	285	239
2 - R135 South	2	B	70	84	103	103	84	70
3 - L3125 West	1	D	225	269	329	329	269	225
4 - R135 North	1	F	291	348	426	426	348	291
4 - R135 North	2	E	25	30	37	37	30	25

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	19	63	18
2 - R135 South	58	20	23
3 - L3125 West	4	57	39
4 - R135 North	58	34	8

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	15.72	22.50	22.50	68.2

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-3.62	24.54	24.54	65.20

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

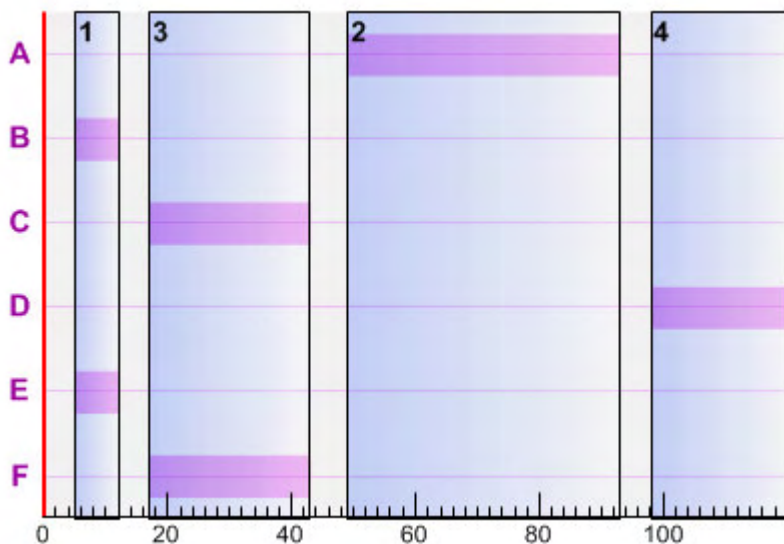
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	26.0	43.0
2	49.0	44.0	93.0
4	98.0	22.0	0.0

Phase Timings

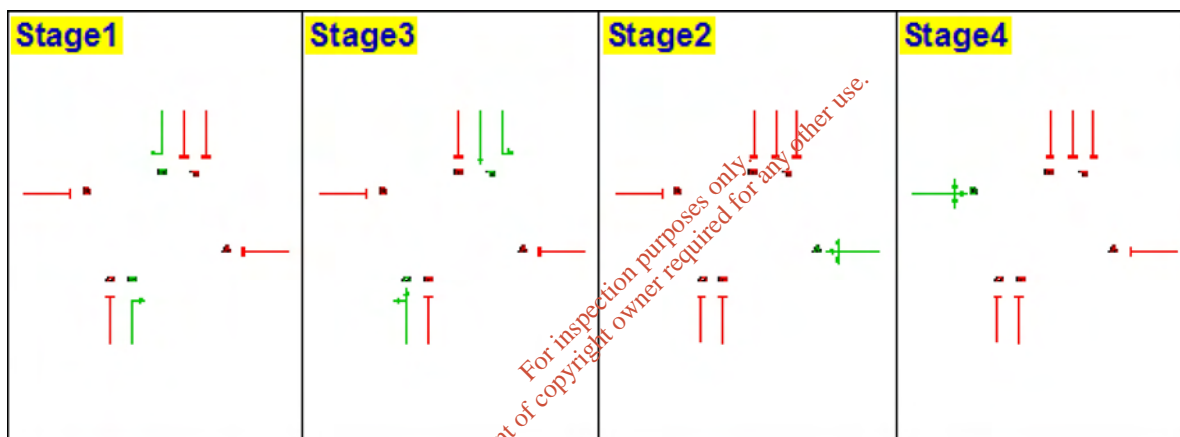
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	49	44.0	93						
B	5	7.0	12						
C	17	26.0	43						
D	98	22.0	0						
E	5	7.0	12						
F	17	26.0	43						

For inspection purposes only. Consent of copyright owner required for any other use.

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	504	A	45.50	42.08	5.89	74.22	21.27	1.55	13.15	11.60	27.10
2	1	291	C	27.50	59.23	4.79	73.66	22.19	1.42	9.36	7.94	10.10
2	2	86	B	8.50	86.16	2.06	65.00	38.47	0.79	3.50	2.71	1.80
3	1	274	D	23.50	68.32	5.20	77.77	15.72	1.86	9.63	7.77	7.80
4	1	355	F	27.50	41.24	4.07	40.93	119.90	0.20	10.02	9.82	20.50
4	2	31	E	8.50	58.14	0.50	23.43	284.15	0.05	1.01	0.97	0.90

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	414	1	45.50	35.32	4.06	60.96	47.63	0.67	10.01	9.34	4.90
2	1	239	3	27.50	50.00	3.32	60.49	48.78	0.62	7.07	6.45	1.80
2	2	70	2	8.50	71.63	1.39	52.90	70.12	0.37	2.57	2.20	0.30
3	1	225	4	23.50	55.23	3.45	63.87	40.92	0.74	7.06	6.32	1.40
4	1	291	6	27.50	39.99	3.23	33.55	168.27	0.12	8.06	7.94	3.00
4	2	25	5	8.50	56.60	0.39	18.89	376.34	0.03	0.81	0.78	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	494	1	45.50	40.82	5.60	72.74	23.72	1.36	12.70	11.34	4.60
2	1	285	3	27.50	57.13	4.52	72.14	24.76	1.22	8.98	7.77	1.70
2	2	84	2	8.50	82.11	1.92	63.48	41.77	0.67	3.31	2.65	0.30
3	1	269	4	23.50	64.92	4.85	76.35	17.87	1.55	9.17	7.62	1.30
4	1	348	6	27.50	41.09	3.97	40.12	124.33	0.19	9.80	9.61	3.40
4	2	30	5	8.50	57.87	0.48	22.67	296.95	0.04	0.98	0.93	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	605	1	45.50	56.09	9.43	89.09	1.02	4.34	18.58	14.24	3.20
2	1	349	3	27.50	75.68	7.34	88.34	1.88	3.49	13.13	9.64	1.30
2	2	103	2	8.50	101.40	2.90	77.84	15.62	1.42	4.68	3.26	0.30
3	1	329	4	23.50	91.08	8.32	93.39	-3.62	4.86	14.30	9.44	1.00
4	1	426	6	27.50	42.79	5.06	49.11	83.25	0.34	12.30	11.97	3.70
4	2	37	5	8.50	59.71	0.61	27.96	221.85	0.07	1.23	1.15	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	605	1	45.50	60.55	10.18	89.09	1.02	4.66	18.90	14.24	3.20
2	1	349	3	27.50	83.30	8.08	88.34	1.88	3.83	13.47	9.64	1.30
2	2	103	2	8.50	110.15	3.15	77.84	15.62	1.54	4.80	3.26	0.30
3	1	329	4	23.50	107.80	9.85	93.39	-3.62	5.75	15.19	9.44	1.00
4	1	426	6	27.50	42.81	5.07	49.11	83.25	0.34	12.30	11.97	3.70
4	2	37	5	8.50	59.78	0.61	27.96	221.85	0.07	1.23	1.15	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	494	1	45.50	43.00	5.90	72.74	23.72	1.49	12.83	11.34	4.60
2	1	285	3	27.50	62.20	4.92	72.14	24.76	1.40	9.17	7.77	1.70
2	2	84	2	8.50	91.58	2.14	63.48	41.77	0.82	3.47	2.65	0.30
3	1	269	4	23.50	78.36	5.86	76.35	17.87	1.91	9.53	7.62	1.30
4	1	348	6	27.50	41.11	3.97	40.12	124.33	0.19	9.80	9.61	3.40
4	2	30	5	8.50	57.96	0.48	22.67	296.95	0.04	0.98	0.93	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	414	1	45.50	35.76	4.11	60.96	47.63	0.70	10.04	9.34	4.90
2	1	239	3	27.50	51.21	3.40	60.49	48.78	0.66	7.11	6.45	1.80
2	2	70	2	8.50	76.22	1.48	52.90	70.12	0.42	2.62	2.20	0.30
3	1	225	4	23.50	57.66	3.60	63.87	40.92	0.82	7.13	6.32	1.40
4	1	291	6	27.50	40.01	3.23	33.55	168.27	0.12	8.06	7.94	3.00
4	2	25	5	8.50	56.72	0.39	18.89	376.34	0.03	0.81	0.78	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads.osc
Report generation date: 05/08/2016 16:07:38

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

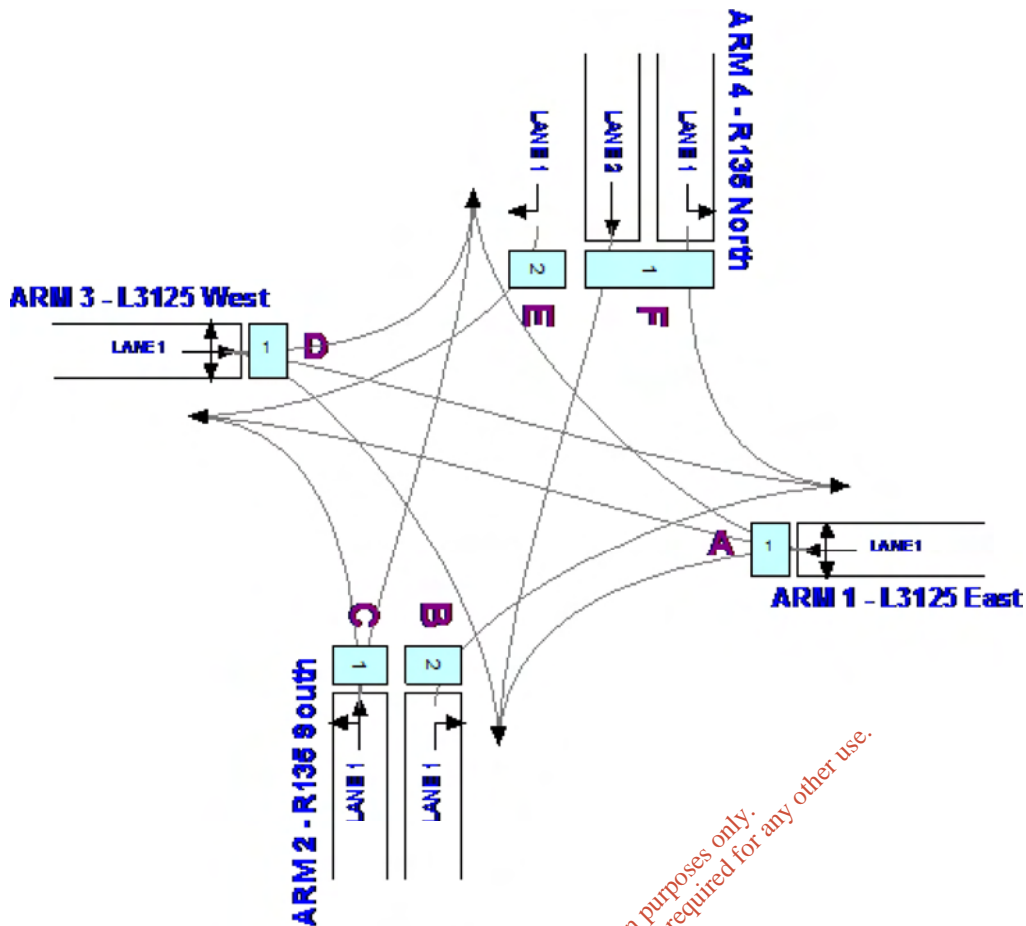
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

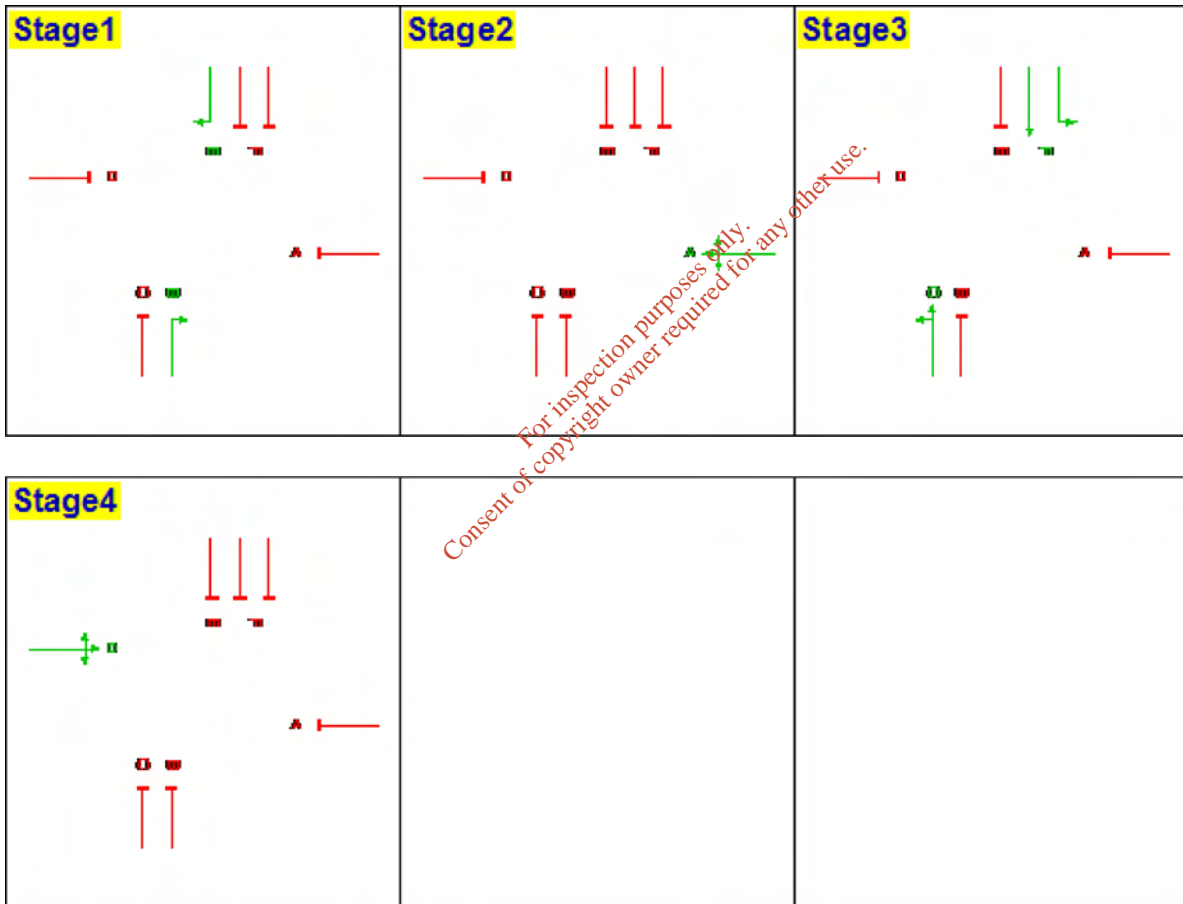
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	Yes	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set5 - 2023 AM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	106	348	98
	Arm 2	92	-	235	87
	Arm 3	171	117	-	12
	Arm 4	246	150	32	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	414	494	605	605	494	414
2 - R135 South	1	C	242	289	354	354	289	242
2 - R135 South	2	B	68	82	100	100	82	68
3 - L3125 West	1	D	225	269	329	329	269	225
4 - R135 North	1	F	295	353	432	432	353	295
4 - R135 North	2	E	22	27	33	33	27	22

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	19	63	18
2 - R135 South	57	21	22
3 - L3125 West	4	57	39
4 - R135 North	57	35	7

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	18.60	22.40	22.40	67.1

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-1.20	24.45	24.45	64.30

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

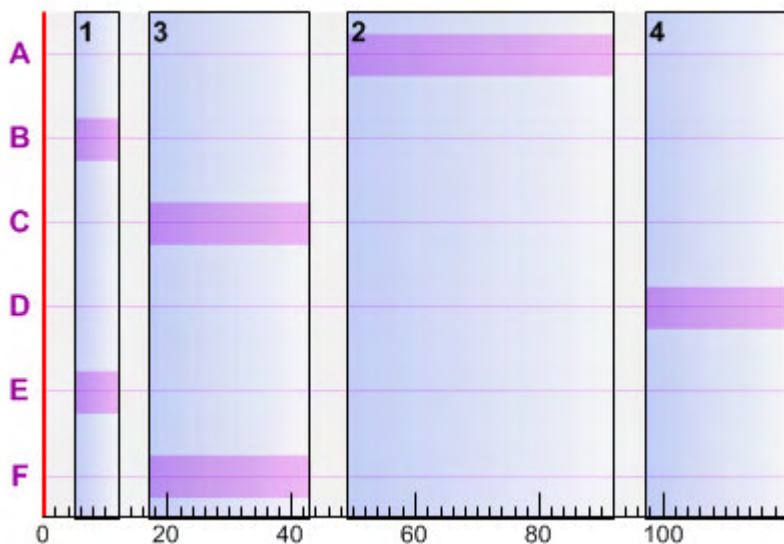
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	26.0	43.0
2	49.0	43.0	92.0
4	97.0	23.0	0.0

Phase Timings

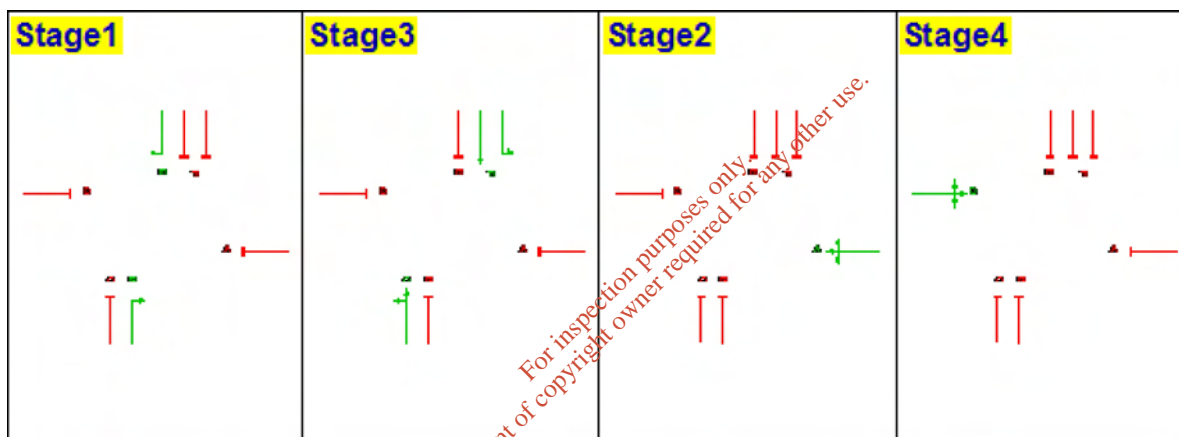
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	49	43.0	92						
B	5	7.0	12						
C	17	26.0	43						
D	97	23.0	0						
E	5	7.0	12						
F	17	26.0	43						

For inspection purposes only. Consent of copyright owner required for any other use.

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	504	A	44.50	44.08	6.17	75.89	18.60	1.73	13.49	11.75	25.40
2	1	295	C	27.50	60.19	4.93	74.67	20.53	1.52	9.57	8.06	10.00
2	2	83	B	8.50	83.26	1.92	62.73	43.48	0.69	3.31	2.61	1.70
3	1	274	D	24.50	63.20	4.81	74.60	20.64	1.50	9.19	7.69	8.60
4	1	360	F	27.50	41.34	4.13	41.50	116.85	0.21	10.18	9.97	20.60
4	2	27	E	8.50	57.15	0.43	20.41	341.06	0.03	0.88	0.84	0.80

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	414	1	44.50	36.55	4.20	62.33	44.38	0.73	10.19	9.47	4.70
2	1	242	3	27.50	50.33	3.38	61.25	46.93	0.65	7.18	6.53	1.80
2	2	68	2	8.50	70.67	1.33	51.39	75.12	0.34	2.48	2.14	0.30
3	1	225	4	24.50	52.92	3.31	61.26	46.92	0.64	6.89	6.25	1.50
4	1	295	6	27.50	40.07	3.28	34.01	164.63	0.12	8.18	8.06	3.10
4	2	22	5	8.50	55.94	0.34	16.63	441.30	0.02	0.71	0.68	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	494	1	44.50	42.59	5.84	74.38	21.00	1.50	13.00	11.50	4.30
2	1	289	3	27.50	57.89	4.65	73.15	23.04	1.29	9.18	7.88	1.70
2	2	82	2	8.50	80.58	1.84	61.97	45.23	0.61	3.20	2.58	0.30
3	1	269	4	24.50	60.86	4.55	73.24	22.89	1.29	8.83	7.54	1.40
4	1	353	6	27.50	41.19	4.04	40.70	121.15	0.20	9.96	9.76	3.40
4	2	27	5	8.50	57.12	0.43	20.41	341.06	0.03	0.88	0.84	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	605	1	44.50	60.26	10.13	91.09	-1.20	5.11	19.54	14.43	2.90
2	1	354	3	27.50	77.96	2.67	89.60	0.44	3.83	13.61	9.79	1.30
2	2	100	2	8.50	97.80	2.72	75.58	19.09	1.26	4.42	3.16	0.30
3	1	329	4	24.50	81.42	7.44	89.57	0.48	3.73	13.08	9.34	1.10
4	1	432	6	27.50	42.94	5.15	49.80	80.71	0.35	12.50	12.15	3.70
4	2	33	5	8.50	58.62	0.54	24.94	260.86	0.05	1.08	1.03	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	605	1	44.50	66.75	11.22	91.09	-1.20	5.62	20.06	14.43	2.80
2	1	354	3	27.50	87.03	8.56	89.60	0.44	4.26	14.05	9.79	1.30
2	2	100	2	8.50	104.65	2.91	75.58	19.09	1.35	4.51	3.16	0.30
3	1	329	4	24.50	91.16	8.33	89.57	0.48	4.17	13.52	9.34	1.10
4	1	432	6	27.50	42.96	5.15	49.80	80.71	0.35	12.50	12.15	3.70
4	2	33	5	8.50	58.68	0.54	24.94	260.86	0.05	1.08	1.03	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	494	1	44.50	45.77	6.28	74.38	21.00	1.67	13.16	11.50	4.30
2	1	289	3	27.50	64.02	5.14	73.15	23.04	1.51	9.39	7.88	1.70
2	2	82	2	8.50	88.22	2.01	61.97	45.23	0.74	3.33	2.58	0.30
3	1	269	4	24.50	67.79	5.07	73.24	22.89	1.52	9.06	7.54	1.40
4	1	353	6	27.50	41.21	4.04	40.70	121.15	0.20	9.96	9.76	3.40
4	2	27	5	8.50	57.21	0.43	20.41	341.06	0.04	0.88	0.84	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	414	1	44.50	37.09	4.27	62.33	44.38	0.76	10.23	9.47	4.70
2	1	242	3	27.50	51.68	3.47	61.25	46.93	0.70	7.23	6.53	1.80
2	2	68	2	8.50	74.70	1.41	51.39	75.12	0.38	2.52	2.14	0.30
3	1	225	4	24.50	54.49	3.41	61.26	46.92	0.70	6.95	6.25	1.50
4	1	295	6	27.50	40.08	3.28	34.01	164.63	0.12	8.18	8.06	3.10
4	2	22	5	8.50	56.02	0.34	16.63	441.30	0.02	0.71	0.68	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads With Link Rd.osc
Report generation date: 05/08/2016 16:25:07

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

For inspection purposes only.
Consent of copyright owner required for any other use.

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

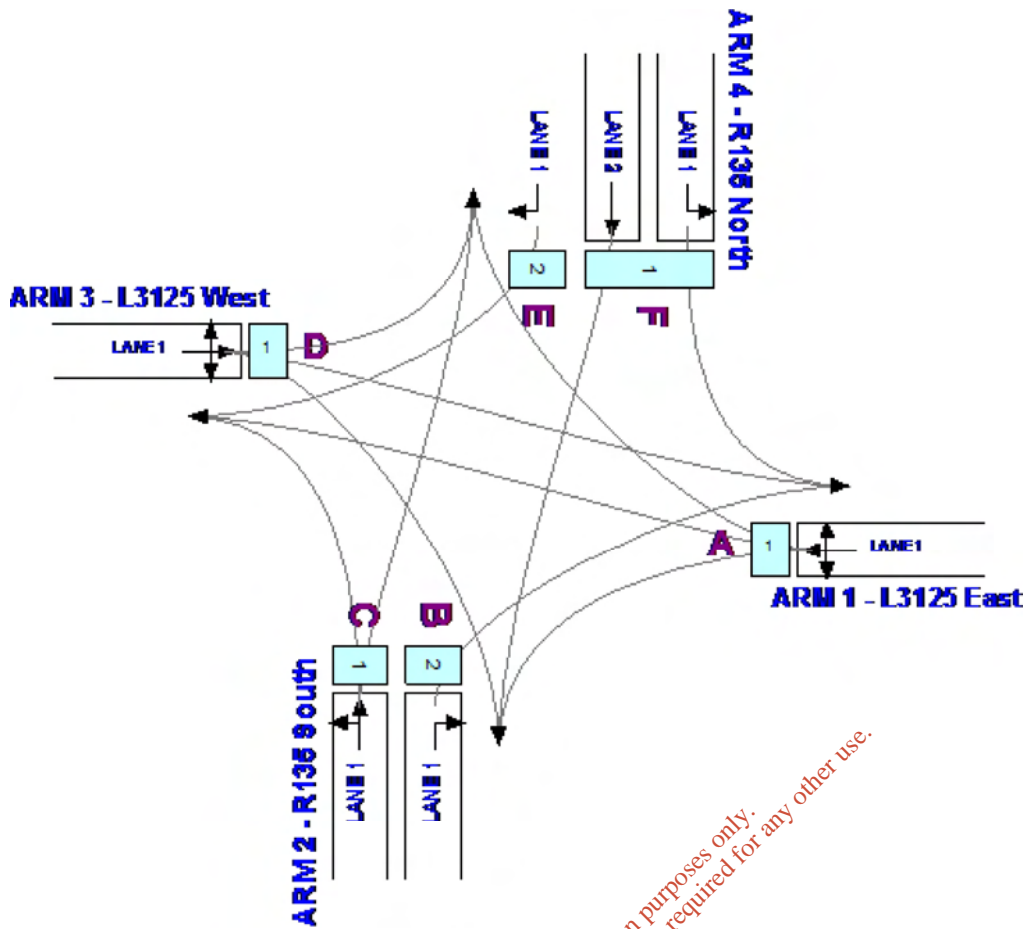
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

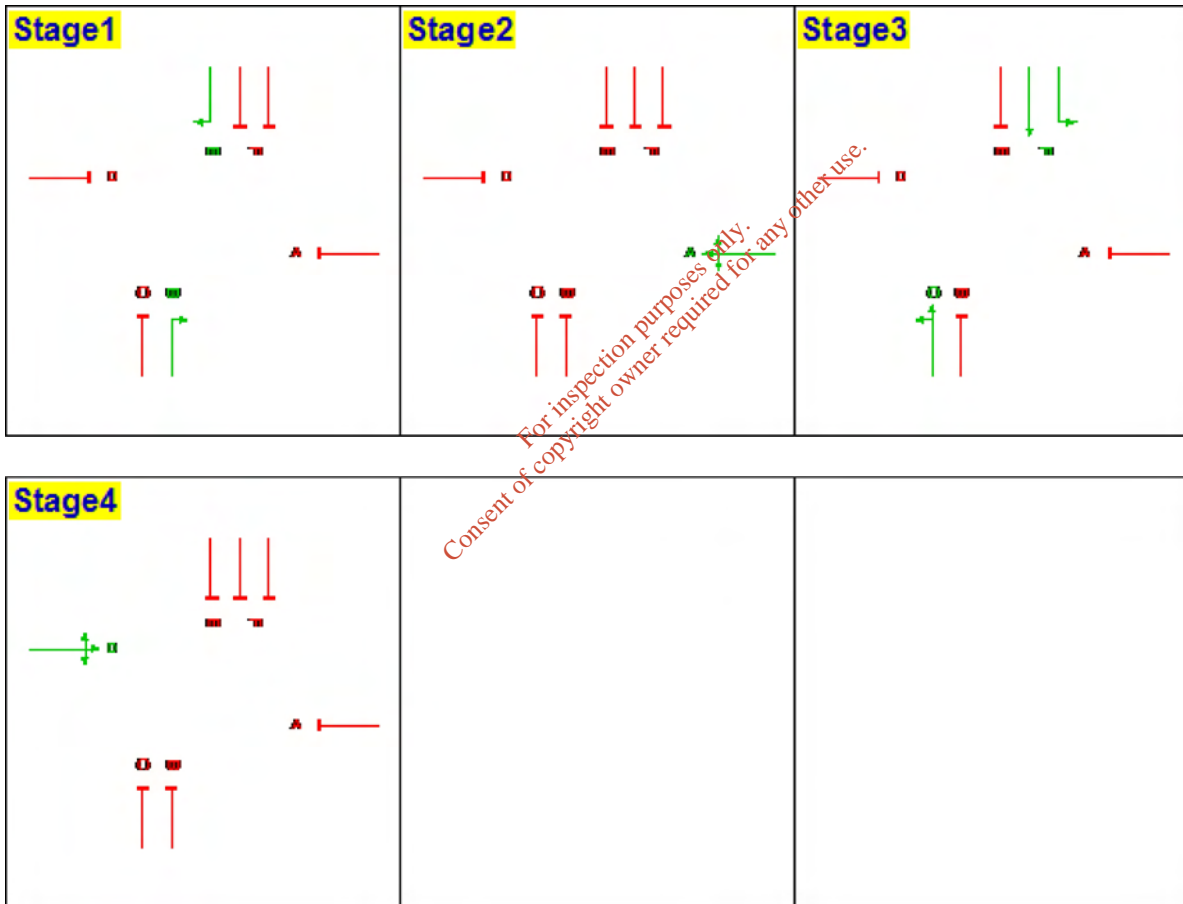
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	Yes	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set5 - 2023 AM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	106	348	48
	Arm 2	92	-	235	87
	Arm 3	171	117	-	12
	Arm 4	123	150	32	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	377	450	551	551	450	377
2 - R135 South	1	C	242	289	354	354	289	242
2 - R135 South	2	B	68	82	100	100	82	68
3 - L3125 West	1	D	225	269	329	329	269	225
4 - R135 North	1	F	204	243	298	298	243	204
4 - R135 North	2	E	23	27	33	33	27	23

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	21	69	10
2 - R135 South	57	21	22
3 - L3125 West	4	57	39
4 - R135 North	40	49	10

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	24.37	19.68	19.68	63.8

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	3.61	21.08	21.08	61.40

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

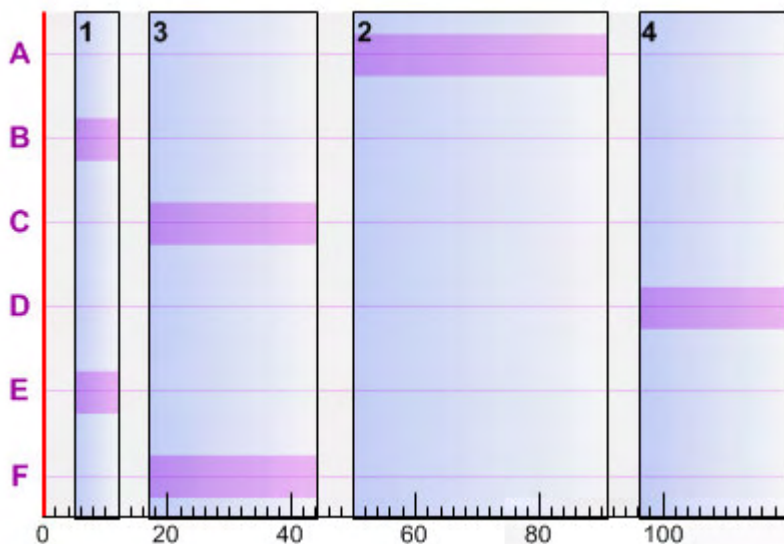
Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	27.0	44.0
2	50.0	41.0	91.0
4	96.0	24.0	0.0

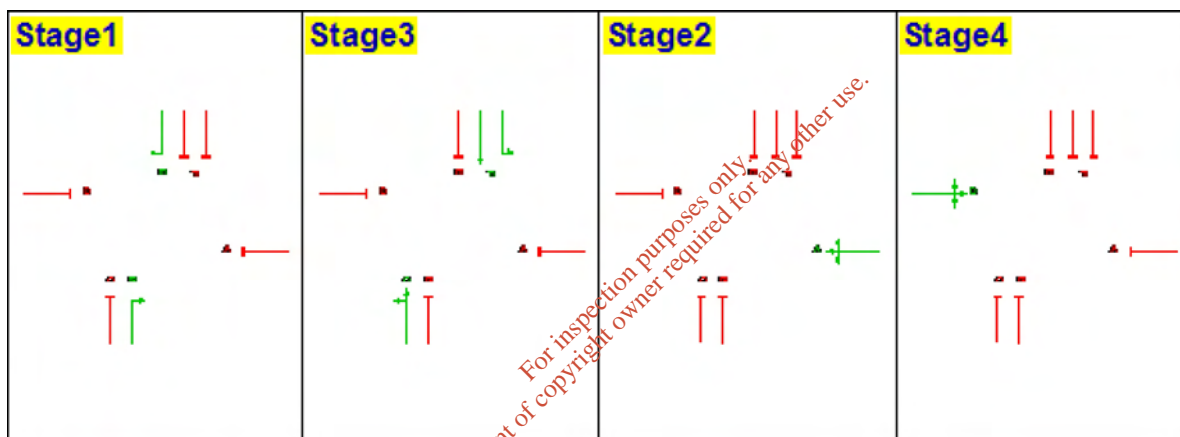
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	50	41.0	91						
B	5	7.0	12						
C	17	27.0	44						
D	96	24.0	0						
E	5	7.0	12						
F	17	27.0	44						

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	459	A	42.50	43.30	5.52	72.36	24.37	1.37	12.25	10.88	24.10
2	1	295	C	28.50	56.68	4.64	72.05	24.92	1.29	9.25	7.97	10.90
2	2	83	B	8.50	83.26	1.92	62.73	43.48	0.69	3.31	2.61	1.70
3	1	274	D	25.50	59.22	4.51	71.67	25.57	1.25	8.86	7.61	9.40
4	1	248	F	28.50	38.34	2.64	27.59	226.23	0.07	6.71	6.63	16.90
4	2	28	E	8.50	57.39	0.45	21.16	325.30	0.04	0.91	0.87	0.80

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	377	1	42.50	36.97	3.87	59.43	51.43	0.61	9.39	8.78	4.20
2	1	242	3	28.50	48.48	3.26	59.10	52.27	0.57	7.04	6.46	1.90
2	2	68	2	8.50	70.67	1.33	51.39	75.12	0.34	2.48	2.14	0.30
3	1	225	4	25.50	50.87	3.18	58.86	52.91	0.56	6.75	6.18	1.60
4	1	204	6	28.50	37.64	2.13	22.69	296.59	0.05	5.45	5.41	2.40
4	2	23	5	8.50	56.16	0.36	17.38	417.76	0.02	0.74	0.72	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	450	1	42.50	42.16	5.27	70.94	26.86	1.20	11.85	10.65	4.10
2	1	289	3	28.50	54.94	4.41	70.58	27.51	1.12	8.91	7.80	1.80
2	2	82	2	8.50	80.58	1.84	61.97	45.23	0.61	3.20	2.58	0.30
3	1	269	4	25.50	57.52	4.30	70.37	27.90	1.09	8.56	7.46	1.60
4	1	243	6	28.50	38.25	2.58	27.03	232.94	0.07	6.56	6.49	2.80
4	2	27	5	8.50	57.12	0.43	20.41	341.06	0.03	0.88	0.84	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	551	1	42.50	55.74	8.53	86.87	3.61	3.55	16.88	13.33	3.10
2	1	354	3	28.50	71.27	7.01	86.46	4.10	3.08	12.76	9.68	1.50
2	2	100	2	8.50	97.80	2.72	75.58	19.09	1.26	4.42	3.16	0.30
3	1	329	4	25.50	73.87	6.75	86.06	4.58	2.94	12.19	9.24	1.30
4	1	298	6	28.50	39.47	3.24	33.15	171.49	0.11	8.17	8.05	3.20
4	2	33	5	8.50	58.62	0.54	24.94	260.86	0.05	1.08	1.03	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	551	1	42.50	59.06	9.04	86.87	3.61	3.76	17.09	13.33	3.00
2	1	354	3	28.50	76.84	7.56	86.46	4.10	3.32	13.00	9.68	1.50
2	2	100	2	8.50	104.65	2.91	75.58	19.09	1.35	4.51	3.16	0.30
3	1	329	4	25.50	79.62	7.28	86.06	4.58	3.17	12.42	9.24	1.30
4	1	298	6	28.50	39.18	3.24	33.15	171.49	0.11	8.17	8.05	3.20
4	2	33	5	8.50	58.68	0.54	24.94	260.86	0.05	1.08	1.03	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	450	1	42.50	43.93	5.49	70.94	26.86	1.31	11.96	10.65	4.00
2	1	289	3	28.50	58.60	4.70	70.58	27.51	1.26	9.06	7.80	1.80
2	2	82	2	8.50	88.22	2.01	61.97	45.23	0.74	3.33	2.58	0.30
3	1	269	4	25.50	61.44	4.59	70.37	27.90	1.24	8.70	7.46	1.60
4	1	243	6	28.50	38.26	2.58	27.03	232.94	0.07	6.56	6.49	2.80
4	2	27	5	8.50	57.21	0.43	20.41	341.06	0.04	0.88	0.84	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	377	1	42.50	37.39	3.92	59.43	51.43	0.63	9.41	8.78	4.20
2	1	242	3	28.50	49.44	3.32	59.10	52.27	0.61	7.07	6.46	1.90
2	2	68	2	8.50	74.70	1.41	51.39	75.12	0.38	2.52	2.14	0.30
3	1	225	4	25.50	51.96	3.25	58.86	52.91	0.60	6.79	6.18	1.60
4	1	204	6	28.50	37.65	2.13	22.69	296.59	0.05	5.45	5.41	2.40
4	2	23	5	8.50	56.24	0.36	17.38	417.76	0.02	0.74	0.72	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads With Link Rd.osc
Report generation date: 04/08/2016 10:32:48

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

For inspection purposes only.
Consent of copyright owner required for any other use.

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

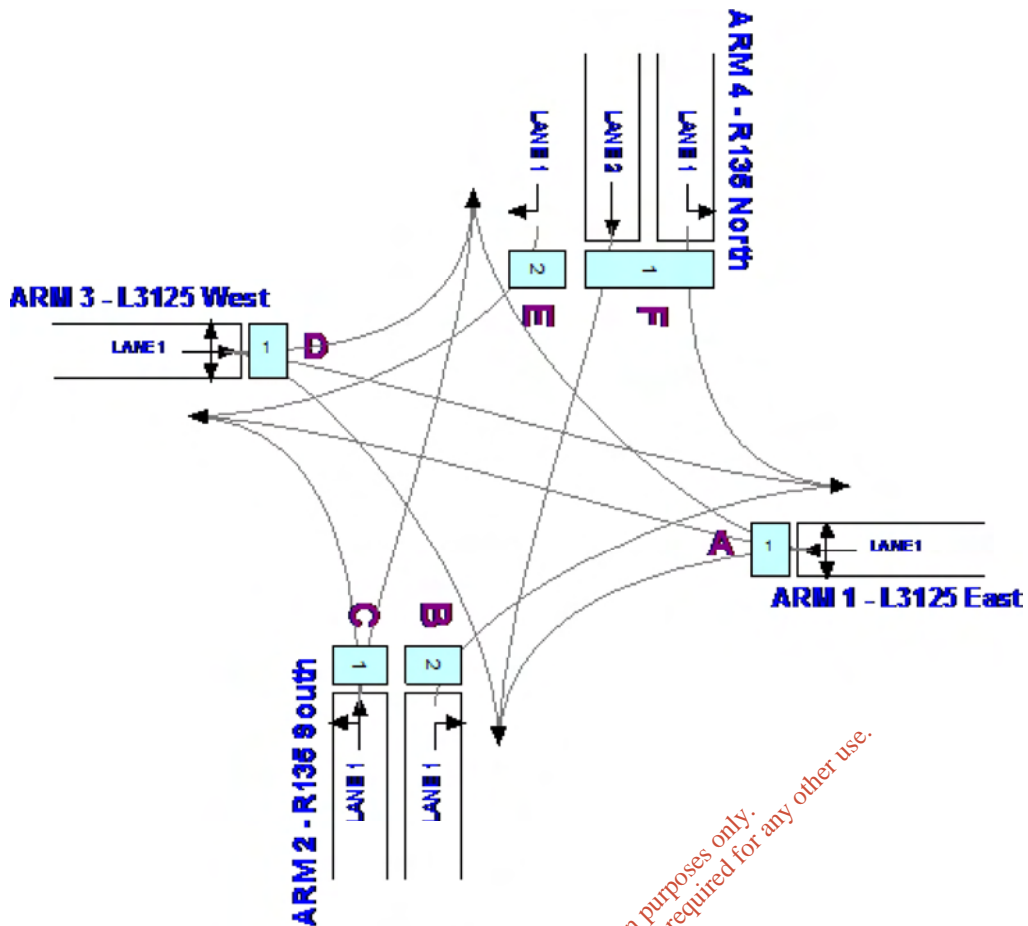
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

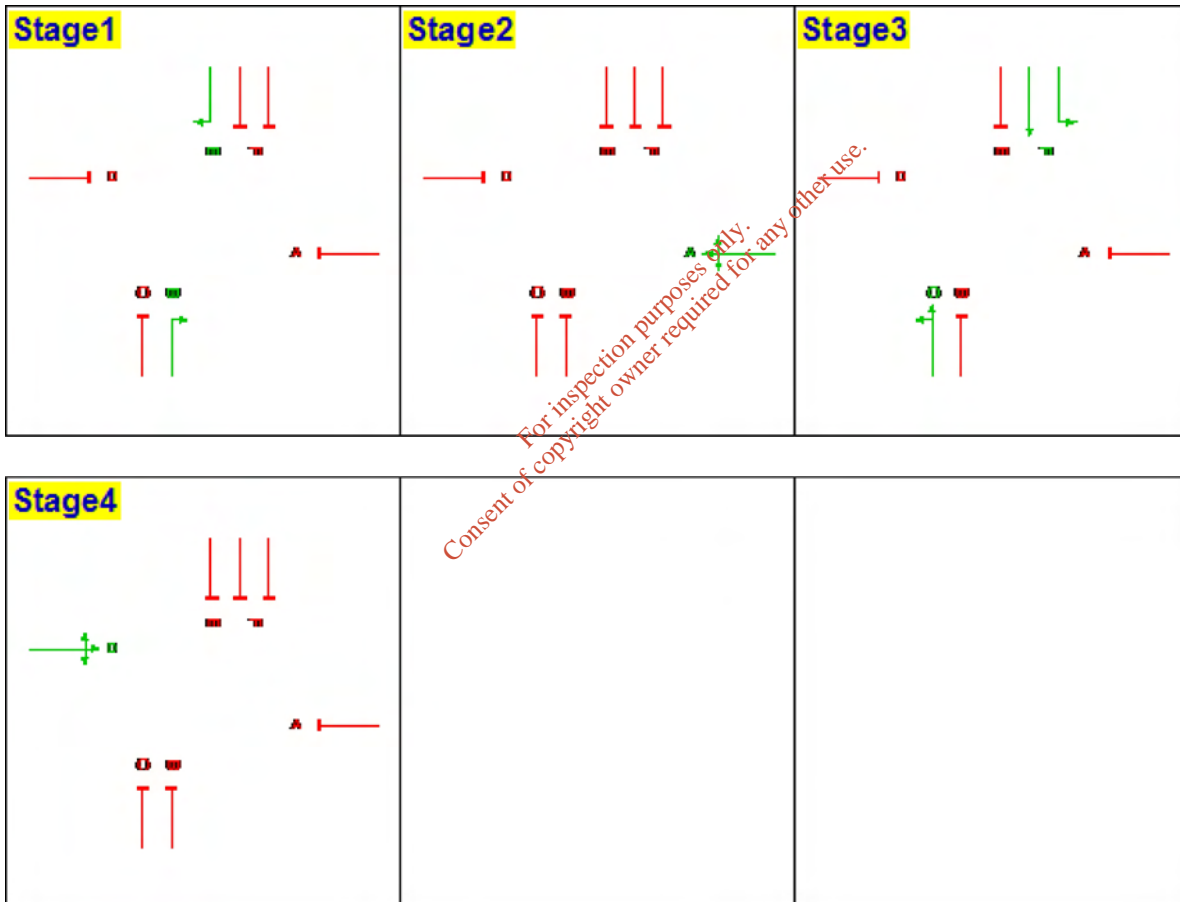
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	Yes	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set5 - 2023 AM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	106	348	48
	Arm 2	92	-	235	103
	Arm 3	171	117	-	12
	Arm 4	123	166	32	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - L3125 East	1	A	377	450	551	551	450	377
2 - R135 South	1	C	255	304	373	373	304	255
2 - R135 South	2	B	68	81	99	99	81	68
3 - L3125 West	1	D	225	269	329	329	269	225
4 - R135 North	1	F	217	259	317	317	259	217
4 - R135 North	2	E	24	29	35	35	29	24

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	21	69	10
2 - R135 South	55	24	21
3 - L3125 West	4	57	39
4 - R135 North	38	52	10

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	21.45	20.33	20.33	64.4

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	1.17	21.90	21.90	61.60

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

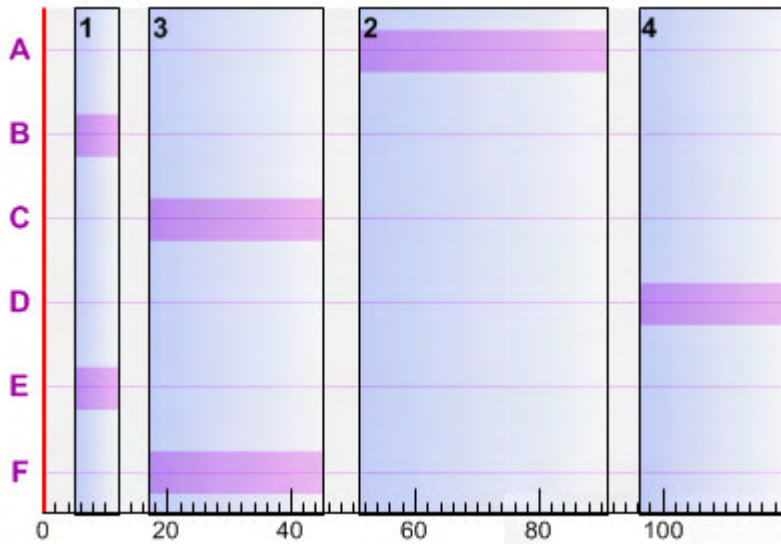
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	7.0	12.0
3	17.0	28.0	45.0
2	51.0	40.0	91.0
4	96.0	24.0	0.0

Phase Timings

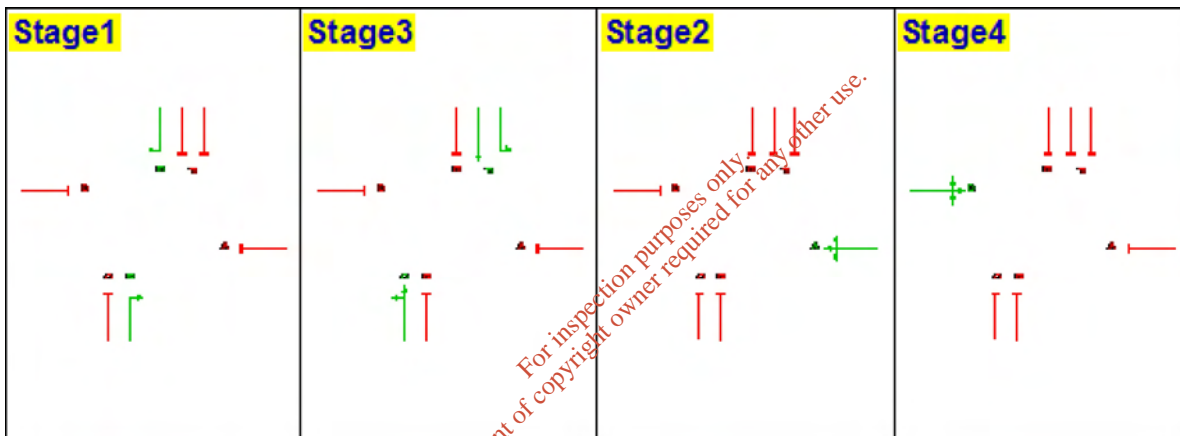
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	51	40.0	91						
B	5	7.0	12						
C	17	28.0	45						
D	96	24.0	0						
E	5	7.0	12						
F	17	28.0	45						

For inspection purposes only. Consent of copyright owner required for any other use.

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	459	A	41.50	45.29	5.77	74.11	21.45	1.53	12.55	11.02	22.50
2	1	311	C	29.50	56.69	4.90	73.38	22.65	1.40	9.74	8.34	11.50
2	2	83	B	8.50	83.26	1.92	62.73	43.48	0.69	3.31	2.61	1.70
3	1	274	D	25.50	59.22	4.51	71.67	25.57	1.25	8.86	7.61	9.40
4	1	264	F	29.50	37.70	2.76	28.37	217.21	0.08	7.08	7.01	18.40
4	2	29	E	8.50	57.64	0.46	21.92	310.64	0.04	0.94	0.90	0.90

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	377	1	41.50	38.23	4.00	60.87	47.86	0.66	9.55	8.89	4.00
2	1	255	3	29.50	48.01	3.40	60.17	49.58	0.61	7.37	6.75	2.00
2	2	68	2	8.50	70.67	1.33	51.39	75.12	0.34	2.48	2.14	0.30
3	1	225	4	25.50	50.87	3.18	58.86	52.91	0.56	6.75	6.18	1.60
4	1	217	6	29.50	36.98	2.23	23.32	285.91	0.05	5.75	5.70	2.60
4	2	24	5	8.50	56.38	0.38	18.14	396.19	0.03	0.77	0.75	0.10

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	450	1	41.50	43.95	5.49	72.65	23.88	1.33	12.12	10.78	3.80
2	1	304	3	29.50	54.70	4.62	71.73	25.47	1.20	9.34	8.14	1.90
2	2	81	2	8.50	79.92	1.80	61.22	47.02	0.59	3.14	2.55	0.30
3	1	269	4	25.50	57.52	4.30	70.37	27.90	1.09	8.56	7.46	1.60
4	1	259	6	29.50	37.62	2.71	27.84	223.33	0.07	6.94	6.87	3.00
4	2	29	5	8.50	57.59	0.46	21.92	310.64	0.04	0.94	0.90	0.10

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	551	1	41.50	59.68	9.13	88.96	1.17	4.17	17.67	13.50	2.70
2	1	373	3	29.50	72.37	7.50	88.01	2.26	3.47	13.61	10.13	1.50
2	2	99	2	8.50	96.58	2.66	74.82	20.29	1.21	4.34	3.13	0.30
3	1	329	4	25.50	73.87	6.75	86.06	4.58	2.94	12.19	9.24	1.30
4	1	317	6	29.50	38.56	3.40	34.07	164.17	0.12	8.63	8.51	3.50
4	2	35	5	8.50	59.17	0.58	26.45	240.24	0.06	1.15	1.09	0.20

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	551	1	41.50	64.56	9.88	88.96	1.17	4.50	18.00	13.50	2.70
2	1	373	3	29.50	79.09	8.19	88.01	2.26	3.79	13.92	10.13	1.50
2	2	99	2	8.50	103.04	2.83	74.82	20.29	1.29	4.42	3.13	0.30
3	1	329	4	25.50	79.62	7.28	86.06	4.58	3.17	12.42	9.24	1.30
4	1	317	6	29.50	38.56	3.40	34.07	164.17	0.12	8.63	8.51	3.50
4	2	35	5	8.50	59.24	0.58	26.45	240.24	0.06	1.15	1.09	0.20

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	450	1	41.50	46.48	5.81	72.65	23.88	1.47	12.25	10.78	3.80
2	1	304	3	29.50	58.97	4.98	71.73	25.47	1.36	9.50	8.14	1.90
2	2	81	2	8.50	86.95	1.96	61.22	47.02	0.71	3.26	2.55	0.30
3	1	269	4	25.50	61.44	4.59	70.37	27.90	1.24	8.70	7.46	1.60
4	1	259	6	29.50	37.62	2.71	27.84	223.33	0.07	6.94	6.87	3.00
4	2	29	5	8.50	57.72	0.46	21.92	310.64	0.04	0.94	0.90	0.10

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	377	1	41.50	38.75	4.06	60.87	47.86	0.69	9.58	8.89	4.00
2	1	255	3	29.50	49.02	3.47	60.17	49.58	0.65	7.41	6.75	2.00
2	2	68	2	8.50	74.51	1.41	51.39	75.12	0.38	2.52	2.14	0.30
3	1	225	4	25.50	51.96	3.25	58.86	52.91	0.60	6.79	6.18	1.60
4	1	217	6	29.50	36.98	2.23	23.32	285.91	0.05	5.75	5.70	2.60
4	2	24	5	8.50	56.49	0.38	18.14	396.19	0.03	0.77	0.75	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads.osc
Report generation date: 05/08/2016 16:08:33

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

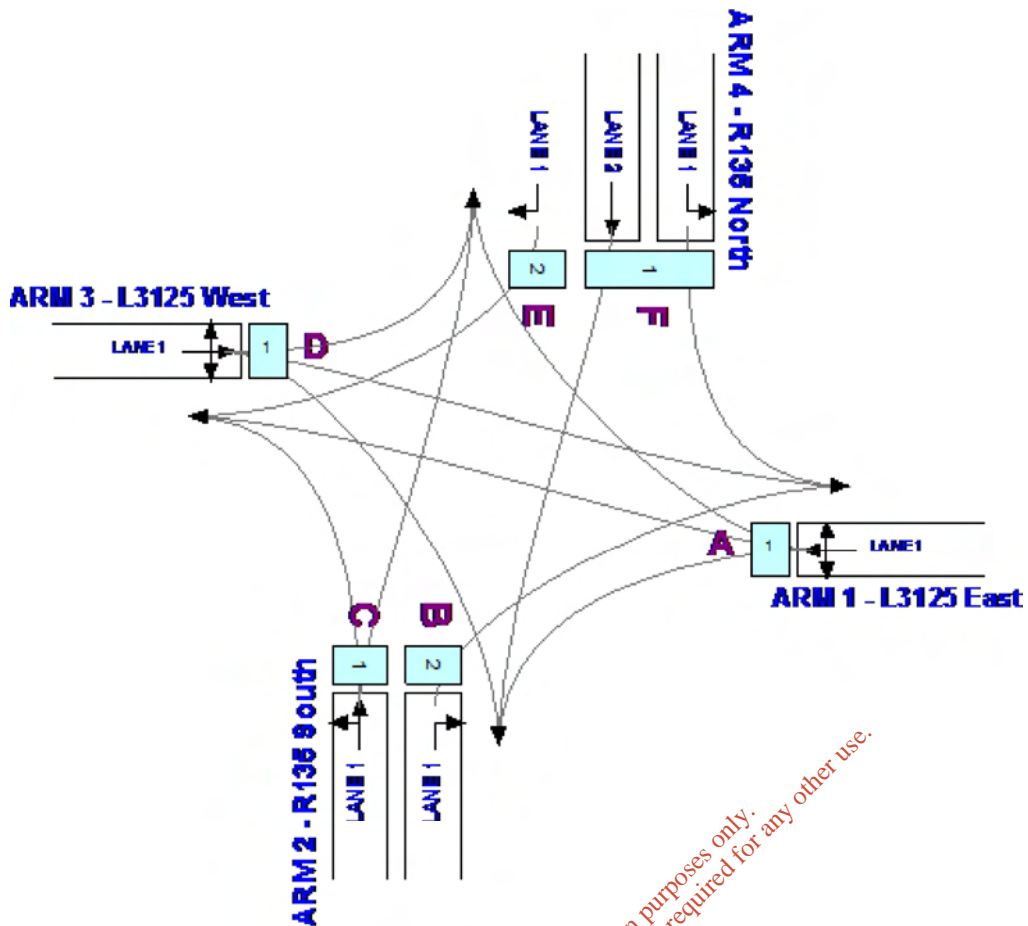
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

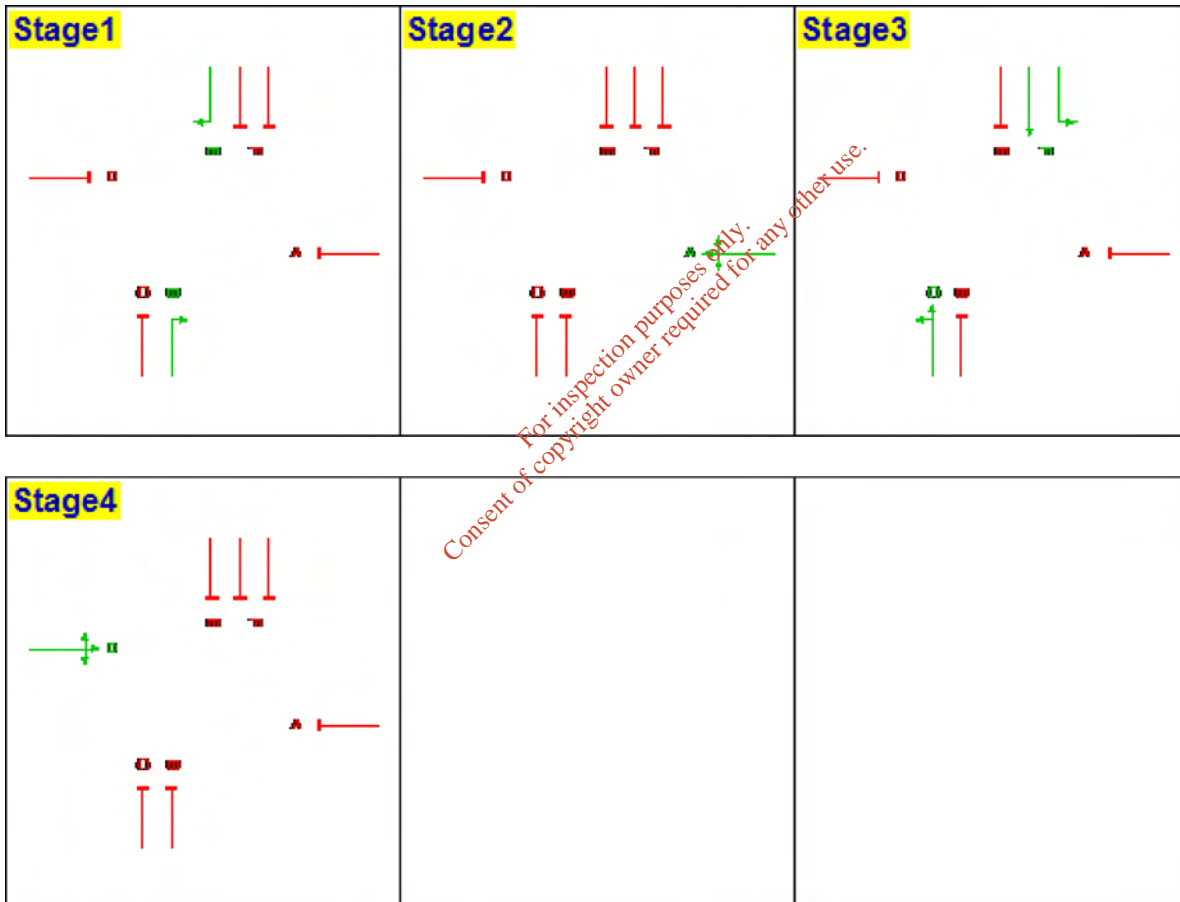
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	Yes	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set6 - 2016 PM Peak Existing

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	91	221	216
	Arm 2	136	-	135	127
	Arm 3	179	175	-	24
	Arm 4	177	97	18	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	396	473	579	579	473	396
2 - R135 South	1	C	197	235	288	288	235	197
2 - R135 South	2	B	101	121	148	148	121	101
3 - L3125 West	1	D	281	335	410	410	335	281
4 - R135 North	1	F	206	246	301	301	246	206
4 - R135 North	2	E	13	16	19	19	16	13

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	34	32	34
3 - L3125 West	6	47	46
4 - R135 North	61	33	6

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	15.83	22.96	22.96	57.3

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-3.47	25.46	25.46	54.40

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

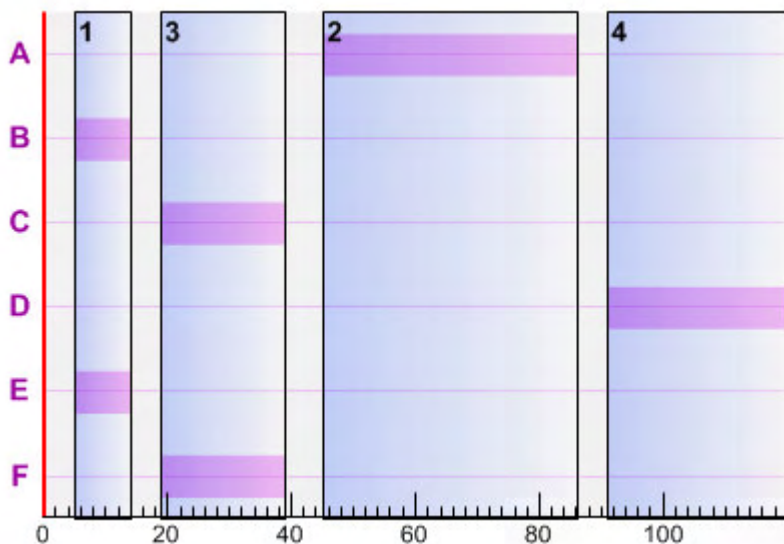
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	9.0	14.0
3	19.0	20.0	39.0
2	45.0	41.0	86.0
4	91.0	29.0	0.0

Phase Timings

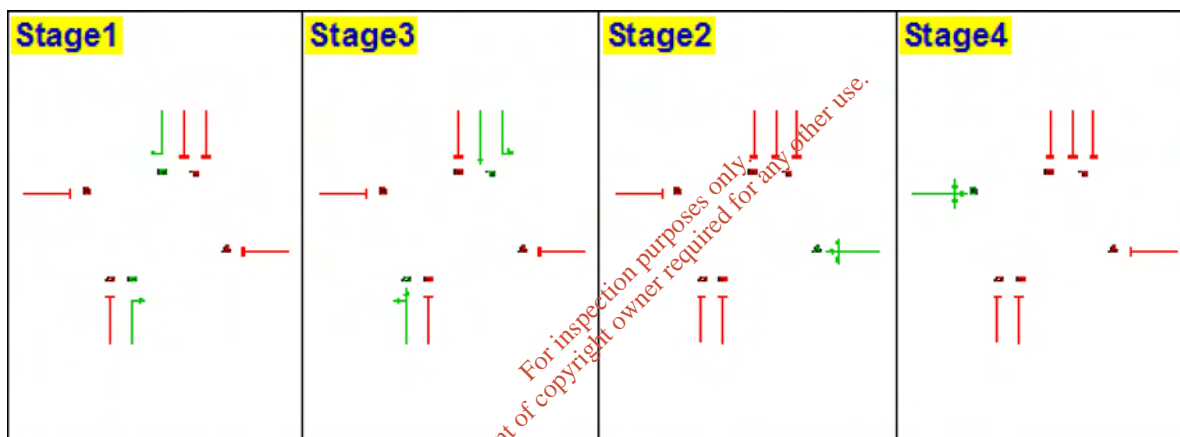
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	45	41.0	86						
B	5	9.0	14						
C	19	20.0	39						
D	91	29.0	0						
E	5	9.0	14						
F	19	20.0	39						

For inspection purposes only. Consent of copyright owner required for any other use.

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	483	A	42.50	45.96	6.17	76.15	18.19	1.76	13.27	11.51	23.10
2	1	240	C	21.50	72.43	4.83	77.70	15.83	1.83	8.73	6.90	6.40
2	2	123	B	10.50	94.46	3.23	75.25	19.60	1.49	5.32	3.84	2.40
3	1	342	D	30.50	56.21	5.34	74.80	20.33	1.55	10.68	9.13	12.50
4	1	251	F	21.50	45.35	3.16	37.01	143.16	0.15	7.38	7.23	12.30
4	2	16	E	10.50	51.97	0.23	9.79	819.41	0.01	0.50	0.49	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	396	1	42.50	38.06	4.19	62.43	44.16	0.73	9.98	9.26	4.20
2	1	197	3	21.50	57.87	3.17	63.78	41.11	0.73	6.34	5.61	1.10
2	2	101	2	10.50	72.84	2.04	61.79	45.65	0.62	3.75	3.14	0.40
3	1	281	4	30.50	47.34	3.70	61.45	46.45	0.66	8.07	7.40	2.20
4	1	206	6	21.50	44.27	2.53	30.38	196.28	0.09	5.97	5.88	1.80
4	2	13	5	10.50	51.56	0.19	7.95	9999.00	0.00	0.40	0.40	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	473	1	42.50	44.33	5.82	74.57	20.69	1.51	12.76	11.25	3.90
2	1	235	3	21.50	68.26	4.46	76.08	18.30	1.49	8.24	6.75	1.10
2	2	121	2	10.50	87.72	2.95	74.03	21.57	1.20	4.98	3.77	0.40
3	1	335	4	30.50	54.24	5.05	73.26	22.84	1.33	10.25	8.93	2.10
4	1	246	6	21.50	45.22	3.09	36.28	148.10	0.14	7.22	7.08	2.00
4	2	16	5	10.50	51.96	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	579	1	42.50	62.54	10.06	91.28	-1.40	5.11	19.21	14.10	2.60
2	1	288	3	21.50	95.68	7.65	93.24	-3.47	4.56	12.93	8.36	0.80
2	2	148	2	10.50	118.53	4.87	90.55	-0.60	3.04	7.68	4.64	0.40
3	1	410	4	30.50	73.02	8.32	89.67	0.37	4.02	15.13	11.10	1.60
4	1	301	6	21.50	46.69	3.90	44.39	102.77	0.25	9.01	8.76	2.30
4	2	19	5	10.50	52.38	0.28	11.62	674.24	0.01	0.59	0.58	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	579	1	42.50	69.60	11.19	91.28	-1.40	5.66	19.75	14.10	2.60
2	1	288	3	21.50	113.91	9.11	93.24	-3.47	5.43	13.79	8.36	0.80
2	2	148	2	10.50	140.92	5.79	90.55	-0.60	3.59	8.23	4.64	0.40
3	1	410	4	30.50	80.83	9.21	89.67	0.37	4.45	15.55	11.10	1.60
4	1	301	6	21.50	46.71	3.91	44.39	102.77	0.25	9.01	8.76	2.30
4	2	19	5	10.50	52.38	0.28	11.62	674.24	0.01	0.59	0.58	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	473	1	42.50	47.90	6.29	74.57	20.69	1.68	12.93	11.25	3.90
2	1	235	3	21.50	84.06	5.49	76.08	18.30	1.88	8.63	6.75	1.10
2	2	121	2	10.50	114.18	3.84	74.03	21.57	1.67	5.44	3.77	0.40
3	1	335	4	30.50	59.07	5.50	73.26	22.84	1.52	10.45	8.93	2.10
4	1	246	6	21.50	45.24	3.09	36.28	148.10	0.14	7.22	7.08	2.00
4	2	16	5	10.50	51.97	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	396	1	42.50	38.67	4.25	62.43	44.16	0.76	10.02	9.26	4.20
2	1	197	3	21.50	60.94	3.33	63.78	41.11	0.81	6.42	5.61	1.10
2	2	101	2	10.50	81.40	2.28	61.79	45.65	0.73	3.86	3.14	0.40
3	1	281	4	30.50	48.37	3.78	61.45	46.45	0.71	8.11	7.40	2.20
4	1	206	6	21.50	44.29	2.53	30.38	196.28	0.09	5.97	5.88	1.80
4	2	13	5	10.50	51.57	0.19	7.95	9999.00	0.00	0.40	0.40	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads.osc
Report generation date: 05/08/2016 16:09:07

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

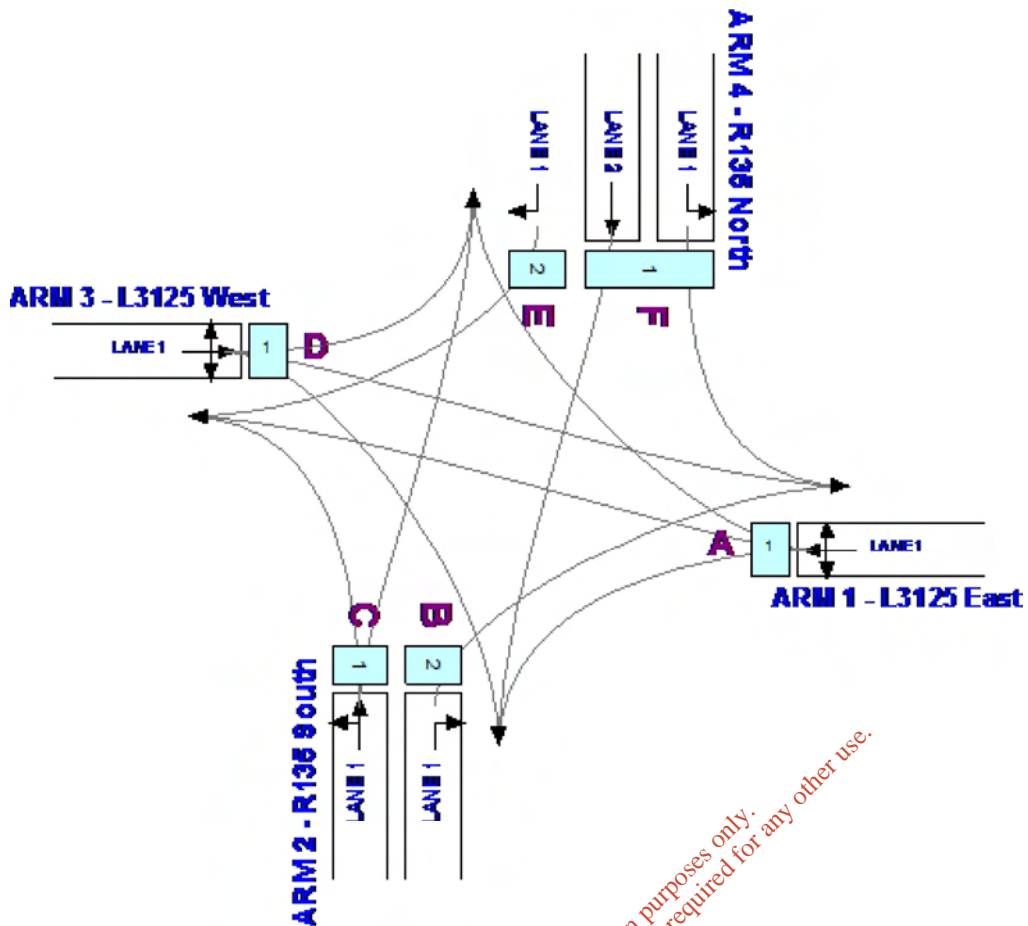
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

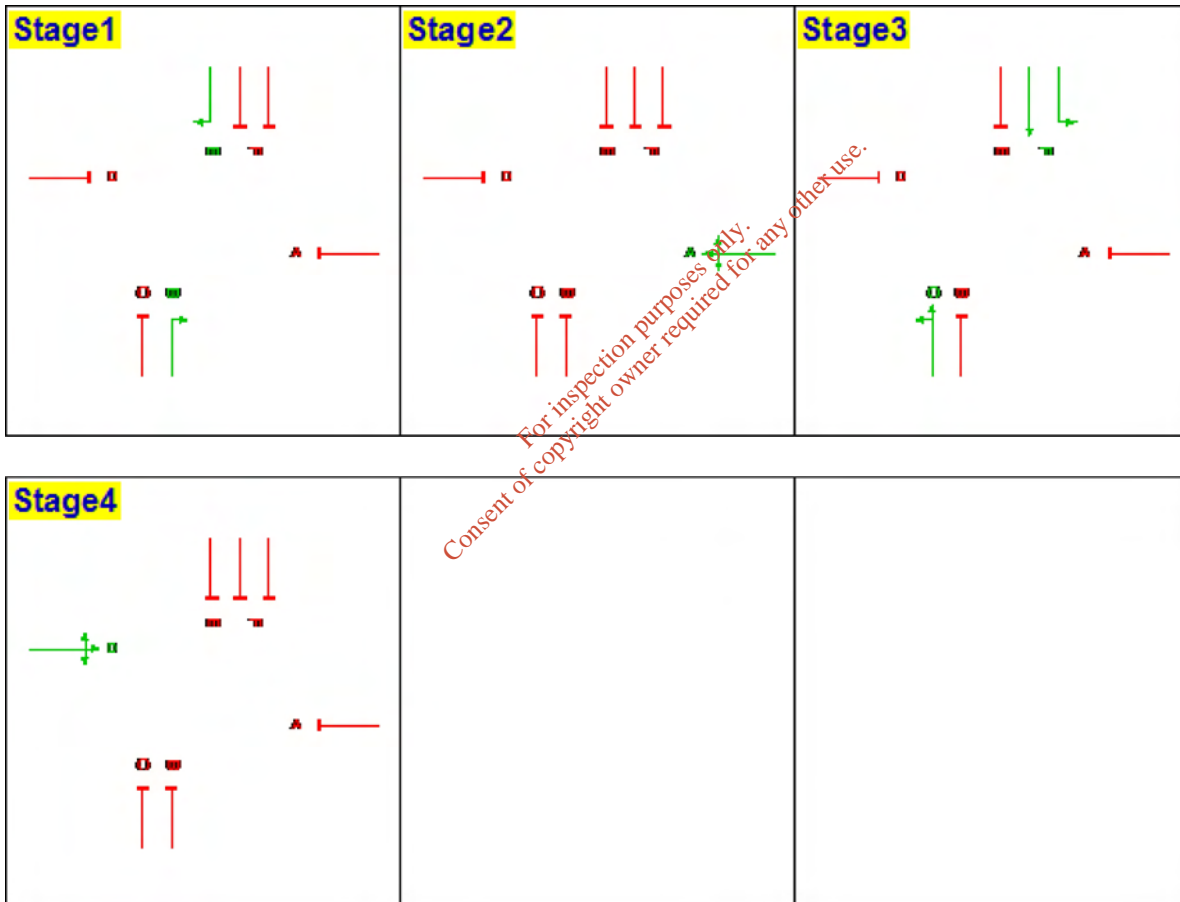
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	Yes	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set7 - 2017 PM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	92	223	218
	Arm 2	137	-	136	128
	Arm 3	180	176	-	24
	Arm 4	178	98	18	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	400	477	585	585	477	400
2 - R135 South	1	C	198	237	290	290	237	198
2 - R135 South	2	B	102	122	150	150	122	102
3 - L3125 West	1	D	282	337	413	413	337	282
4 - R135 North	1	F	207	248	303	303	248	207
4 - R135 North	2	E	13	16	19	19	16	13

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	34	32	34
3 - L3125 West	6	47	46
4 - R135 North	61	33	6

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	14.87	23.42	23.42	57.2

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-4.14	26.12	26.12	54.00

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

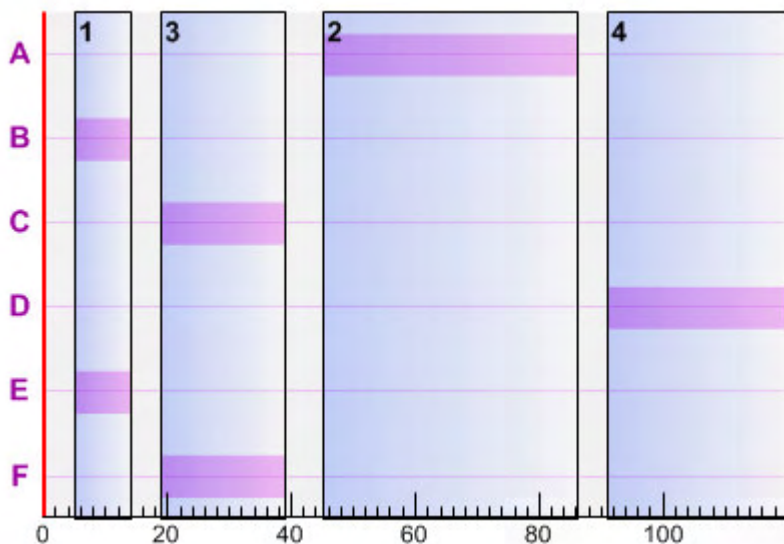
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	9.0	14.0
3	19.0	20.0	39.0
2	45.0	41.0	86.0
4	91.0	29.0	0.0

Phase Timings

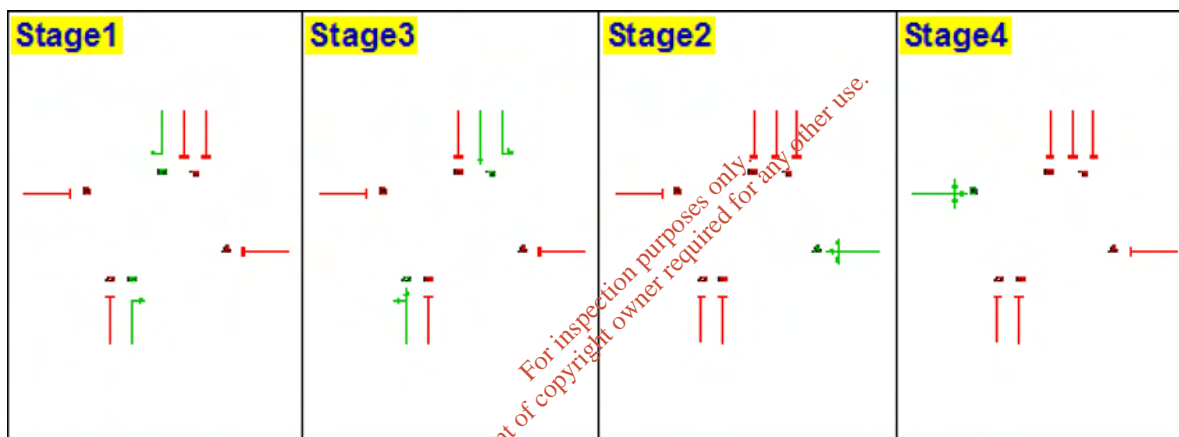
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	45	41.0	86						
B	5	9.0	14						
C	19	20.0	39						
D	91	29.0	0						
E	5	9.0	14						
F	19	20.0	39						

For inspection purposes only. Consent of copyright owner required for any other use.

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	487	A	42.50	46.47	6.29	76.78	17.22	1.83	13.45	11.62	22.90
2	1	242	C	21.50	73.39	4.93	78.35	14.87	1.91	8.87	6.96	6.40
2	2	125	B	10.50	97.02	3.37	76.48	17.68	1.61	5.51	3.90	2.40
3	1	344	D	30.50	56.61	5.41	75.23	19.63	1.60	10.78	9.18	12.50
4	1	253	F	21.50	45.40	3.19	37.31	141.24	0.15	7.45	7.29	12.40
4	2	16	E	10.50	51.97	0.23	9.79	819.41	0.01	0.50	0.49	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	400	1	42.50	38.31	4.26	63.06	42.72	0.75	10.11	9.36	4.20
2	1	198	3	21.50	58.05	3.19	64.10	40.40	0.74	6.39	5.64	1.10
2	2	102	2	10.50	73.29	2.08	62.40	44.22	0.64	3.81	3.17	0.40
3	1	282	4	30.50	47.43	3.72	61.67	45.93	0.67	8.10	7.43	2.20
4	1	207	6	21.50	44.29	2.55	30.52	194.85	0.09	6.00	5.91	1.80
4	2	13	5	10.50	51.56	0.19	7.95	9999.00	0.00	0.40	0.40	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	477	1	42.50	44.75	5.93	75.20	19.68	1.57	12.92	11.35	3.90
2	1	237	3	21.50	68.91	4.54	76.73	17.30	1.55	8.36	6.81	1.10
2	2	122	2	10.50	88.60	3.00	74.64	20.58	1.25	5.05	3.80	0.40
3	1	337	4	30.50	54.55	5.11	73.70	22.11	1.36	10.35	8.98	2.10
4	1	248	6	21.50	45.27	3.12	36.57	146.10	0.15	7.29	7.14	2.00
4	2	16	5	10.50	51.96	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	585	1	42.50	64.24	10.44	92.23	-2.41	5.55	19.81	14.26	2.50
2	1	290	3	21.50	97.28	7.84	93.89	-4.14	4.78	13.20	8.42	0.80
2	2	150	2	10.50	121.27	5.05	91.77	-1.93	3.25	7.96	4.70	0.40
3	1	413	4	30.50	74.18	8.51	90.32	-0.36	4.23	15.42	11.19	1.60
4	1	303	6	21.50	46.75	3.93	44.68	101.43	0.25	9.08	8.83	2.30
4	2	19	5	10.50	52.38	0.28	11.62	674.24	0.01	0.59	0.58	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	585	1	42.50	72.56	11.79	92.23	-2.41	6.23	20.49	14.26	2.50
2	1	290	3	21.50	116.89	9.42	93.89	-4.14	5.74	14.16	8.42	0.80
2	2	150	2	10.50	146.58	6.11	91.77	-1.93	3.90	8.61	4.70	0.40
3	1	413	4	30.50	82.82	9.50	90.32	-0.36	4.71	15.91	11.19	1.50
4	1	303	6	21.50	46.77	3.94	44.68	101.43	0.25	9.08	8.83	2.30
4	2	19	5	10.50	52.38	0.28	11.62	674.24	0.01	0.59	0.58	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	477	1	42.50	48.94	6.48	75.20	19.68	1.76	13.12	11.35	3.80
2	1	237	3	21.50	86.48	5.69	76.73	17.30	1.98	8.79	6.81	1.10
2	2	122	2	10.50	118.83	4.03	74.64	20.58	1.75	5.56	3.80	0.40
3	1	337	4	30.50	59.89	5.61	73.70	22.11	1.57	10.55	8.98	2.10
4	1	248	6	21.50	45.29	3.12	36.57	146.10	0.15	7.29	7.14	2.00
4	2	16	5	10.50	51.97	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	400	1	42.50	38.96	4.33	63.06	42.72	0.79	10.15	9.36	4.20
2	1	198	3	21.50	61.35	3.37	64.10	40.40	0.83	6.47	5.64	1.10
2	2	102	2	10.50	82.55	2.34	62.40	44.22	0.76	3.93	3.17	0.40
3	1	282	4	30.50	48.50	3.80	61.67	45.93	0.72	8.15	7.43	2.20
4	1	207	6	21.50	44.31	2.55	30.52	194.85	0.09	6.00	5.91	1.80
4	2	13	5	10.50	51.57	0.19	7.95	9999.00	0.00	0.40	0.40	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads.osc
Report generation date: 05/08/2016 16:09:54

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

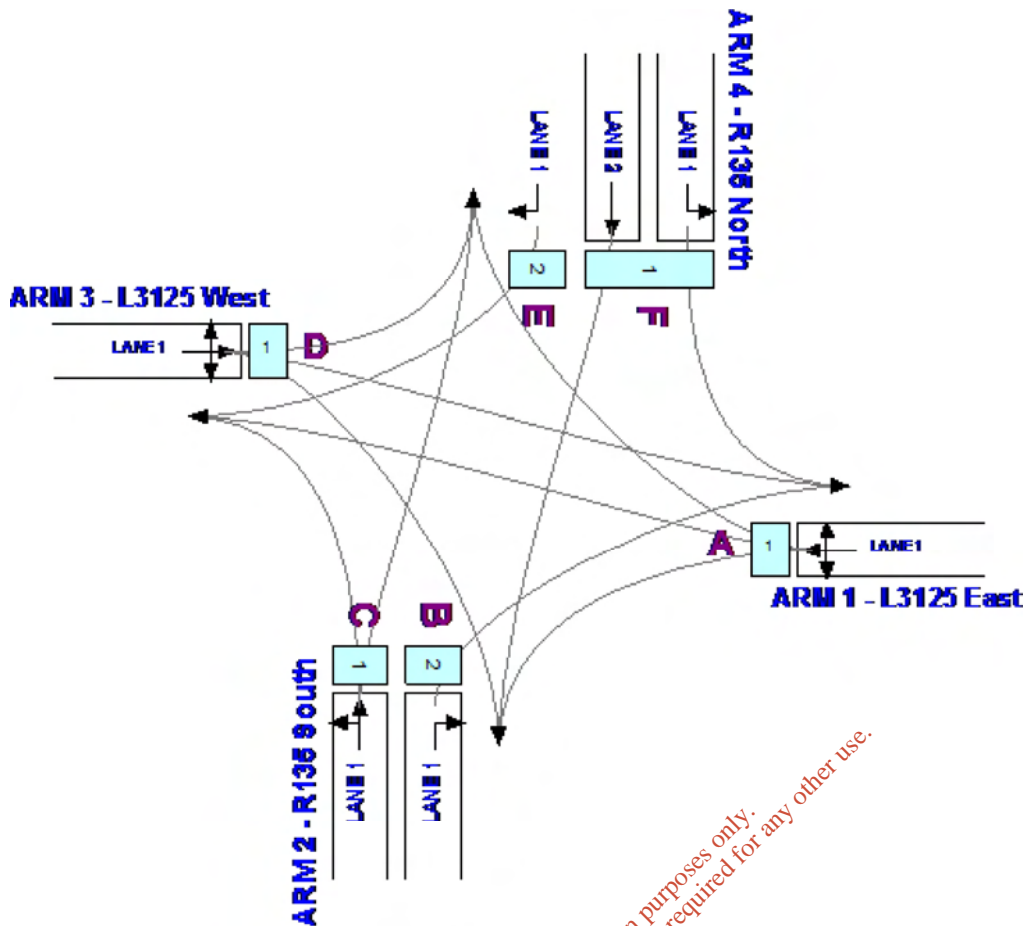
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

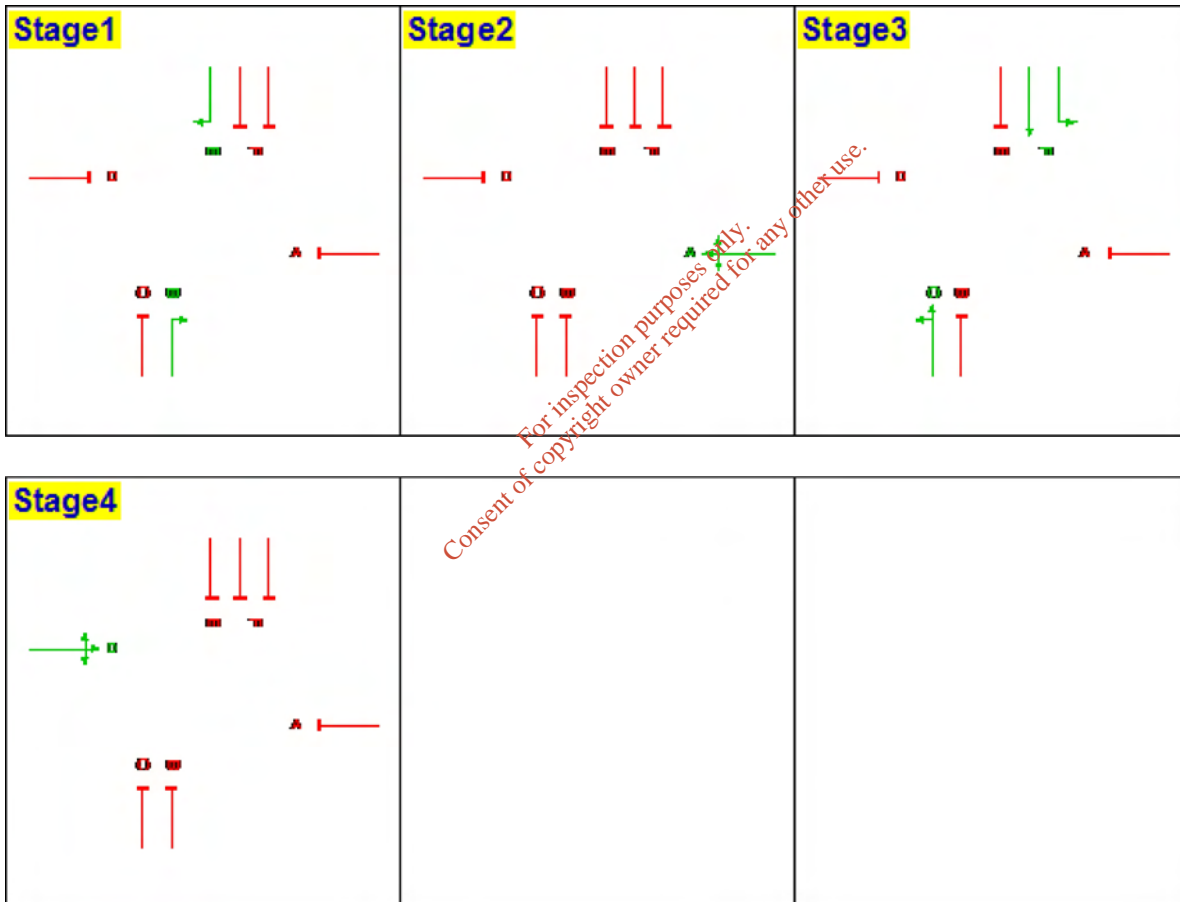
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	Yes	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set8 - 2017 PM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	92	223	218
	Arm 2	137	-	136	134
	Arm 3	180	176	-	24
	Arm 4	178	104	18	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	400	477	585	585	477	400
2 - R135 South	1	C	201	241	295	295	241	201
2 - R135 South	2	B	104	124	152	152	124	104
3 - L3125 West	1	D	282	337	413	413	337	282
4 - R135 North	1	F	211	253	309	309	253	211
4 - R135 North	2	E	14	16	20	20	16	14

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	33	33	34
3 - L3125 West	6	47	46
4 - R135 North	59	35	6

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	15.71	23.69	23.69	57.5

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-3.63	26.50	26.50	54.40

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

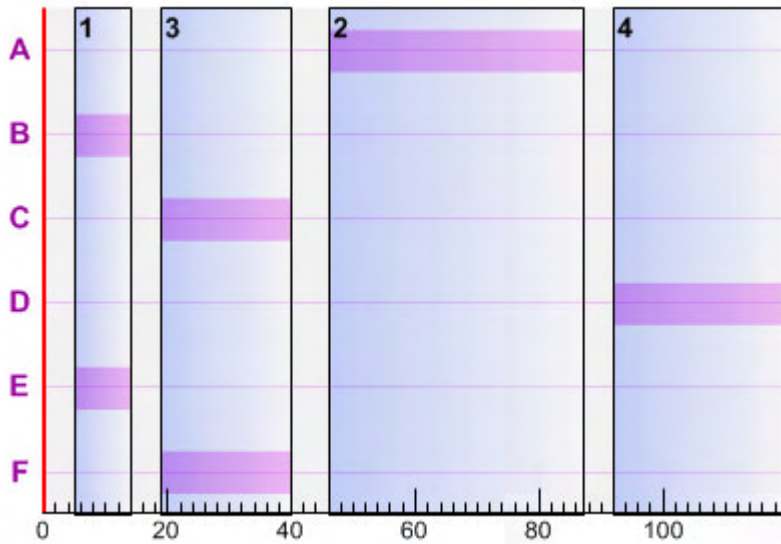
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	9.0	14.0
3	19.0	21.0	40.0
2	46.0	41.0	87.0
4	92.0	28.0	0.0

Phase Timings

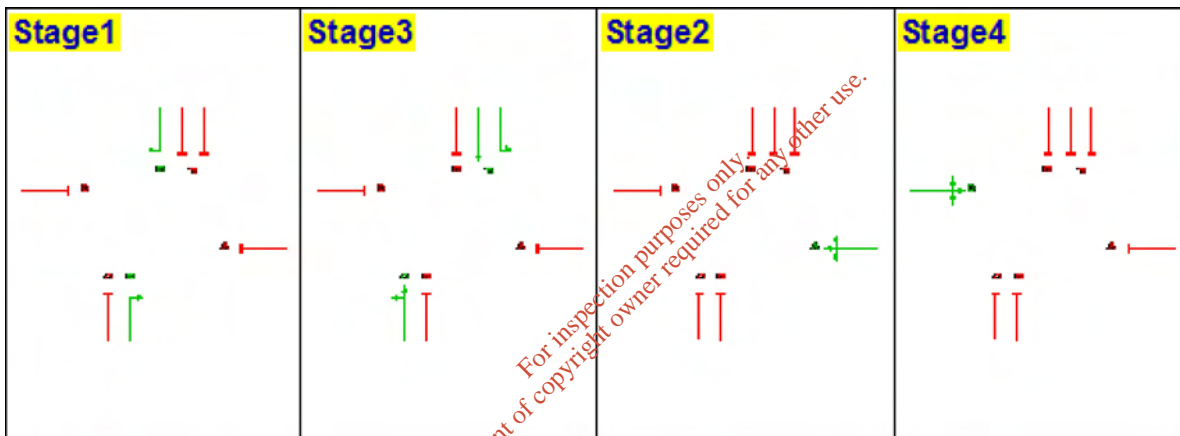
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	46	41.0	87						
B	5	9.0	14						
C	19	21.0	40						
D	92	28.0	0						
E	5	9.0	14						
F	19	21.0	40						

For inspection purposes only. Consent of copyright owner required for any other use.

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	487	A	42.50	46.47	6.29	76.78	17.22	1.83	13.45	11.62	22.90
2	1	246	C	22.50	68.63	4.69	76.10	18.26	1.65	8.65	7.01	7.00
2	2	127	B	10.50	99.78	3.52	77.70	15.83	1.75	5.71	3.96	2.40
3	1	344	D	29.50	60.33	5.76	77.78	15.71	1.89	11.18	9.29	11.40
4	1	258	F	22.50	44.42	3.18	36.35	147.57	0.14	7.51	7.37	13.20
4	2	17	E	10.50	52.10	0.25	10.40	765.32	0.01	0.53	0.52	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	400	1	42.50	38.31	4.26	63.06	42.72	0.75	10.11	9.36	4.20
2	1	201	3	22.50	55.89	3.12	62.18	44.74	0.67	6.34	5.67	1.20
2	2	104	2	10.50	74.22	2.14	63.63	41.45	0.68	3.91	3.23	0.40
3	1	282	4	29.50	49.26	3.86	63.76	41.14	0.76	8.27	7.51	2.10
4	1	211	6	22.50	43.35	2.54	29.73	202.71	0.09	6.05	5.97	1.90
4	2	14	5	10.50	51.69	0.20	8.57	950.75	0.01	0.43	0.43	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	477	1	42.50	44.75	5.93	75.20	19.68	1.57	12.92	11.35	3.90
2	1	241	3	22.50	65.29	4.37	74.56	20.72	1.37	8.23	6.86	1.20
2	2	124	2	10.50	90.43	3.11	75.86	18.63	1.33	5.20	3.87	0.40
3	1	337	4	29.50	57.58	5.39	76.20	18.11	1.58	10.67	9.08	1.90
4	1	253	6	22.50	44.29	3.11	35.65	152.46	0.14	7.35	7.22	2.20
4	2	16	5	10.50	51.96	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	585	1	42.50	64.24	10.44	92.23	-2.41	5.55	19.81	14.26	2.50
2	1	295	3	22.50	89.41	7.33	91.26	-1.38	4.04	12.53	8.49	0.90
2	2	152	2	10.50	124.73	5.27	92.99	-3.22	3.49	8.26	4.77	0.40
3	1	413	4	29.50	81.65	9.37	93.39	-3.63	5.32	16.63	11.32	1.40
4	1	309	6	22.50	45.69	3.92	43.54	106.71	0.23	9.15	8.92	2.40
4	2	20	5	10.50	52.52	0.29	12.24	635.52	0.01	0.62	0.61	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	585	1	42.50	72.56	11.79	92.23	-2.41	6.23	20.49	14.26	2.50
2	1	295	3	22.50	103.11	8.45	91.26	-1.38	4.65	13.14	8.49	0.90
2	2	152	2	10.50	152.69	6.45	92.99	-3.22	4.25	9.02	4.77	0.40
3	1	413	4	29.50	95.57	10.96	93.39	-3.63	6.22	17.53	11.32	1.30
4	1	309	6	22.50	45.71	3.92	43.54	106.71	0.23	9.15	8.92	2.40
4	2	20	5	10.50	52.52	0.29	12.24	635.52	0.01	0.62	0.61	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	477	1	42.50	48.94	6.48	75.20	19.68	1.76	13.12	11.35	3.80
2	1	241	3	22.50	76.23	5.10	74.56	20.72	1.67	8.53	6.86	1.20
2	2	124	2	10.50	125.53	4.32	75.86	18.63	1.93	5.80	3.87	0.40
3	1	337	4	29.50	66.99	6.27	76.20	18.11	1.88	10.97	9.08	1.90
4	1	253	6	22.50	44.31	3.11	35.65	152.46	0.14	7.36	7.22	2.20
4	2	16	5	10.50	51.97	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	400	1	42.50	38.96	4.33	63.06	42.72	0.79	10.15	9.36	4.20
2	1	201	3	22.50	58.22	3.25	62.18	44.74	0.74	6.41	5.67	1.20
2	2	104	2	10.50	84.89	2.45	63.63	41.45	0.82	4.05	3.23	0.40
3	1	282	4	29.50	50.76	3.98	63.76	41.14	0.81	8.33	7.51	2.10
4	1	211	6	22.50	43.37	2.54	29.73	202.71	0.09	6.05	5.97	1.90
4	2	14	5	10.50	51.71	0.20	8.57	950.75	0.01	0.43	0.43	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads.osc
Report generation date: 05/08/2016 16:10:36

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

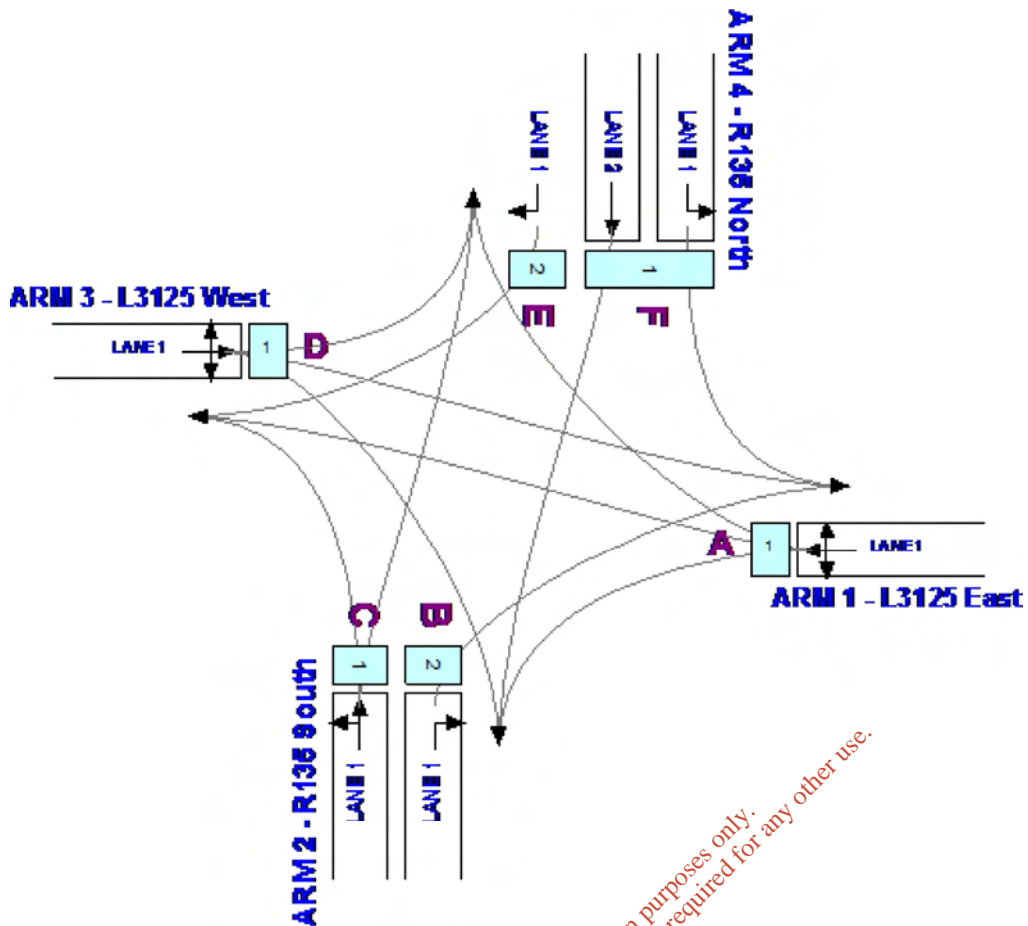
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

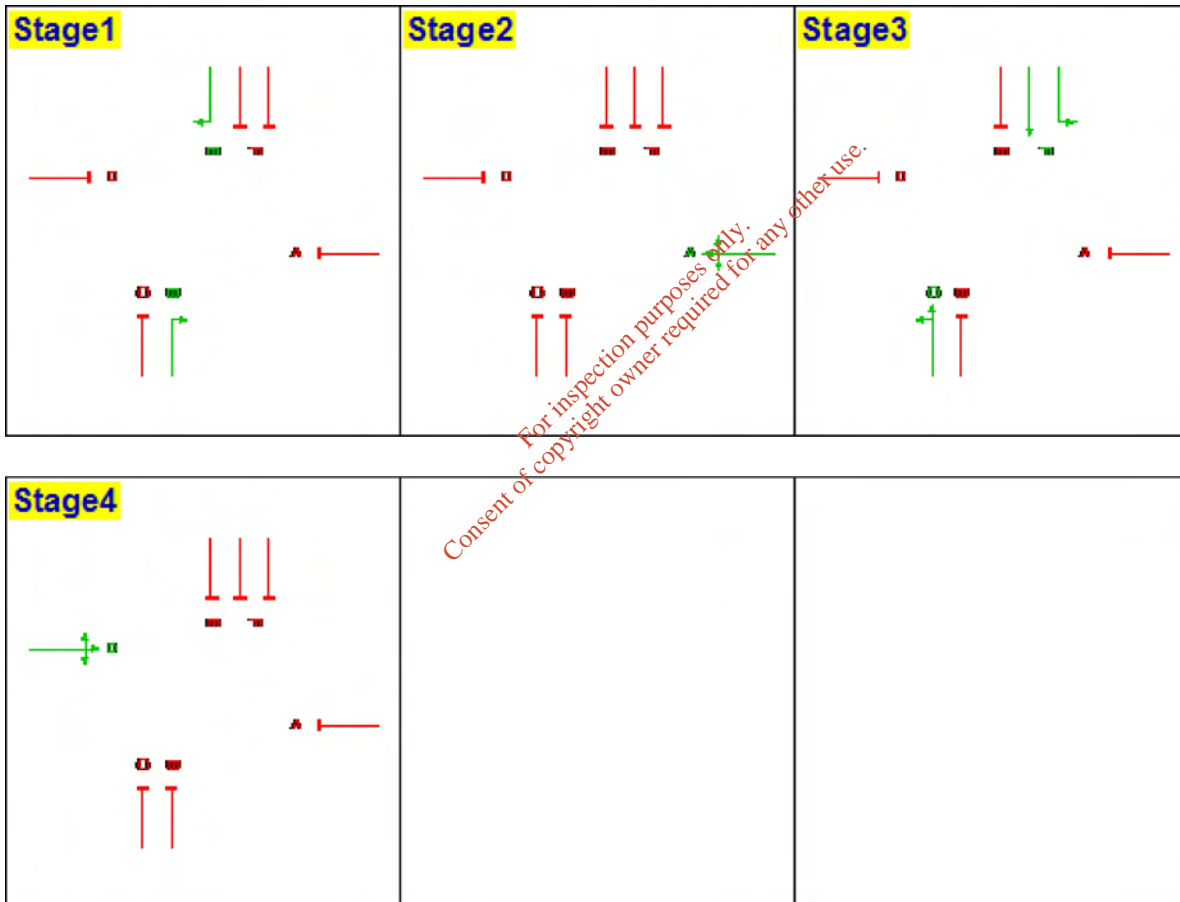
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	Yes	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set9 - 2023 PM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	96	232	227
	Arm 2	143	-	142	133
	Arm 3	188	184	-	25
	Arm 4	186	102	19	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	416	497	609	609	497	416
2 - R135 South	1	C	207	247	303	303	247	207
2 - R135 South	2	B	107	127	156	156	127	107
3 - L3125 West	1	D	295	352	431	431	352	295
4 - R135 North	1	F	216	258	317	317	258	216
4 - R135 North	2	E	14	16	20	20	16	14

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	34	32	34
3 - L3125 West	6	47	46
4 - R135 North	61	33	6

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	10.32	25.83	25.83	55.6

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-8.25	30.07	30.07	52.10

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

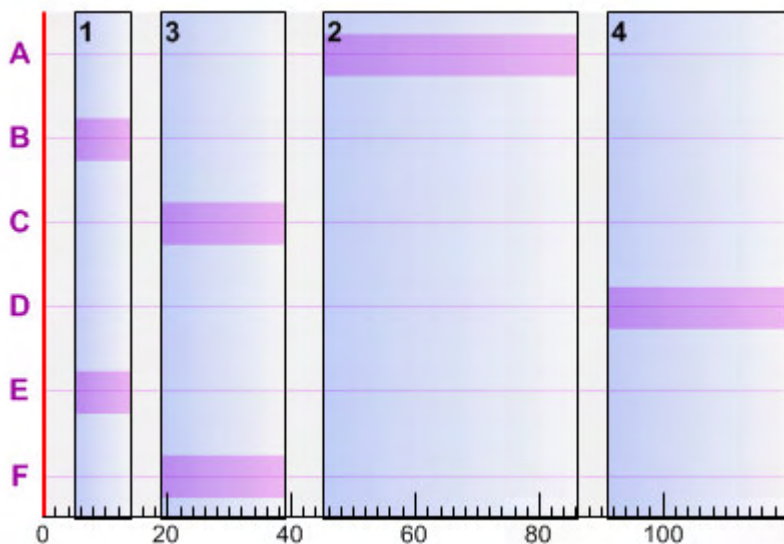
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	9.0	14.0
3	19.0	20.0	39.0
2	45.0	41.0	86.0
4	91.0	29.0	0.0

Phase Timings

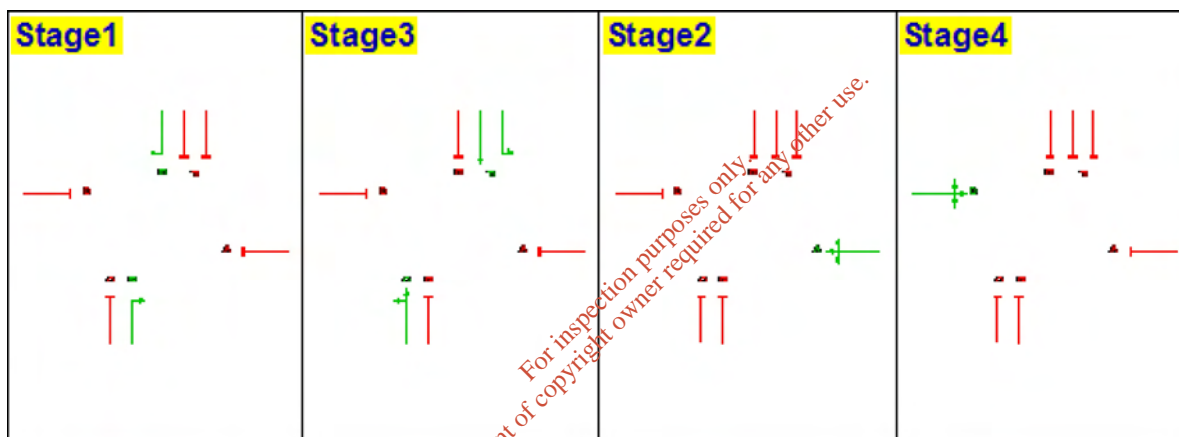
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	45	41.0	86						
B	5	9.0	14						
C	19	20.0	39						
D	91	29.0	0						
E	5	9.0	14						
F	19	20.0	39						

For inspection purposes only. Consent of copyright owner required for any other use.

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	507	A	42.50	49.36	6.95	79.93	12.60	2.30	14.45	12.15	21.70
2	1	252	C	21.50	78.99	5.53	81.58	10.32	2.42	9.68	7.26	6.20
2	2	130	B	10.50	104.39	3.77	79.54	13.16	1.98	6.04	4.06	2.40
3	1	359	D	30.50	59.98	5.98	78.51	14.63	2.00	11.62	9.62	12.00
4	1	264	F	21.50	45.69	3.35	38.93	131.19	0.17	7.80	7.63	12.70
4	2	17	E	10.50	52.10	0.25	10.40	765.32	0.01	0.53	0.52	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	416	1	42.50	39.33	4.54	65.58	37.23	0.87	10.64	9.77	4.20
2	1	207	3	21.50	59.76	3.44	67.02	34.30	0.88	6.79	5.91	1.10
2	2	107	2	10.50	75.67	2.25	65.46	37.48	0.75	4.08	3.33	0.40
3	1	295	4	30.50	48.69	3.99	64.52	39.50	0.79	8.59	7.79	2.20
4	1	216	6	21.50	44.50	2.67	31.85	182.56	0.10	6.28	6.18	1.90
4	2	14	5	10.50	51.69	0.20	8.57	950.75	0.01	0.43	0.43	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	42.50	47.06	6.50	78.35	14.87	1.93	13.81	11.88	3.70
2	1	247	3	21.50	72.72	4.99	79.97	12.55	1.89	9.00	7.11	1.00
2	2	127	2	10.50	93.32	3.29	77.70	15.83	1.48	5.44	3.96	0.40
3	1	352	4	30.50	57.19	5.59	76.98	16.91	1.67	11.09	9.41	2.00
4	1	258	6	21.50	45.52	3.26	38.04	136.56	0.16	7.60	7.44	2.10
4	2	16	5	10.50	51.96	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	609	1	42.50	72.29	12.23	96.01	-6.26	7.82	22.75	14.93	2.10
2	1	303	3	21.50	108.25	9.11	98.10	-8.25	6.43	15.26	8.83	0.70
2	2	156	2	10.50	131.49	5.70	95.44	-5.70	4.01	8.91	4.90	0.40
3	1	431	4	30.50	82.25	9.85	94.26	-4.52	5.78	17.51	11.73	1.40
4	1	317	6	21.50	47.47	4.15	46.75	92.53	0.29	9.55	9.26	2.30
4	2	20	5	10.50	52.52	0.29	12.24	635.52	0.01	0.62	0.61	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	609	1	42.50	87.90	14.87	96.01	-6.26	9.48	24.41	14.93	2.00
2	1	303	3	21.50	139.59	11.75	98.10	-8.25	8.35	17.17	8.83	0.70
2	2	156	2	10.50	165.93	7.19	95.44	-5.70	5.05	9.95	4.90	0.40
3	1	431	4	30.50	97.56	11.68	94.26	-4.52	6.86	18.58	11.73	1.30
4	1	317	6	21.50	47.19	4.16	46.75	92.53	0.29	9.55	9.26	2.30
4	2	20	5	10.50	52.52	0.29	12.24	635.52	0.01	0.62	0.61	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	42.50	56.04	7.74	78.35	14.87	2.25	14.13	11.88	3.60
2	1	247	3	21.50	107.47	7.37	79.97	12.55	2.62	9.73	7.11	1.00
2	2	127	2	10.50	140.11	4.94	77.70	15.83	2.25	6.21	3.96	0.40
3	1	352	4	30.50	67.69	6.62	76.98	16.91	2.00	11.42	9.41	2.00
4	1	258	6	21.50	45.55	3.26	38.04	136.56	0.16	7.61	7.44	2.10
4	2	16	5	10.50	51.97	0.23	9.79	819.41	0.01	0.50	0.49	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	416	1	42.50	40.27	4.65	65.58	37.23	0.93	10.69	9.77	4.20
2	1	207	3	21.50	64.77	3.72	67.02	34.30	1.00	6.91	5.91	1.10
2	2	107	2	10.50	89.14	2.65	65.46	37.48	0.92	4.25	3.33	0.40
3	1	295	4	30.50	50.24	4.12	64.52	39.50	0.85	8.65	7.79	2.20
4	1	216	6	21.50	44.52	2.67	31.85	182.56	0.10	6.28	6.18	1.90
4	2	14	5	10.50	51.71	0.20	8.57	950.75	0.01	0.43	0.43	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads.osc
Report generation date: 05/08/2016 16:11:12

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

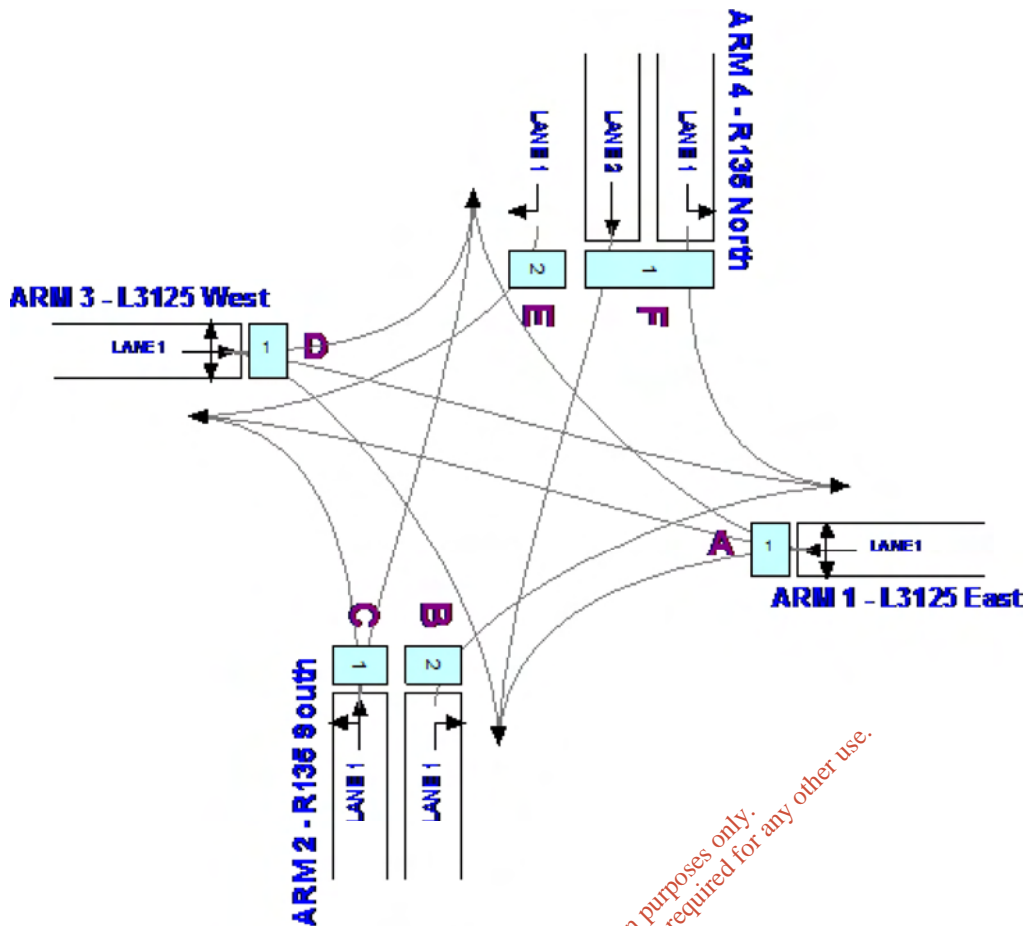
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

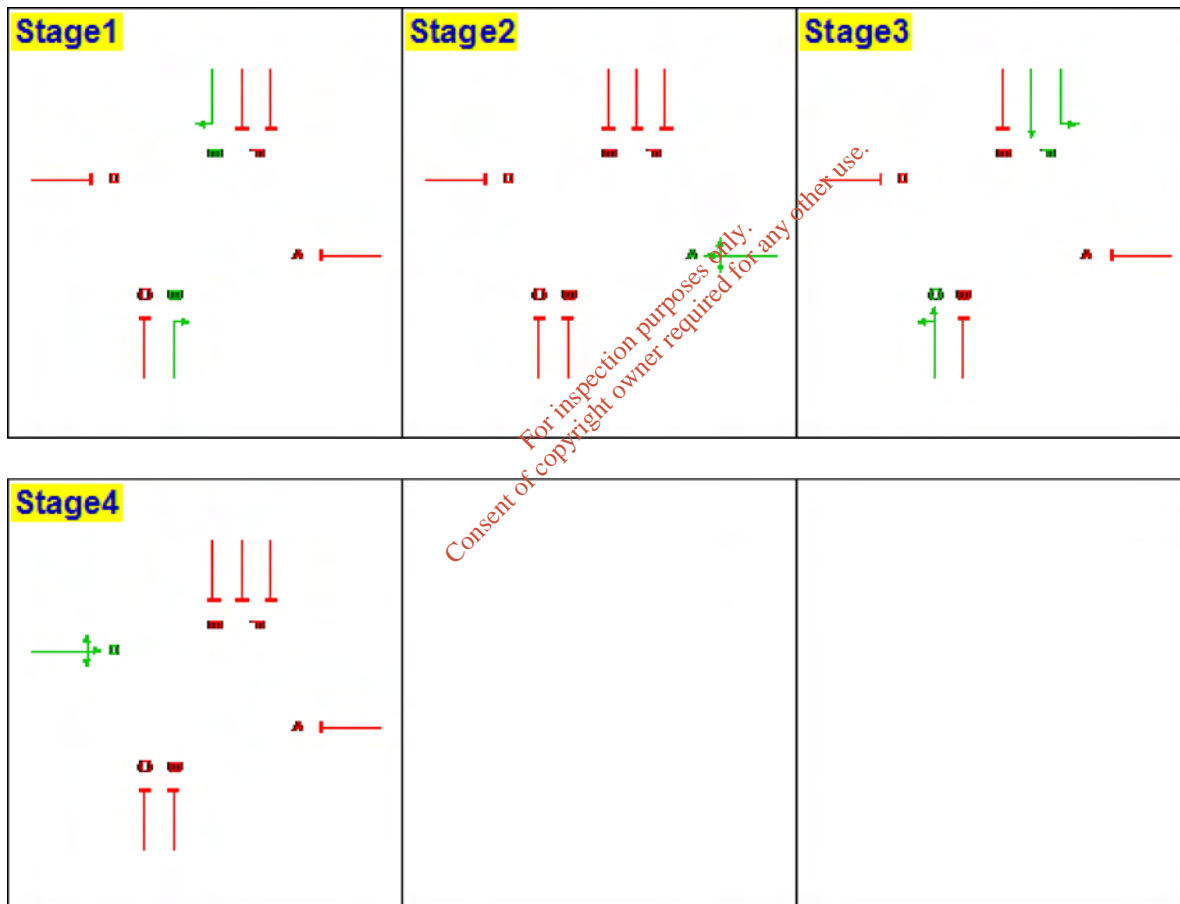
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	Yes	16:30	18:00	ODTAB	No	D1

Demand Set10 - 2023 PM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	96	232	227
	Arm 2	143	-	142	139
	Arm 3	188	184	-	25
	Arm 4	186	108	19	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	416	497	609	609	497	416
2 - R135 South	1	C	210	251	307	307	251	210
2 - R135 South	2	B	108	129	158	158	129	108
3 - L3125 West	1	D	295	352	431	431	352	295
4 - R135 North	1	F	221	263	323	323	263	221
4 - R135 North	2	E	14	17	21	21	17	14

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	17	42	41
2 - R135 South	33	33	34
3 - L3125 West	6	47	46
4 - R135 North	59	35	6

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	10.87	26.12	26.12	56.0

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-7.65	30.51	30.51	52.70

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

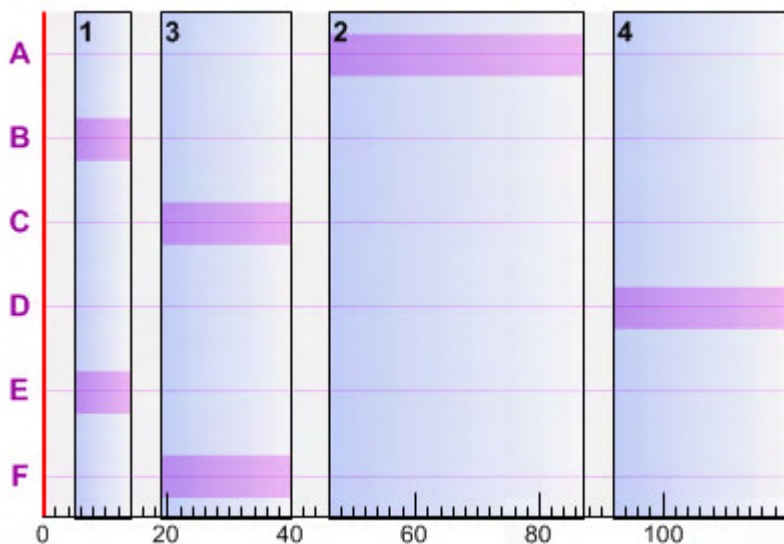
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	9.0	14.0
3	19.0	21.0	40.0
2	46.0	41.0	87.0
4	92.0	28.0	0.0

Phase Timings

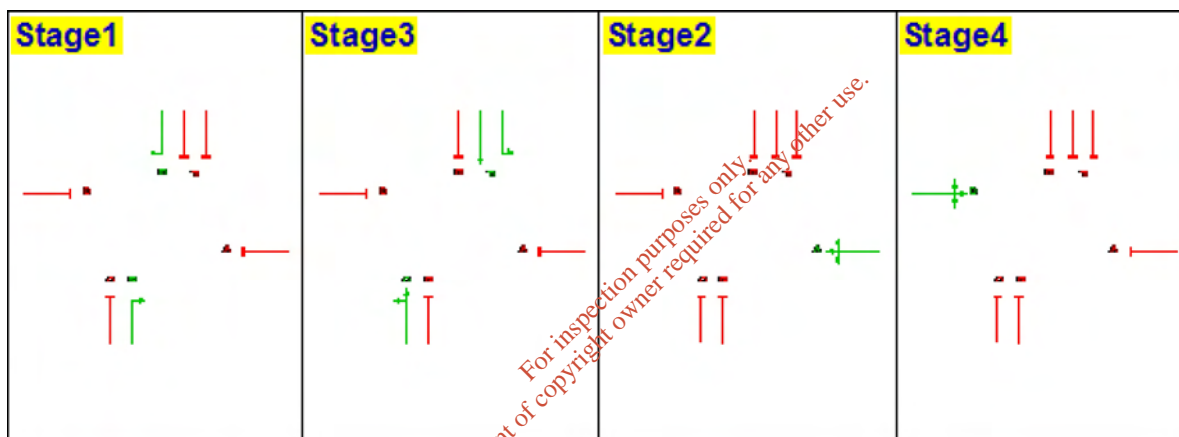
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	46	41.0	87						
B	5	9.0	14						
C	19	21.0	40						
D	92	28.0	0						
E	5	9.0	14						
F	19	21.0	40						

For inspection purposes only. Consent of copyright owner required for any other use.

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	507	A	42.50	49.36	6.95	79.93	12.60	2.30	14.45	12.15	21.70
2	1	256	C	22.50	72.97	5.19	79.20	13.64	2.04	9.34	7.31	6.80
2	2	132	B	10.50	107.81	3.95	80.76	11.44	2.15	6.28	4.12	2.40
3	1	359	D	29.50	64.65	6.45	81.18	10.87	2.42	12.15	9.72	10.90
4	1	269	F	22.50	44.68	3.34	37.90	137.44	0.16	7.86	7.70	13.60
4	2	17	E	10.50	52.10	0.25	10.40	765.32	0.01	0.53	0.52	0.60

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	416	1	42.50	39.33	4.54	65.58	37.23	0.87	10.64	9.77	4.20
2	1	210	3	22.50	57.38	3.35	64.97	38.54	0.79	6.72	5.94	1.20
2	2	108	2	10.50	76.18	2.29	66.08	36.21	0.78	4.13	3.36	0.40
3	1	295	4	29.50	50.68	4.15	66.70	34.92	0.90	8.78	7.88	2.10
4	1	221	6	22.50	43.57	2.67	31.14	189.01	0.10	6.36	6.26	2.00
4	2	14	5	10.50	51.69	0.20	8.57	950.75	0.01	0.43	0.43	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	42.50	47.06	6.50	78.35	14.87	1.93	13.81	11.88	3.70
2	1	251	3	22.50	68.42	4.77	77.65	15.91	1.65	8.81	7.16	1.20
2	2	129	2	10.50	95.17	3.41	78.92	14.03	1.58	5.61	4.03	0.40
3	1	352	4	29.50	60.76	5.94	79.59	13.08	1.97	11.48	9.52	1.90
4	1	263	6	22.50	44.53	3.25	37.06	142.86	0.15	7.67	7.52	2.20
4	2	17	5	10.50	52.09	0.25	10.40	765.32	0.01	0.53	0.52	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	609	1	42.50	72.29	12.23	96.01	-6.26	7.82	22.75	14.93	2.10
2	1	307	3	22.50	98.06	8.36	94.97	-5.24	5.27	14.12	8.86	0.80
2	2	158	2	10.50	135.33	5.94	96.67	-6.90	4.31	9.27	4.96	0.40
3	1	431	4	29.50	91.53	10.96	97.46	-7.65	7.38	19.24	11.86	1.20
4	1	323	6	22.50	46.08	4.13	45.51	97.75	0.27	9.62	9.35	2.50
4	2	21	5	10.50	52.67	0.31	12.85	600.50	0.01	0.65	0.64	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	609	1	42.50	87.90	14.87	96.01	-6.26	9.48	24.41	14.93	2.00
2	1	307	3	22.50	119.58	10.20	94.97	-5.24	6.43	15.29	8.86	0.80
2	2	158	2	10.50	173.03	7.59	96.67	-6.90	5.50	10.46	4.96	0.40
3	1	431	4	29.50	115.61	13.84	97.46	-7.65	9.39	21.24	11.86	1.10
4	1	323	6	22.50	46.10	4.14	45.51	97.75	0.27	9.62	9.35	2.50
4	2	21	5	10.50	52.67	0.31	12.85	600.50	0.01	0.65	0.64	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	497	1	42.50	56.04	7.74	78.35	14.87	2.25	14.13	11.88	3.60
2	1	251	3	22.50	88.18	6.15	77.65	15.91	2.13	9.29	7.16	1.10
2	2	129	2	10.50	149.38	5.35	78.92	14.03	2.49	6.52	4.03	0.40
3	1	352	4	29.50	81.15	7.94	79.59	13.08	2.49	12.01	9.52	1.80
4	1	263	6	22.50	44.55	3.25	37.06	142.86	0.15	7.67	7.52	2.20
4	2	17	5	10.50	52.11	0.25	10.40	765.32	0.01	0.53	0.52	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	416	1	42.50	40.27	4.65	65.58	37.23	0.93	10.69	9.77	4.20
2	1	210	3	22.50	60.74	3.54	64.97	38.54	0.87	6.81	5.94	1.20
2	2	108	2	10.50	91.76	2.75	66.08	36.21	0.96	4.32	3.36	0.40
3	1	295	4	29.50	52.97	4.34	66.70	34.92	0.98	8.86	7.88	2.10
4	1	221	6	22.50	43.58	2.68	31.14	189.01	0.10	6.36	6.26	2.00
4	2	14	5	10.50	51.71	0.20	8.57	950.75	0.01	0.43	0.43	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads With Link Rd.osc
Report generation date: 05/08/2016 16:27:44

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

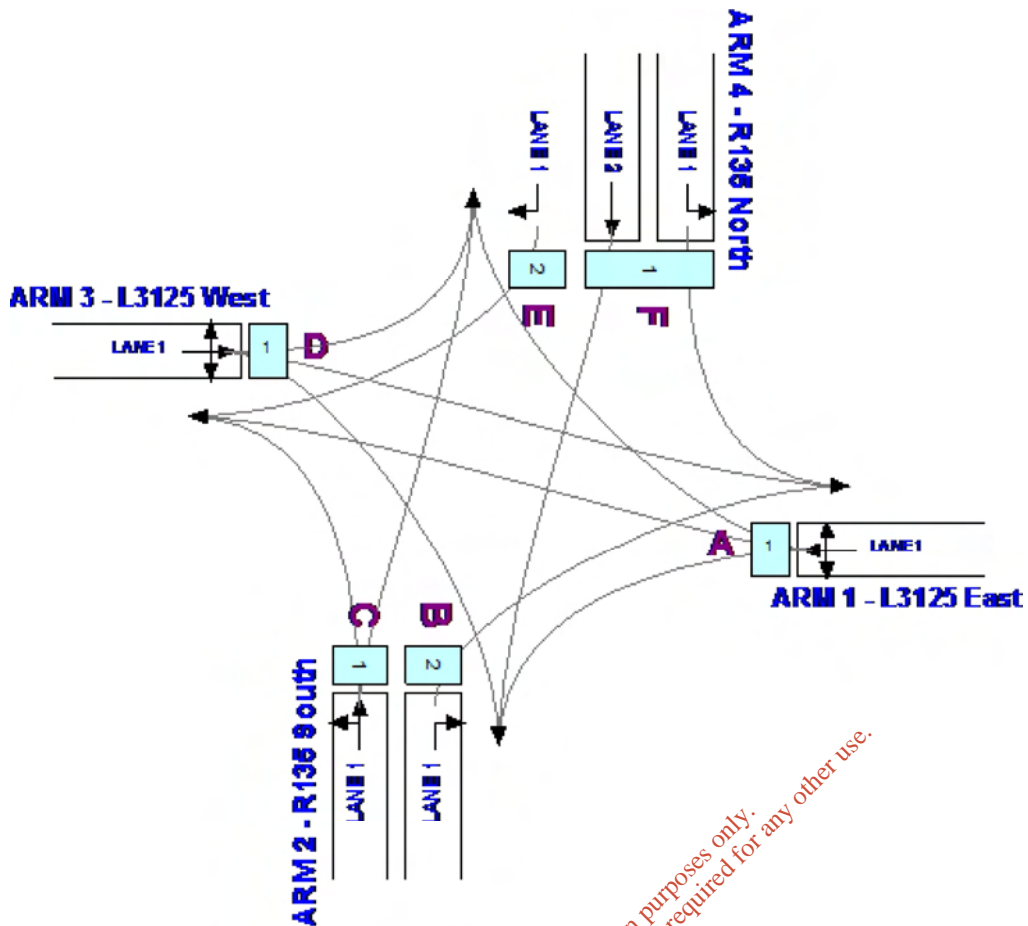
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

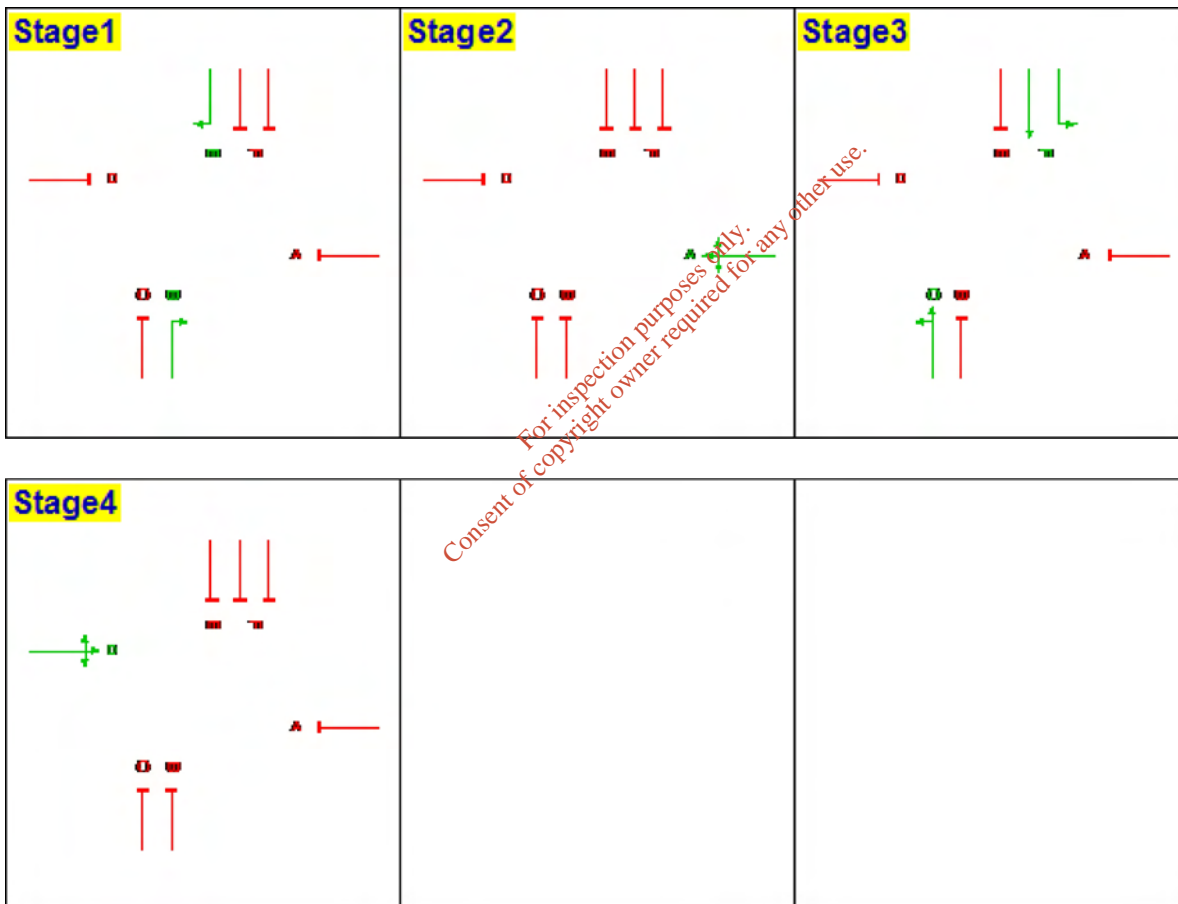
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	Yes	16:30	18:00	ODTAB	No	D1

Demand Set10 - 2023 PM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	96	232	114
	Arm 2	143	-	142	139
	Arm 3	188	184	-	25
	Arm 4	92	108	19	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	332	396	485	485	396	332
2 - R135 South	1	C	210	251	307	307	251	210
2 - R135 South	2	B	108	129	158	158	129	108
3 - L3125 West	1	D	295	352	431	431	352	295
4 - R135 North	1	F	149	178	219	219	178	149
4 - R135 North	2	E	15	18	22	22	18	15

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	22	52	26
2 - R135 South	33	33	34
3 - L3125 West	6	47	46
4 - R135 North	42	49	9

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	21.36	20.84	20.84	54.8

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	1.09	22.70	22.70	52.20

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

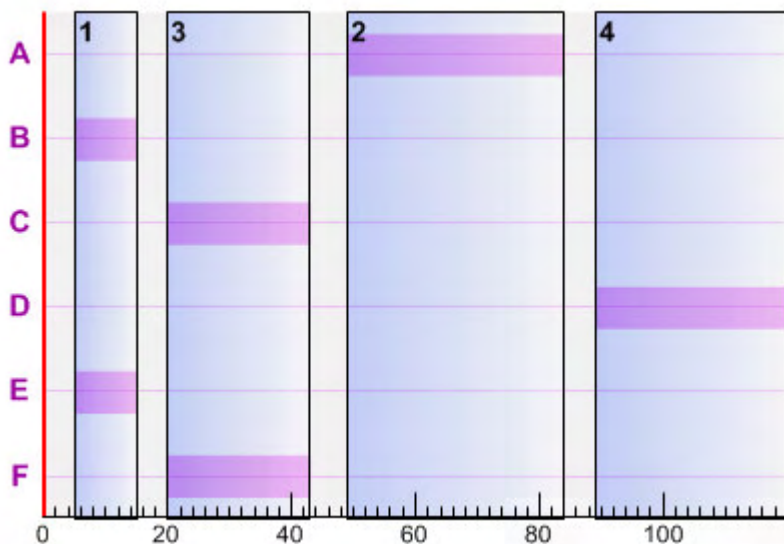
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	10.0	15.0
3	20.0	23.0	43.0
2	49.0	35.0	84.0
4	89.0	31.0	0.0

Phase Timings

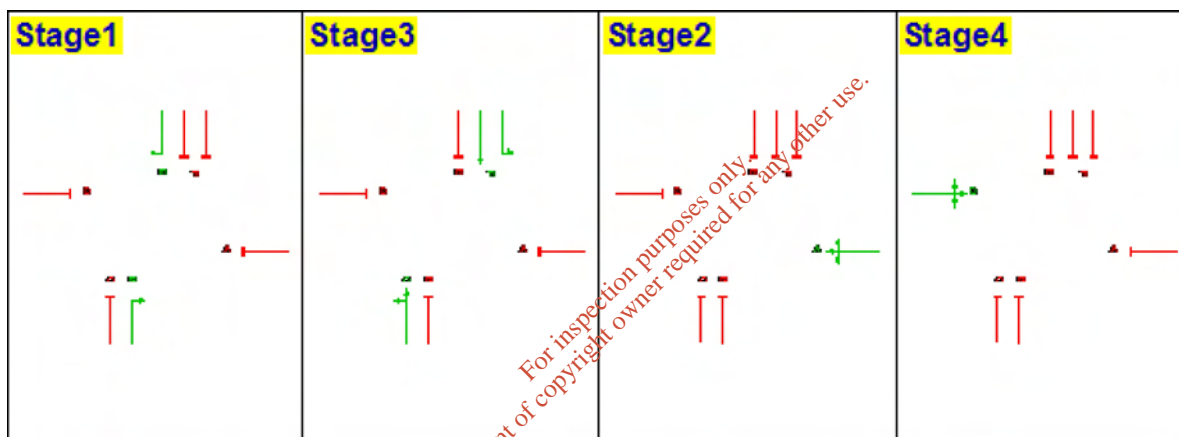
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	49	35.0	84						
B	5	10.0	15						
C	20	23.0	43						
D	89	31.0	0						
E	5	10.0	15						
F	20	23.0	43						

For inspection purposes only. Consent of copyright owner required for any other use.

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	404	A	36.50	49.76	5.58	74.16	21.36	1.51	11.71	10.19	17.50
2	1	256	C	24.50	62.10	4.42	72.73	23.74	1.33	8.49	7.16	8.30
2	2	132	B	11.50	87.81	3.22	73.74	22.06	1.36	5.45	4.09	2.80
3	1	359	D	32.50	53.19	5.30	73.68	22.15	1.45	10.85	9.40	14.20
4	1	182	F	24.50	40.87	2.07	23.55	282.14	0.05	5.06	5.01	11.30
4	2	18	E	11.50	51.01	0.26	10.05	795.08	0.01	0.55	0.54	0.70

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	332	1	36.50	42.13	3.89	60.94	47.68	0.65	8.90	8.25	3.10
2	1	210	3	24.50	52.56	3.07	59.66	50.85	0.59	6.40	5.82	1.40
2	2	108	2	11.50	69.65	2.09	60.33	49.18	0.58	3.90	3.33	0.40
3	1	295	4	32.50	45.24	3.71	60.55	48.65	0.63	8.25	7.62	2.50
4	1	149	6	24.50	40.29	1.67	19.28	366.77	0.03	4.11	4.07	1.60
4	2	15	5	11.50	50.66	0.21	8.38	974.10	0.01	0.46	0.45	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	396	1	36.50	48.23	5.31	72.69	23.81	1.31	11.29	9.98	3.00
2	1	251	3	24.50	59.95	4.18	71.31	26.21	1.14	8.15	7.01	1.40
2	2	129	2	11.50	82.09	2.94	72.06	24.90	1.10	5.09	3.99	0.50
3	1	352	4	32.50	51.56	5.04	72.25	24.58	1.26	10.46	9.20	2.40
4	1	178	6	24.50	40.80	2.02	23.03	290.73	0.05	4.94	4.90	1.90
4	2	18	5	11.50	51.00	0.26	10.05	795.08	0.01	0.55	0.54	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	485	1	36.50	64.95	8.72	89.03	1.09	4.04	16.49	12.46	2.20
2	1	307	3	24.50	78.28	6.68	87.22	3.19	3.12	11.80	8.68	1.10
2	2	158	2	11.50	108.76	4.77	88.26	1.97	2.74	7.66	4.92	0.40
3	1	431	4	32.50	68.47	8.20	88.46	1.74	3.74	15.20	11.46	1.80
4	1	219	6	24.50	41.55	2.53	28.34	217.58	0.08	6.15	6.08	2.20
4	2	22	5	11.50	51.49	0.31	12.29	632.34	0.01	0.68	0.67	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	485	1	36.50	70.73	9.53	89.03	1.09	4.38	16.84	12.46	2.10
2	1	307	3	24.50	85.69	7.31	87.22	3.19	3.41	12.09	8.68	1.10
2	2	158	2	11.50	125.51	5.51	88.26	1.97	3.15	8.06	4.92	0.40
3	1	431	4	32.50	74.52	8.92	88.46	1.74	4.06	15.52	11.46	1.80
4	1	219	6	24.50	41.55	2.53	28.34	217.58	0.08	6.15	6.08	2.20
4	2	22	5	11.50	51.49	0.31	12.29	632.34	0.01	0.68	0.67	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	396	1	36.50	51.44	5.66	72.69	23.81	1.47	11.44	9.98	2.90
2	1	251	3	24.50	65.26	4.55	71.31	26.21	1.32	8.34	7.01	1.40
2	2	129	2	11.50	99.82	3.58	72.06	24.90	1.44	5.43	3.99	0.50
3	1	352	4	32.50	55.15	5.39	72.25	24.58	1.42	10.62	9.20	2.40
4	1	178	6	24.50	40.80	2.02	23.03	290.73	0.05	4.94	4.90	1.90
4	2	18	5	11.50	51.02	0.26	10.05	795.08	0.01	0.55	0.54	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	332	1	36.50	42.81	3.95	60.94	47.68	0.69	8.94	8.25	3.10
2	1	210	3	24.50	53.96	3.15	59.66	50.85	0.63	6.45	5.82	1.40
2	2	108	2	11.50	75.48	2.26	60.33	49.18	0.66	3.99	3.33	0.40
3	1	295	4	32.50	46.05	3.77	60.55	48.65	0.67	8.29	7.62	2.50
4	1	149	6	24.50	40.29	1.67	19.28	366.77	0.03	4.11	4.07	1.60
4	2	15	5	11.50	50.67	0.21	8.38	974.10	0.01	0.46	0.45	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry
TIA\Reports\Appendices\Crossroads With Link Rd.osc
Report generation date: 04/08/2016 10:34:21

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

For inspection purposes only.
Consent of copyright owner required for any other use.

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	L3125 East	50.0	10	10	80	0
2	R135 South	50.0	10	10	80	0
3	L3125 West	50.0	10	10	80	0
4	R135 North	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1791	Yes	0	A	-
2	1	Traffic		1724	Yes	0	C	-
2	2	Traffic		1868	Yes	0	B	-
3	1	Traffic		1799	Yes	0	D	-
4	1	Traffic		3785	Yes	0	F	-
4	2	Traffic		1868	Yes	0	E	-

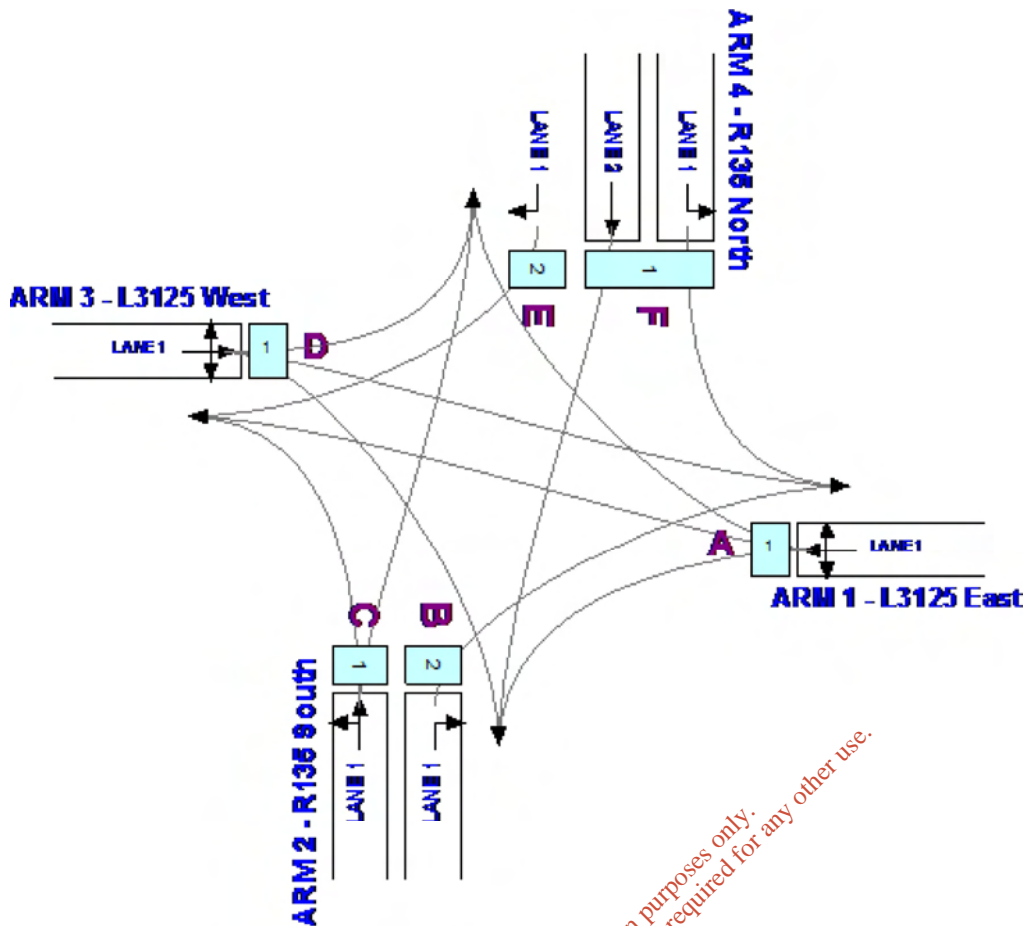
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
4	1	0.0	0.0	90	100	0	0.0	1.10	0
4	2	0.0	0.0	90	100	0	0.0	1.10	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2	3	4	0.37	8	Yes	3.00	0.0	0
2	1	1		3	4		0.74	10	Yes	3.00	0.0	0
2	2	1				1	1.00	15	No	3.00	0.0	0
3	1	1		4	1	2	0.43	10	Yes	3.00	0.0	0
4	1	1		1			1.00	14	Yes	3.00	0.0	0
4	1	2			2		0.00	10	No	3.00	0.0	0
4	2	1				3	1.00	15	No	3.00	0.0	0

For inspection purposes only. Consent of copyright owner required for any other use.

Junction Diagram



Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No

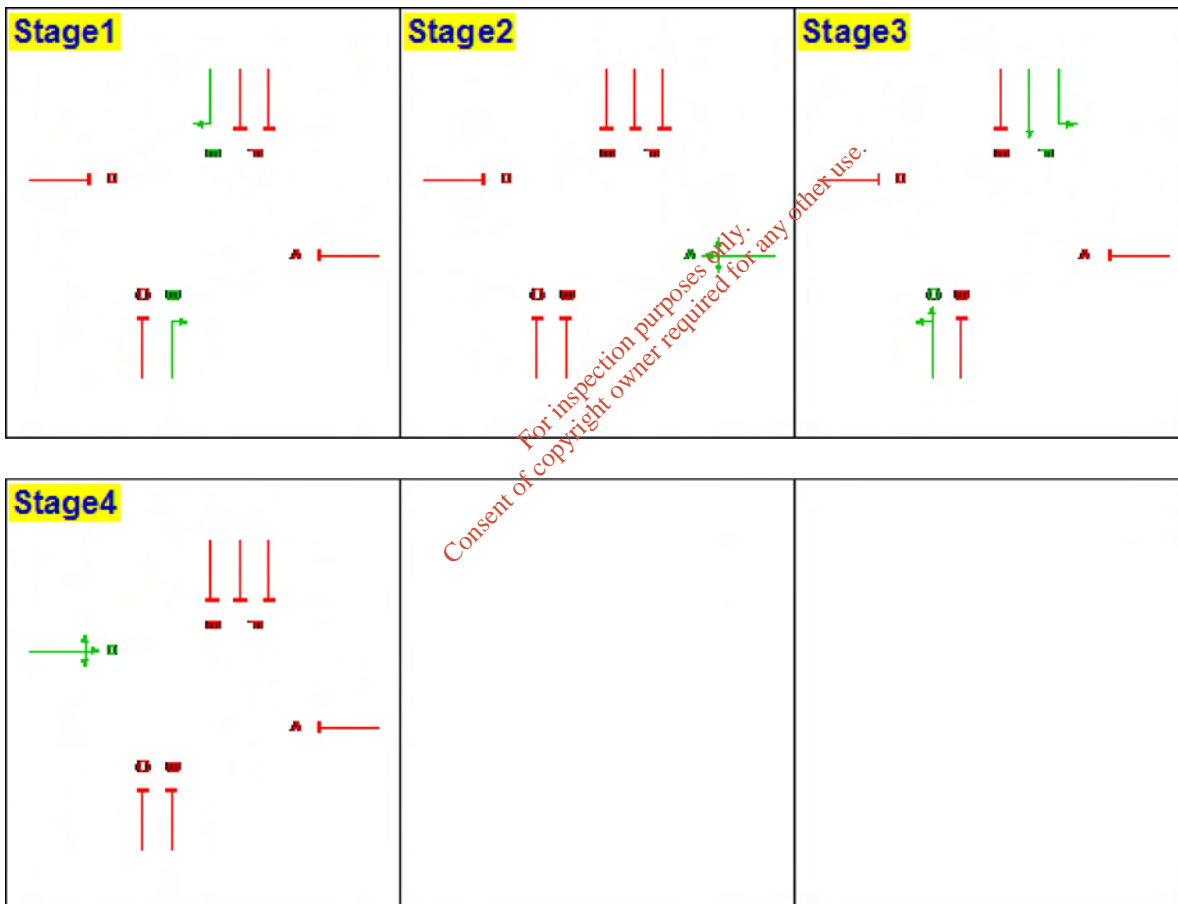
Intergreen Matrix

		To					
		A	B	C	D	E	F
From	A	-	5	5	5	5	5
	B	6	-	5	6		5
	C	6	5	-	6	5	
	D	5	5	5	-	5	5
	E	6		5	6	-	5
	F	6	5		6	5	-

Stages

Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	B,E	Yes
2	-1	A	Yes
3	-1	C,F	Yes
4	-1	D	Yes

Stage Diagrams



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,3,4
2		1,4,3,2
3		1,3,2,4
4		1,4,2,3
5		1,2,4,3
6		1,3,4,2

Constraints

(No constraints)

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	Yes	16:30	18:00	ODTAB	No	D1

Demand Set10 - 2023 PM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To			
		Arm 1	Arm 2	Arm 3	Arm 4
From	Arm 1	-	96	232	114
	Arm 2	143	-	142	155
	Arm 3	188	184	-	25
	Arm 4	92	124	19	-

Average pedestrian flow on each pedestrian stream (if applicable): 0 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - L3125 East	1	A	332	396	485	485	396	332
2 - R135 South	1	C	221	264	323	323	264	221
2 - R135 South	2	B	109	130	159	159	130	109
3 - L3125 West	1	D	295	352	431	431	352	295
4 - R135 North	1	F	162	194	237	237	194	162
4 - R135 North	2	E	14	17	21	21	17	14

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - L3125 East	22	52	26
2 - R135 South	32	35	33
3 - L3125 West	6	47	46
4 - R135 North	39	53	8

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	18.39	21.52	21.52	55.6

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	-1.39	23.63	23.63	53.00

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

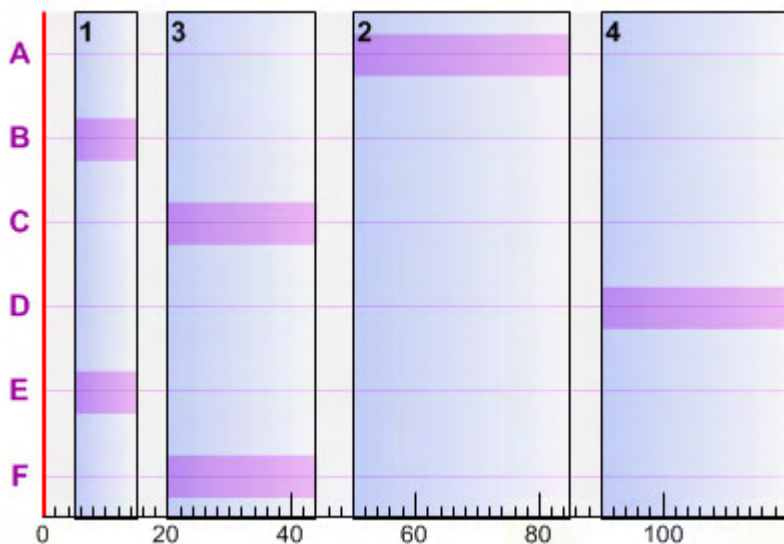
Stage	Start Time (s)	Duration (s)	End Time (s)
1	5.0	10.0	15.0
3	20.0	24.0	44.0
2	50.0	35.0	85.0
4	90.0	30.0	0.0

Phase Timings

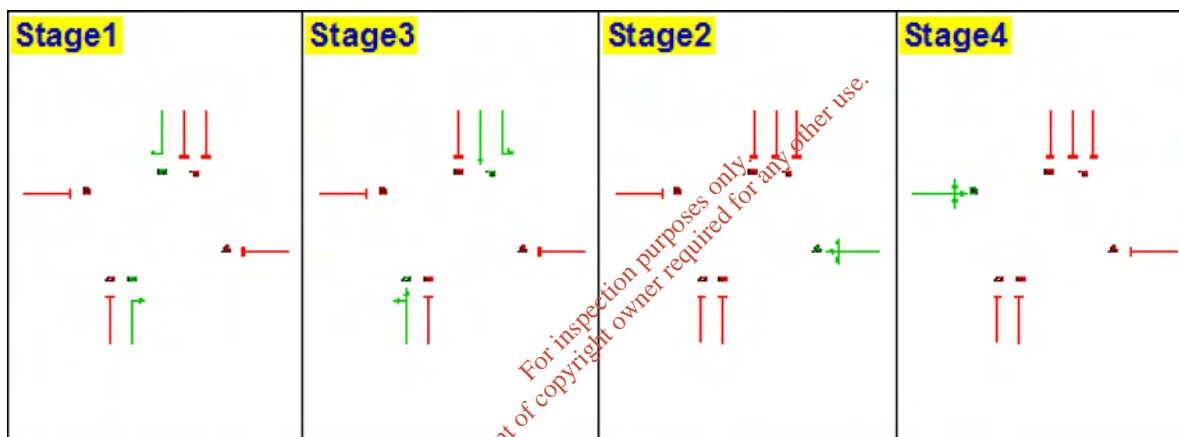
Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	50	35.0	85						
B	5	10.0	15						
C	20	24.0	44						
D	90	30.0	0						
E	5	10.0	15						
F	20	24.0	44						

For inspection purposes only. Consent of copyright owner required for any other use.

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	404	A	36.50	49.76	5.58	74.16	21.36	1.51	11.71	10.19	17.50
2	1	269	C	25.50	61.47	4.59	73.43	22.57	1.39	8.86	7.46	8.90
2	2	133	B	11.50	88.77	3.28	74.29	21.14	1.41	5.53	4.12	2.80
3	1	359	D	31.50	56.26	5.61	76.02	18.39	1.69	11.20	9.51	13.10
4	1	198	F	25.50	40.23	2.21	24.62	265.60	0.06	5.47	5.41	12.60
4	2	17	E	11.50	50.89	0.24	9.50	847.74	0.01	0.52	0.51	0.70

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	332	1	36.50	42.13	3.89	60.94	47.68	0.65	8.90	8.25	3.10
2	1	221	3	25.50	51.86	3.18	60.32	49.19	0.61	6.68	6.07	1.50
2	2	109	2	11.50	70.03	2.12	60.89	47.81	0.59	3.95	3.36	0.40
3	1	295	4	31.50	46.89	3.84	62.47	44.07	0.71	8.41	7.71	2.40
4	1	162	6	25.50	39.62	1.78	20.14	346.84	0.03	4.43	4.39	1.80
4	2	14	5	11.50	50.54	0.20	7.82	9999.00	0.00	0.43	0.42	0.10

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	396	1	36.50	48.23	5.31	72.69	23.81	1.31	11.29	9.98	3.00
2	1	264	3	25.50	59.34	4.35	72.06	24.89	1.20	8.52	7.32	1.50
2	2	130	2	11.50	82.82	2.99	72.62	23.93	1.13	5.15	4.02	0.50
3	1	352	4	31.50	54.16	5.30	74.54	20.74	1.44	10.75	9.31	2.20
4	1	194	6	25.50	40.16	2.16	24.12	273.13	0.05	5.35	5.30	2.10
4	2	17	5	11.50	50.89	0.24	9.50	847.74	0.01	0.52	0.51	0.10

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	485	1	36.50	64.95	8.72	89.03	1.09	4.04	16.49	12.46	2.20
2	1	323	3	25.50	78.34	7.03	88.17	2.08	3.37	12.44	9.06	1.20
2	2	159	2	11.50	110.08	4.86	88.82	1.33	2.83	7.78	4.95	0.40
3	1	431	4	31.50	74.67	8.94	91.27	-1.39	4.60	16.20	11.59	1.60
4	1	237	6	25.50	40.93	2.69	29.47	205.44	0.09	6.62	6.53	2.40
4	2	21	5	11.50	51.37	0.30	11.73	667.21	0.01	0.65	0.64	0.10

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	485	1	36.50	70.73	9.53	89.03	1.09	4.38	16.84	12.46	2.10
2	1	323	3	25.50	86.44	7.76	88.17	2.08	3.72	12.78	9.06	1.20
2	2	159	2	11.50	127.72	5.64	88.82	1.33	3.27	8.22	4.95	0.40
3	1	431	4	31.50	84.30	10.09	91.27	-1.39	5.19	16.78	11.59	1.60
4	1	237	6	25.50	40.93	2.69	29.47	205.44	0.09	6.62	6.53	2.40
4	2	21	5	11.50	51.37	0.30	11.73	667.21	0.01	0.65	0.64	0.10

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	396	1	36.50	51.44	5.66	72.69	23.81	1.47	11.44	9.98	2.90
2	1	264	3	25.50	65.00	4.77	72.06	24.89	1.39	8.71	7.32	1.50
2	2	130	2	11.50	101.77	3.67	72.62	23.93	1.49	5.52	4.02	0.50
3	1	352	4	31.50	60.07	5.87	74.54	20.74	1.66	10.97	9.31	2.20
4	1	194	6	25.50	40.16	2.16	24.12	273.13	0.05	5.35	5.30	2.10
4	2	17	5	11.50	50.90	0.24	9.50	847.74	0.01	0.52	0.51	0.10

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	332	1	36.50	42.81	3.95	60.94	47.68	0.69	8.94	8.25	3.10
2	1	221	3	25.50	53.26	3.27	60.32	49.19	0.66	6.73	6.07	1.50
2	2	109	2	11.50	76.21	2.31	60.89	47.81	0.69	4.04	3.36	0.40
3	1	295	4	31.50	47.99	3.93	62.47	44.07	0.75	8.46	7.71	2.40
4	1	162	6	25.50	39.62	1.78	20.14	346.84	0.03	4.43	4.39	1.80
4	2	14	5	11.50	50.55	0.20	7.82	9999.00	0.00	0.43	0.42	0.10

For inspection purposes only.
Consent of copyright owner required for any other use.

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry TIA\Reports\Appendices\T-Junction.osc

Report generation date: 05/08/2016 16:45:41

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	R135 North	50.0	10	10	80	0
2	Elm Road	50.0	10	10	80	0
3	R135 South	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1936	Yes	0	B	-
1	2	Traffic		2105	Yes	0	A	-
2	1	Traffic		1747	Yes	0	D	-
2	2	Traffic		1871	Yes	0	C	-
3	1	Traffic		1965	Yes	0	F	-
3	2	Traffic		1830	Yes	0	E	-
(Ped)	1	Pedestrian		10000	Yes	0	J	-
(Ped)	2	Pedestrian		10000	Yes	0	I	-
(Ped)	3	Pedestrian		10000	Yes	0	H	-
(Ped)	4	Pedestrian		10000	Yes	0	G	-

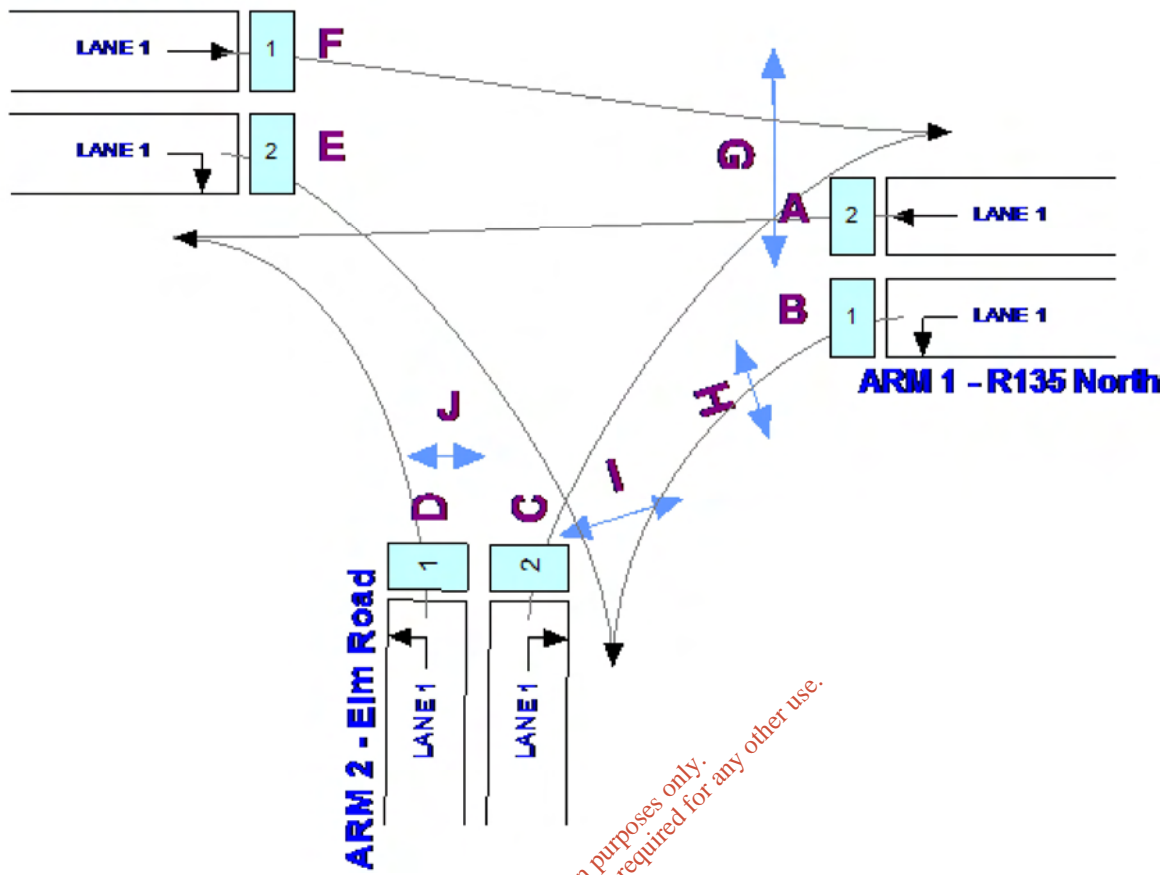
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
1	2	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
3	2	0.0	0.0	90	100	0	0.0	1.10	0
(Ped)	1	0.0	0.0	90	100	0	0.0	-	0
(Ped)	2	0.0	0.0	90	100	0	0.0	-	0
(Ped)	3	0.0	0.0	90	100	0	0.0	-	0
(Ped)	4	0.0	0.0	90	100	0	0.0	-	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2			1.00	100	Yes	3.50	0.0	0
1	2	1			3		0.00	10	No	3.50	0.0	0
2	1	1		3			1.00	12	Yes	3.50	0.0	0
2	2	1				1	1.00	12	No	3.50	0.0	0
3	1	1				1	0.00	10	Yes	3.50	0.0	0
3	2	1				2	1.00	10	No	3.50	0.0	0

Junction Diagram

ARM 3 - R135 South



Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No
G	(Name)	Pedestrian	-	7.0	0.0	No
H	(Name)	Pedestrian	-	7.0	0.0	No
I	(Name)	Pedestrian	-	7.0	0.0	No
J	(Name)	Pedestrian	-	7.0	0.0	No

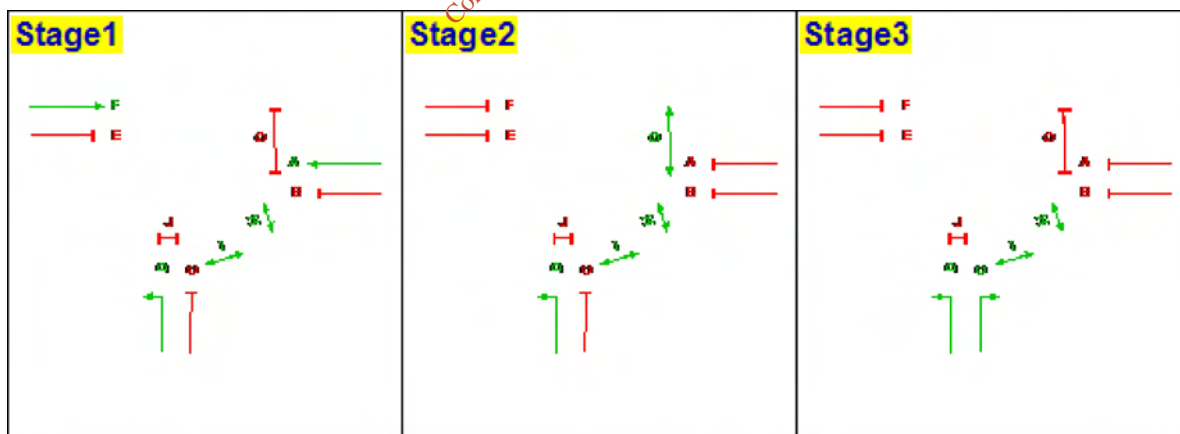
Intergreen Matrix

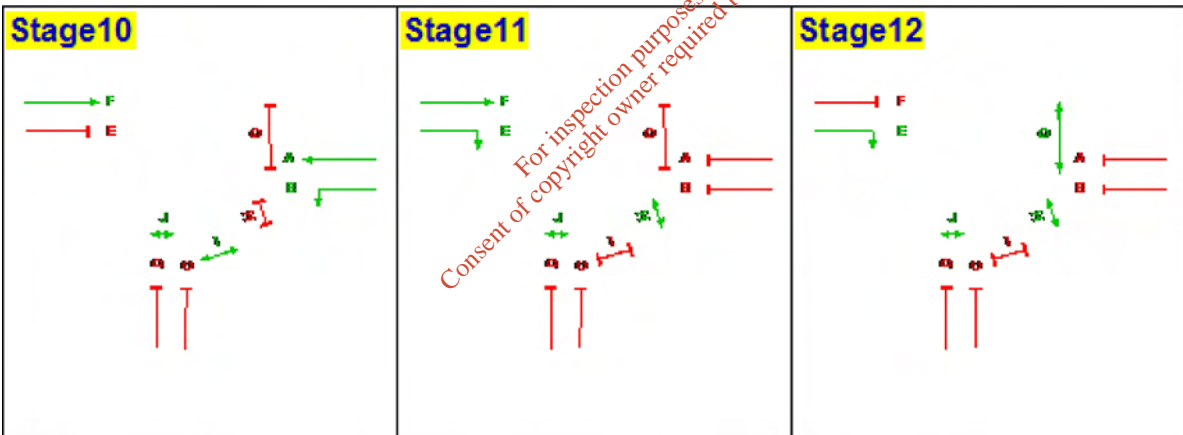
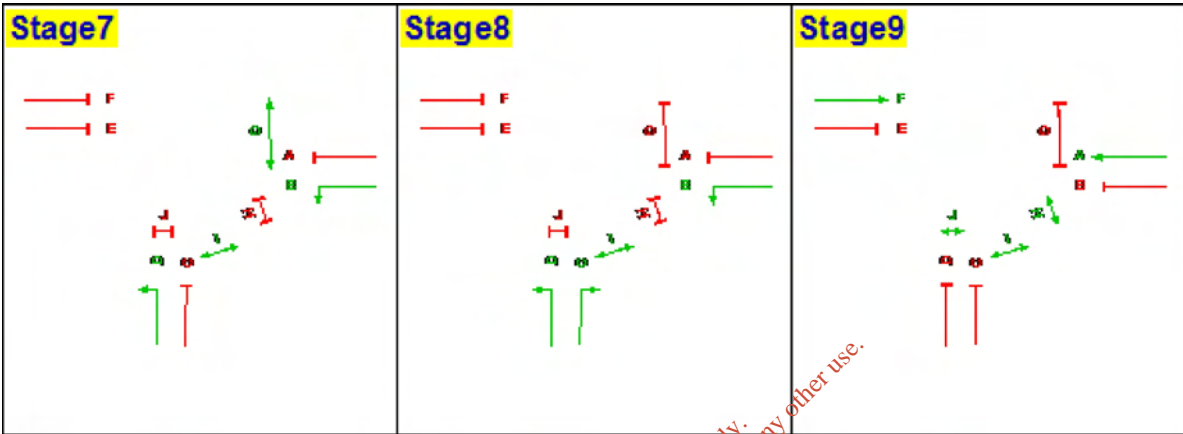
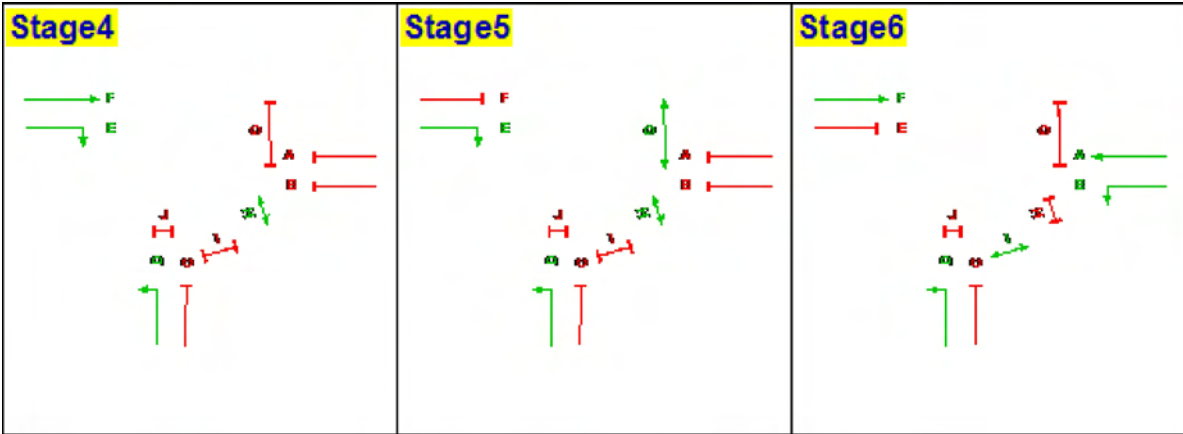
		To									
		A	B	C	D	E	F	G	H	I	J
From	A	-		5		5		5			
	B		-			5			5		
	C	5		-		5	5	8			5
	D				-						5
	E	5	6	5		-				7	
	F			5			-	8			
	G	5		5			5	-			
	H		6						-		
	I					5				-	
	J			5	7						-

Stages

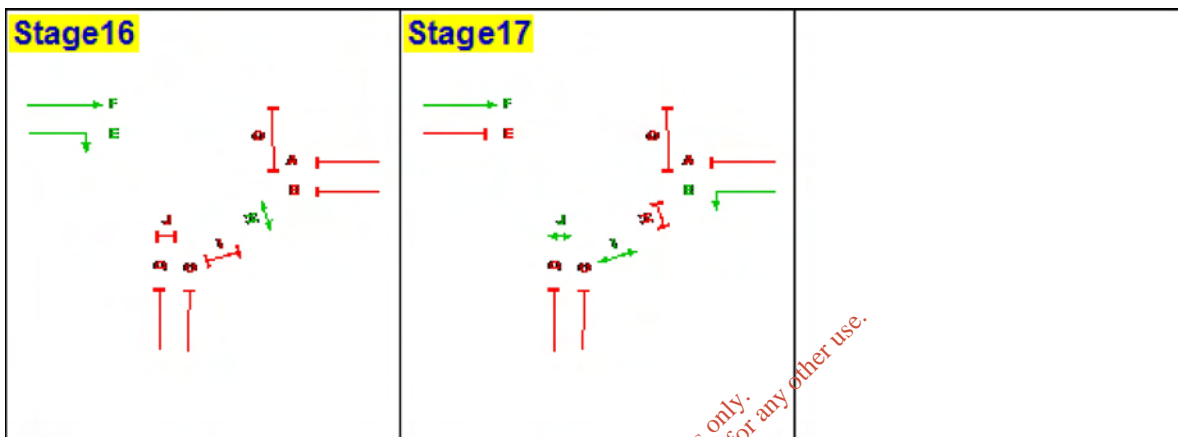
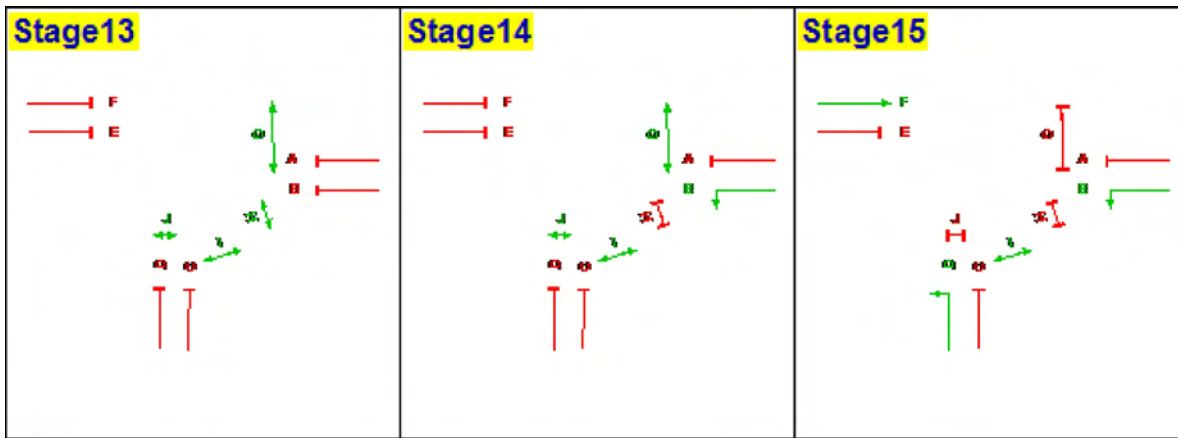
Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	A,D,F,H,I	Yes
2	-1	D,G,H,I	Yes
3	-1	C,D,H,I	Yes
4	-1	D,E,F,H	Yes
5	-1	D,E,G,H	Yes
6	-1	A,B,D,F,I	Yes
7	-1	B,D,G,I	Yes
8	-1	B,C,D,I	Yes
9	-1	A,F,H,I,J	Yes
10	-1	A,B,F,I,J	Yes
11	-1	E,F,H,J	Yes
12	-1	E,G,H,J	Yes
13	-1	G,H,I,J	Yes
14	-1	B,G,I,J	Yes
15	-1	B,D,F,I	Yes
16	-1	E,F,H	Yes
17	-1	B,F,I,J	Yes

Stage Diagrams





For inspection purposes only.
 Consent of copyright owner required for any other use.



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,13,12,13,14,7,8,6
2		1,6,8,7,14,13,12,13,2
3		1,2,13,14,13,2,3,4
4		1,4,3,2,13,14,13,2
5		1,2,13,12,13,14,13,2,3
6		1,3,2,13,14,13,12,13,2
7		1,2,13,14,13,12,13,2,3
8		1,3,2,13,12,13,14,13,2

Constraints

(No constraints)

For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	Yes	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set1 - 2016 AM Peak Existing

ODTAB Data (PCU/hr during central 60 min peak period)

		To		
		Arm 1	Arm 2	Arm 3
From	Arm 1	-	283	71
	Arm 2	22	-	4
	Arm 3	371	173	-

Average pedestrian flow on each pedestrian stream (if applicable): 1 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - R135 North	1	B	212	254	311	311	254	212
1 - R135 North	2	A	53	63	78	78	63	53
2 - Elm Road	1	D	3	3	4	4	3	3
2 - Elm Road	2	C	17	20	24	24	20	17
3 - R135 South	1	F	277	331	406	406	331	277
3 - R135 South	2	E	131	156	191	191	156	131
Pedestrians	1	J	1	1	1	1	1	1
Pedestrians	2	I	1	1	1	1	1	1
Pedestrians	3	H	1	1	1	1	1	1
Pedestrians	4	G	1	1	1	1	1	1

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - R135 North	80	20	-
2 - Elm Road	15	-	85
3 - R135 South	-	68	32

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	275.50	3.46	3.46	143.4

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	212.59	3.48	3.48	143.20

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
4	5.0	42.0	47.0
6	54.0	39.0	93.0
14	101.0	7.0	108.0
8	115.0	5.0	0.0

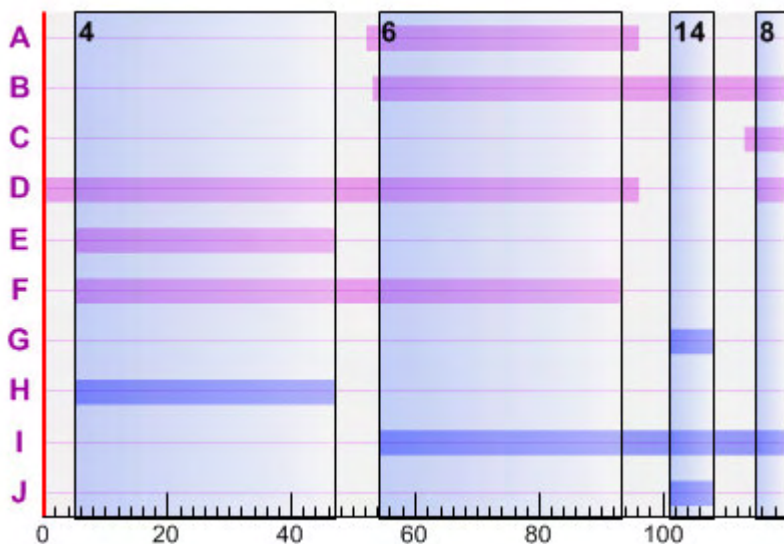
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	52	44.0	96						
B	53	67.0	0						
C	113	7.0	0						
D	115	101.0	96						
E	5	42.0	47						
F	5	88.0	93						
G	101	7.0	108						
H	5	42.0	47						
I	54	66.0	0						
J	101	7.0	108						

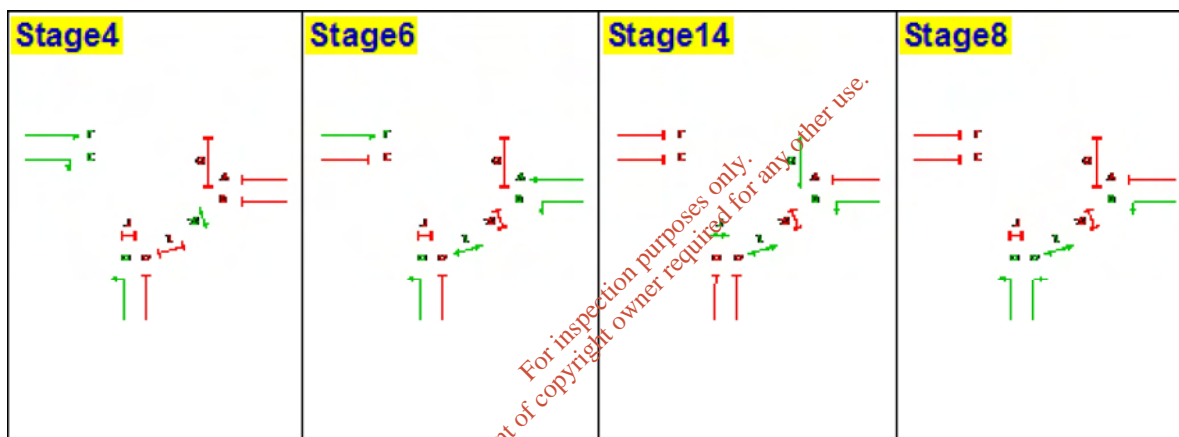
Phase Delays

Type	Phase	Terminating Stage	Starting Stage	Absolute Delay (s)	Relative Delay (s)
Losing	A	6	14	3.00	
Losing	D	6	14	3.00	

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	259	B	68.50	13.42	0.97	23.44	284.02	0.05	3.96	3.91	42.70
1	2	65	A	45.50	24.13	0.44	8.14	9999.00	0.00	1.37	1.36	7.80
2	1	3	D	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.90
2	2	20	C	8.50	55.54	0.31	15.09	496.38	0.02	0.64	0.62	0.60
3	1	338	F	89.50	5.17	0.49	23.06	290.24	0.05	3.12	3.07	74.90
3	2	159	E	43.50	27.83	1.23	23.97	275.50	0.05	3.54	3.49	16.50
Ped	1	1	J	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	I	67.50	11.49	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	H	43.50	24.39	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	G	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	212	2	68.50	12.92	0.76	19.18	369.16	0.03	3.20	3.17	6.00
1	2	53	1	45.50	23.93	0.35	6.64	9999.00	0.00	1.11	1.11	1.10
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	17	3	8.50	54.88	0.26	12.83	601.63	0.01	0.54	0.53	0.10
3	1	277	6	89.50	4.89	0.38	18.90	376.18	0.03	2.52	2.48	10.40
3	2	131	5	43.50	27.14	0.99	19.75	355.75	0.03	2.89	2.86	2.30
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	67.50	11.49	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	43.50	24.39	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	254	2	68.50	13.36	0.94	22.98	291.58	0.05	3.88	3.83	7.00
1	2	63	1	45.50	24.09	0.42	7.89	9999.00	0.00	1.32	1.32	1.30
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	20	3	8.50	55.53	0.31	15.09	496.38	0.02	0.64	0.62	0.10
3	1	331	6	89.50	5.13	0.47	22.59	298.49	0.05	3.05	3.00	12.30
3	2	156	5	43.50	27.76	1.20	23.52	282.72	0.05	3.47	3.42	2.70
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	67.50	11.49	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	43.50	24.39	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	311	2	68.50	14.01	1.21	28.14	219.81	0.08	4.82	4.74	8.30
1	2	78	1	45.50	24.34	0.53	9.77	820.94	0.01	1.65	1.64	1.60
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	24	3	8.50	56.42	0.38	18.11	396.98	0.03	0.77	0.75	0.10
3	1	406	6	89.50	5.51	0.62	27.70	224.88	0.08	3.82	3.74	14.70
3	2	191	5	43.50	28.67	1.52	28.79	212.59	0.08	4.30	4.22	3.20
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	67.50	11.49	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	43.50	24.39	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	311	2	68.50	14.01	1.21	28.14	219.81	0.08	4.82	4.74	8.30
1	2	78	1	45.50	24.34	0.53	9.77	820.94	0.01	1.65	1.64	1.60
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	24	3	8.50	56.42	0.38	18.11	396.98	0.03	0.77	0.75	0.10
3	1	406	6	89.50	5.51	0.62	27.70	224.88	0.08	3.82	3.74	14.70
3	2	191	5	43.50	28.67	1.52	28.79	212.59	0.08	4.30	4.22	3.20
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	67.50	11.49	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	43.50	24.39	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	254	2	68.50	13.36	0.94	22.98	291.58	0.05	3.88	3.83	7.00
1	2	63	1	45.50	24.10	0.42	7.89	9999.00	0.00	1.32	1.32	1.30
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	20	3	8.50	55.55	0.31	15.09	496.38	0.02	0.64	0.62	0.10
3	1	331	6	89.50	5.13	0.47	22.59	298.49	0.05	3.05	3.00	12.30
3	2	156	5	43.50	27.76	1.20	23.52	282.72	0.05	3.47	3.42	2.70
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	67.50	11.49	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	43.50	24.39	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	212	2	68.50	12.92	0.76	19.18	369.16	0.03	3.20	3.17	6.00
1	2	53	1	45.50	23.93	0.35	6.64	9999.00	0.00	1.11	1.11	1.10
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	17	3	8.50	54.93	0.26	12.83	601.63	0.01	0.54	0.53	0.10
3	1	277	6	89.50	4.89	0.38	18.90	376.18	0.03	2.52	2.48	10.40
3	2	131	5	43.50	27.15	0.99	19.75	355.75	0.03	2.89	2.86	2.30
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	67.50	11.49	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	43.50	24.39	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry TIA\Reports\Appendices\T-Junction.osc

Report generation date: 05/08/2016 16:46:24

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	R135 North	50.0	10	10	80	0
2	Elm Road	50.0	10	10	80	0
3	R135 South	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1936	Yes	0	B	-
1	2	Traffic		2105	Yes	0	A	-
2	1	Traffic		1747	Yes	0	D	-
2	2	Traffic		1871	Yes	0	C	-
3	1	Traffic		1965	Yes	0	F	-
3	2	Traffic		1830	Yes	0	E	-
(Ped)	1	Pedestrian		10000	Yes	0	J	-
(Ped)	2	Pedestrian		10000	Yes	0	I	-
(Ped)	3	Pedestrian		10000	Yes	0	H	-
(Ped)	4	Pedestrian		10000	Yes	0	G	-

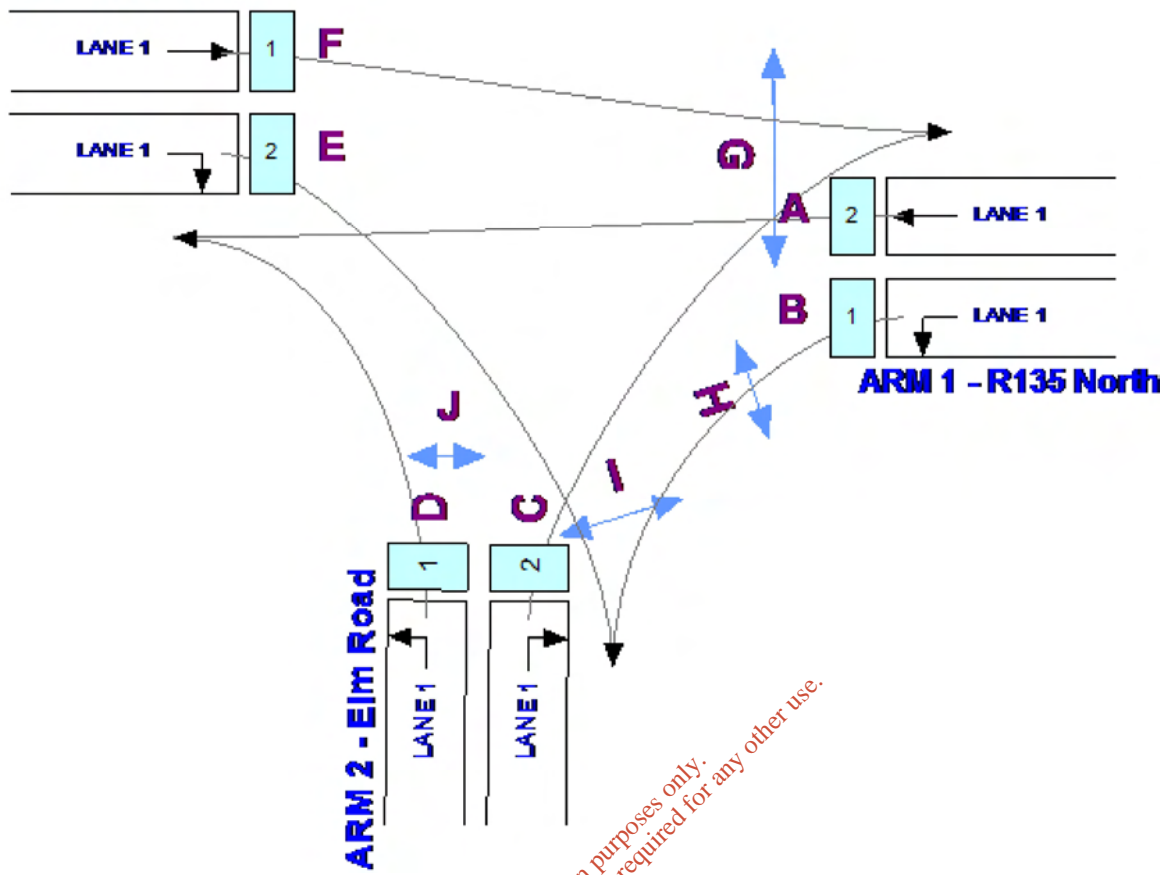
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
1	2	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
3	2	0.0	0.0	90	100	0	0.0	1.10	0
(Ped)	1	0.0	0.0	90	100	0	0.0	-	0
(Ped)	2	0.0	0.0	90	100	0	0.0	-	0
(Ped)	3	0.0	0.0	90	100	0	0.0	-	0
(Ped)	4	0.0	0.0	90	100	0	0.0	-	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2			1.00	100	Yes	3.50	0.0	0
1	2	1			3		0.00	10	No	3.50	0.0	0
2	1	1		3			1.00	12	Yes	3.50	0.0	0
2	2	1				1	1.00	12	No	3.50	0.0	0
3	1	1				1	0.00	10	Yes	3.50	0.0	0
3	2	1				2	1.00	10	No	3.50	0.0	0

Junction Diagram

ARM 3 - R135 South



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No
G	(Name)	Pedestrian	-	7.0	0.0	No
H	(Name)	Pedestrian	-	7.0	0.0	No
I	(Name)	Pedestrian	-	7.0	0.0	No
J	(Name)	Pedestrian	-	7.0	0.0	No

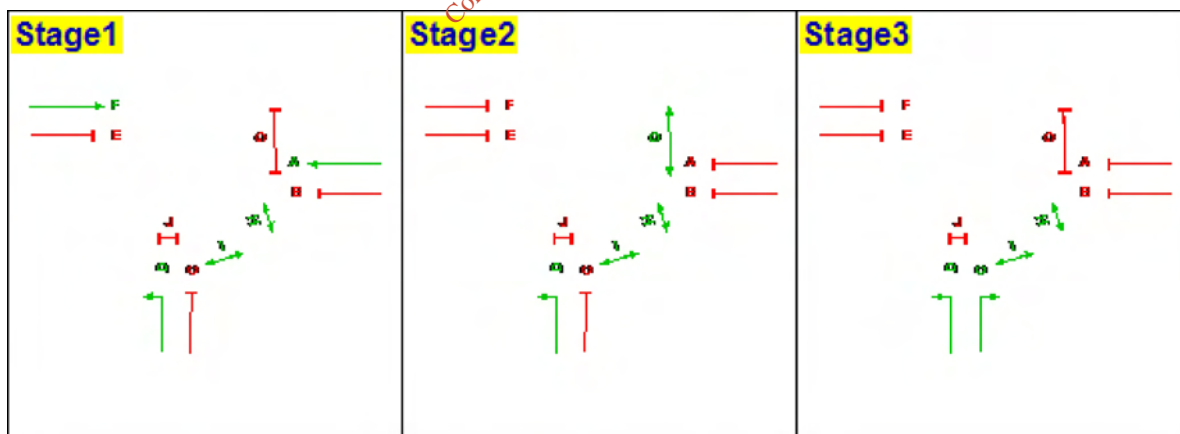
Intergreen Matrix

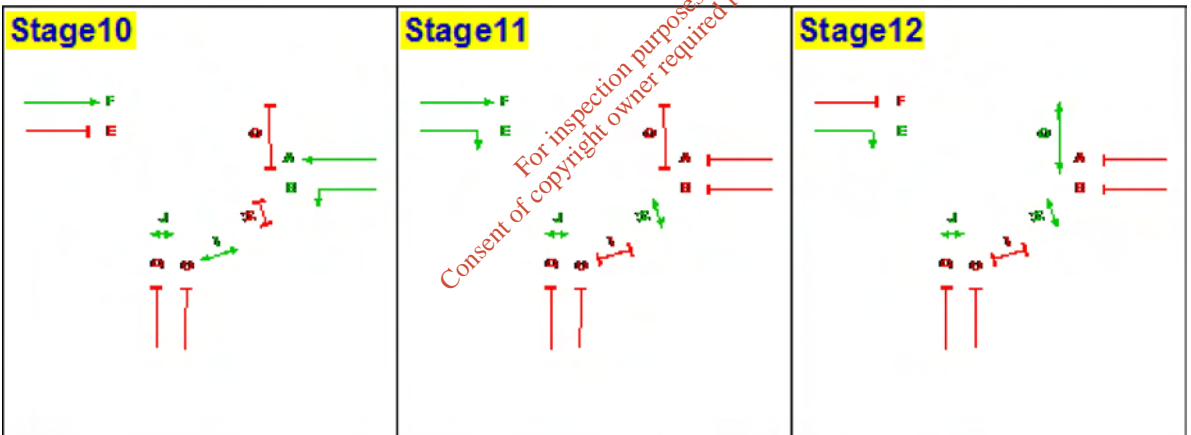
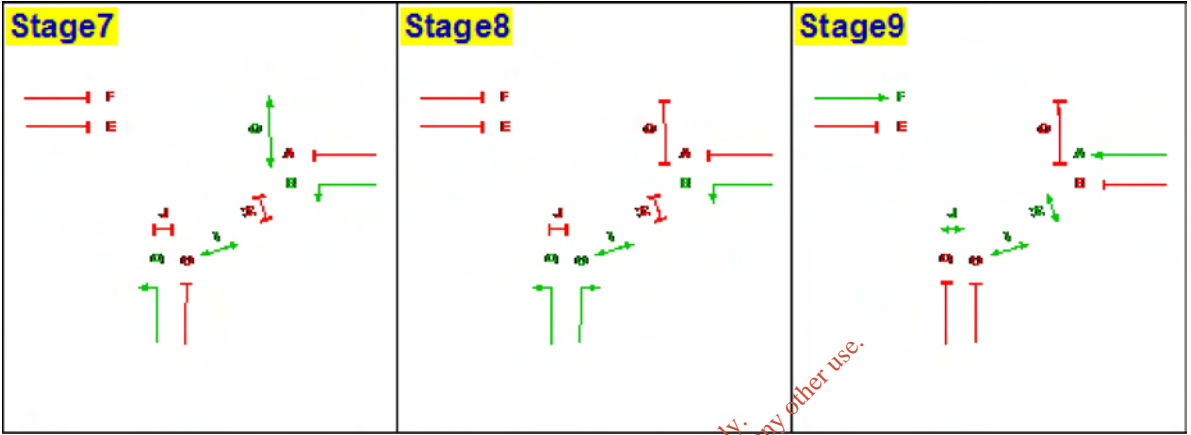
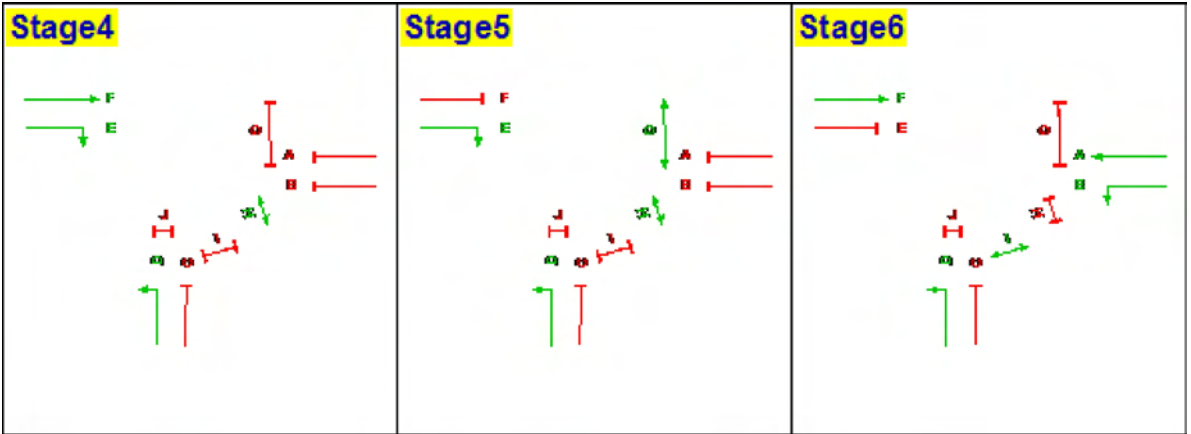
		To									
		A	B	C	D	E	F	G	H	I	J
From	A	-		5		5		5			
	B		-			5			5		
	C	5		-		5	5	8			5
	D				-						5
	E	5	6	5		-				7	
	F			5			-	8			
	G	5		5			5	-			
	H		6						-		
	I					5				-	
	J			5	7						-

Stages

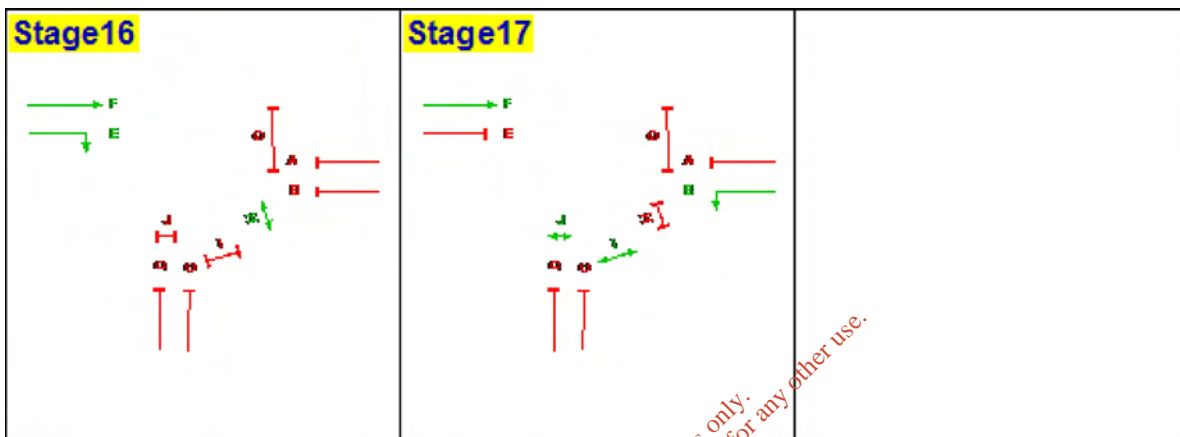
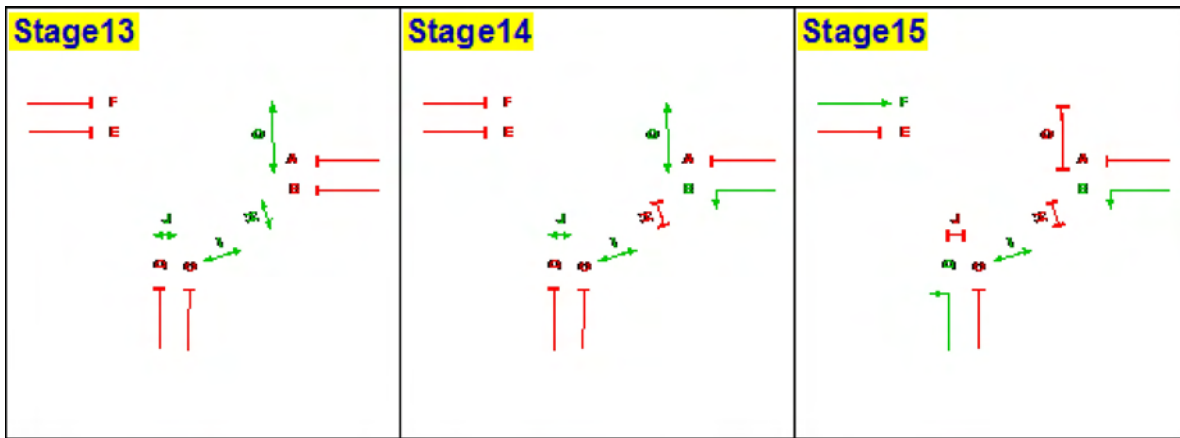
Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	A,D,F,H,I	Yes
2	-1	D,G,H,I	Yes
3	-1	C,D,H,I	Yes
4	-1	D,E,F,H	Yes
5	-1	D,E,G,H	Yes
6	-1	A,B,D,F,I	Yes
7	-1	B,D,G,I	Yes
8	-1	B,C,D,I	Yes
9	-1	A,F,H,I,J	Yes
10	-1	A,B,F,I,J	Yes
11	-1	E,F,H,J	Yes
12	-1	E,G,H,J	Yes
13	-1	G,H,I,J	Yes
14	-1	B,G,I,J	Yes
15	-1	B,D,F,I	Yes
16	-1	E,F,H	Yes
17	-1	B,F,I,J	Yes

Stage Diagrams





For inspection purposes only.
 Consent of copyright owner required for any other use.



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,13,12,13,14,7,8,6
2		1,6,8,7,14,13,12,13,2
3		1,2,13,14,13,2,3,4
4		1,4,3,2,13,14,13,2
5		1,2,13,12,13,14,13,2,3
6		1,3,2,13,14,13,12,13,2
7		1,2,13,14,13,12,13,2,3
8		1,3,2,13,12,13,14,13,2

Constraints

(No constraints)

For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	Yes	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set2 - 2017 AM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To		
		Arm 1	Arm 2	Arm 3
From	Arm 1	-	285	72
	Arm 2	22	-	4
	Arm 3	374	174	-

Average pedestrian flow on each pedestrian stream (if applicable): 1 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - R135 North	1	B	214	256	313	313	256	214
1 - R135 North	2	A	54	64	78	78	64	54
2 - Elm Road	1	D	3	3	4	4	3	3
2 - Elm Road	2	C	17	20	24	24	20	17
3 - R135 South	1	F	279	334	409	409	334	279
3 - R135 South	2	E	132	157	192	192	157	132
Pedestrians	1	J	1	1	1	1	1	1
Pedestrians	2	I	1	1	1	1	1	1
Pedestrians	3	H	1	1	1	1	1	1
Pedestrians	4	G	1	1	1	1	1	1

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - R135 North	80	20	-
2 - Elm Road	15	-	85
3 - R135 South	-	68	32

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	275.52	3.50	3.50	143.9

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	213.13	3.53	3.53	143.60

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
4	5.0	43.0	48.0
10	55.0	5.0	60.0
6	67.0	26.0	93.0
7	101.0	7.0	108.0
8	113.0	7.0	0.0

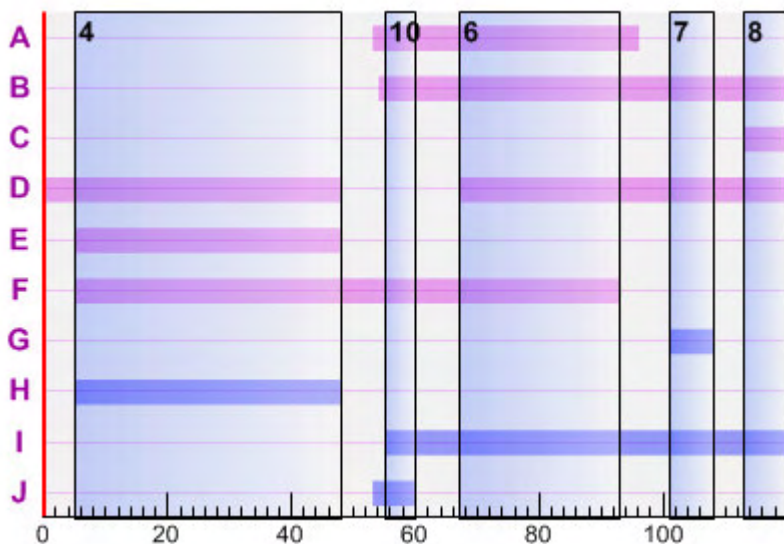
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	53	43.0	96						
B	54	66.0	0						
C	113	7.0	0						
D	67	101.0	48						
E	5	43.0	48						
F	5	88.0	93						
G	101	7.0	108						
H	5	43.0	48						
I	55	65.0	0						
J	53	7.0	60						

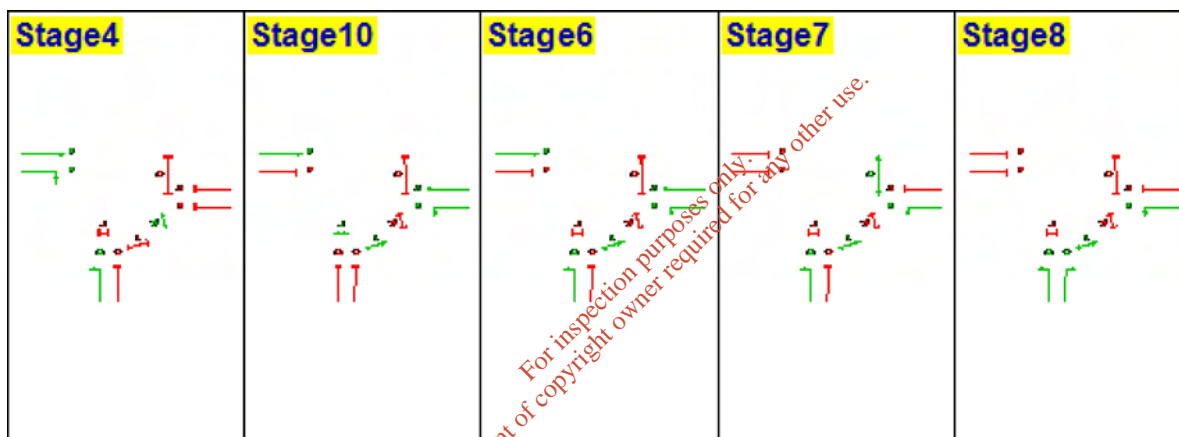
Phase Delays

Type	Phase	Terminating Stage	Starting Stage	Absolute Delay (s)	Relative Delay (s)
Losing	A	6	7	3.00	

Phase Timings Diagram



Final Stage Sequence



For inspection purposes only. Consent of copyright owner required for any other use.

Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	261	B	67.50	13.96	1.01	23.97	275.52	0.05	4.07	4.02	42.20
1	2	65	A	44.50	24.78	0.45	8.33	980.84	0.01	1.39	1.38	7.70
2	1	3	D	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.90
2	2	20	C	8.50	55.54	0.31	15.09	496.38	0.02	0.64	0.62	0.60
3	1	341	F	89.50	5.18	0.49	23.27	286.81	0.05	3.15	3.10	75.50
3	2	160	E	44.50	27.11	1.20	23.58	281.73	0.05	3.52	3.47	17.00
Ped	1	1	J	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	I	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	H	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	G	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	214	2	67.50	13.44	0.80	19.65	357.99	0.03	3.29	3.26	5.90
1	2	54	1	44.50	24.60	0.37	6.92	9999.00	0.00	1.15	1.14	1.10
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	17	3	8.50	54.88	0.26	12.83	601.63	0.01	0.54	0.53	0.10
3	1	279	6	89.50	4.90	0.38	19.04	372.76	0.03	2.53	2.50	10.50
3	2	132	5	44.50	26.44	0.97	19.45	362.70	0.03	2.88	2.84	2.40
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	256	2	67.50	13.90	0.99	23.51	282.85	0.05	3.98	3.93	6.90
1	2	64	1	44.50	24.77	0.44	8.20	997.72	0.00	1.36	1.36	1.30
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	20	3	8.50	55.53	0.31	15.09	496.38	0.02	0.64	0.62	0.10
3	1	334	6	89.50	5.15	0.48	22.79	294.91	0.05	3.08	3.03	12.40
3	2	157	5	44.50	27.03	1.18	23.14	289.02	0.05	3.45	3.40	2.80
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	313	2	67.50	14.58	1.27	28.74	213.13	0.08	4.95	4.87	8.20
1	2	78	1	44.50	25.00	0.54	9.99	800.70	0.01	1.67	1.66	1.50
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	24	3	8.50	56.42	0.38	18.11	396.98	0.03	0.77	0.75	0.10
3	1	409	6	89.50	5.52	0.63	27.91	222.50	0.08	3.85	3.77	14.80
3	2	192	5	44.50	27.91	1.49	28.29	218.11	0.08	4.26	4.19	3.30
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	313	2	67.50	14.58	1.27	28.74	213.13	0.08	4.95	4.87	8.20
1	2	78	1	44.50	25.00	0.54	9.99	800.70	0.01	1.67	1.66	1.50
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	24	3	8.50	56.42	0.38	18.11	396.98	0.03	0.77	0.75	0.10
3	1	409	6	89.50	5.52	0.63	27.91	222.50	0.08	3.85	3.77	14.80
3	2	192	5	44.50	27.92	1.49	28.29	218.11	0.08	4.26	4.19	3.30
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	256	2	67.50	13.91	0.99	23.51	282.85	0.05	3.99	3.93	6.90
1	2	64	1	44.50	24.77	0.44	8.20	997.72	0.00	1.36	1.36	1.30
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	20	3	8.50	55.55	0.31	15.09	496.38	0.02	0.64	0.62	0.10
3	1	334	6	89.50	5.15	0.48	22.79	294.91	0.05	3.08	3.03	12.40
3	2	157	5	44.50	27.04	1.18	23.14	289.02	0.05	3.45	3.40	2.80
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	214	2	67.50	13.45	0.80	19.65	357.99	0.03	3.29	3.26	5.90
1	2	54	1	44.50	24.60	0.37	6.92	9999.00	0.00	1.15	1.14	1.10
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	17	3	8.50	54.93	0.26	12.83	601.63	0.01	0.54	0.53	0.10
3	1	279	6	89.50	4.90	0.38	19.04	372.76	0.03	2.53	2.50	10.50
3	2	132	5	44.50	26.44	0.97	19.45	362.70	0.03	2.88	2.84	2.40
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry TIA\Reports\Appendices\T-Junction.osc

Report generation date: 05/08/2016 16:47:04

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	R135 North	50.0	10	10	80	0
2	Elm Road	50.0	10	10	80	0
3	R135 South	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1936	Yes	0	B	-
1	2	Traffic		2105	Yes	0	A	-
2	1	Traffic		1747	Yes	0	D	-
2	2	Traffic		1871	Yes	0	C	-
3	1	Traffic		1965	Yes	0	F	-
3	2	Traffic		1830	Yes	0	E	-
(Ped)	1	Pedestrian		10000	Yes	0	J	-
(Ped)	2	Pedestrian		10000	Yes	0	I	-
(Ped)	3	Pedestrian		10000	Yes	0	H	-
(Ped)	4	Pedestrian		10000	Yes	0	G	-

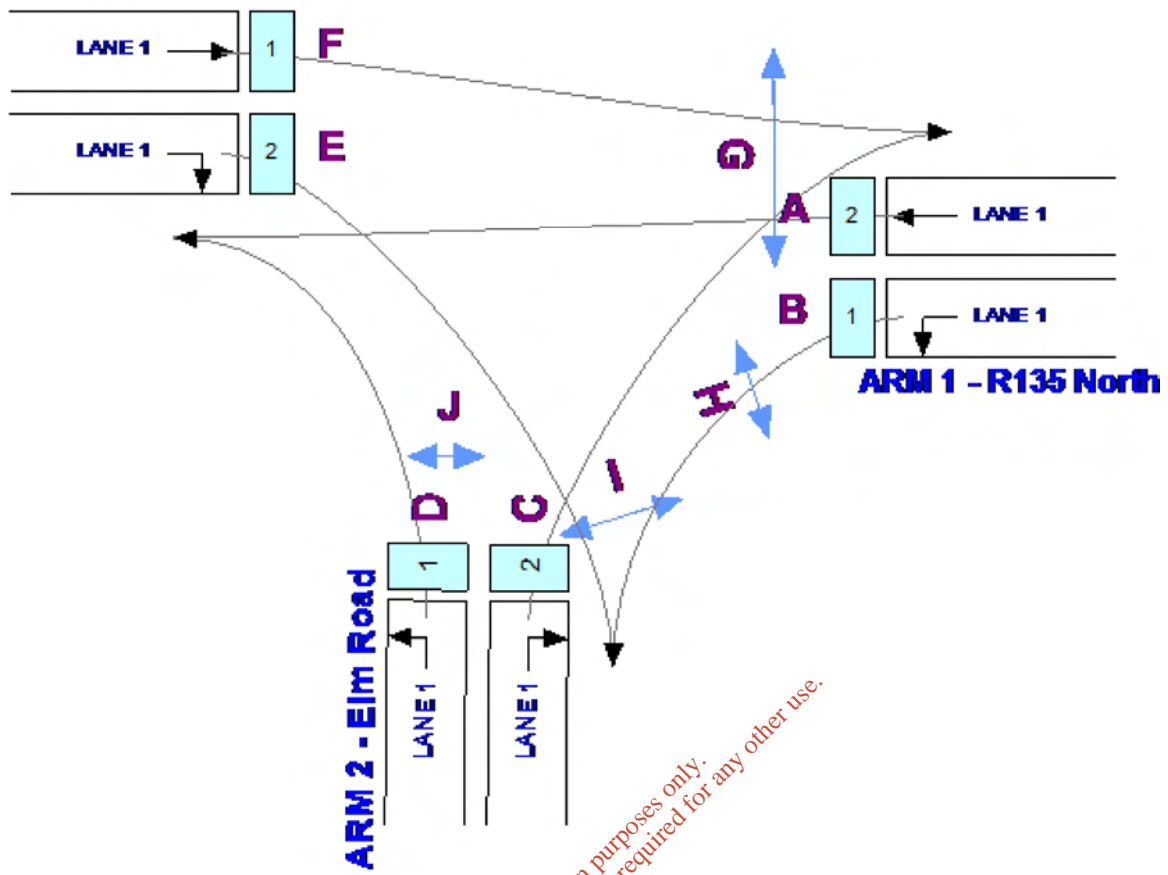
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
1	2	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
3	2	0.0	0.0	90	100	0	0.0	1.10	0
(Ped)	1	0.0	0.0	90	100	0	0.0	-	0
(Ped)	2	0.0	0.0	90	100	0	0.0	-	0
(Ped)	3	0.0	0.0	90	100	0	0.0	-	0
(Ped)	4	0.0	0.0	90	100	0	0.0	-	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2			1.00	100	Yes	3.50	0.0	0
1	2	1			3		0.00	10	No	3.50	0.0	0
2	1	1		3			1.00	12	Yes	3.50	0.0	0
2	2	1				1	1.00	12	No	3.50	0.0	0
3	1	1				1	0.00	10	Yes	3.50	0.0	0
3	2	1				2	1.00	10	No	3.50	0.0	0

Junction Diagram

ARM 3 - R135 South



Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No
G	(Name)	Pedestrian	-	7.0	0.0	No
H	(Name)	Pedestrian	-	7.0	0.0	No
I	(Name)	Pedestrian	-	7.0	0.0	No
J	(Name)	Pedestrian	-	7.0	0.0	No

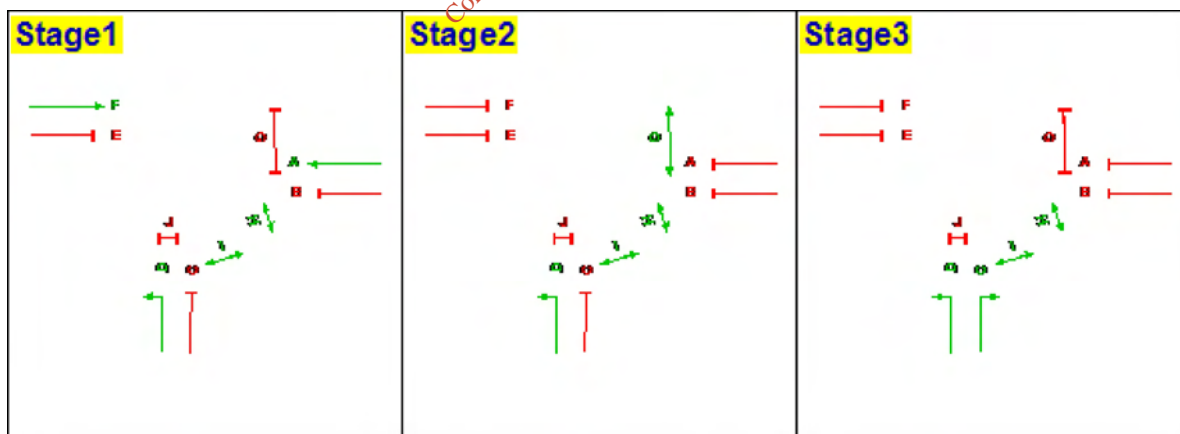
Intergreen Matrix

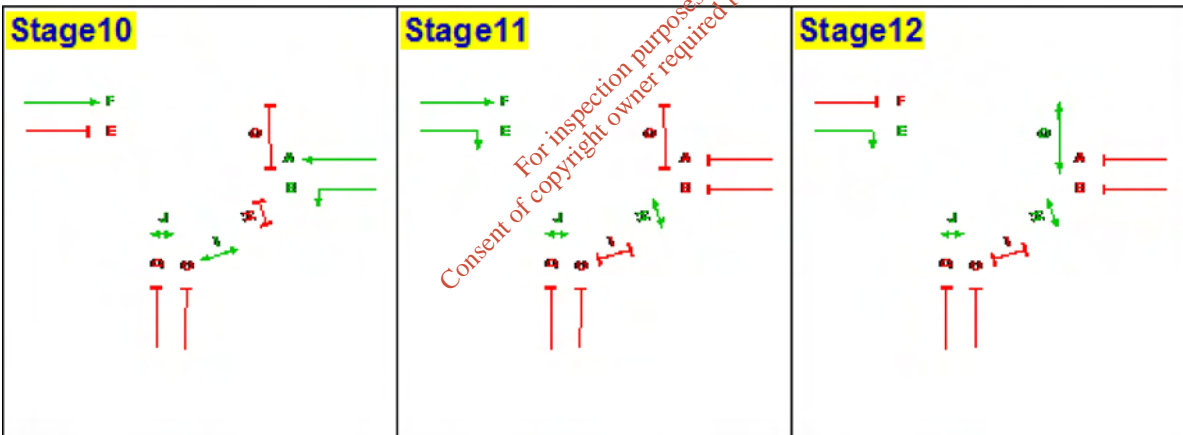
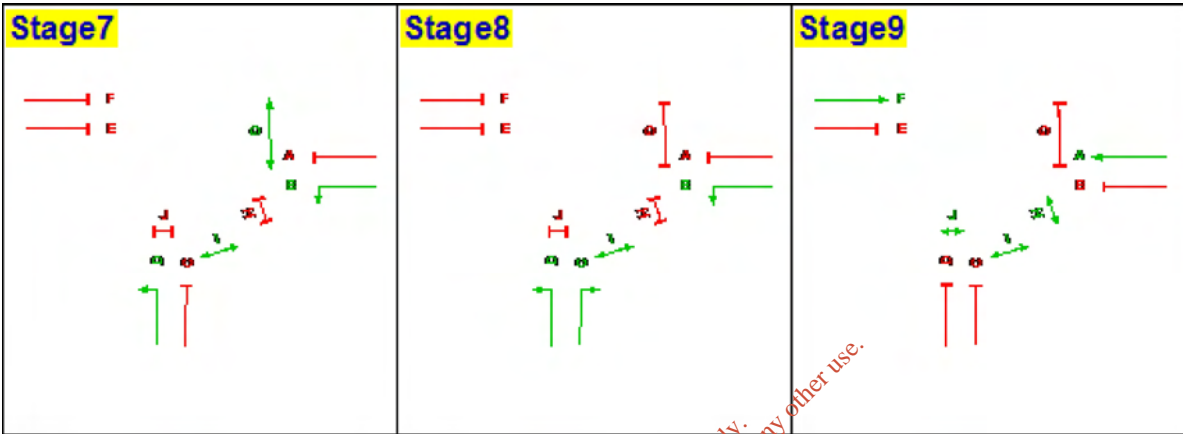
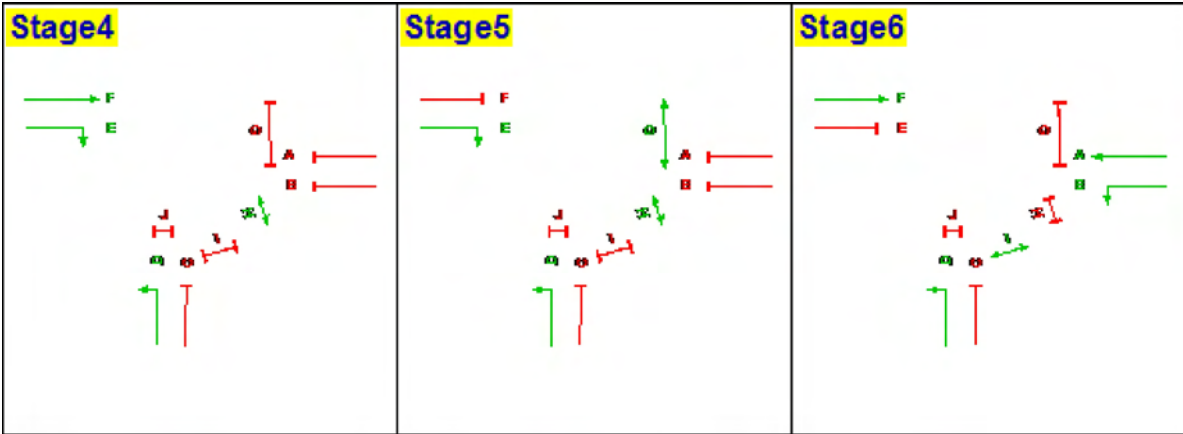
		To									
		A	B	C	D	E	F	G	H	I	J
From	A	-		5		5		5			
	B		-			5			5		
	C	5		-		5	5	8			5
	D				-						5
	E	5	6	5		-				7	
	F			5			-	8			
	G	5		5			5	-			
	H		6						-		
	I					5				-	
	J			5	7						-

Stages

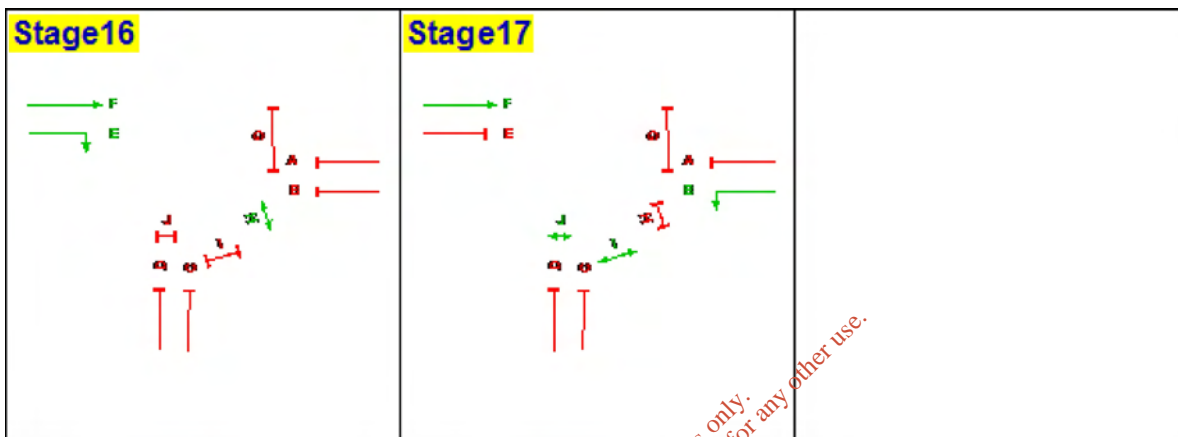
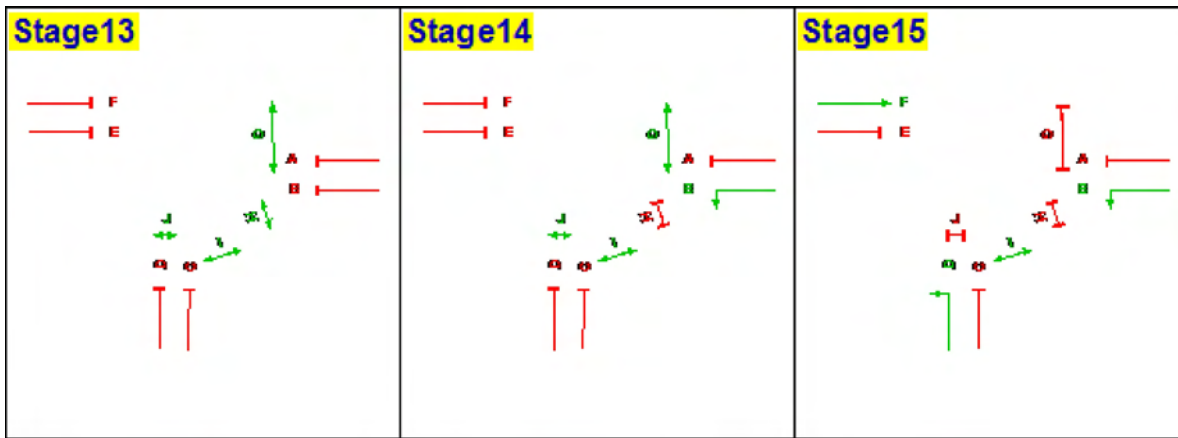
Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	A,D,F,H,I	Yes
2	-1	D,G,H,I	Yes
3	-1	C,D,H,I	Yes
4	-1	D,E,F,H	Yes
5	-1	D,E,G,H	Yes
6	-1	A,B,D,F,I	Yes
7	-1	B,D,G,I	Yes
8	-1	B,C,D,I	Yes
9	-1	A,F,H,I,J	Yes
10	-1	A,B,F,I,J	Yes
11	-1	E,F,H,J	Yes
12	-1	E,G,H,J	Yes
13	-1	G,H,I,J	Yes
14	-1	B,G,I,J	Yes
15	-1	B,D,F,I	Yes
16	-1	E,F,H	Yes
17	-1	B,F,I,J	Yes

Stage Diagrams





For inspection purposes only.
 Consent of copyright owner required for any other use.



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,13,12,13,14,7,8,6
2		1,6,8,7,14,13,12,13,2
3		1,2,13,14,13,2,3,4
4		1,4,3,2,13,14,13,2
5		1,2,13,12,13,14,13,2,3
6		1,3,2,13,14,13,12,13,2
7		1,2,13,14,13,12,13,2,3
8		1,3,2,13,12,13,14,13,2

Constraints

(No constraints)

For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	Yes	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set3 - 2017 AM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To		
		Arm 1	Arm 2	Arm 3
From	Arm 1	-	285	78
	Arm 2	22	-	4
	Arm 3	380	180	-

Average pedestrian flow on each pedestrian stream (if applicable): 1 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - R135 North	1	B	215	257	315	315	257	215
1 - R135 North	2	A	57	68	84	84	68	57
2 - Elm Road	1	D	3	3	4	4	3	3
2 - Elm Road	2	C	17	20	24	24	20	17
3 - R135 South	1	F	286	341	418	418	341	286
3 - R135 South	2	E	134	160	197	197	160	134
Pedestrians	1	J	1	1	1	1	1	1
Pedestrians	2	I	1	1	1	1	1	1
Pedestrians	3	H	1	1	1	1	1	1
Pedestrians	4	G	1	1	1	1	1	1

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - R135 North	79	21	-
2 - Elm Road	15	-	85
3 - R135 South	-	68	32

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	272.42	3.59	3.59	146.4

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	210.03	3.60	3.60	145.20

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
4	7.0	29.0	36.0
6	43.0	38.0	81.0
7	89.0	7.0	96.0
8	101.0	7.0	108.0
11	113.0	7.0	0.0

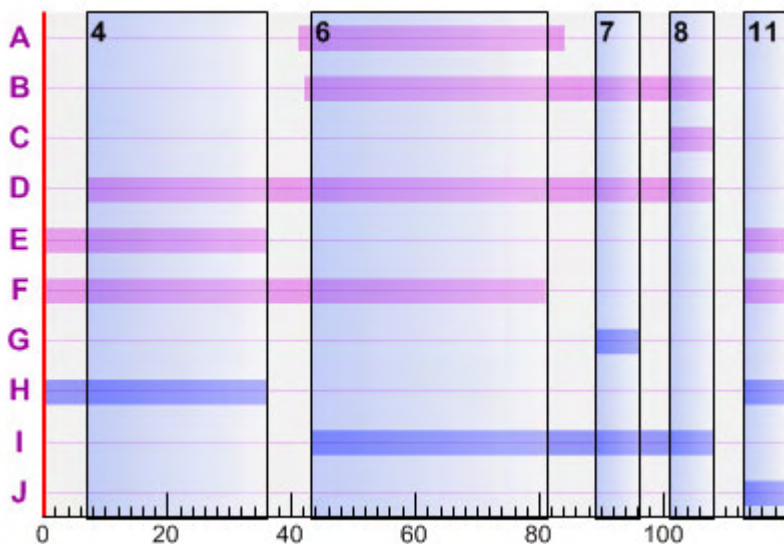
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	41	43.0	84						
B	42	66.0	108						
C	101	7.0	108						
D	7	101.0	108						
E	113	43.0	36						
F	113	88.0	81						
G	89	7.0	96						
H	113	43.0	36						
I	43	65.0	108						
J	113	7.0	0						

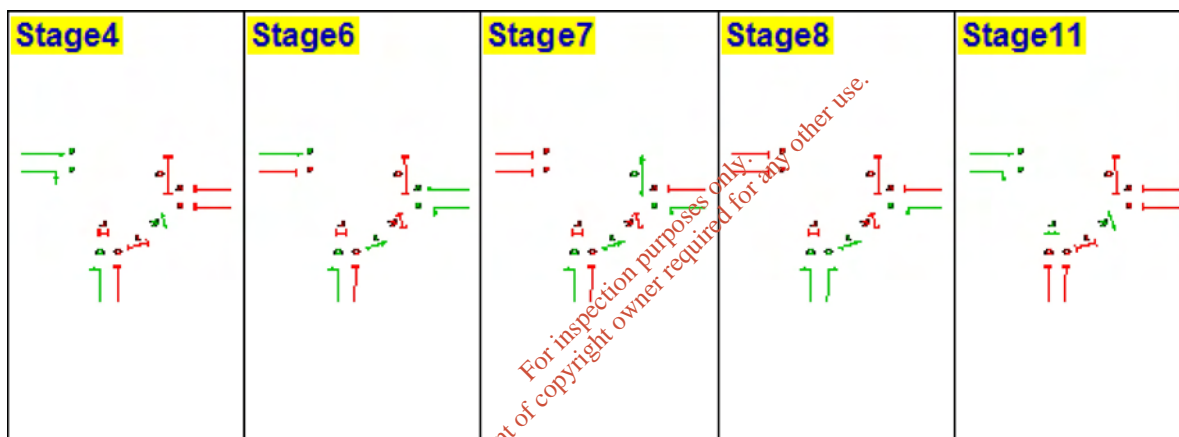
Phase Delays

Type	Phase	Terminating Stage	Starting Stage	Absolute Delay (s)	Relative Delay (s)
Losing	A	6	7	3.00	

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	262	B	67.50	13.97	1.02	24.06	274.08	0.05	4.09	4.03	42.40
1	2	70	A	44.50	24.87	0.48	8.97	903.63	0.01	1.49	1.49	8.20
2	1	3	D	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.90
2	2	20	C	8.50	55.54	0.31	15.09	496.38	0.02	0.64	0.62	0.60
3	1	348	F	89.50	5.21	0.50	23.75	279.02	0.05	3.22	3.17	77.00
3	2	164	E	44.50	27.20	1.24	24.17	272.42	0.05	3.61	3.56	17.30
Ped	1	1	J	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	I	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	H	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	G	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	215	2	67.50	13.45	0.80	19.74	355.86	0.03	3.31	3.28	5.90
1	2	57	1	44.50	24.65	0.39	7.30	9999.00	0.00	1.21	1.21	1.10
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	17	3	8.50	54.88	0.26	12.83	601.63	0.01	0.54	0.53	0.10
3	1	286	6	89.50	4.93	0.39	19.51	361.19	0.03	2.60	2.57	10.70
3	2	134	5	44.50	26.49	0.99	19.75	355.79	0.03	2.92	2.89	2.40
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	257	2	67.50	13.92	0.99	23.60	281.36	0.05	4.00	3.95	6.90
1	2	68	1	44.50	24.83	0.47	8.71	933.15	0.01	1.45	1.45	1.30
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	20	3	8.50	55.53	0.31	15.09	496.38	0.02	0.64	0.62	0.10
3	1	341	6	89.50	5.18	0.49	23.27	286.81	0.05	3.15	3.10	12.60
3	2	160	5	44.50	27.10	1.20	23.58	281.73	0.05	3.52	3.47	2.80
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	315	2	67.50	14.60	1.28	28.93	211.14	0.08	4.99	4.90	8.20
1	2	84	1	44.50	25.10	0.59	10.76	736.36	0.01	1.80	1.79	1.60
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	24	3	8.50	56.42	0.38	18.11	396.98	0.03	0.77	0.75	0.10
3	1	418	6	89.50	5.57	0.65	28.52	215.55	0.08	3.95	3.86	15.10
3	2	197	5	44.50	28.04	1.53	29.03	210.03	0.08	4.38	4.30	3.30
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	315	2	67.50	14.60	1.28	28.93	211.14	0.08	4.99	4.90	8.20
1	2	84	1	44.50	25.11	0.59	10.76	736.36	0.01	1.80	1.79	1.60
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	24	3	8.50	56.42	0.38	18.11	396.98	0.03	0.77	0.75	0.10
3	1	418	6	89.50	5.57	0.65	28.52	215.55	0.08	3.95	3.86	15.10
3	2	197	5	44.50	28.05	1.53	29.03	210.03	0.08	4.38	4.30	3.30
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	257	2	67.50	13.92	0.99	23.60	281.36	0.05	4.00	3.95	6.90
1	2	68	1	44.50	24.83	0.47	8.71	933.15	0.01	1.45	1.45	1.30
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	20	3	8.50	55.55	0.31	15.09	496.38	0.02	0.64	0.62	0.10
3	1	341	6	89.50	5.18	0.49	23.27	286.81	0.05	3.15	3.10	12.60
3	2	160	5	44.50	27.11	1.20	23.58	281.73	0.05	3.52	3.47	2.80
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	215	2	67.50	13.46	0.80	19.74	355.86	0.03	3.31	3.28	5.90
1	2	57	1	44.50	24.65	0.39	7.30	9999.00	0.00	1.21	1.21	1.10
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	17	3	8.50	54.93	0.26	12.83	601.63	0.01	0.54	0.53	0.10
3	1	286	6	89.50	4.93	0.39	19.51	361.19	0.03	2.60	2.57	10.70
3	2	134	5	44.50	26.49	0.99	19.75	355.79	0.03	2.92	2.89	2.40
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry TIA\Reports\Appendices\T-Junction.osc

Report generation date: 05/08/2016 16:47:44

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	R135 North	50.0	10	10	80	0
2	Elm Road	50.0	10	10	80	0
3	R135 South	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1936	Yes	0	B	-
1	2	Traffic		2105	Yes	0	A	-
2	1	Traffic		1747	Yes	0	D	-
2	2	Traffic		1871	Yes	0	C	-
3	1	Traffic		1965	Yes	0	F	-
3	2	Traffic		1830	Yes	0	E	-
(Ped)	1	Pedestrian		10000	Yes	0	J	-
(Ped)	2	Pedestrian		10000	Yes	0	I	-
(Ped)	3	Pedestrian		10000	Yes	0	H	-
(Ped)	4	Pedestrian		10000	Yes	0	G	-

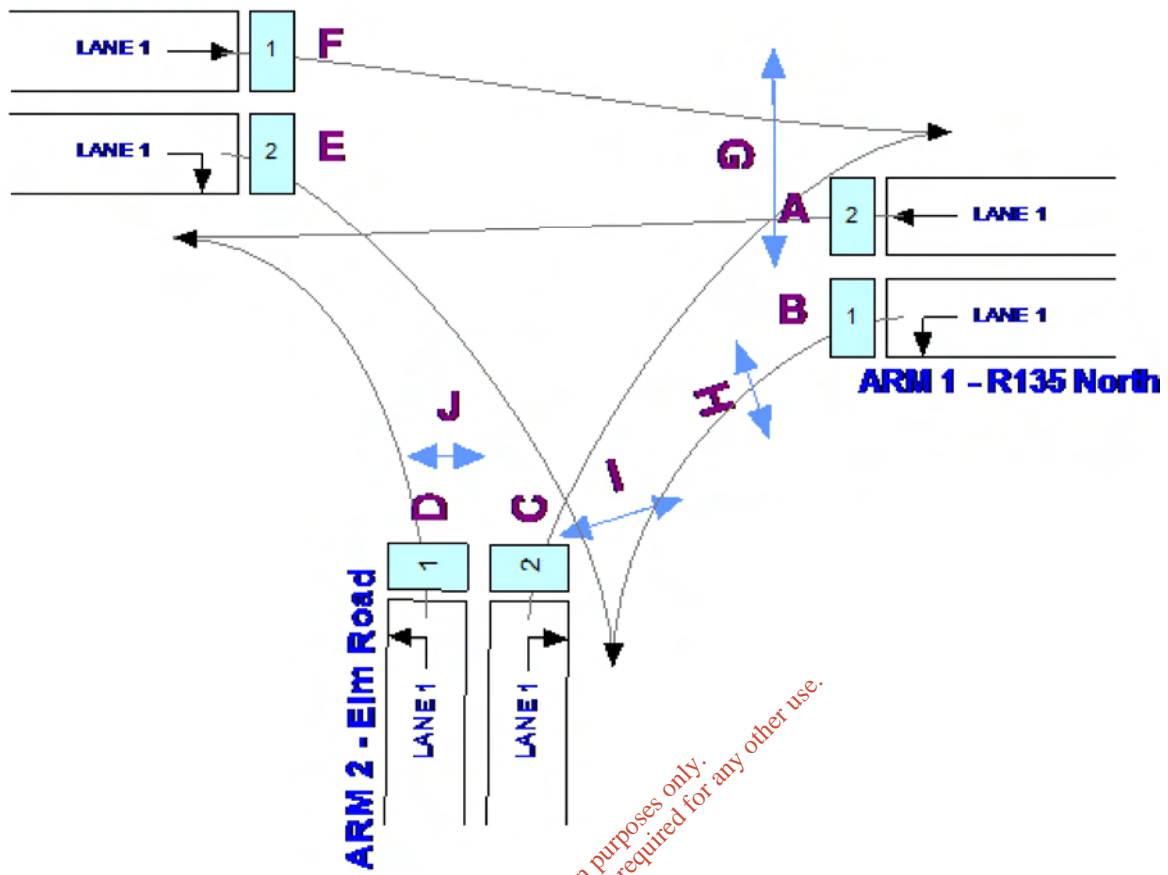
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
1	2	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
3	2	0.0	0.0	90	100	0	0.0	1.10	0
(Ped)	1	0.0	0.0	90	100	0	0.0	-	0
(Ped)	2	0.0	0.0	90	100	0	0.0	-	0
(Ped)	3	0.0	0.0	90	100	0	0.0	-	0
(Ped)	4	0.0	0.0	90	100	0	0.0	-	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2			1.00	100	Yes	3.50	0.0	0
1	2	1			3		0.00	10	No	3.50	0.0	0
2	1	1		3			1.00	12	Yes	3.50	0.0	0
2	2	1				1	1.00	12	No	3.50	0.0	0
3	1	1				1	0.00	10	Yes	3.50	0.0	0
3	2	1				2	1.00	10	No	3.50	0.0	0

Junction Diagram

ARM 3 - R135 South



Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No
G	(Name)	Pedestrian	-	7.0	0.0	No
H	(Name)	Pedestrian	-	7.0	0.0	No
I	(Name)	Pedestrian	-	7.0	0.0	No
J	(Name)	Pedestrian	-	7.0	0.0	No

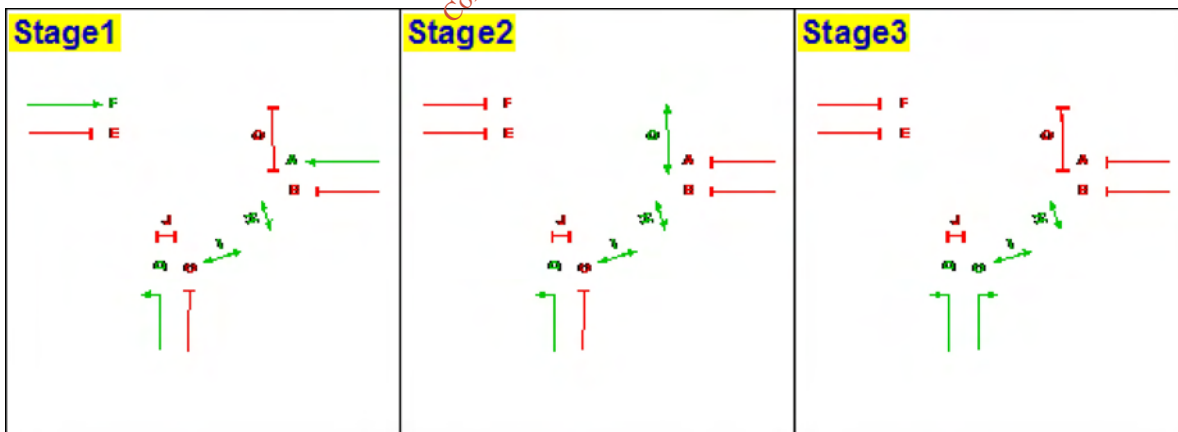
Intergreen Matrix

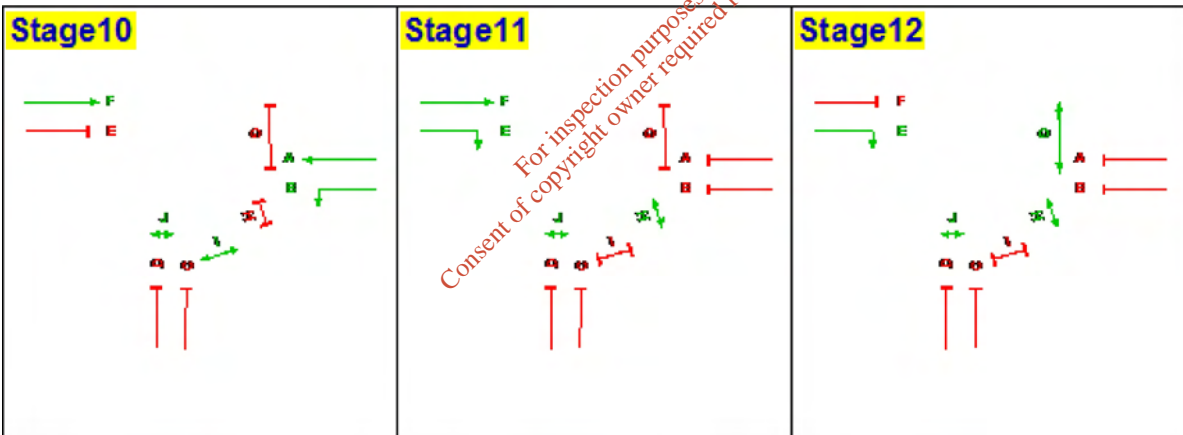
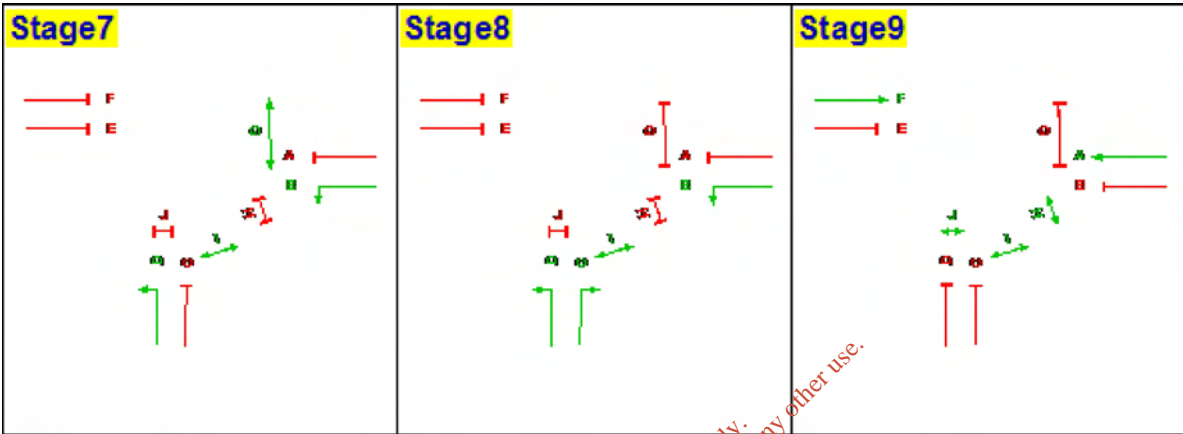
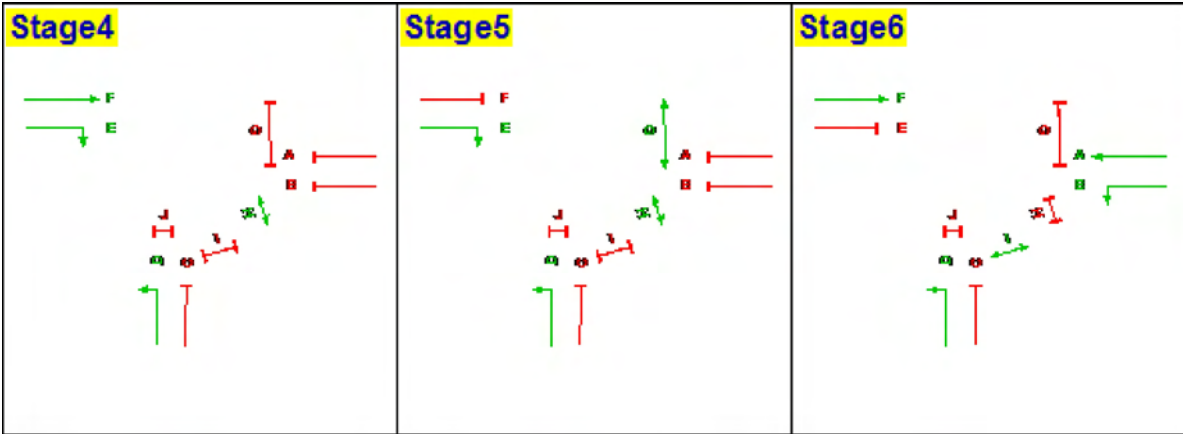
		To									
		A	B	C	D	E	F	G	H	I	J
From	A	-		5		5		5			
	B		-			5			5		
	C	5		-		5	5	8			5
	D				-						5
	E	5	6	5		-				7	
	F			5			-	8			
	G	5		5			5	-			
	H		6						-		
	I					5				-	
	J			5	7						-

Stages

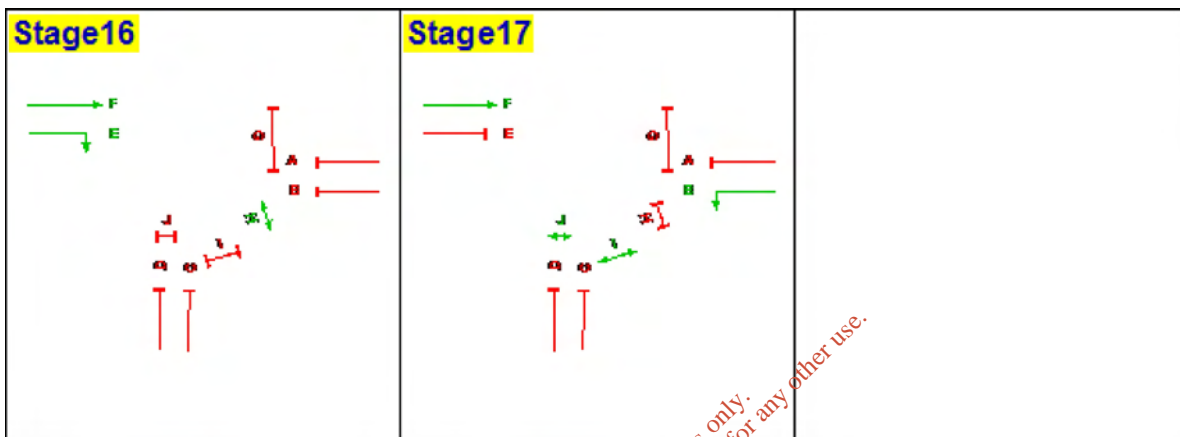
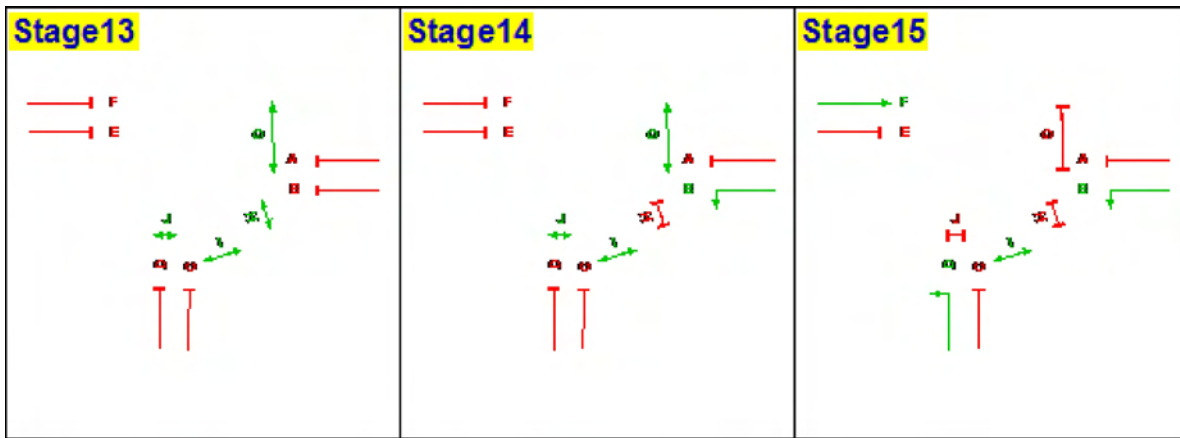
Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	A,D,F,H,I	Yes
2	-1	D,G,H,I	Yes
3	-1	C,D,H,I	Yes
4	-1	D,E,F,H	Yes
5	-1	D,E,G,H	Yes
6	-1	A,B,D,F,I	Yes
7	-1	B,D,G,I	Yes
8	-1	B,C,D,I	Yes
9	-1	A,F,H,I,J	Yes
10	-1	A,B,F,I,J	Yes
11	-1	E,F,H,J	Yes
12	-1	E,G,H,J	Yes
13	-1	G,H,I,J	Yes
14	-1	B,G,I,J	Yes
15	-1	B,D,F,I	Yes
16	-1	E,F,H	Yes
17	-1	B,F,I,J	Yes

Stage Diagrams





For inspection purposes only.
Consent of copyright owner required for any other use.



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,13,12,13,14,7,8,6
2		1,6,8,7,14,13,12,13,2
3		1,2,13,14,13,2,3,4
4		1,4,3,2,13,14,13,2
5		1,2,13,12,13,14,13,2,3
6		1,3,2,13,14,13,12,13,2
7		1,2,13,14,13,12,13,2,3
8		1,3,2,13,12,13,14,13,2

Constraints

(No constraints)

For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	Yes	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set4 - 2023 AM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To		
		Arm 1	Arm 2	Arm 3
From	Arm 1	-	297	75
	Arm 2	23	-	4
	Arm 3	390	182	-

Average pedestrian flow on each pedestrian stream (if applicable): 1 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - R135 North	1	B	223	267	326	326	267	223
1 - R135 North	2	A	56	67	82	82	67	56
2 - Elm Road	1	D	3	4	4	4	4	3
2 - Elm Road	2	C	17	21	25	25	21	17
3 - R135 South	1	F	292	348	427	427	348	292
3 - R135 South	2	E	137	164	201	201	164	137
Pedestrians	1	J	1	1	1	1	1	1
Pedestrians	2	I	1	1	1	1	1	1
Pedestrians	3	H	1	1	1	1	1	1
Pedestrians	4	G	1	1	1	1	1	1

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - R135 North	80	20	-
2 - Elm Road	15	-	85
3 - R135 South	-	68	32

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	260.33	3.73	3.73	148.6

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	200.64	3.74	3.74	148.00

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
8	7.0	7.0	14.0
11	19.0	43.0	62.0
10	69.0	38.0	107.0
14	115.0	5.0	0.0

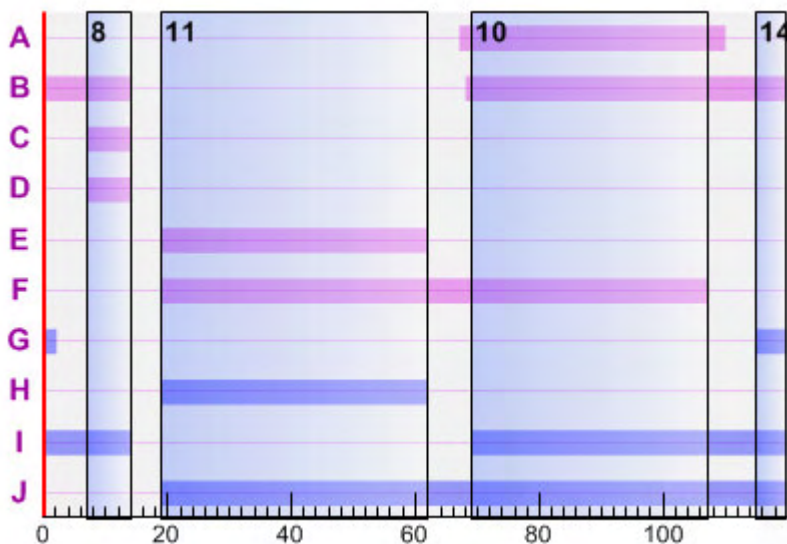
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	67	43.0	110						
B	68	66.0	14						
C	7	7.0	14						
D	7	7.0	14						
E	19	43.0	62						
F	19	88.0	107						
G	115	7.0	2						
H	19	43.0	62						
I	69	65.0	14						
J	19	101.0	0						

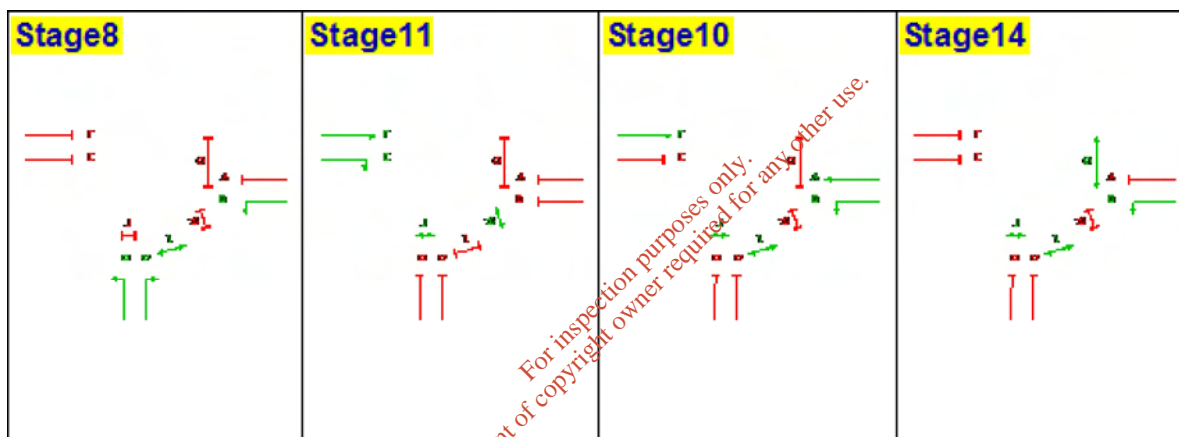
Phase Delays

Type	Phase	Terminating Stage	Starting Stage	Absolute Delay (s)	Relative Delay (s)
Losing	A	10	14	3.00	
Losing	G	14	8	2.00	

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	272	B	67.50	14.09	1.06	24.98	260.33	0.06	4.25	4.19	43.70
1	2	68	A	44.50	24.83	0.47	8.71	933.15	0.01	1.45	1.45	8.10
2	1	4	D	8.50	52.56	0.06	3.23	9999.00	0.00	0.12	0.12	0.10
2	2	21	C	8.50	55.76	0.33	15.85	467.98	0.02	0.67	0.65	0.60
3	1	356	F	89.50	5.25	0.52	24.29	270.51	0.06	3.30	3.25	78.50
3	2	167	E	44.50	27.28	1.27	24.61	265.73	0.05	3.68	3.62	17.60
Ped	1	1	J	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	I	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	H	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	G	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	223	2	67.50	13.54	0.84	20.48	339.51	0.04	3.44	3.40	6.10
1	2	56	1	44.50	24.64	0.38	7.17	9999.00	0.00	1.19	1.19	1.10
2	1	3	4	8.50	52.37	0.04	2.42	9999.00	0.00	0.09	0.09	0.00
2	2	17	3	8.50	54.88	0.26	12.83	601.63	0.01	0.54	0.53	0.10
3	1	292	6	89.50	4.96	0.40	19.92	351.71	0.03	2.66	2.63	10.90
3	2	137	5	44.50	26.55	1.01	20.19	345.81	0.03	2.99	2.95	2.50
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	267	2	67.50	14.03	1.04	24.52	267.08	0.06	4.17	4.11	7.20
1	2	67	1	44.50	24.82	0.46	8.58	948.57	0.01	1.43	1.42	1.30
2	1	4	4	8.50	52.56	0.06	3.23	9999.00	0.00	0.12	0.12	0.00
2	2	21	3	8.50	55.74	0.33	15.85	462.98	0.02	0.67	0.65	0.10
3	1	348	6	89.50	5.21	0.50	23.75	279.02	0.05	3.22	3.17	12.80
3	2	164	5	44.50	27.20	1.24	24.17	272.42	0.05	3.61	3.56	2.90
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	326	2	67.50	14.74	1.33	29.94	200.64	0.09	5.18	5.09	8.50
1	2	82	1	44.50	25.07	0.57	10.50	756.76	0.01	1.76	1.75	1.60
2	1	4	4	8.50	52.56	0.06	3.23	9999.00	0.00	0.12	0.12	0.00
2	2	25	3	8.50	56.64	0.39	18.86	377.11	0.03	0.81	0.78	0.10
3	1	427	6	89.50	5.62	0.67	29.14	208.90	0.09	4.04	3.96	15.40
3	2	201	5	44.50	28.15	1.57	29.62	203.86	0.09	4.48	4.39	3.40
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	326	2	67.50	14.74	1.33	29.94	200.64	0.09	5.18	5.09	8.50
1	2	82	1	44.50	25.07	0.57	10.50	756.76	0.01	1.76	1.75	1.60
2	1	4	4	8.50	52.56	0.06	3.23	9999.00	0.00	0.12	0.12	0.00
2	2	25	3	8.50	56.67	0.39	18.86	377.11	0.03	0.81	0.78	0.10
3	1	427	6	89.50	5.62	0.67	29.14	208.90	0.09	4.04	3.96	15.40
3	2	201	5	44.50	28.15	1.57	29.62	203.86	0.09	4.48	4.39	3.40
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	267	2	67.50	14.03	1.04	24.52	267.08	0.06	4.17	4.11	7.20
1	2	67	1	44.50	24.82	0.46	8.58	948.57	0.01	1.43	1.42	1.30
2	1	4	4	8.50	52.56	0.06	3.23	9999.00	0.00	0.12	0.12	0.00
2	2	21	3	8.50	55.79	0.33	15.85	462.98	0.02	0.67	0.65	0.10
3	1	348	6	89.50	5.22	0.50	23.75	279.02	0.05	3.22	3.17	12.80
3	2	164	5	44.50	27.21	1.24	24.17	272.42	0.05	3.61	3.56	2.90
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	223	2	67.50	13.54	0.84	20.48	339.51	0.04	3.44	3.40	6.10
1	2	56	1	44.50	24.64	0.38	7.17	9999.00	0.00	1.19	1.19	1.10
2	1	3	4	8.50	52.37	0.04	2.42	9999.00	0.00	0.09	0.09	0.00
2	2	17	3	8.50	54.93	0.26	12.83	601.63	0.01	0.54	0.53	0.10
3	1	292	6	89.50	4.96	0.40	19.92	351.71	0.03	2.66	2.63	10.90
3	2	137	5	44.50	26.56	1.01	20.19	345.81	0.03	2.99	2.95	2.50
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry TIA\Reports\Appendices\T-Junction.osc

Report generation date: 05/08/2016 17:02:10

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

For inspection purposes only.
Consent of copyright owner required for any other use.

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	R135 North	50.0	10	10	80	0
2	Elm Road	50.0	10	10	80	0
3	R135 South	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1936	Yes	0	B	-
1	2	Traffic		2105	Yes	0	A	-
2	1	Traffic		1747	Yes	0	D	-
2	2	Traffic		1871	Yes	0	C	-
3	1	Traffic		1965	Yes	0	F	-
3	2	Traffic		1830	Yes	0	E	-
(Ped)	1	Pedestrian		10000	Yes	0	J	-
(Ped)	2	Pedestrian		10000	Yes	0	I	-
(Ped)	3	Pedestrian		10000	Yes	0	H	-
(Ped)	4	Pedestrian		10000	Yes	0	G	-

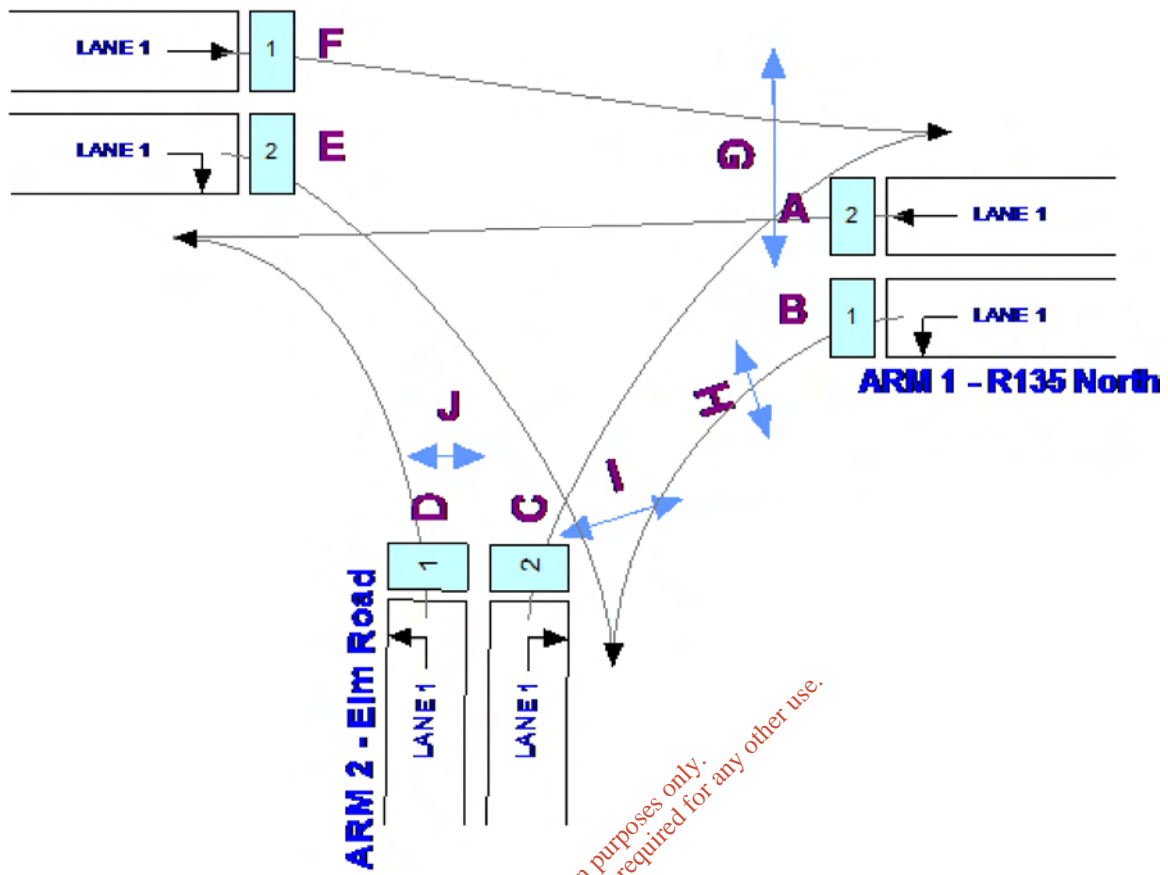
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
1	2	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
3	2	0.0	0.0	90	100	0	0.0	1.10	0
(Ped)	1	0.0	0.0	90	100	0	0.0	-	0
(Ped)	2	0.0	0.0	90	100	0	0.0	-	0
(Ped)	3	0.0	0.0	90	100	0	0.0	-	0
(Ped)	4	0.0	0.0	90	100	0	0.0	-	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2			1.00	100	Yes	3.50	0.0	0
1	2	1			3		0.00	10	No	3.50	0.0	0
2	1	1		3			1.00	12	Yes	3.50	0.0	0
2	2	1				1	1.00	12	No	3.50	0.0	0
3	1	1				1	0.00	10	Yes	3.50	0.0	0
3	2	1				2	1.00	10	No	3.50	0.0	0

Junction Diagram

ARM 3 - R135 South



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No
G	(Name)	Pedestrian	-	7.0	0.0	No
H	(Name)	Pedestrian	-	7.0	0.0	No
I	(Name)	Pedestrian	-	7.0	0.0	No
J	(Name)	Pedestrian	-	7.0	0.0	No

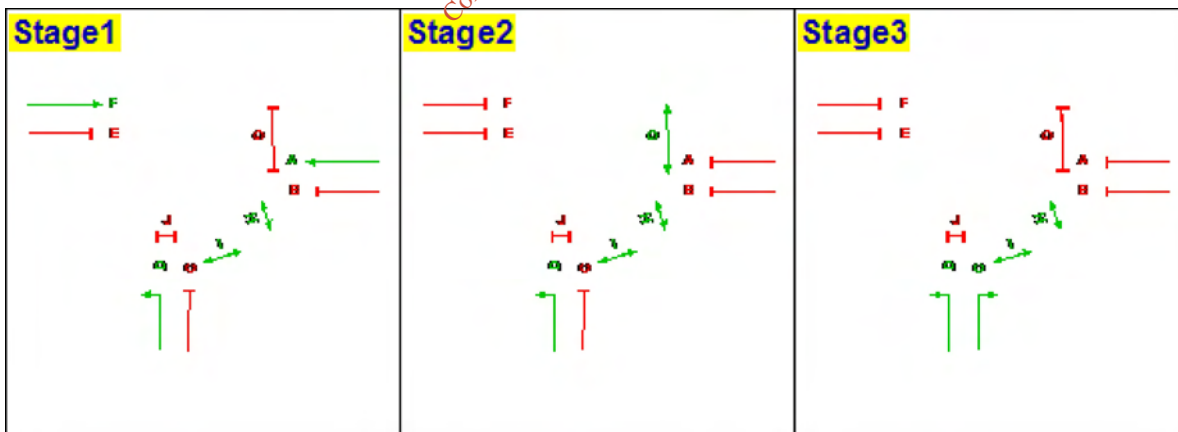
Intergreen Matrix

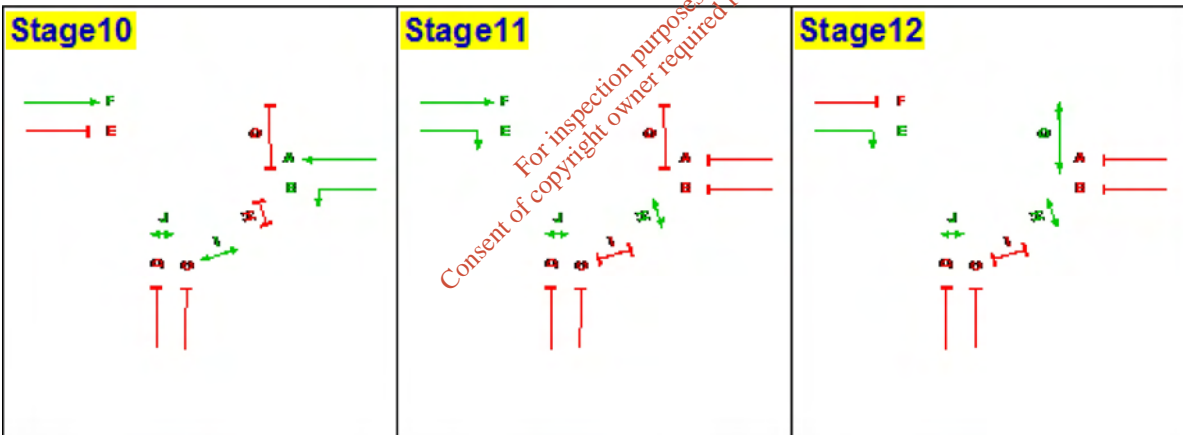
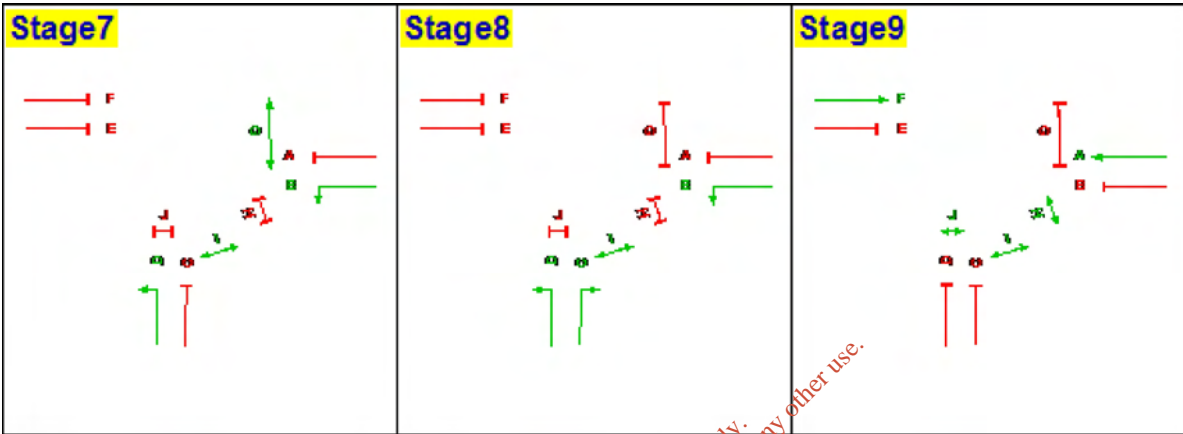
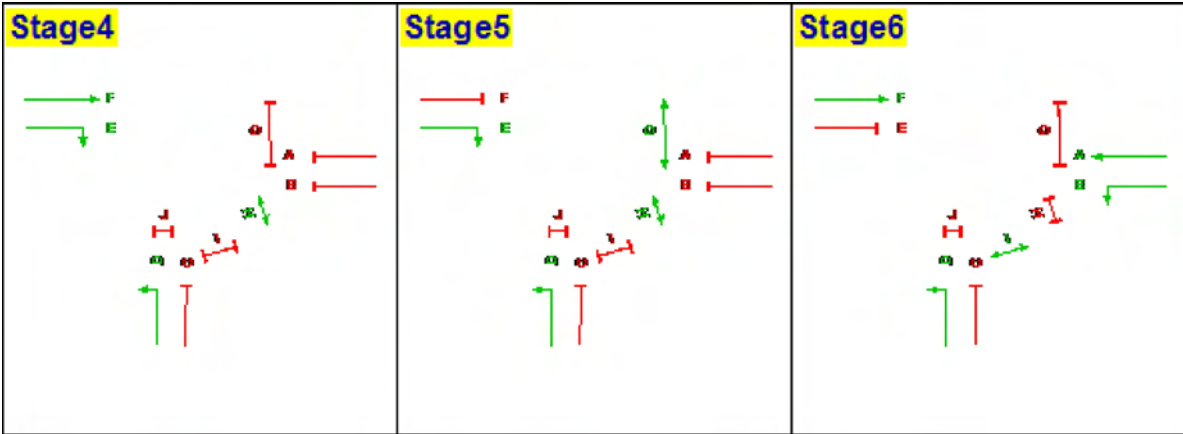
		To									
		A	B	C	D	E	F	G	H	I	J
From	A	-		5		5		5			
	B		-			5			5		
	C	5		-		5	5	8			5
	D				-						5
	E	5	6	5		-				7	
	F			5			-	8			
	G	5		5			5	-			
	H		6						-		
	I					5				-	
	J			5	7						-

Stages

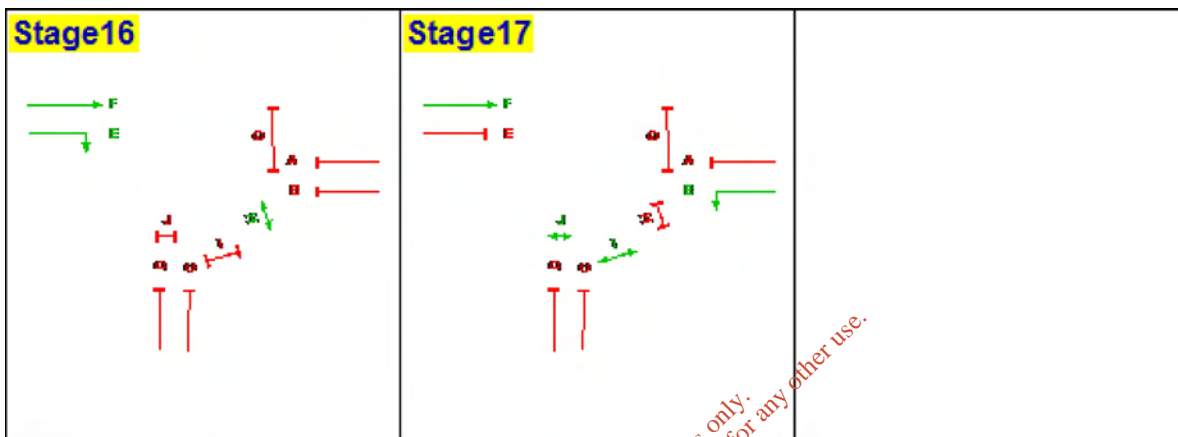
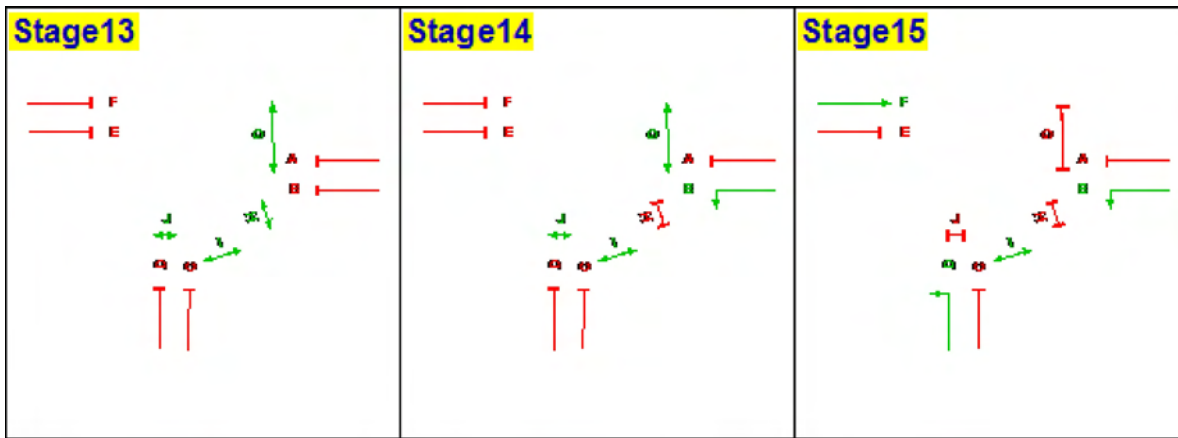
Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	A,D,F,H,I	Yes
2	-1	D,G,H,I	Yes
3	-1	C,D,H,I	Yes
4	-1	D,E,F,H	Yes
5	-1	D,E,G,H	Yes
6	-1	A,B,D,F,I	Yes
7	-1	B,D,G,I	Yes
8	-1	B,C,D,I	Yes
9	-1	A,F,H,I,J	Yes
10	-1	A,B,F,I,J	Yes
11	-1	E,F,H,J	Yes
12	-1	E,G,H,J	Yes
13	-1	G,H,I,J	Yes
14	-1	B,G,I,J	Yes
15	-1	B,D,F,I	Yes
16	-1	E,F,H	Yes
17	-1	B,F,I,J	Yes

Stage Diagrams





For inspection purposes only.
 Consent of copyright owner required for any other use.



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,13,12,13,14,7,8,6
2		1,6,8,7,14,13,12,13,2
3		1,2,13,14,13,2,3,4
4		1,4,3,2,13,14,13,2
5		1,2,13,12,13,14,13,2,3
6		1,3,2,13,14,13,12,13,2
7		1,2,13,14,13,12,13,2,3
8		1,3,2,13,12,13,14,13,2

Constraints

(No constraints)

For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	Yes	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set5 - 2023 AM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To		
		Arm 1	Arm 2	Arm 3
From	Arm 1	-	297	81
	Arm 2	23	-	4
	Arm 3	396	188	-

Average pedestrian flow on each pedestrian stream (if applicable): 1 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - R135 North	1	B	224	267	328	328	267	224
1 - R135 North	2	A	60	71	87	87	71	60
2 - Elm Road	1	D	3	4	4	4	4	3
2 - Elm Road	2	C	17	21	25	25	21	17
3 - R135 South	1	F	298	356	436	436	356	298
3 - R135 South	2	E	140	167	205	205	167	140
Pedestrians	1	J	1	1	1	1	1	1
Pedestrians	2	I	1	1	1	1	1	1
Pedestrians	3	H	1	1	1	1	1	1
Pedestrians	4	G	1	1	1	1	1	1

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - R135 North	79	21	-
2 - Elm Road	15	-	85
3 - R135 South	-	68	32

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	257.17	3.77	3.77	152.0

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	197.93	3.78	3.78	151.40

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
4	5.0	43.0	48.0
6	55.0	38.0	93.0
14	101.0	7.0	108.0
8	115.0	5.0	0.0

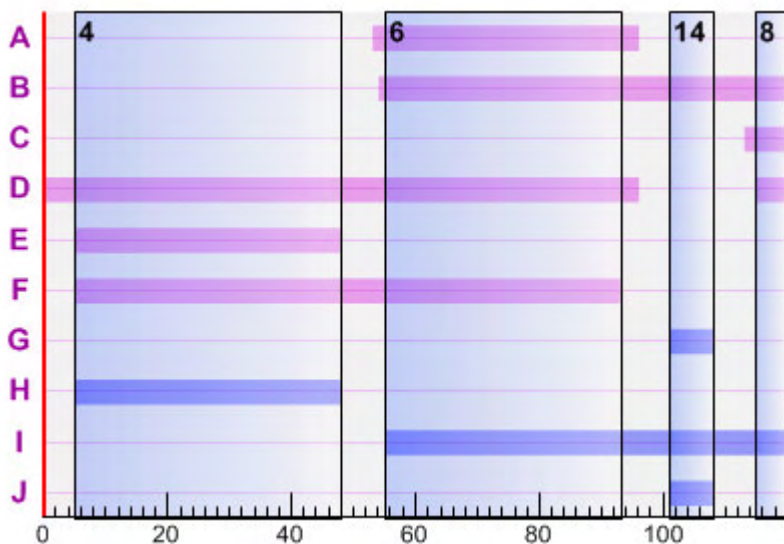
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	53	43.0	96						
B	54	66.0	0						
C	113	7.0	0						
D	115	101.0	96						
E	5	43.0	48						
F	5	88.0	93						
G	101	7.0	108						
H	5	43.0	48						
I	55	65.0	0						
J	101	7.0	108						

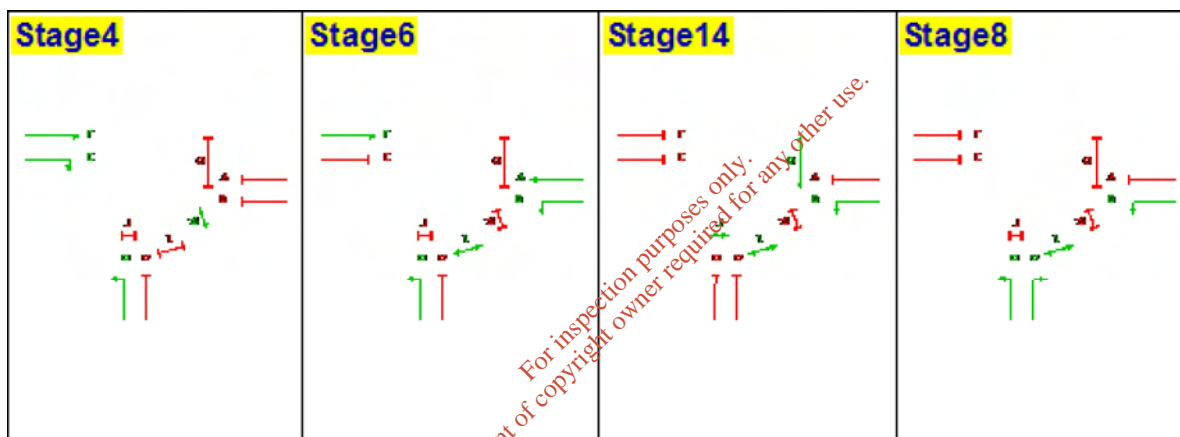
Phase Delays

Type	Phase	Terminating Stage	Starting Stage	Absolute Delay (s)	Relative Delay (s)
Losing	A	6	14	3.00	
Losing	D	6	14	3.00	

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	273	B	67.50	14.10	1.07	25.07	259.01	0.06	4.27	4.21	43.90
1	2	73	A	44.50	24.92	0.51	9.35	862.39	0.01	1.56	1.55	8.60
2	1	4	D	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	1.00
2	2	21	C	8.50	55.76	0.33	15.85	467.98	0.02	0.67	0.65	0.60
3	1	363	F	89.50	5.29	0.53	24.77	263.36	0.06	3.37	3.32	80.00
3	2	171	E	44.50	27.38	1.30	25.20	257.17	0.06	3.77	3.71	17.90
Ped	1	1	J	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	I	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	H	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	G	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	224	2	67.50	13.55	0.84	20.57	337.54	0.04	3.46	3.42	6.20
1	2	60	1	44.50	24.70	0.41	7.69	9999.00	0.00	1.28	1.27	1.20
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	17	3	8.50	54.88	0.26	12.83	601.63	0.01	0.54	0.53	0.10
3	1	298	6	89.50	4.98	0.41	20.33	342.62	0.04	2.72	2.68	11.10
3	2	140	5	44.50	26.62	1.04	20.63	336.26	0.04	3.06	3.02	2.50
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	267	2	67.50	14.03	1.04	24.52	267.08	0.06	4.17	4.11	7.20
1	2	71	1	44.50	24.88	0.49	9.10	889.50	0.01	1.52	1.51	1.40
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	21	3	8.50	55.74	0.33	15.85	462.98	0.02	0.67	0.65	0.10
3	1	356	6	89.50	5.25	0.52	24.29	270.51	0.06	3.30	3.25	13.10
3	2	167	5	44.50	27.28	1.27	24.61	265.73	0.05	3.68	3.62	2.90
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	328	2	67.50	14.77	1.35	30.12	198.81	0.09	5.21	5.12	8.50
1	2	87	1	44.50	25.16	0.61	11.15	707.52	0.01	1.87	1.86	1.70
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	25	3	8.50	56.64	0.39	18.86	377.11	0.03	0.81	0.78	0.10
3	1	436	6	89.50	5.67	0.69	29.75	202.52	0.09	4.14	4.05	15.70
3	2	205	5	44.50	28.26	1.61	30.21	197.93	0.09	4.57	4.48	3.40
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	328	2	67.50	14.77	1.35	30.12	198.81	0.09	5.21	5.12	8.50
1	2	87	1	44.50	25.16	0.61	11.15	707.52	0.01	1.87	1.86	1.70
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	25	3	8.50	56.67	0.39	18.86	377.11	0.03	0.81	0.78	0.10
3	1	436	6	89.50	5.67	0.69	29.75	202.52	0.09	4.14	4.05	15.70
3	2	205	5	44.50	28.26	1.61	30.21	197.93	0.09	4.57	4.48	3.40
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	267	2	67.50	14.03	1.04	24.52	267.08	0.06	4.17	4.11	7.20
1	2	71	1	44.50	24.88	0.49	9.10	889.50	0.01	1.52	1.51	1.40
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	21	3	8.50	55.79	0.33	15.85	467.98	0.02	0.67	0.65	0.10
3	1	356	6	89.50	5.25	0.52	24.29	270.51	0.06	3.30	3.25	13.10
3	2	167	5	44.50	27.28	1.27	24.61	265.73	0.06	3.68	3.62	2.90
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	224	2	67.50	13.55	0.84	20.57	337.54	0.04	3.46	3.42	6.20
1	2	60	1	44.50	24.70	0.41	7.69	9999.00	0.00	1.28	1.27	1.20
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	17	3	8.50	54.93	0.26	12.83	601.63	0.01	0.54	0.53	0.10
3	1	298	6	89.50	4.98	0.41	20.33	342.62	0.04	2.72	2.68	11.10
3	2	140	5	44.50	26.63	1.04	20.63	336.26	0.04	3.06	3.02	2.50
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	66.50	11.93	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	44.50	23.75	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry TIA\Reports\Appendices\T-Junction.osc

Report generation date: 04/08/2016 10:38:26

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	R135 North	50.0	10	10	80	0
2	Elm Road	50.0	10	10	80	0
3	R135 South	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1936	Yes	0	B	-
1	2	Traffic		2105	Yes	0	A	-
2	1	Traffic		1747	Yes	0	D	-
2	2	Traffic		1871	Yes	0	C	-
3	1	Traffic		1965	Yes	0	F	-
3	2	Traffic		1830	Yes	0	E	-
(Ped)	1	Pedestrian		10000	Yes	0	J	-
(Ped)	2	Pedestrian		10000	Yes	0	I	-
(Ped)	3	Pedestrian		10000	Yes	0	H	-
(Ped)	4	Pedestrian		10000	Yes	0	G	-

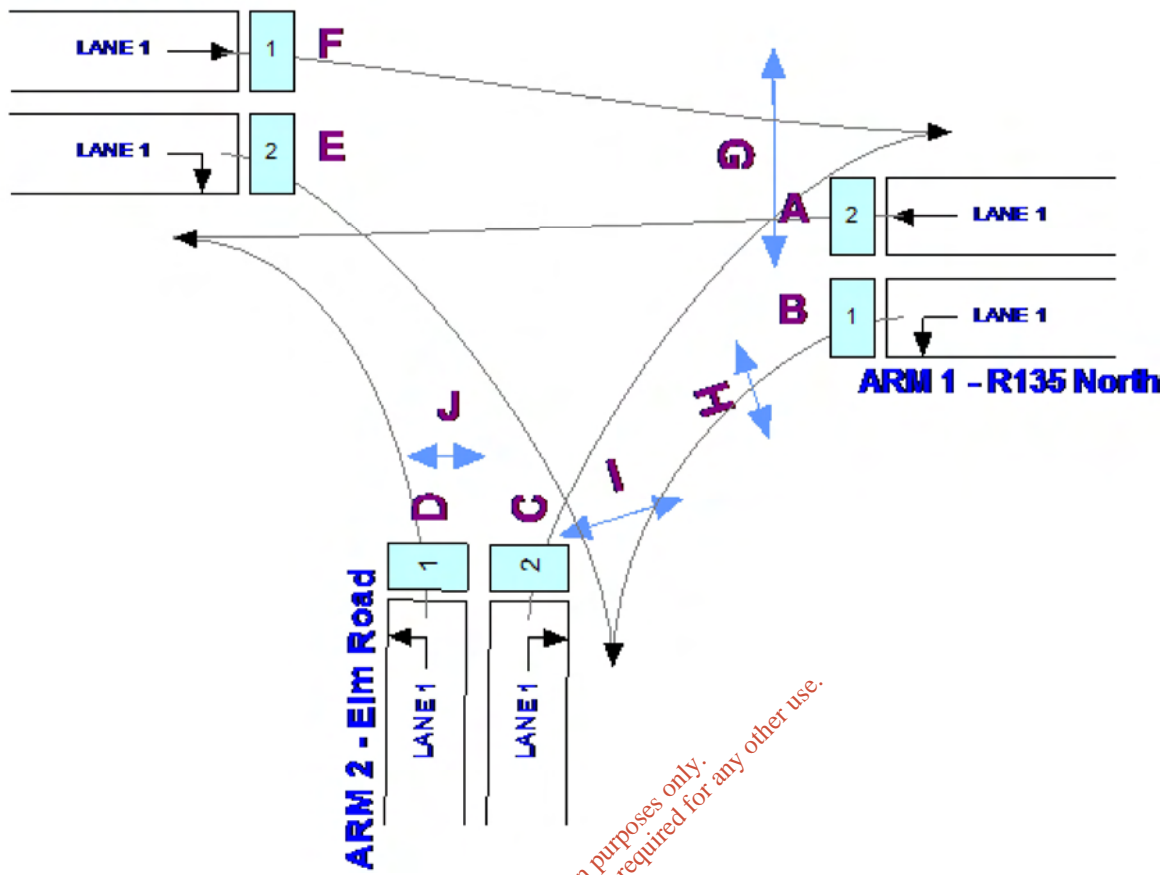
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
1	2	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
3	2	0.0	0.0	90	100	0	0.0	1.10	0
(Ped)	1	0.0	0.0	90	100	0	0.0	-	0
(Ped)	2	0.0	0.0	90	100	0	0.0	-	0
(Ped)	3	0.0	0.0	90	100	0	0.0	-	0
(Ped)	4	0.0	0.0	90	100	0	0.0	-	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2			1.00	100	Yes	3.50	0.0	0
1	2	1			3		0.00	10	No	3.50	0.0	0
2	1	1		3			1.00	12	Yes	3.50	0.0	0
2	2	1				1	1.00	12	No	3.50	0.0	0
3	1	1				1	0.00	10	Yes	3.50	0.0	0
3	2	1				2	1.00	10	No	3.50	0.0	0

Junction Diagram

ARM 3 - R135 South



Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No
G	(Name)	Pedestrian	-	7.0	0.0	No
H	(Name)	Pedestrian	-	7.0	0.0	No
I	(Name)	Pedestrian	-	7.0	0.0	No
J	(Name)	Pedestrian	-	7.0	0.0	No

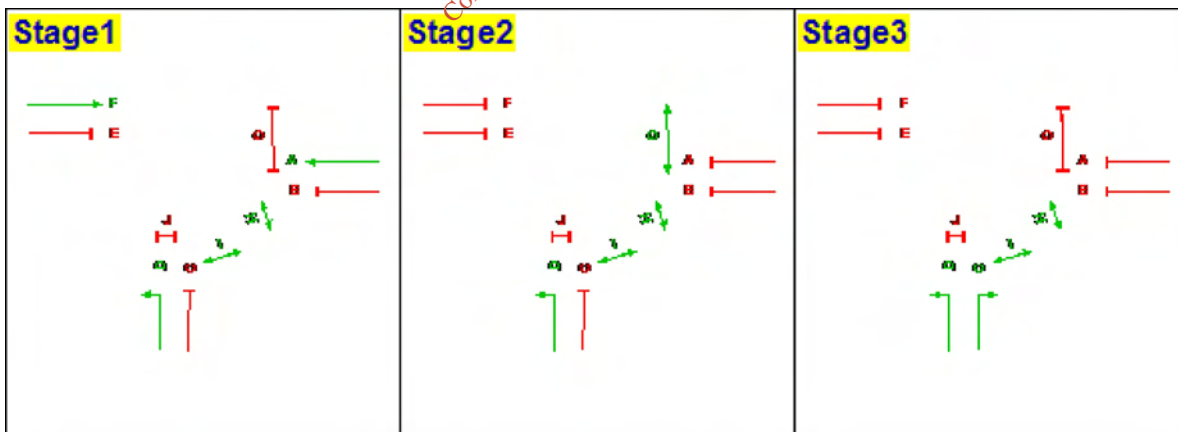
Intergreen Matrix

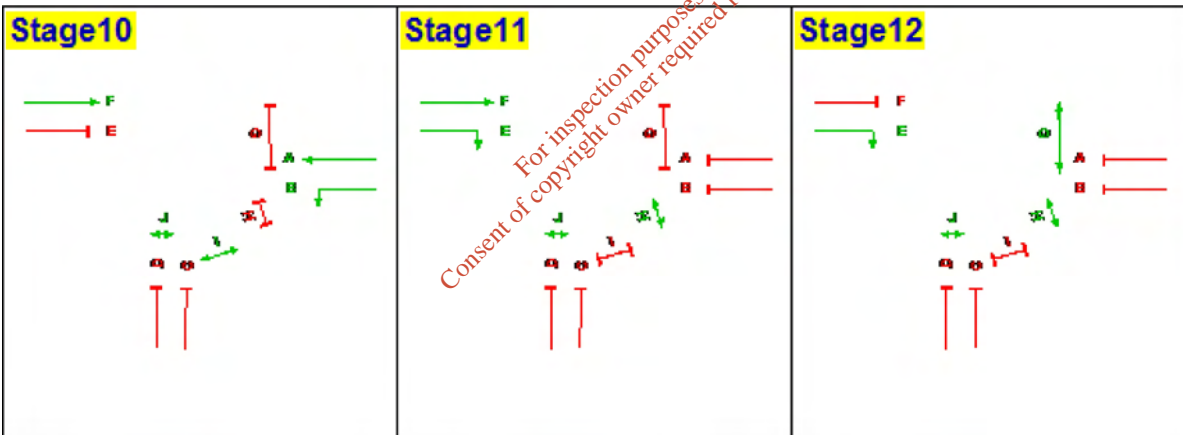
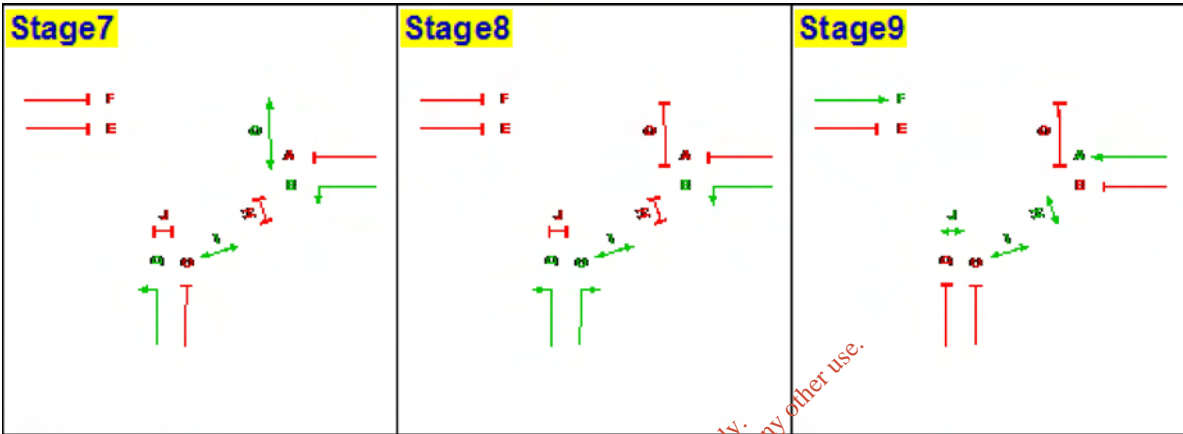
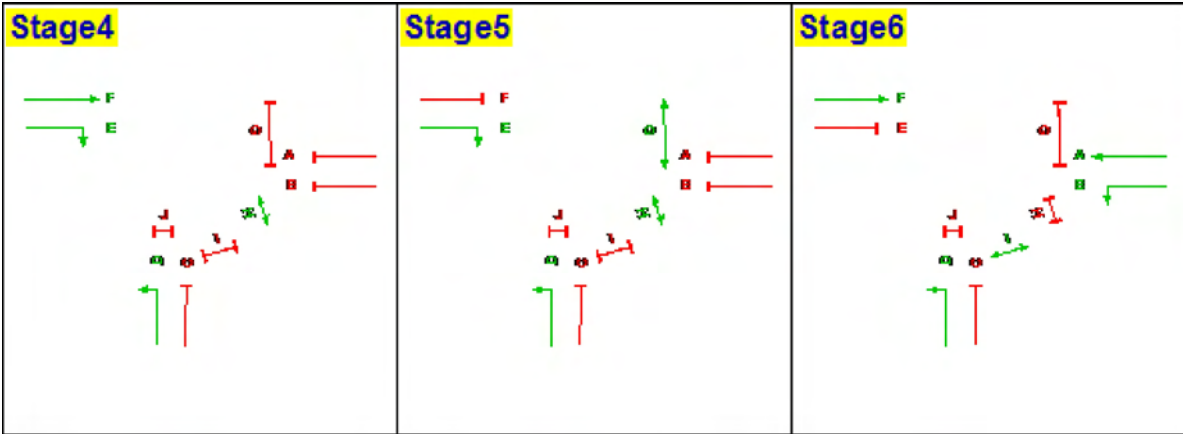
		To									
		A	B	C	D	E	F	G	H	I	J
From	A	-		5		5		5			
	B		-			5			5		
	C	5		-		5	5	8			5
	D				-						5
	E	5	6	5		-				7	
	F			5			-	8			
	G	5		5			5	-			
	H		6						-		
	I					5				-	
	J			5	7						-

Stages

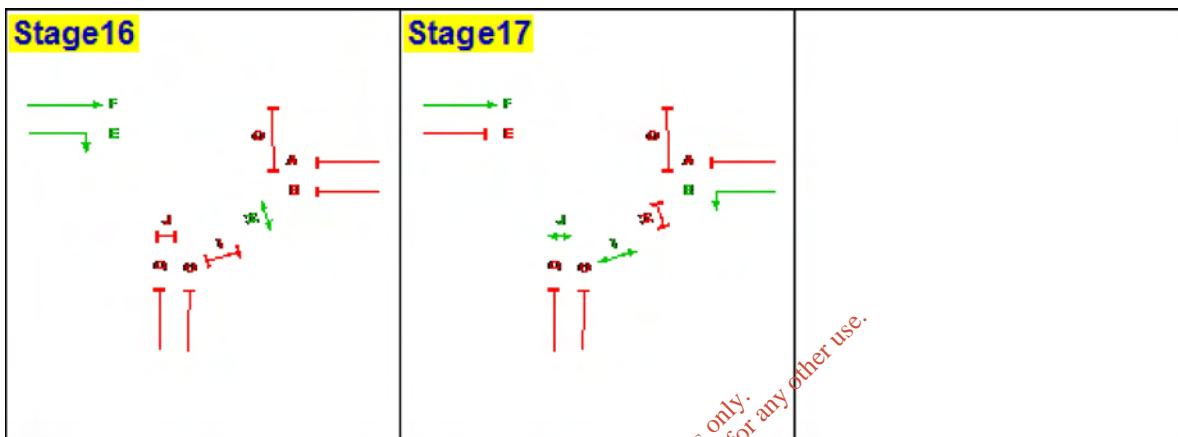
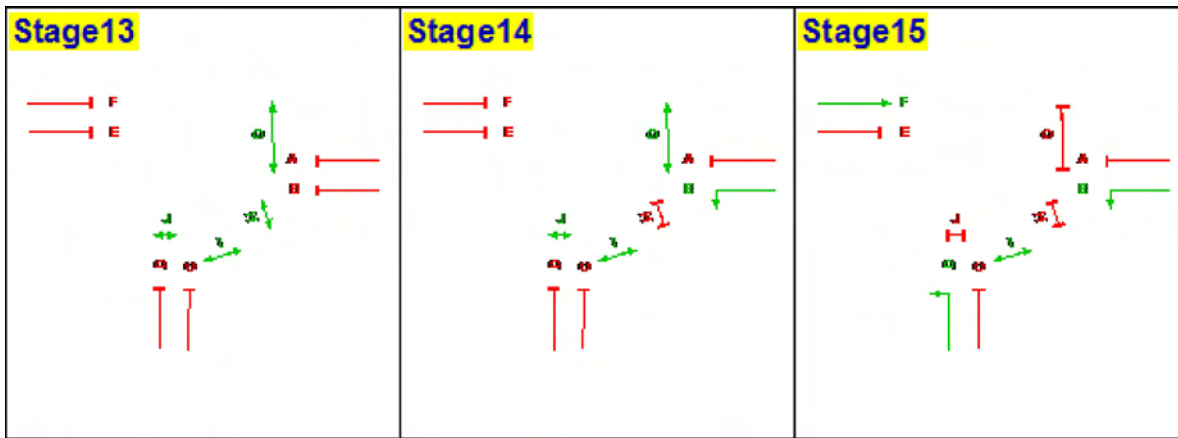
Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	A,D,F,H,I	Yes
2	-1	D,G,H,I	Yes
3	-1	C,D,H,I	Yes
4	-1	D,E,F,H	Yes
5	-1	D,E,G,H	Yes
6	-1	A,B,D,F,I	Yes
7	-1	B,D,G,I	Yes
8	-1	B,C,D,I	Yes
9	-1	A,F,H,I,J	Yes
10	-1	A,B,F,I,J	Yes
11	-1	E,F,H,J	Yes
12	-1	E,G,H,J	Yes
13	-1	G,H,I,J	Yes
14	-1	B,G,I,J	Yes
15	-1	B,D,F,I	Yes
16	-1	E,F,H	Yes
17	-1	B,F,I,J	Yes

Stage Diagrams





For inspection purposes only.
 Consent of copyright owner required for any other use.



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,13,12,13,14,7,8,6
2		1,6,8,7,14,13,12,13,2
3		1,2,13,14,13,2,3,4
4		1,4,3,2,13,14,13,2
5		1,2,13,12,13,14,13,2,3
6		1,3,2,13,14,13,12,13,2
7		1,2,13,14,13,12,13,2,3
8		1,3,2,13,12,13,14,13,2

Constraints

(No constraints)

For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	Yes	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set5 - 2023 AM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To		
		Arm 1	Arm 2	Arm 3
From	Arm 1	-	297	97
	Arm 2	23	-	4
	Arm 3	412	204	-

Average pedestrian flow on each pedestrian stream (if applicable): 1 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	07:15-07:30	07:30-07:45	07:45-08:00	08:00-08:15	08:15-08:30	08:30-08:45
1 - R135 North	1	B	222	265	324	324	265	222
1 - R135 North	2	A	74	88	108	108	88	74
2 - Elm Road	1	D	3	4	4	4	4	3
2 - Elm Road	2	C	17	21	25	25	21	17
3 - R135 South	1	F	310	370	453	453	370	310
3 - R135 South	2	E	152	182	223	223	182	152
Pedestrians	1	J	1	1	1	1	1	1
Pedestrians	2	I	1	1	1	1	1	1
Pedestrians	3	H	1	1	1	1	1	1
Pedestrians	4	G	1	1	1	1	1	1

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - R135 North	75	25	-
2 - Elm Road	15	-	85
3 - R135 South	-	67	33

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	246.87	4.11	4.11	156.0

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	189.06	4.12	4.12	155.40

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
4	7.0	32.0	39.0
6	46.0	35.0	81.0
7	89.0	7.0	96.0
8	101.0	7.0	108.0
11	113.0	7.0	0.0

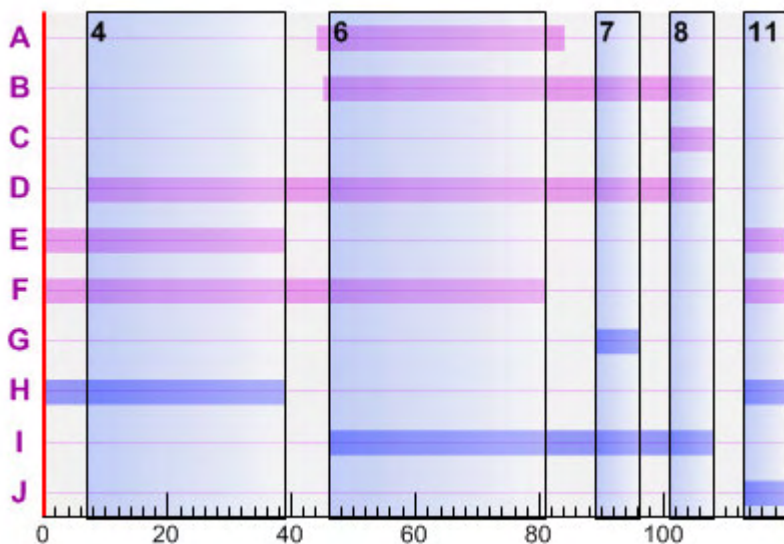
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	44	40.0	84						
B	45	63.0	108						
C	101	7.0	108						
D	7	101.0	108						
E	113	46.0	39						
F	113	88.0	81						
G	89	7.0	96						
H	113	46.0	39						
I	46	62.0	108						
J	113	7.0	0						

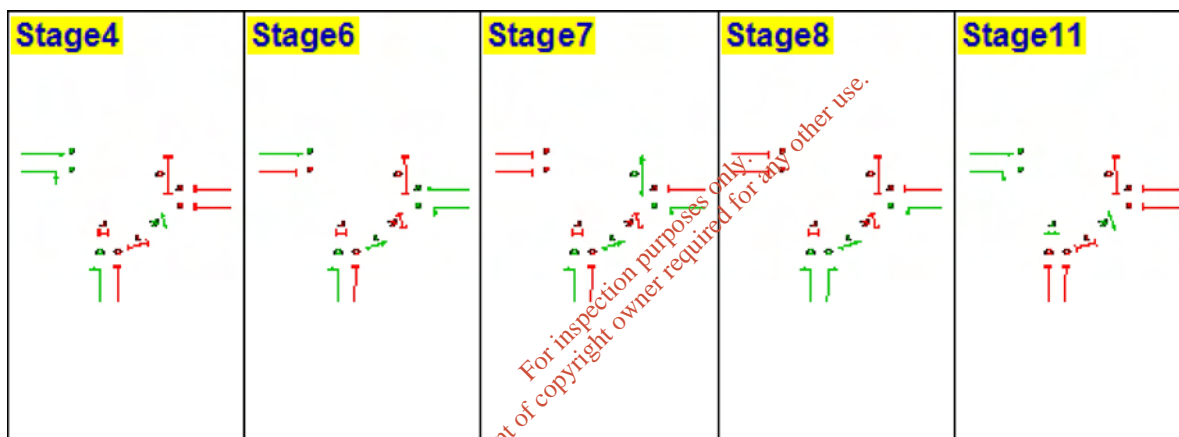
Phase Delays

Type	Phase	Terminating Stage	Starting Stage	Absolute Delay (s)	Relative Delay (s)
Losing	A	6	7	3.00	

Phase Timings Diagram



Final Stage Sequence



For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	270	B	64.50	15.71	1.18	25.95	246.87	0.06	4.46	4.40	41.10
1	2	90	A	41.50	27.28	0.68	12.36	627.98	0.01	2.01	2.00	9.70
2	1	4	D	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	1.00
2	2	21	C	8.50	55.76	0.33	15.85	467.98	0.02	0.67	0.65	0.60
3	1	378	F	89.50	5.36	0.56	25.79	248.94	0.06	3.53	3.46	82.90
3	2	186	E	47.50	25.51	1.32	25.68	250.50	0.06	3.95	3.89	20.70
Ped	1	1	J	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	I	63.50	13.30	0.00	0.02	9999.00	0.00	0.02	0.02	0.00
Ped	3	1	H	47.50	21.90	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	G	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:15-07:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	222	2	64.50	15.11	0.93	21.33	321.86	0.04	3.62	3.58	5.80
1	2	74	1	41.50	26.98	0.55	10.17	785.38	0.01	1.65	1.64	1.40
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	17	3	8.50	54.88	0.26	12.83	601.63	0.01	0.54	0.53	0.10
3	1	310	6	89.50	5.04	0.43	21.15	325.49	0.04	2.84	2.80	11.50
3	2	152	5	47.50	24.75	1.05	20.98	328.91	0.04	3.19	3.16	2.90
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	63.50	13.30	0.00	0.02	9999.00	0.00	0.02	0.02	0.00
Ped	3	1	8	47.50	21.90	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:30-07:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	265	2	64.50	15.65	1.15	25.47	253.41	0.06	4.37	4.31	6.70
1	2	88	1	41.50	27.24	0.67	12.09	644.52	0.01	1.96	1.95	1.60
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	21	3	8.50	55.74	0.33	15.85	462.98	0.02	0.67	0.65	0.10
3	1	370	6	89.50	5.32	0.55	25.25	256.49	0.06	3.45	3.39	13.60
3	2	182	5	47.50	25.42	1.28	25.13	258.21	0.06	3.86	3.80	3.40
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	63.50	13.30	0.00	0.02	9999.00	0.00	0.02	0.02	0.00
Ped	3	1	8	47.50	21.90	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (07:45-08:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	324	2	64.50	16.44	1.48	31.14	189.06	0.10	5.44	5.34	7.90
1	2	108	1	41.50	27.63	0.83	14.84	506.65	0.02	2.42	2.41	1.90
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	25	3	8.50	56.64	0.39	18.86	377.11	0.03	0.81	0.78	0.10
3	1	453	6	89.50	5.76	0.72	30.91	191.17	0.10	4.32	4.22	16.20
3	2	223	5	47.50	26.39	1.63	30.79	192.35	0.09	4.79	4.70	4.00
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	63.50	13.30	0.00	0.02	9999.00	0.00	0.02	0.02	0.00
Ped	3	1	8	47.50	21.90	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:00-08:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	324	2	64.50	16.45	1.48	31.14	189.06	0.10	5.44	5.34	7.90
1	2	108	1	41.50	27.63	0.83	14.84	506.65	0.02	2.42	2.41	1.90
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	25	3	8.50	56.67	0.39	18.86	377.11	0.03	0.81	0.78	0.10
3	1	453	6	89.50	5.76	0.73	30.91	191.17	0.10	4.32	4.22	16.20
3	2	223	5	47.50	26.40	1.64	30.79	192.35	0.09	4.79	4.70	4.00
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	63.50	13.30	0.00	0.02	9999.00	0.00	0.02	0.02	0.00
Ped	3	1	8	47.50	21.90	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:15-08:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	265	2	64.50	15.65	1.15	25.47	253.41	0.06	4.37	4.31	6.70
1	2	88	1	41.50	27.25	0.67	12.09	644.52	0.01	1.96	1.95	1.60
2	1	4	4	102.50	1.28	0.00	0.27	9999.00	0.00	0.02	0.02	0.20
2	2	21	3	8.50	55.79	0.33	15.85	467.98	0.02	0.67	0.65	0.10
3	1	370	6	89.50	5.32	0.55	25.25	256.49	0.06	3.45	3.39	13.60
3	2	182	5	47.50	25.42	1.29	25.13	258.21	0.06	3.86	3.80	3.40
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	63.50	13.30	0.00	0.02	9999.00	0.00	0.02	0.02	0.00
Ped	3	1	8	47.50	21.90	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (08:30-08:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	222	2	64.50	15.12	0.93	21.33	321.86	0.04	3.62	3.58	5.80
1	2	74	1	41.50	26.98	0.55	10.17	785.38	0.01	1.65	1.64	1.40
2	1	3	4	102.50	1.28	0.00	0.20	9999.00	0.00	0.01	0.01	0.10
2	2	17	3	8.50	54.93	0.26	12.83	601.63	0.01	0.54	0.53	0.10
3	1	310	6	89.50	5.04	0.43	21.15	325.49	0.04	2.84	2.80	11.50
3	2	152	5	47.50	24.76	1.05	20.98	328.91	0.04	3.20	3.16	2.90
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	63.50	13.30	0.00	0.02	9999.00	0.00	0.02	0.02	0.00
Ped	3	1	8	47.50	21.90	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry TIA\Reports\Appendices\T-Junction.osc

Report generation date: 05/08/2016 16:51:25

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

For inspection purposes only.
Consent of copyright owner required for any other use.

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	R135 North	50.0	10	10	80	0
2	Elm Road	50.0	10	10	80	0
3	R135 South	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1936	Yes	0	B	-
1	2	Traffic		2105	Yes	0	A	-
2	1	Traffic		1747	Yes	0	D	-
2	2	Traffic		1871	Yes	0	C	-
3	1	Traffic		1965	Yes	0	F	-
3	2	Traffic		1830	Yes	0	E	-
(Ped)	1	Pedestrian		10000	Yes	0	J	-
(Ped)	2	Pedestrian		10000	Yes	0	I	-
(Ped)	3	Pedestrian		10000	Yes	0	H	-
(Ped)	4	Pedestrian		10000	Yes	0	G	-

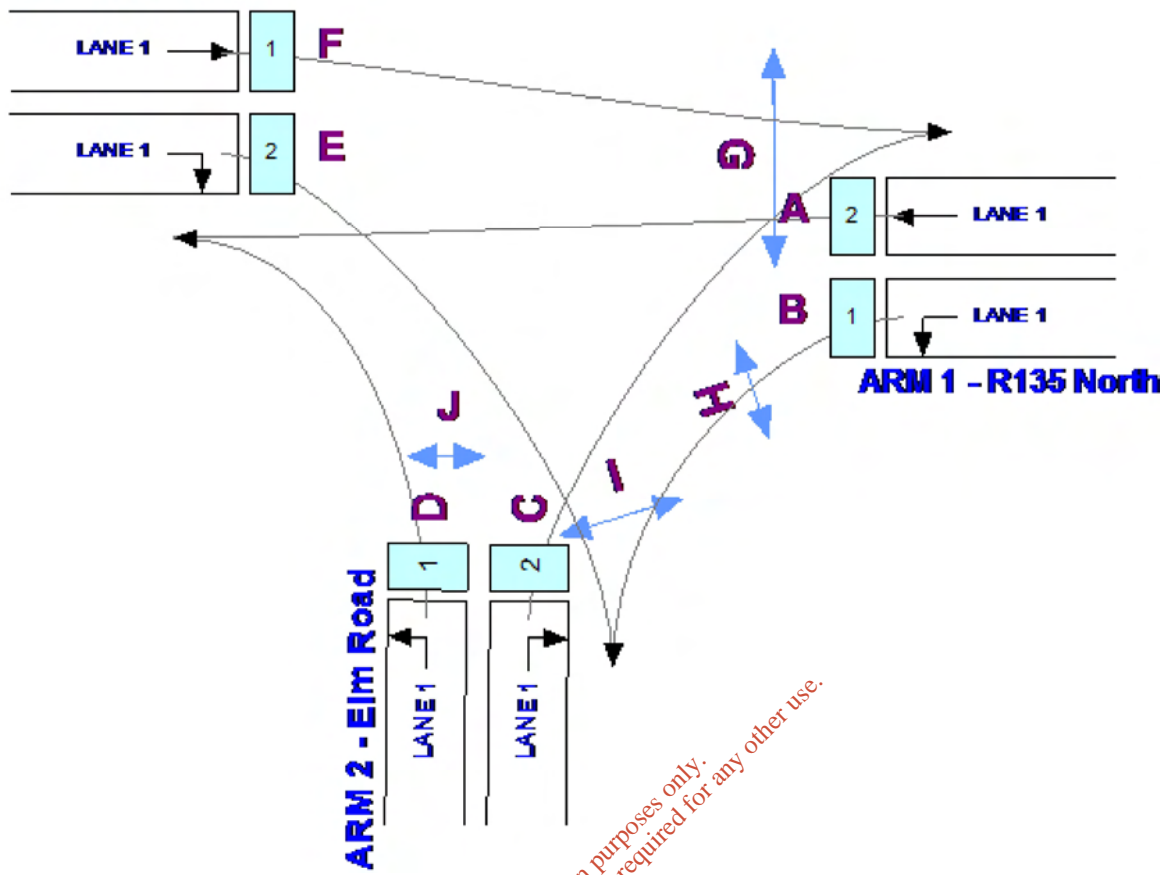
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
1	2	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
3	2	0.0	0.0	90	100	0	0.0	1.10	0
(Ped)	1	0.0	0.0	90	100	0	0.0	-	0
(Ped)	2	0.0	0.0	90	100	0	0.0	-	0
(Ped)	3	0.0	0.0	90	100	0	0.0	-	0
(Ped)	4	0.0	0.0	90	100	0	0.0	-	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2			1.00	100	Yes	3.50	0.0	0
1	2	1			3		0.00	10	No	3.50	0.0	0
2	1	1		3			1.00	12	Yes	3.50	0.0	0
2	2	1				1	1.00	12	No	3.50	0.0	0
3	1	1				1	0.00	10	Yes	3.50	0.0	0
3	2	1				2	1.00	10	No	3.50	0.0	0

Junction Diagram

ARM 3 - R135 South



Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No
G	(Name)	Pedestrian	-	7.0	0.0	No
H	(Name)	Pedestrian	-	7.0	0.0	No
I	(Name)	Pedestrian	-	7.0	0.0	No
J	(Name)	Pedestrian	-	7.0	0.0	No

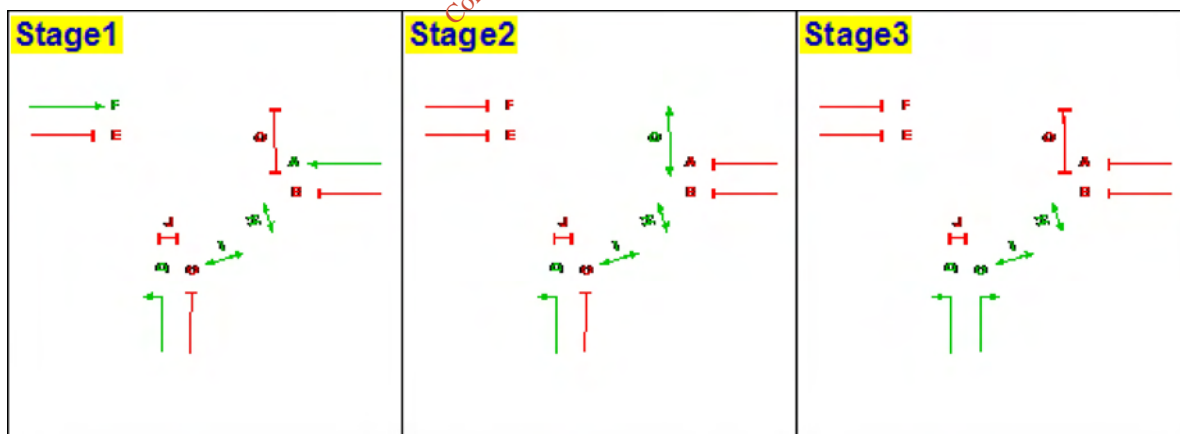
Intergreen Matrix

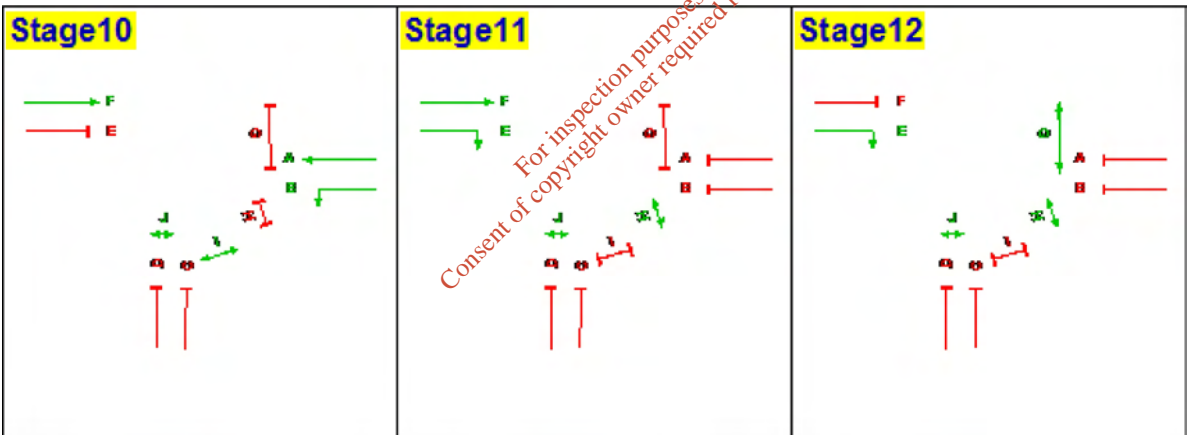
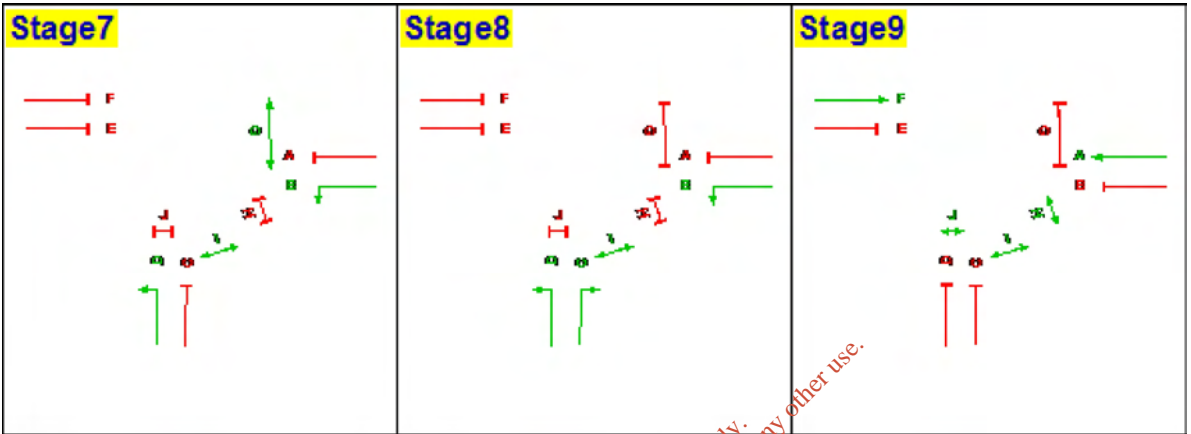
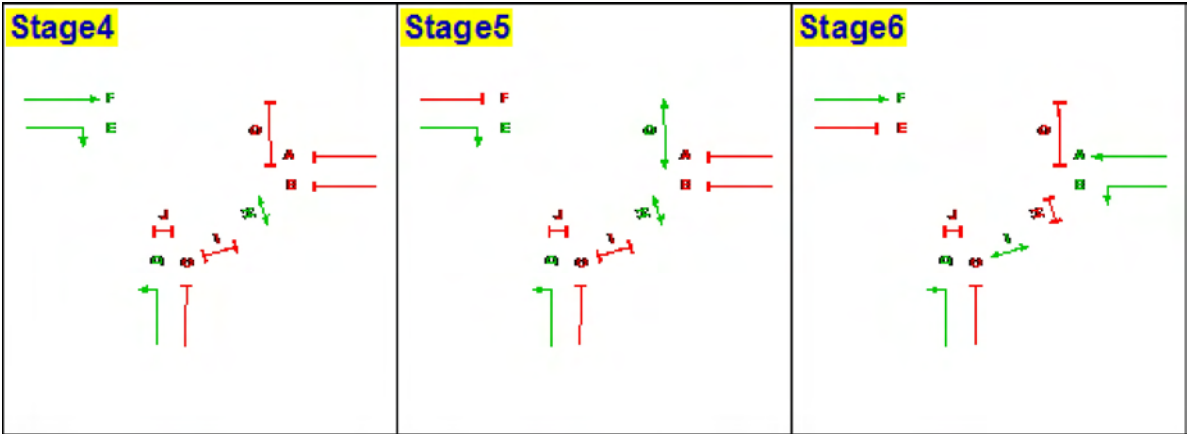
		To									
		A	B	C	D	E	F	G	H	I	J
From	A	-		5		5		5			
	B		-			5			5		
	C	5		-		5	5	8			5
	D				-						5
	E	5	6	5		-				7	
	F			5			-	8			
	G	5		5			5	-			
	H		6						-		
	I					5				-	
	J			5	7						-

Stages

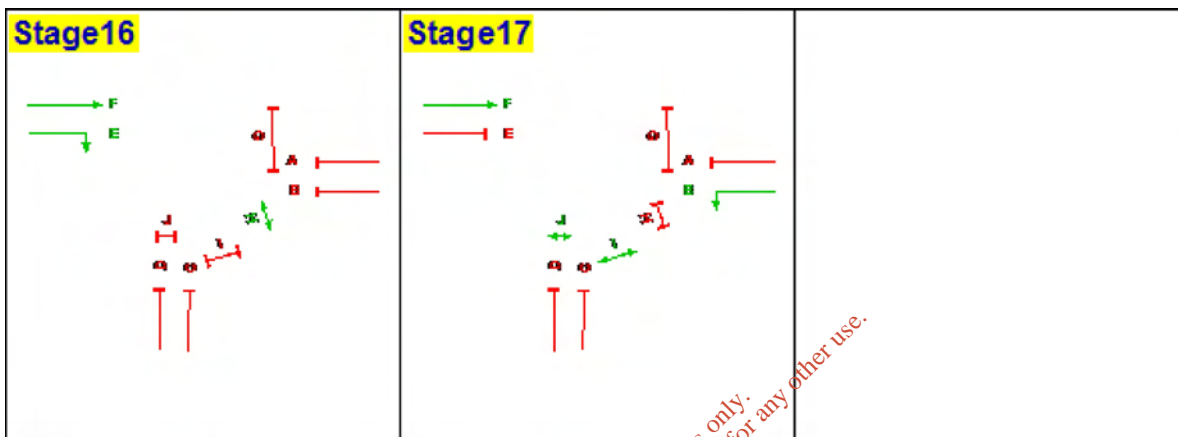
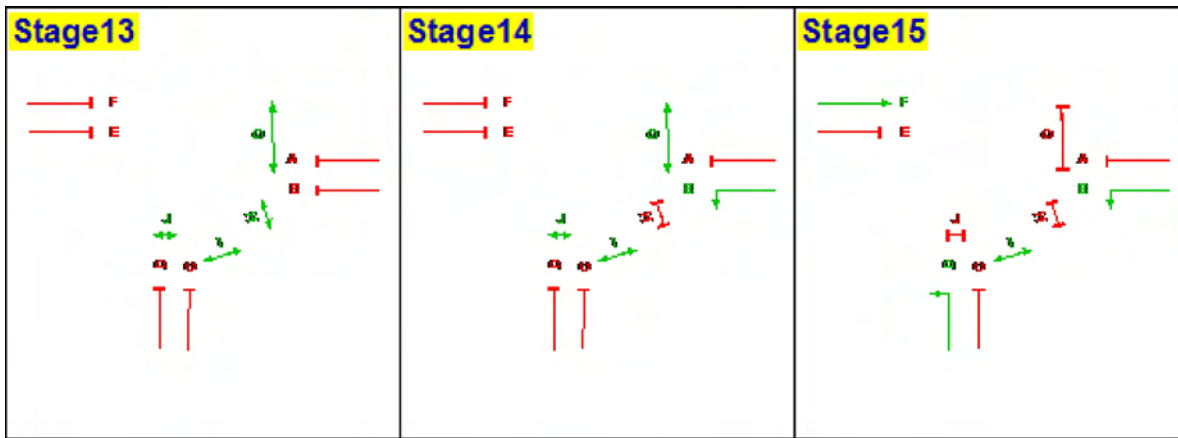
Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	A,D,F,H,I	Yes
2	-1	D,G,H,I	Yes
3	-1	C,D,H,I	Yes
4	-1	D,E,F,H	Yes
5	-1	D,E,G,H	Yes
6	-1	A,B,D,F,I	Yes
7	-1	B,D,G,I	Yes
8	-1	B,C,D,I	Yes
9	-1	A,F,H,I,J	Yes
10	-1	A,B,F,I,J	Yes
11	-1	E,F,H,J	Yes
12	-1	E,G,H,J	Yes
13	-1	G,H,I,J	Yes
14	-1	B,G,I,J	Yes
15	-1	B,D,F,I	Yes
16	-1	E,F,H	Yes
17	-1	B,F,I,J	Yes

Stage Diagrams





For inspection purposes only.
 Consent of copyright owner required for any other use.



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,13,12,13,14,7,8,6
2		1,6,8,7,14,13,12,13,2
3		1,2,13,14,13,2,3,4
4		1,4,3,2,13,14,13,2
5		1,2,13,12,13,14,13,2,3
6		1,3,2,13,14,13,12,13,2
7		1,2,13,14,13,12,13,2,3
8		1,3,2,13,12,13,14,13,2

Constraints

(No constraints)

For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	Yes	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set6 - 2016 PM Peak Existing

ODTAB Data (PCU/hr during central 60 min peak period)

		To		
		Arm 1	Arm 2	Arm 3
From	Arm 1	-	337	28
	Arm 2	30	-	2
	Arm 3	378	164	-

Average pedestrian flow on each pedestrian stream (if applicable): 1 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - R135 North	1	B	252	301	368	368	301	252
1 - R135 North	2	A	22	26	32	32	26	22
2 - Elm Road	1	D	1	2	2	2	2	1
2 - Elm Road	2	C	23	27	33	33	27	23
3 - R135 South	1	F	285	340	416	416	340	285
3 - R135 South	2	E	122	146	178	178	146	122
Pedestrians	1	J	1	1	1	1	1	1
Pedestrians	2	I	1	1	1	1	1	1
Pedestrians	3	H	1	1	1	1	1	1
Pedestrians	4	G	1	1	1	1	1	1

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - R135 North	92	8	-
2 - Elm Road	6	-	94
3 - R135 South	-	70	30

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	245.43	3.41	3.41	149.8

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	189.15	3.41	3.41	149.40

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
4	7.0	22.0	29.0
6	36.0	45.0	81.0
7	89.0	7.0	96.0
8	101.0	7.0	108.0
11	113.0	7.0	0.0

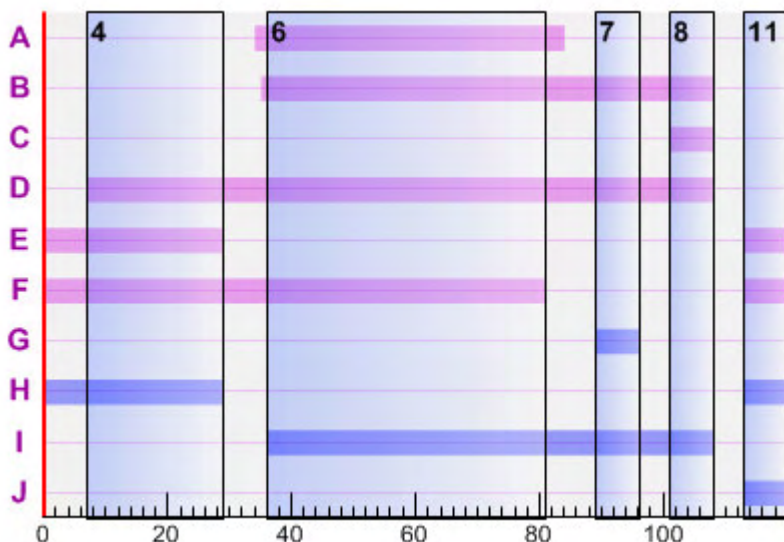
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	34	50.0	84						
B	35	73.0	108						
C	101	7.0	108						
D	7	101.0	108						
E	113	36.0	29						
F	113	88.0	81						
G	89	7.0	96						
H	113	36.0	29						
I	36	72.0	108						
J	113	7.0	0						

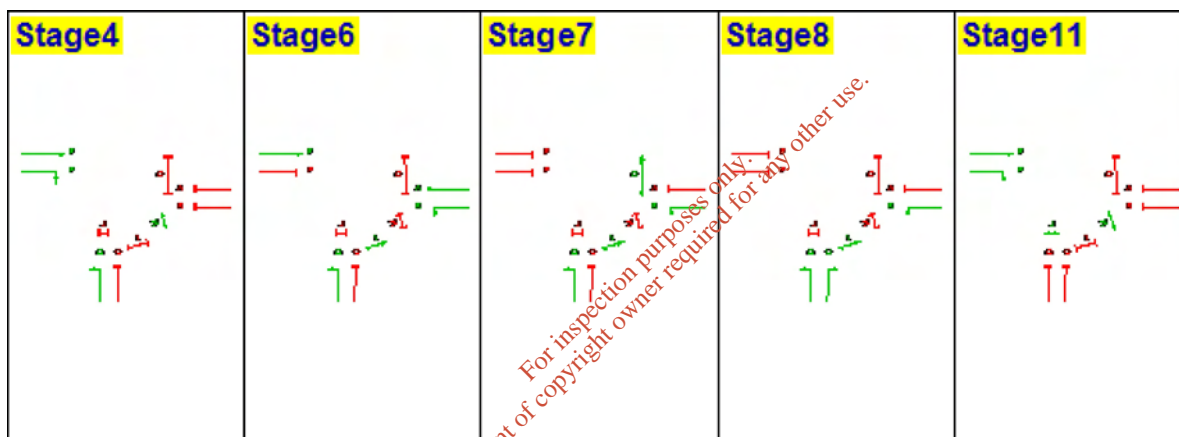
Phase Delays

Type	Phase	Terminating Stage	Starting Stage	Absolute Delay (s)	Relative Delay (s)
Losing	A	6	7	3.00	

Phase Timings Diagram



Final Stage Sequence



For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	307	B	74.50	10.93	0.93	25.54	252.36	0.06	4.20	4.13	54.80
1	2	27	A	51.50	19.89	0.15	2.99	9999.00	0.00	0.52	0.52	3.70
2	1	2	D	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.50
2	2	28	C	8.50	57.37	0.45	21.13	325.99	0.04	0.91	0.87	0.80
3	1	347	F	89.50	5.21	0.50	23.68	280.12	0.05	3.21	3.16	76.80
3	2	149	E	37.50	32.34	1.34	26.05	245.43	0.06	3.58	3.52	13.20
Ped	1	1	J	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	I	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	H	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	G	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	252	2	74.50	10.44	0.73	20.97	329.26	0.04	3.39	3.35	7.70
1	2	22	1	51.50	19.82	0.12	2.44	9999.00	0.00	0.42	0.42	0.50
2	1	1	4	102.50	1.28	0.00	0.07	9999.00	0.00	0.00	0.00	0.00
2	2	23	3	8.50	56.14	0.36	17.35	418.59	0.02	0.74	0.72	0.10
3	1	285	6	89.50	4.92	0.39	19.45	362.81	0.03	2.59	2.56	10.70
3	2	122	5	37.50	31.51	1.07	21.33	321.87	0.04	2.90	2.87	1.90
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	301	2	74.50	10.87	0.91	25.04	259.38	0.06	4.11	4.05	9.00
1	2	26	1	51.50	19.87	0.14	2.88	9999.00	0.00	0.50	0.50	0.60
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	27	3	8.50	57.10	0.43	20.37	341.76	0.03	0.87	0.84	0.10
3	1	340	6	89.50	5.18	0.49	23.20	287.94	0.05	3.14	3.09	12.60
3	2	146	5	37.50	32.24	1.31	25.53	252.53	0.06	3.51	3.45	2.20
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	368	2	74.50	11.52	1.18	30.62	193.95	0.10	5.12	5.02	10.60
1	2	32	1	51.50	19.95	0.18	3.54	9999.00	0.00	0.61	0.61	0.70
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	33	3	8.50	58.60	0.54	24.90	261.44	0.05	1.08	1.03	0.20
3	1	416	6	89.50	5.56	0.64	28.39	217.07	0.08	3.93	3.84	15.10
3	2	178	5	37.50	33.28	1.65	31.13	189.15	0.10	4.33	4.23	2.50
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	368	2	74.50	11.52	1.18	30.62	193.95	0.10	5.12	5.02	10.60
1	2	32	1	51.50	19.95	0.18	3.54	9999.00	0.00	0.61	0.61	0.70
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	33	3	8.50	58.66	0.54	24.90	261.44	0.05	1.08	1.03	0.20
3	1	416	6	89.50	5.56	0.64	28.39	217.07	0.08	3.93	3.84	15.10
3	2	178	5	37.50	33.29	1.65	31.13	189.15	0.10	4.33	4.23	2.50
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	301	2	74.50	10.88	0.91	25.04	259.38	0.06	4.11	4.05	9.00
1	2	26	1	51.50	19.87	0.14	2.88	9999.00	0.00	0.50	0.50	0.60
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	27	3	8.50	57.19	0.43	20.37	341.76	0.03	0.88	0.84	0.10
3	1	340	6	89.50	5.18	0.49	23.20	287.94	0.05	3.14	3.09	12.60
3	2	146	5	37.50	32.25	1.31	25.53	252.53	0.06	3.51	3.45	2.20
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	252	2	74.50	10.44	0.73	20.97	329.26	0.04	3.39	3.35	7.70
1	2	22	1	51.50	19.82	0.12	2.44	9999.00	0.00	0.42	0.42	0.50
2	1	1	4	102.50	1.28	0.00	0.07	9999.00	0.00	0.00	0.00	0.00
2	2	23	3	8.50	56.23	0.36	17.35	418.59	0.02	0.74	0.72	0.10
3	1	285	6	89.50	4.93	0.39	19.45	362.81	0.03	2.59	2.56	10.70
3	2	122	5	37.50	31.51	1.07	21.33	321.87	0.04	2.91	2.87	1.90
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry TIA\Reports\Appendices\T-Junction.osc

Report generation date: 05/08/2016 16:52:04

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	R135 North	50.0	10	10	80	0
2	Elm Road	50.0	10	10	80	0
3	R135 South	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1936	Yes	0	B	-
1	2	Traffic		2105	Yes	0	A	-
2	1	Traffic		1747	Yes	0	D	-
2	2	Traffic		1871	Yes	0	C	-
3	1	Traffic		1965	Yes	0	F	-
3	2	Traffic		1830	Yes	0	E	-
(Ped)	1	Pedestrian		10000	Yes	0	J	-
(Ped)	2	Pedestrian		10000	Yes	0	I	-
(Ped)	3	Pedestrian		10000	Yes	0	H	-
(Ped)	4	Pedestrian		10000	Yes	0	G	-

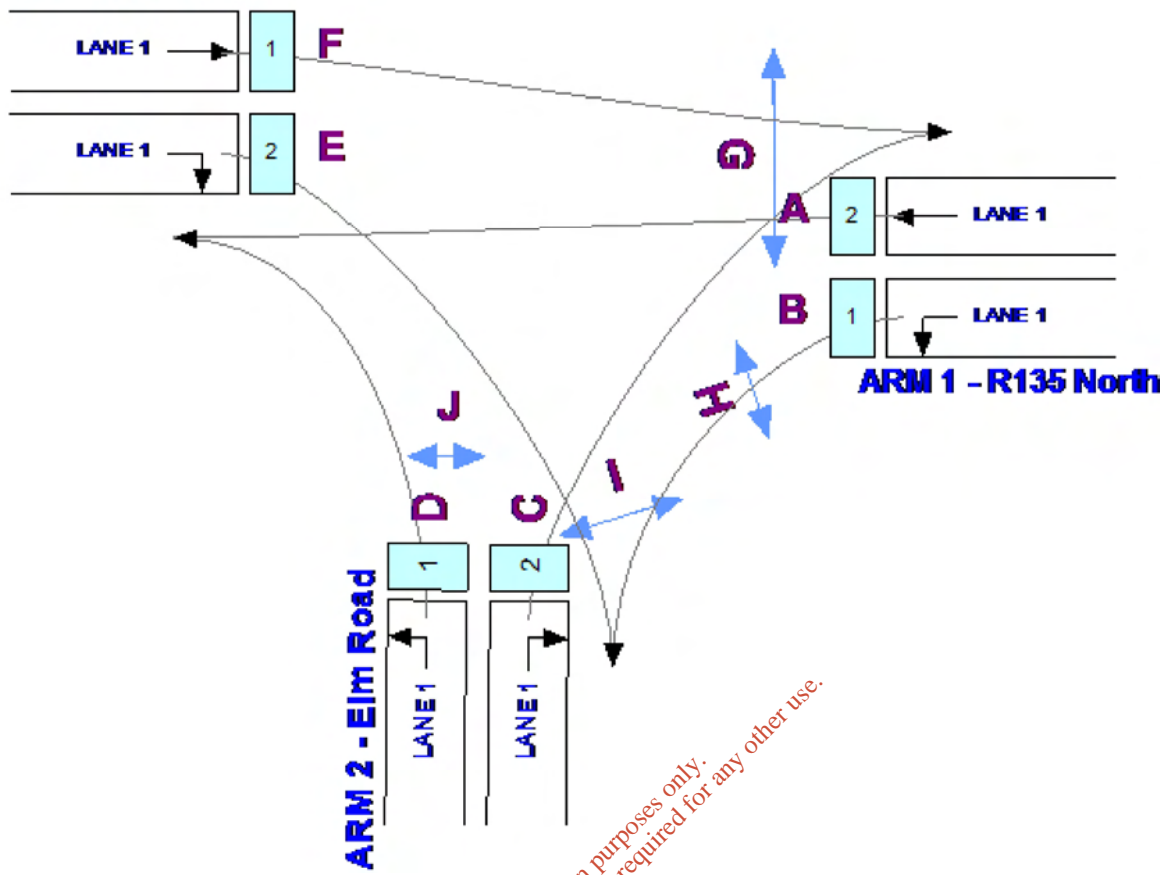
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
1	2	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
3	2	0.0	0.0	90	100	0	0.0	1.10	0
(Ped)	1	0.0	0.0	90	100	0	0.0	-	0
(Ped)	2	0.0	0.0	90	100	0	0.0	-	0
(Ped)	3	0.0	0.0	90	100	0	0.0	-	0
(Ped)	4	0.0	0.0	90	100	0	0.0	-	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2			1.00	100	Yes	3.50	0.0	0
1	2	1			3		0.00	10	No	3.50	0.0	0
2	1	1		3			1.00	12	Yes	3.50	0.0	0
2	2	1				1	1.00	12	No	3.50	0.0	0
3	1	1				1	0.00	10	Yes	3.50	0.0	0
3	2	1				2	1.00	10	No	3.50	0.0	0

Junction Diagram

ARM 3 - R135 South



Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No
G	(Name)	Pedestrian	-	7.0	0.0	No
H	(Name)	Pedestrian	-	7.0	0.0	No
I	(Name)	Pedestrian	-	7.0	0.0	No
J	(Name)	Pedestrian	-	7.0	0.0	No

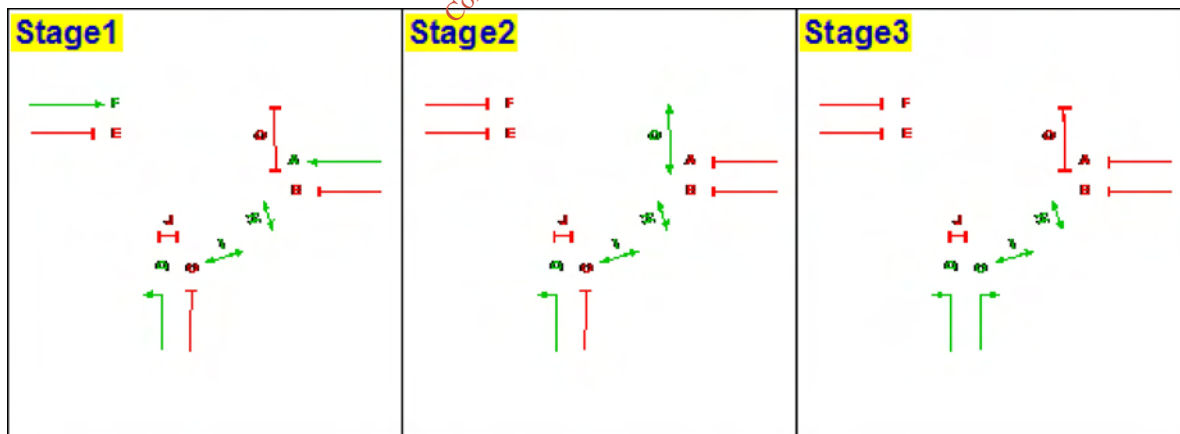
Intergreen Matrix

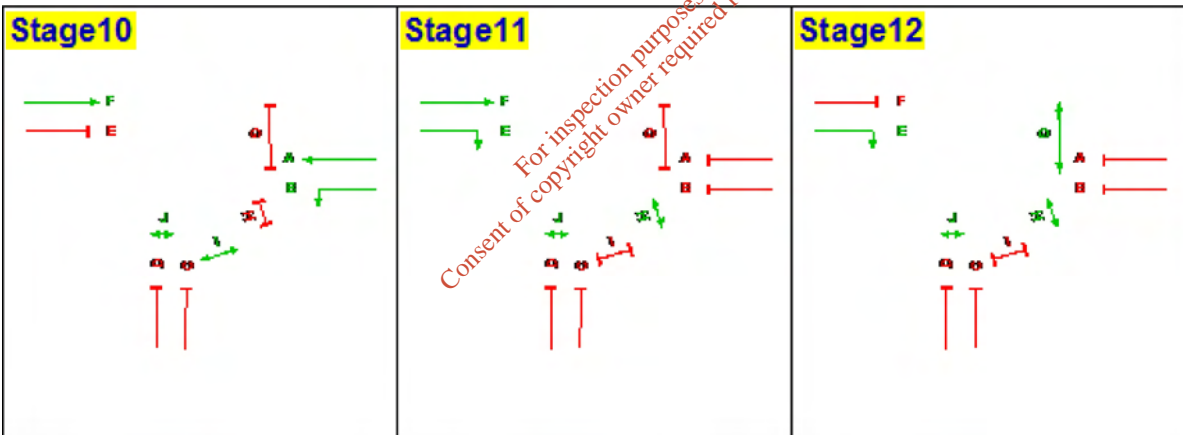
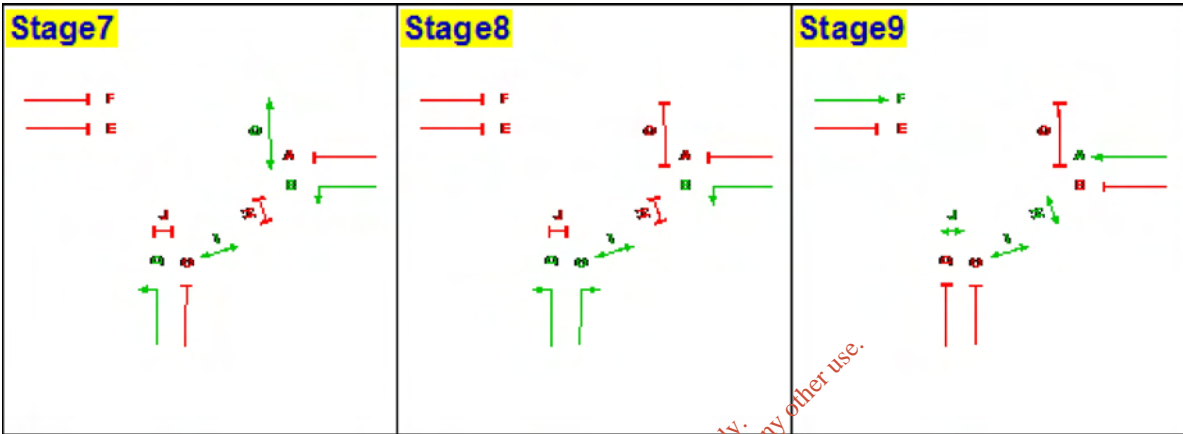
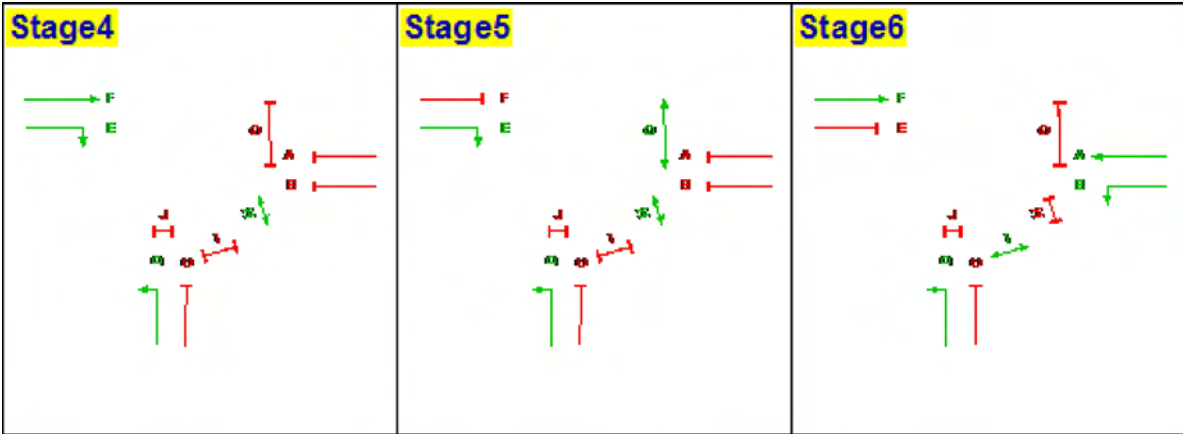
		To									
		A	B	C	D	E	F	G	H	I	J
From	A	-		5		5		5			
	B		-			5			5		
	C	5		-		5	5	8			5
	D				-						5
	E	5	6	5		-				7	
	F			5			-	8			
	G	5		5			5	-			
	H		6						-		
	I					5				-	
	J			5	7						-

Stages

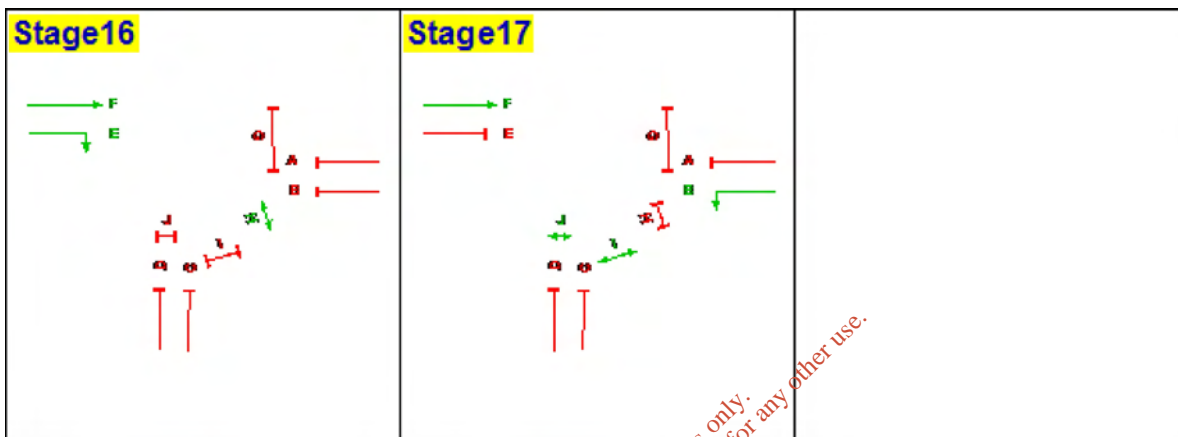
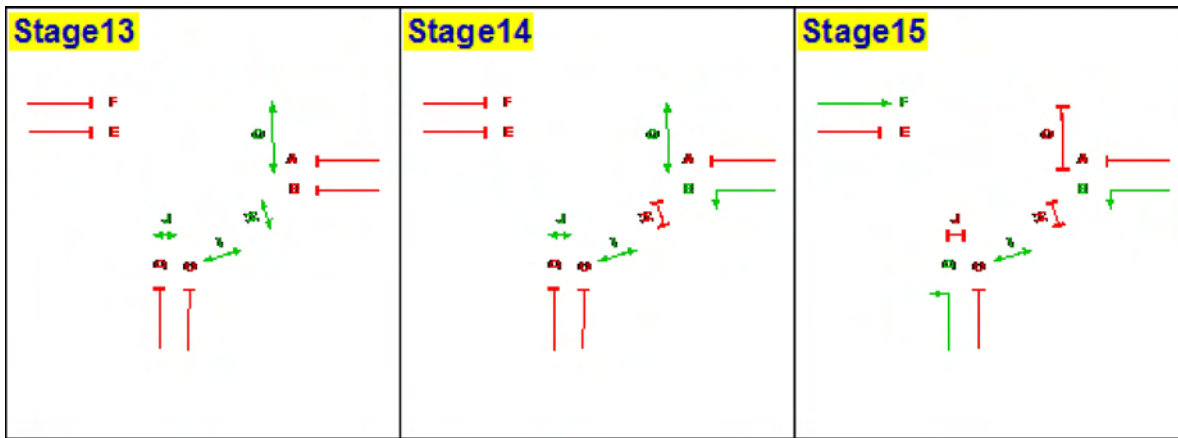
Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	A,D,F,H,I	Yes
2	-1	D,G,H,I	Yes
3	-1	C,D,H,I	Yes
4	-1	D,E,F,H	Yes
5	-1	D,E,G,H	Yes
6	-1	A,B,D,F,I	Yes
7	-1	B,D,G,I	Yes
8	-1	B,C,D,I	Yes
9	-1	A,F,H,I,J	Yes
10	-1	A,B,F,I,J	Yes
11	-1	E,F,H,J	Yes
12	-1	E,G,H,J	Yes
13	-1	G,H,I,J	Yes
14	-1	B,G,I,J	Yes
15	-1	B,D,F,I	Yes
16	-1	E,F,H	Yes
17	-1	B,F,I,J	Yes

Stage Diagrams





For inspection purposes only.
 Consent of copyright owner required for any other use.



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,13,12,13,14,7,8,6
2		1,6,8,7,14,13,12,13,2
3		1,2,13,14,13,2,3,4
4		1,4,3,2,13,14,13,2
5		1,2,13,12,13,14,13,2,3
6		1,3,2,13,14,13,12,13,2
7		1,2,13,14,13,12,13,2,3
8		1,3,2,13,12,13,14,13,2

Constraints

(No constraints)

For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	Yes	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set7 - 2017 PM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To		
		Arm 1	Arm 2	Arm 3
From	Arm 1	-	339	28
	Arm 2	30	-	2
	Arm 3	381	165	-

Average pedestrian flow on each pedestrian stream (if applicable): 1 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - R135 North	1	B	253	302	370	370	302	253
1 - R135 North	2	A	22	26	32	32	26	22
2 - Elm Road	1	D	1	2	2	2	2	1
2 - Elm Road	2	C	23	27	33	33	27	23
3 - R135 South	1	F	287	342	419	419	342	287
3 - R135 South	2	E	123	147	180	180	147	123
Pedestrians	1	J	1	1	1	1	1	1
Pedestrians	2	I	1	1	1	1	1	1
Pedestrians	3	H	1	1	1	1	1	1
Pedestrians	4	G	1	1	1	1	1	1

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - R135 North	92	8	-
2 - Elm Road	6	-	94
3 - R135 South	-	70	30

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	243.13	3.44	3.44	150.1

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	185.94	3.44	3.44	149.20

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
8	7.0	7.0	14.0
11	19.0	36.0	55.0
10	62.0	45.0	107.0
14	115.0	5.0	0.0

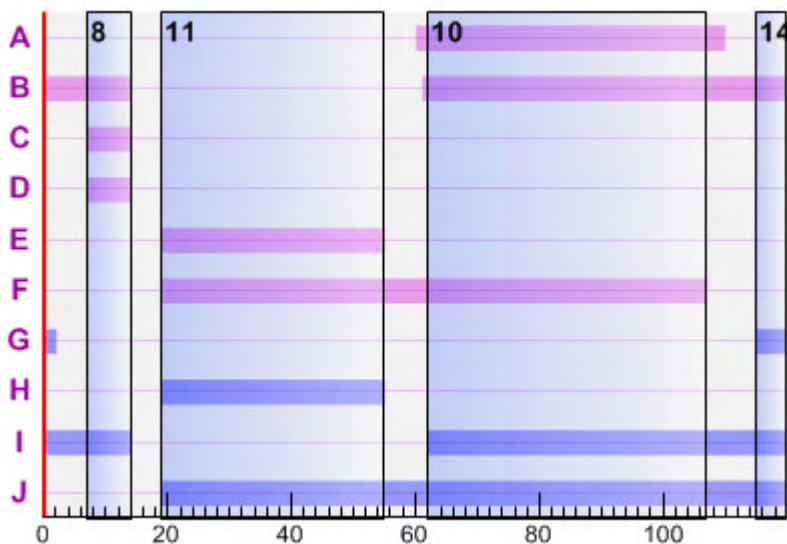
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	60	50.0	110						
B	61	73.0	14						
C	7	7.0	14						
D	7	7.0	14						
E	19	36.0	55						
F	19	88.0	107						
G	115	7.0	2						
H	19	36.0	55						
I	62	72.0	14						
J	19	101.0	0						

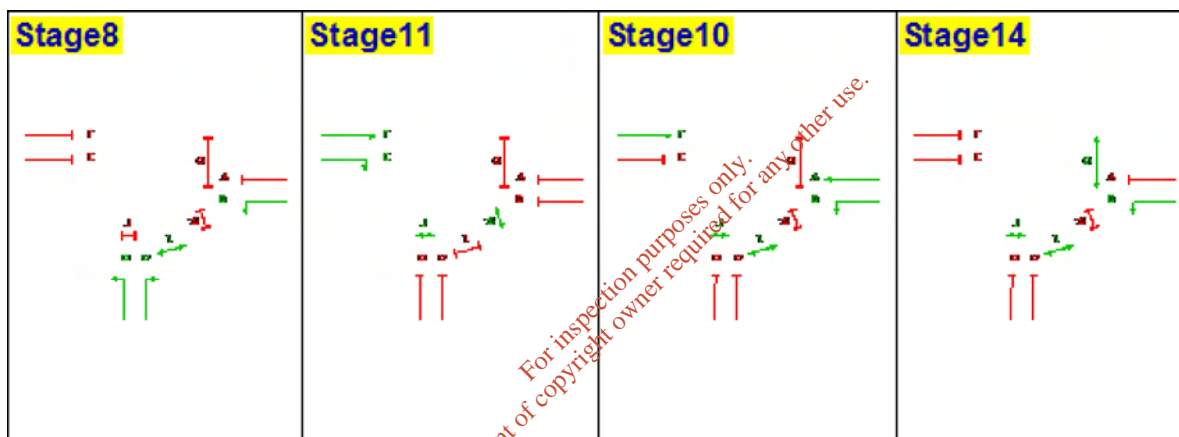
Phase Delays

Type	Phase	Terminating Stage	Starting Stage	Absolute Delay (s)	Relative Delay (s)
Losing	A	10	14	3.00	
Losing	G	14	8	2.00	

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	308	B	74.50	10.94	0.94	25.63	251.21	0.06	4.21	4.15	55.00
1	2	27	A	51.50	19.89	0.15	2.99	9999.00	0.00	0.52	0.52	3.70
2	1	2	D	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.10
2	2	28	C	8.50	57.37	0.45	21.13	325.99	0.04	0.91	0.87	0.80
3	1	349	F	89.50	5.22	0.51	23.81	277.94	0.05	3.23	3.18	77.20
3	2	150	E	37.50	32.37	1.35	26.23	243.13	0.06	3.61	3.54	13.30
Ped	1	1	J	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	I	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	H	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	G	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	253	2	74.50	10.45	0.73	21.05	327.57	0.04	3.41	3.37	7.70
1	2	22	1	51.50	19.82	0.12	2.44	9999.00	0.00	0.42	0.42	0.50
2	1	1	4	8.50	51.99	0.01	0.81	9999.00	0.00	0.03	0.03	0.00
2	2	23	3	8.50	56.14	0.36	17.35	418.59	0.02	0.74	0.72	0.10
3	1	287	6	89.50	4.93	0.39	19.58	359.58	0.03	2.61	2.58	10.70
3	2	123	5	37.50	31.53	1.08	21.51	318.45	0.04	2.93	2.89	1.90
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	302	2	74.50	10.88	0.91	25.13	258.19	0.06	4.12	4.06	9.00
1	2	26	1	51.50	19.87	0.14	2.88	9999.00	0.00	0.50	0.50	0.60
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	27	3	8.50	57.10	0.43	20.37	341.76	0.03	0.87	0.84	0.10
3	1	342	6	89.50	5.19	0.49	23.34	285.67	0.05	3.16	3.11	12.60
3	2	147	5	37.50	32.27	1.32	25.70	250.13	0.06	3.53	3.47	2.20
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	370	2	74.50	11.54	1.19	30.78	192.36	0.10	5.15	5.05	10.70
1	2	32	1	51.50	19.95	0.18	3.54	9999.00	0.00	0.61	0.61	0.70
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	33	3	8.50	58.60	0.54	24.90	261.44	0.05	1.08	1.03	0.20
3	1	419	6	89.50	5.57	0.65	28.59	214.80	0.08	3.96	3.87	15.10
3	2	180	5	37.50	33.35	1.67	31.48	185.94	0.10	4.38	4.28	2.50
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	370	2	74.50	11.54	1.19	30.78	192.36	0.10	5.15	5.05	10.70
1	2	32	1	51.50	19.95	0.18	3.54	9999.00	0.00	0.61	0.61	0.70
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	33	3	8.50	58.66	0.54	24.90	261.44	0.05	1.08	1.03	0.20
3	1	419	6	89.50	5.58	0.65	28.59	214.80	0.08	3.96	3.87	15.10
3	2	180	5	37.50	33.36	1.67	31.48	185.94	0.10	4.38	4.28	2.50
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	302	2	74.50	10.88	0.91	25.13	258.19	0.06	4.12	4.06	9.00
1	2	26	1	51.50	19.87	0.14	2.88	9999.00	0.00	0.50	0.50	0.60
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	27	3	8.50	57.19	0.43	20.37	341.76	0.03	0.88	0.84	0.10
3	1	342	6	89.50	5.19	0.49	23.34	285.67	0.05	3.16	3.11	12.60
3	2	147	5	37.50	32.28	1.32	25.70	250.13	0.06	3.53	3.47	2.20
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	253	2	74.50	10.45	0.73	21.05	327.57	0.04	3.41	3.37	7.70
1	2	22	1	51.50	19.82	0.12	2.44	9999.00	0.00	0.42	0.42	0.50
2	1	1	4	8.50	51.99	0.01	0.81	9999.00	0.00	0.03	0.03	0.00
2	2	23	3	8.50	56.23	0.36	17.35	418.59	0.02	0.74	0.72	0.10
3	1	287	6	89.50	4.93	0.39	19.58	359.58	0.03	2.61	2.58	10.70
3	2	123	5	37.50	31.54	1.08	21.51	318.45	0.04	2.93	2.89	1.90
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry TIA\Reports\Appendices\T-Junction.osc

Report generation date: 05/08/2016 16:52:56

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	R135 North	50.0	10	10	80	0
2	Elm Road	50.0	10	10	80	0
3	R135 South	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1936	Yes	0	B	-
1	2	Traffic		2105	Yes	0	A	-
2	1	Traffic		1747	Yes	0	D	-
2	2	Traffic		1871	Yes	0	C	-
3	1	Traffic		1965	Yes	0	F	-
3	2	Traffic		1830	Yes	0	E	-
(Ped)	1	Pedestrian		10000	Yes	0	J	-
(Ped)	2	Pedestrian		10000	Yes	0	I	-
(Ped)	3	Pedestrian		10000	Yes	0	H	-
(Ped)	4	Pedestrian		10000	Yes	0	G	-

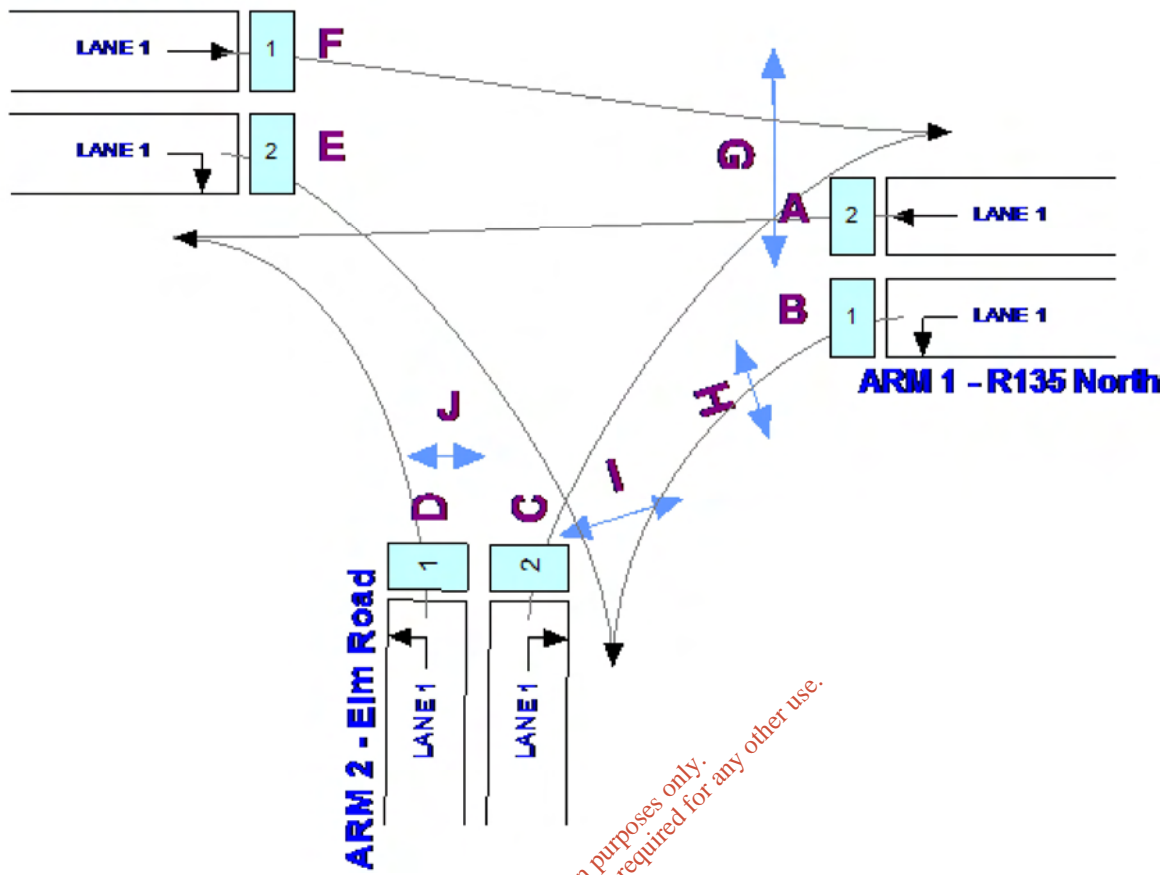
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
1	2	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
3	2	0.0	0.0	90	100	0	0.0	1.10	0
(Ped)	1	0.0	0.0	90	100	0	0.0	-	0
(Ped)	2	0.0	0.0	90	100	0	0.0	-	0
(Ped)	3	0.0	0.0	90	100	0	0.0	-	0
(Ped)	4	0.0	0.0	90	100	0	0.0	-	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2			1.00	100	Yes	3.50	0.0	0
1	2	1			3		0.00	10	No	3.50	0.0	0
2	1	1		3			1.00	12	Yes	3.50	0.0	0
2	2	1				1	1.00	12	No	3.50	0.0	0
3	1	1				1	0.00	10	Yes	3.50	0.0	0
3	2	1				2	1.00	10	No	3.50	0.0	0

Junction Diagram

ARM 3 - R135 South



Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No
G	(Name)	Pedestrian	-	7.0	0.0	No
H	(Name)	Pedestrian	-	7.0	0.0	No
I	(Name)	Pedestrian	-	7.0	0.0	No
J	(Name)	Pedestrian	-	7.0	0.0	No

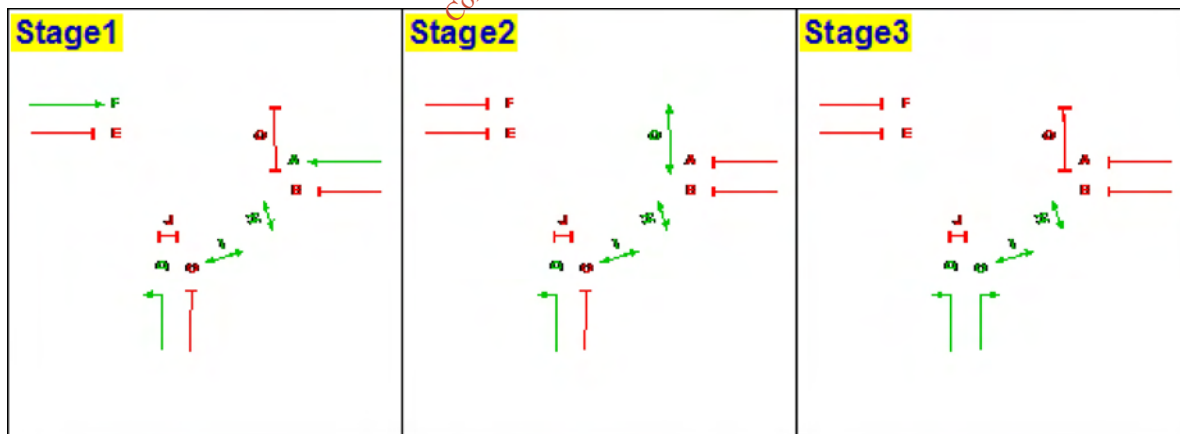
Intergreen Matrix

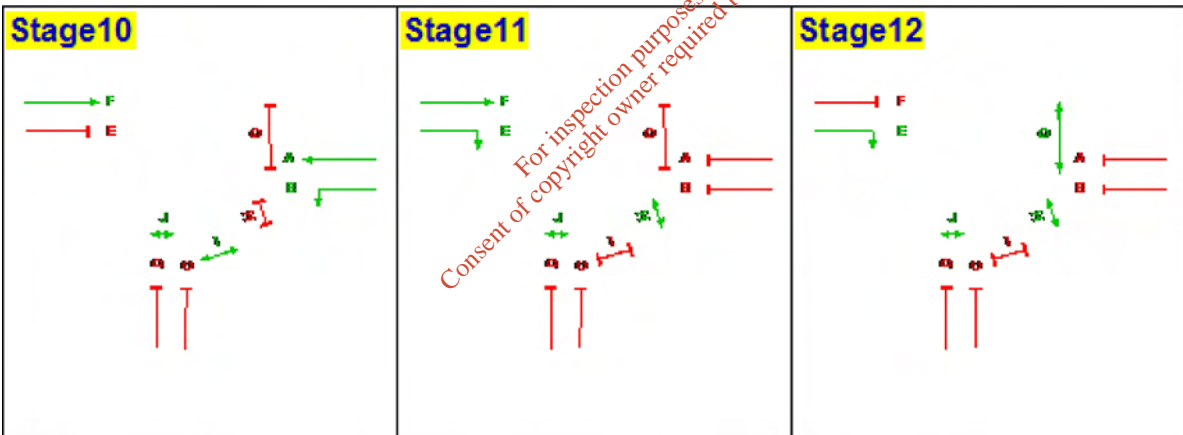
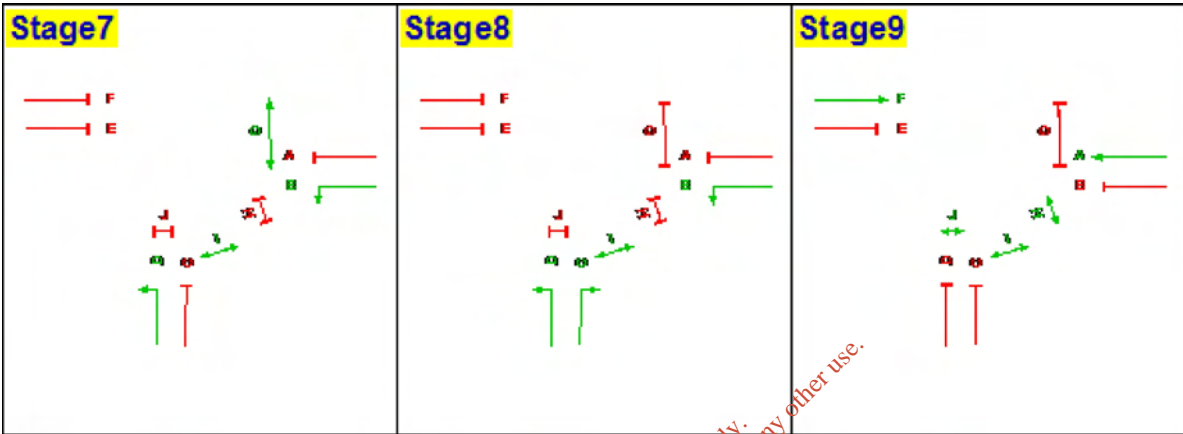
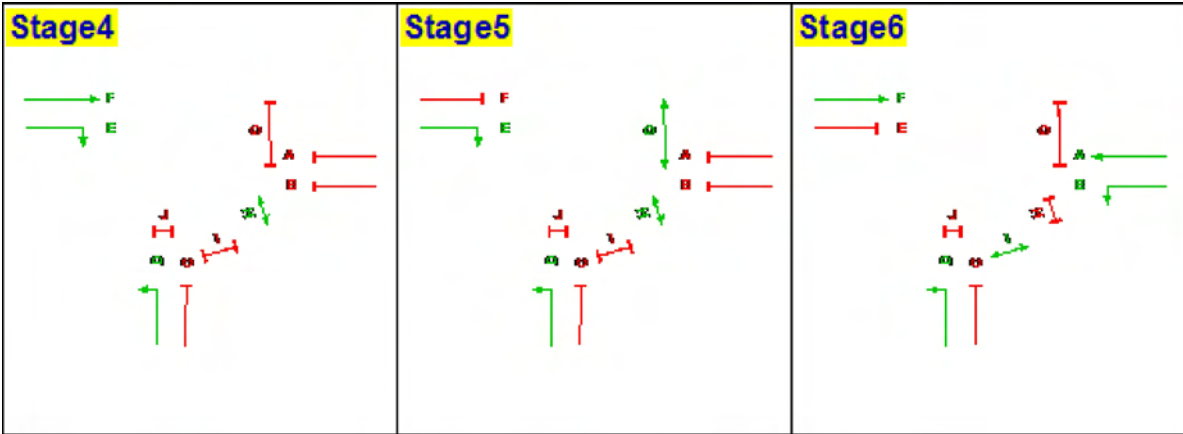
		To									
		A	B	C	D	E	F	G	H	I	J
From	A	-		5		5		5			
	B		-			5			5		
	C	5		-		5	5	8			5
	D				-						5
	E	5	6	5		-				7	
	F			5			-	8			
	G	5		5			5	-			
	H		6						-		
	I					5				-	
	J			5	7						-

Stages

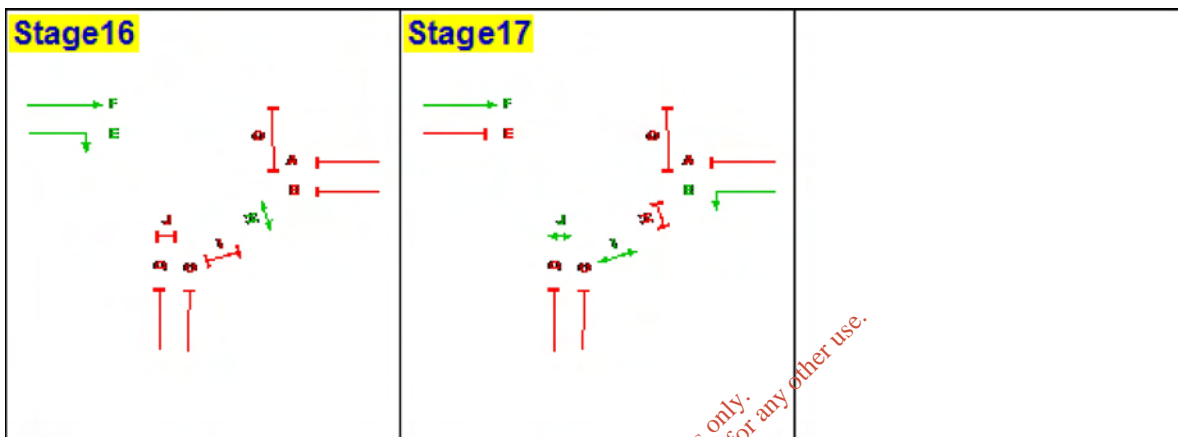
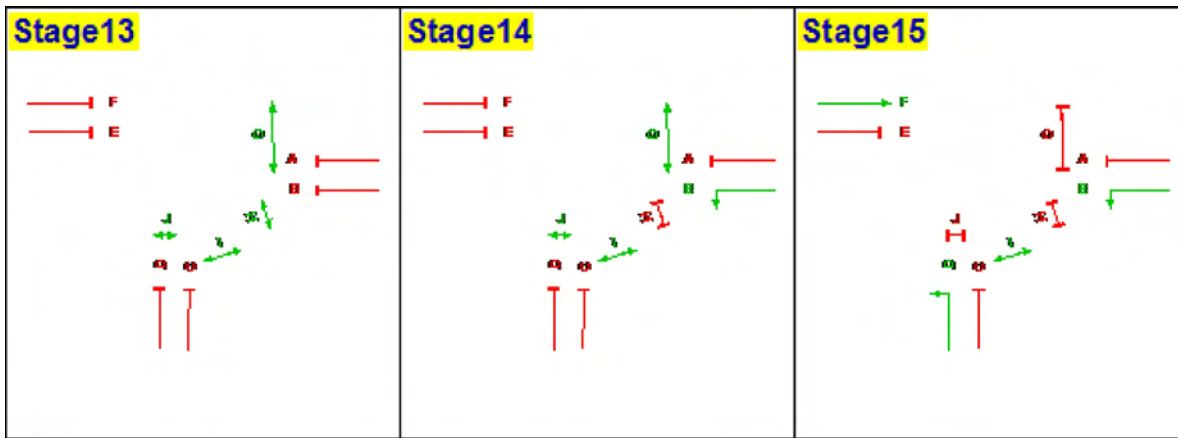
Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	A,D,F,H,I	Yes
2	-1	D,G,H,I	Yes
3	-1	C,D,H,I	Yes
4	-1	D,E,F,H	Yes
5	-1	D,E,G,H	Yes
6	-1	A,B,D,F,I	Yes
7	-1	B,D,G,I	Yes
8	-1	B,C,D,I	Yes
9	-1	A,F,H,I,J	Yes
10	-1	A,B,F,I,J	Yes
11	-1	E,F,H,J	Yes
12	-1	E,G,H,J	Yes
13	-1	G,H,I,J	Yes
14	-1	B,G,I,J	Yes
15	-1	B,D,F,I	Yes
16	-1	E,F,H	Yes
17	-1	B,F,I,J	Yes

Stage Diagrams





For inspection purposes only.
 Consent of copyright owner required for any other use.



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,13,12,13,14,7,8,6
2		1,6,8,7,14,13,12,13,2
3		1,2,13,14,13,2,3,4
4		1,4,3,2,13,14,13,2
5		1,2,13,12,13,14,13,2,3
6		1,3,2,13,14,13,12,13,2
7		1,2,13,14,13,12,13,2,3
8		1,3,2,13,12,13,14,13,2

Constraints

(No constraints)

For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	Yes	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set8 - 2017 PM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To		
		Arm 1	Arm 2	Arm 3
From	Arm 1	-	339	34
	Arm 2	30	-	2
	Arm 3	387	171	-

Average pedestrian flow on each pedestrian stream (if applicable): 1 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - R135 North	1	B	255	304	372	372	304	255
1 - R135 North	2	A	25	30	37	37	30	25
2 - Elm Road	1	D	1	2	2	2	2	1
2 - Elm Road	2	C	23	27	33	33	27	23
3 - R135 South	1	F	289	345	422	422	345	289
3 - R135 South	2	E	130	155	190	190	155	130
Pedestrians	1	J	1	1	1	1	1	1
Pedestrians	2	I	1	1	1	1	1	1
Pedestrians	3	H	1	1	1	1	1	1
Pedestrians	4	G	1	1	1	1	1	1

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - R135 North	91	9	-
2 - Elm Road	6	-	94
3 - R135 South	-	69	31

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	239.58	3.57	3.57	151.3

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	182.98	3.58	3.58	150.50

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
4	7.0	24.0	31.0
6	38.0	43.0	81.0
7	89.0	7.0	96.0
8	101.0	7.0	108.0
11	113.0	7.0	0.0

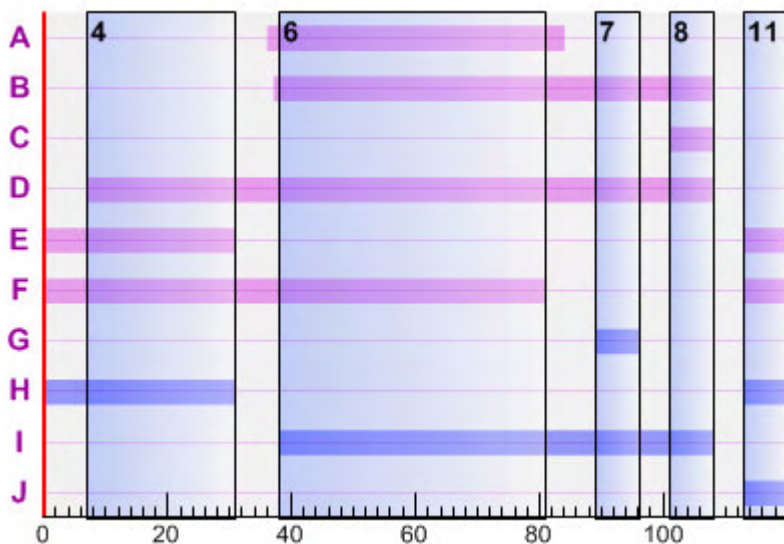
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	36	48.0	84						
B	37	71.0	108						
C	101	7.0	108						
D	7	101.0	108						
E	113	38.0	31						
F	113	88.0	81						
G	89	7.0	96						
H	113	38.0	31						
I	38	70.0	108						
J	113	7.0	0						

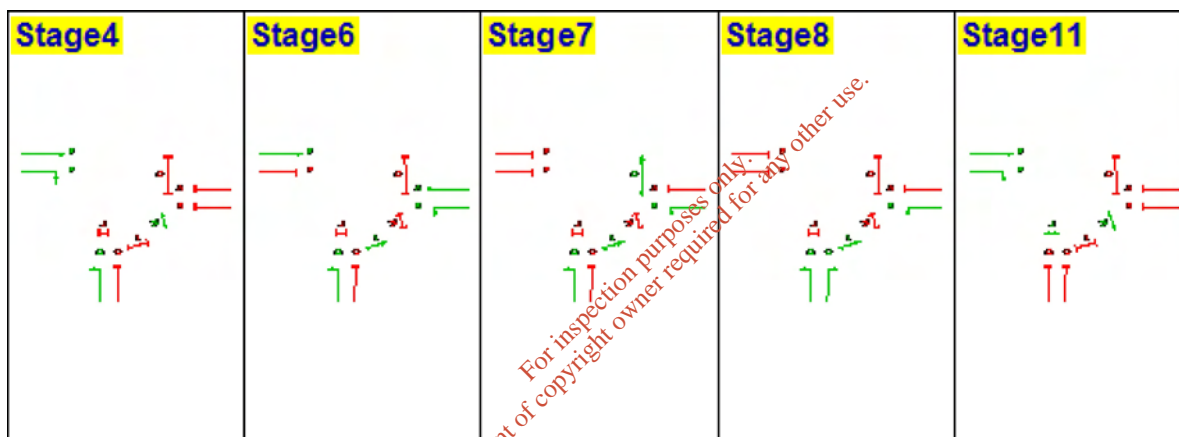
Phase Delays

Type	Phase	Terminating Stage	Starting Stage	Absolute Delay (s)	Relative Delay (s)
Losing	A	6	7	3.00	

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	310	B	72.50	11.93	1.03	26.50	239.58	0.07	4.43	4.36	53.40
1	2	31	A	49.50	21.12	0.18	3.57	9999.00	0.00	0.61	0.61	4.10
2	1	2	D	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.50
2	2	28	C	8.50	57.37	0.45	21.13	325.99	0.04	0.91	0.87	0.80
3	1	352	F	89.50	5.23	0.51	24.02	274.72	0.05	3.26	3.21	77.80
3	2	158	E	39.50	30.95	1.36	26.23	243.13	0.06	3.71	3.65	14.70
Ped	1	1	J	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	I	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	H	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	G	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	255	2	72.50	11.39	0.81	21.80	312.82	0.04	3.59	3.55	7.50
1	2	25	1	49.50	21.04	0.15	2.88	9999.00	0.00	0.49	0.49	0.60
2	1	1	4	102.50	1.28	0.00	0.07	9999.00	0.00	0.00	0.00	0.00
2	2	23	3	8.50	56.14	0.36	17.35	418.59	0.02	0.74	0.72	0.10
3	1	289	6	89.50	4.94	0.40	19.72	356.40	0.03	2.63	2.60	10.80
3	2	130	5	39.50	30.15	1.09	21.58	317.03	0.04	3.02	2.98	2.10
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	304	2	72.50	11.86	1.00	25.99	246.28	0.06	4.33	4.27	8.70
1	2	30	1	49.50	21.11	0.18	3.45	9999.00	0.00	0.59	0.59	0.70
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	27	3	8.50	57.10	0.43	20.37	341.76	0.03	0.87	0.84	0.10
3	1	345	6	89.50	5.20	0.50	23.54	282.32	0.05	3.19	3.14	12.70
3	2	155	5	39.50	30.86	1.33	25.73	249.77	0.06	3.64	3.58	2.40
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	372	2	72.50	12.58	1.30	31.80	182.98	0.11	5.41	5.30	10.30
1	2	37	1	49.50	21.20	0.22	4.26	9999.00	0.00	0.73	0.73	0.80
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	33	3	8.50	58.60	0.54	24.90	261.44	0.05	1.08	1.03	0.20
3	1	422	6	89.50	5.59	0.66	28.79	212.56	0.08	3.99	3.90	15.20
3	2	190	5	39.50	31.94	1.69	31.54	185.34	0.10	4.52	4.42	2.80
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	372	2	72.50	12.58	1.30	31.80	182.98	0.11	5.41	5.30	10.30
1	2	37	1	49.50	21.20	0.22	4.26	9999.00	0.00	0.73	0.73	0.80
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	33	3	8.50	58.66	0.54	24.90	261.44	0.05	1.08	1.03	0.20
3	1	422	6	89.50	5.59	0.66	28.79	212.56	0.08	3.99	3.90	15.20
3	2	190	5	39.50	31.95	1.69	31.54	185.34	0.10	4.52	4.42	2.80
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	304	2	72.50	11.87	1.00	25.99	246.28	0.06	4.34	4.27	8.80
1	2	30	1	49.50	21.11	0.18	3.45	9999.00	0.00	0.59	0.59	0.70
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	27	3	8.50	57.19	0.43	20.37	341.76	0.03	0.88	0.84	0.10
3	1	345	6	89.50	5.20	0.50	23.54	282.32	0.05	3.19	3.14	12.70
3	2	155	5	39.50	30.87	1.33	25.73	249.77	0.06	3.64	3.58	2.40
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	255	2	72.50	11.39	0.81	21.80	312.82	0.04	3.59	3.55	7.50
1	2	25	1	49.50	21.04	0.15	2.88	9999.00	0.00	0.49	0.49	0.60
2	1	1	4	102.50	1.28	0.00	0.07	9999.00	0.00	0.00	0.00	0.00
2	2	23	3	8.50	56.23	0.36	17.35	418.59	0.02	0.74	0.72	0.10
3	1	289	6	89.50	4.94	0.40	19.72	356.40	0.03	2.63	2.60	10.80
3	2	130	5	39.50	30.15	1.09	21.58	317.03	0.04	3.02	2.98	2.10
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry TIA\Reports\Appendices\T-Junction.osc

Report generation date: 05/08/2016 16:53:40

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	R135 North	50.0	10	10	80	0
2	Elm Road	50.0	10	10	80	0
3	R135 South	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1936	Yes	0	B	-
1	2	Traffic		2105	Yes	0	A	-
2	1	Traffic		1747	Yes	0	D	-
2	2	Traffic		1871	Yes	0	C	-
3	1	Traffic		1965	Yes	0	F	-
3	2	Traffic		1830	Yes	0	E	-
(Ped)	1	Pedestrian		10000	Yes	0	J	-
(Ped)	2	Pedestrian		10000	Yes	0	I	-
(Ped)	3	Pedestrian		10000	Yes	0	H	-
(Ped)	4	Pedestrian		10000	Yes	0	G	-

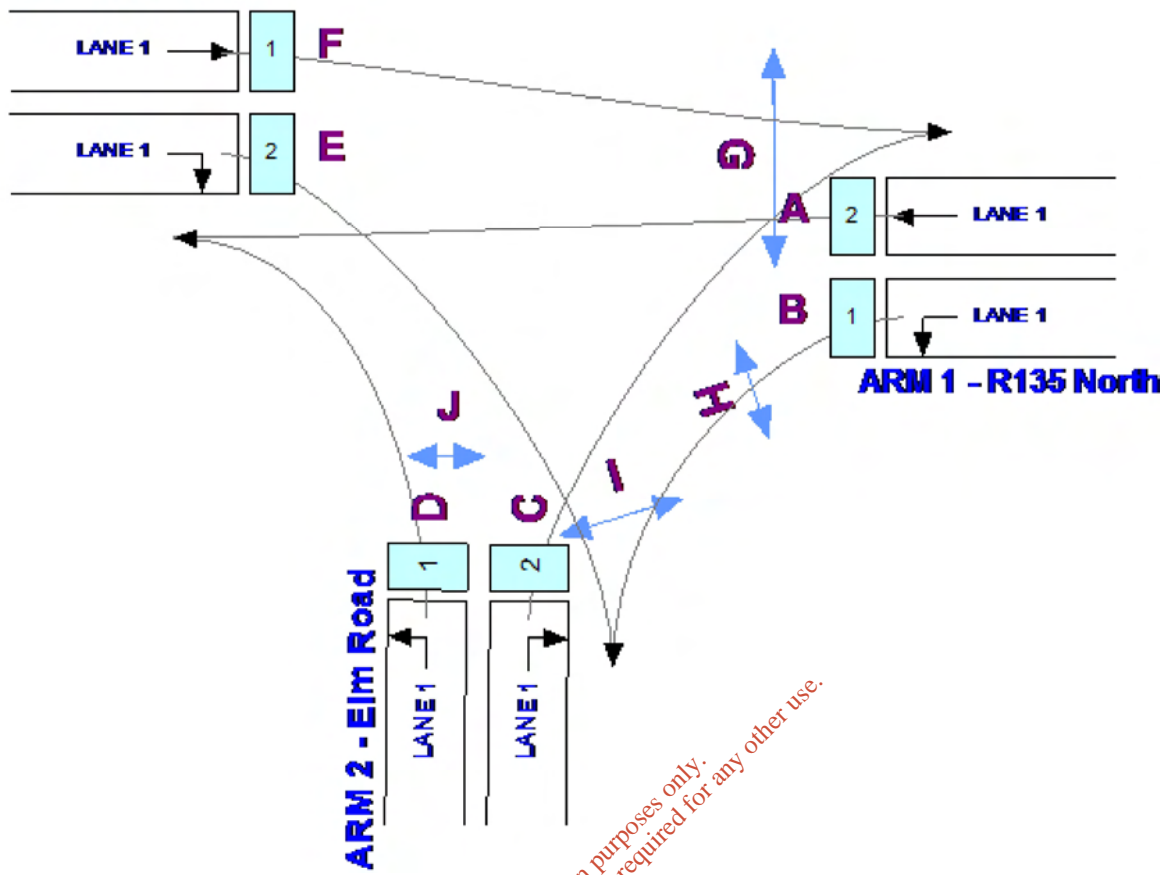
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
1	2	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
3	2	0.0	0.0	90	100	0	0.0	1.10	0
(Ped)	1	0.0	0.0	90	100	0	0.0	-	0
(Ped)	2	0.0	0.0	90	100	0	0.0	-	0
(Ped)	3	0.0	0.0	90	100	0	0.0	-	0
(Ped)	4	0.0	0.0	90	100	0	0.0	-	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2			1.00	100	Yes	3.50	0.0	0
1	2	1			3		0.00	10	No	3.50	0.0	0
2	1	1		3			1.00	12	Yes	3.50	0.0	0
2	2	1				1	1.00	12	No	3.50	0.0	0
3	1	1				1	0.00	10	Yes	3.50	0.0	0
3	2	1				2	1.00	10	No	3.50	0.0	0

Junction Diagram

ARM 3 - R135 South



For inspection purposes only.
Consent of copyright owner required for any other use.

Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No
G	(Name)	Pedestrian	-	7.0	0.0	No
H	(Name)	Pedestrian	-	7.0	0.0	No
I	(Name)	Pedestrian	-	7.0	0.0	No
J	(Name)	Pedestrian	-	7.0	0.0	No

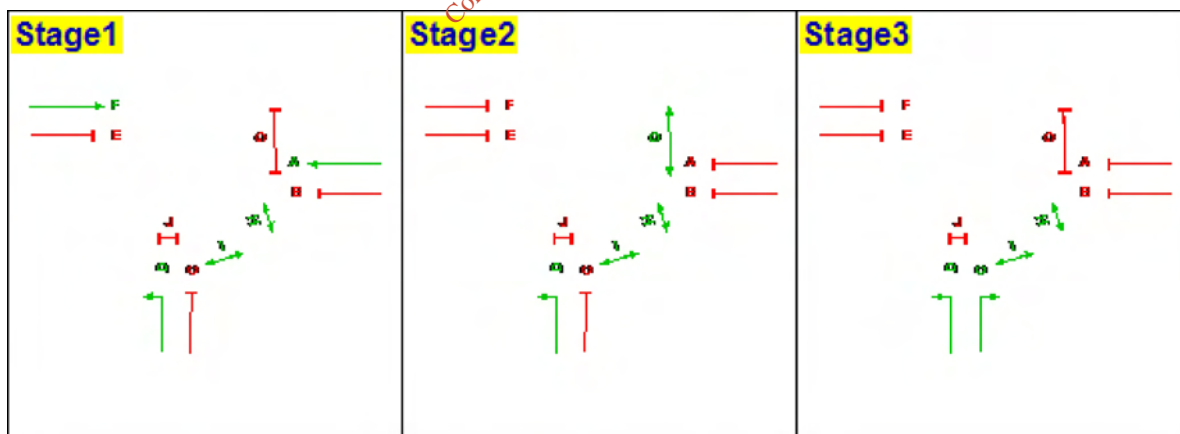
Intergreen Matrix

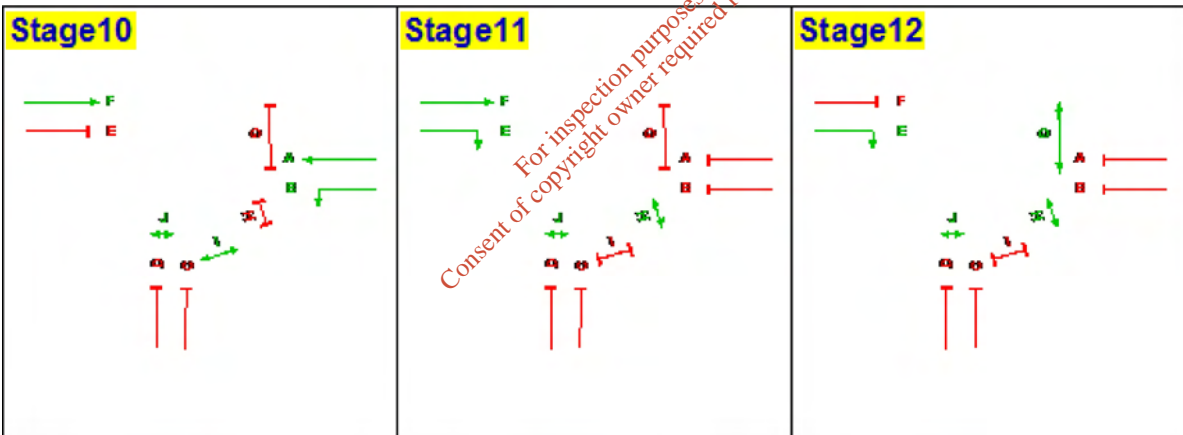
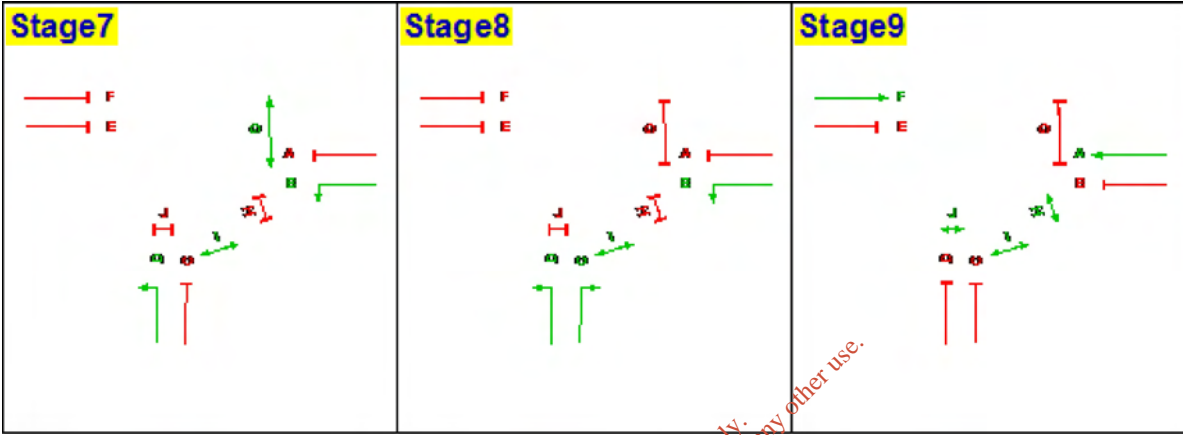
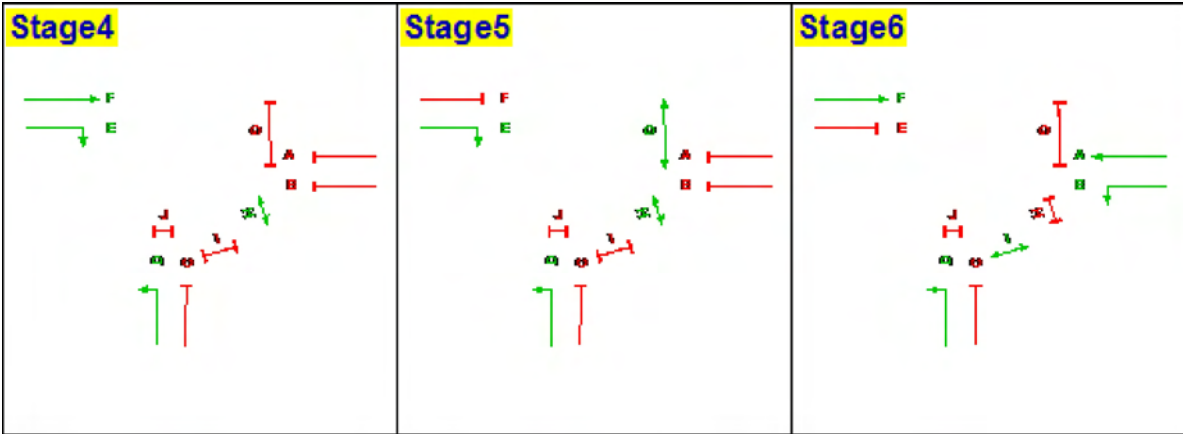
		To									
		A	B	C	D	E	F	G	H	I	J
From	A	-		5		5		5			
	B		-			5			5		
	C	5		-		5	5	8			5
	D				-						5
	E	5	6	5		-				7	
	F			5			-	8			
	G	5		5			5	-			
	H		6						-		
	I					5				-	
	J			5	7						-

Stages

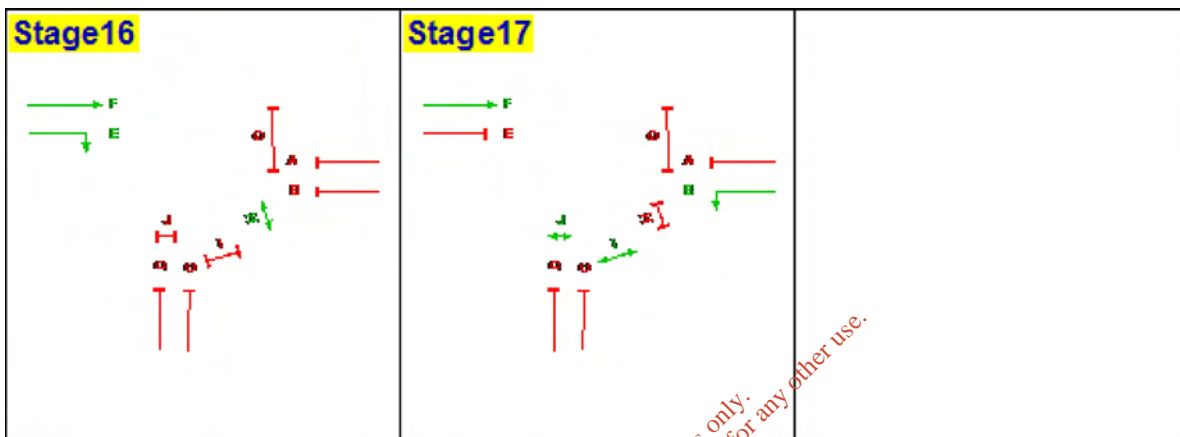
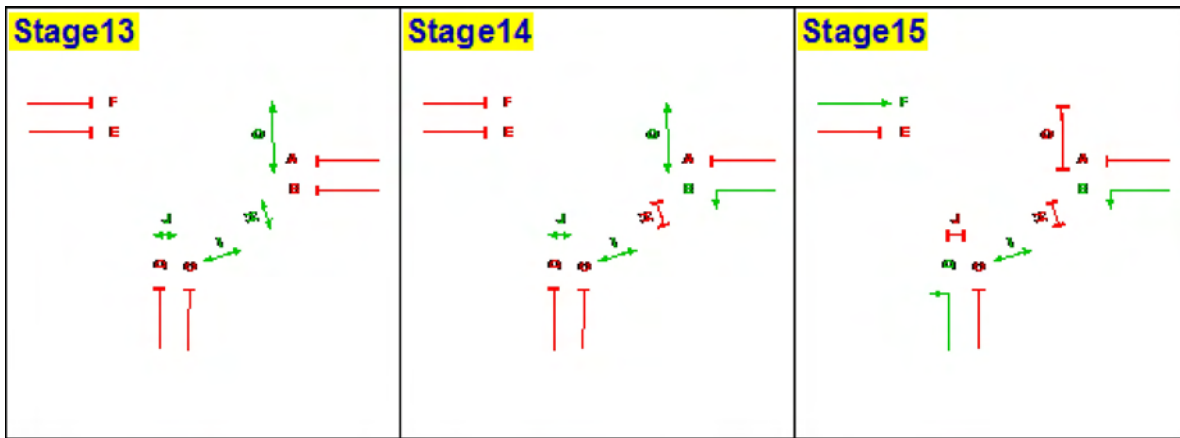
Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	A,D,F,H,I	Yes
2	-1	D,G,H,I	Yes
3	-1	C,D,H,I	Yes
4	-1	D,E,F,H	Yes
5	-1	D,E,G,H	Yes
6	-1	A,B,D,F,I	Yes
7	-1	B,D,G,I	Yes
8	-1	B,C,D,I	Yes
9	-1	A,F,H,I,J	Yes
10	-1	A,B,F,I,J	Yes
11	-1	E,F,H,J	Yes
12	-1	E,G,H,J	Yes
13	-1	G,H,I,J	Yes
14	-1	B,G,I,J	Yes
15	-1	B,D,F,I	Yes
16	-1	E,F,H	Yes
17	-1	B,F,I,J	Yes

Stage Diagrams





For inspection purposes only.
 Consent of copyright owner required for any other use.



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,13,12,13,14,7,8,6
2		1,6,8,7,14,13,12,13,2
3		1,2,13,14,13,2,3,4
4		1,4,3,2,13,14,13,2
5		1,2,13,12,13,14,13,2,3
6		1,3,2,13,14,13,12,13,2
7		1,2,13,14,13,12,13,2,3
8		1,3,2,13,12,13,14,13,2

Constraints

(No constraints)

For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	Yes	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1

Demand Set9 - 2023 PM Peak No Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To		
		Arm 1	Arm 2	Arm 3
From	Arm 1	-	354	29
	Arm 2	32	-	2
	Arm 3	397	172	-

Average pedestrian flow on each pedestrian stream (if applicable): 1 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - R135 North	1	B	264	316	386	386	316	264
1 - R135 North	2	A	23	27	34	34	27	23
2 - Elm Road	1	D	2	2	2	2	2	2
2 - Elm Road	2	C	24	29	35	35	29	24
3 - R135 South	1	F	299	357	437	437	357	299
3 - R135 South	2	E	128	153	187	187	153	128
Pedestrians	1	J	1	1	1	1	1	1
Pedestrians	2	I	1	1	1	1	1	1
Pedestrians	3	H	1	1	1	1	1	1
Pedestrians	4	G	1	1	1	1	1	1

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - R135 North	92	8	-
2 - Elm Road	6	-	94
3 - R135 South	-	70	30

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	229.93	3.59	3.59	156.2

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	175.23	3.62	3.62	155.40

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
4	7.0	22.0	29.0
6	36.0	45.0	81.0
7	89.0	7.0	96.0
8	101.0	7.0	108.0
11	113.0	7.0	0.0

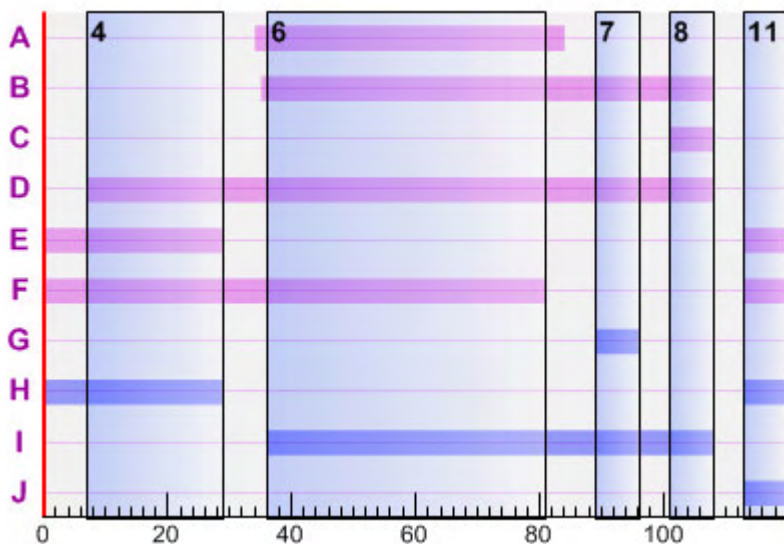
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	34	50.0	84						
B	35	73.0	108						
C	101	7.0	108						
D	7	101.0	108						
E	113	36.0	29						
F	113	88.0	81						
G	89	7.0	96						
H	113	36.0	29						
I	36	72.0	108						
J	113	7.0	0						

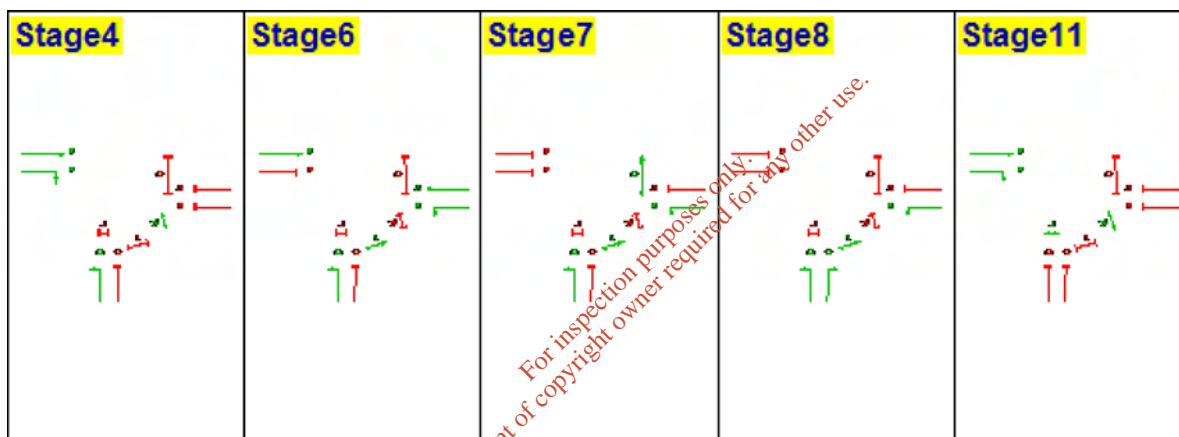
Phase Delays

Type	Phase	Terminating Stage	Starting Stage	Absolute Delay (s)	Relative Delay (s)
Losing	A	6	7	3.00	

Phase Timings Diagram



Final Stage Sequence



Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	322	B	74.50	11.07	0.99	26.79	235.94	0.07	4.42	4.35	57.00
1	2	28	A	51.50	19.90	0.15	3.10	9999.00	0.00	0.54	0.54	3.90
2	1	2	D	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.50
2	2	29	C	8.50	57.62	0.46	21.88	311.30	0.04	0.94	0.90	0.90
3	1	364	F	89.50	5.29	0.54	24.84	262.36	0.06	3.38	3.33	80.20
3	2	156	E	37.50	32.56	1.41	27.28	229.93	0.07	3.76	3.69	13.70
Ped	1	1	J	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	I	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	H	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	G	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	264	2	74.50	10.54	0.77	21.96	309.75	0.04	3.57	3.52	8.00
1	2	23	1	51.50	19.84	0.13	2.55	9999.00	0.00	0.44	0.44	0.50
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	24	3	8.50	56.36	0.38	18.11	396.98	0.03	0.77	0.75	0.10
3	1	299	6	89.50	4.99	0.41	20.40	341.14	0.04	2.73	2.69	11.20
3	2	128	5	37.50	31.68	1.13	22.38	302.10	0.04	3.05	3.01	1.90
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	316	2	74.50	11.01	0.97	26.29	242.32	0.07	4.33	4.26	9.40
1	2	27	1	51.50	19.89	0.15	2.99	9999.00	0.00	0.52	0.52	0.60
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	29	3	8.50	57.58	0.46	21.88	311.30	0.04	0.94	0.90	0.10
3	1	357	6	89.50	5.26	0.52	24.36	269.47	0.06	3.31	3.26	13.10
3	2	153	5	37.50	32.46	1.38	26.75	236.40	0.07	3.68	3.62	2.20
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	386	2	74.50	11.71	1.26	32.11	180.24	0.11	5.40	5.29	11.00
1	2	34	1	51.50	19.97	0.19	3.76	9999.00	0.00	0.65	0.65	0.80
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	35	3	8.50	59.15	0.58	26.41	240.79	0.06	1.15	1.09	0.20
3	1	437	6	89.50	5.67	0.69	29.82	201.83	0.09	4.15	4.06	15.70
3	2	187	5	37.50	33.60	1.75	32.70	175.23	0.11	4.56	4.45	2.60
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	386	2	74.50	11.71	1.26	32.11	180.24	0.11	5.40	5.29	11.00
1	2	34	1	51.50	19.97	0.19	3.76	9999.00	0.00	0.65	0.65	0.80
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	35	3	8.50	59.21	0.58	26.41	240.79	0.06	1.15	1.09	0.20
3	1	437	6	89.50	5.67	0.69	29.82	201.83	0.09	4.15	4.06	15.70
3	2	187	5	37.50	33.60	1.75	32.70	175.23	0.11	4.56	4.45	2.60
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	316	2	74.50	11.01	0.97	26.29	242.32	0.07	4.33	4.26	9.40
1	2	27	1	51.50	19.89	0.15	2.99	9999.00	0.00	0.52	0.52	0.60
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	29	3	8.50	57.70	0.46	21.88	311.30	0.04	0.94	0.90	0.10
3	1	357	6	89.50	5.26	0.52	24.36	269.47	0.06	3.31	3.26	13.10
3	2	153	5	37.50	32.47	1.38	26.75	236.40	0.07	3.68	3.62	2.20
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	264	2	74.50	10.55	0.77	21.96	309.75	0.04	3.57	3.52	8.00
1	2	23	1	51.50	19.84	0.13	2.55	9999.00	0.00	0.44	0.44	0.50
2	1	2	4	102.50	1.28	0.00	0.13	9999.00	0.00	0.01	0.01	0.10
2	2	24	3	8.50	56.48	0.38	18.11	396.98	0.03	0.77	0.75	0.10
3	1	299	6	89.50	4.99	0.41	20.40	341.14	0.04	2.73	2.69	11.20
3	2	128	5	37.50	31.69	1.13	22.38	302.10	0.04	3.05	3.01	1.90
Ped	1	1	10	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00
Ped	2	1	9	73.50	9.01	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	37.50	28.36	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry TIA\Reports\Appendices\T-Junction.osc

Report generation date: 05/08/2016 16:55:19

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	R135 North	50.0	10	10	80	0
2	Elm Road	50.0	10	10	80	0
3	R135 South	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1936	Yes	0	B	-
1	2	Traffic		2105	Yes	0	A	-
2	1	Traffic		1747	Yes	0	D	-
2	2	Traffic		1871	Yes	0	C	-
3	1	Traffic		1965	Yes	0	F	-
3	2	Traffic		1830	Yes	0	E	-
(Ped)	1	Pedestrian		10000	Yes	0	J	-
(Ped)	2	Pedestrian		10000	Yes	0	I	-
(Ped)	3	Pedestrian		10000	Yes	0	H	-
(Ped)	4	Pedestrian		10000	Yes	0	G	-

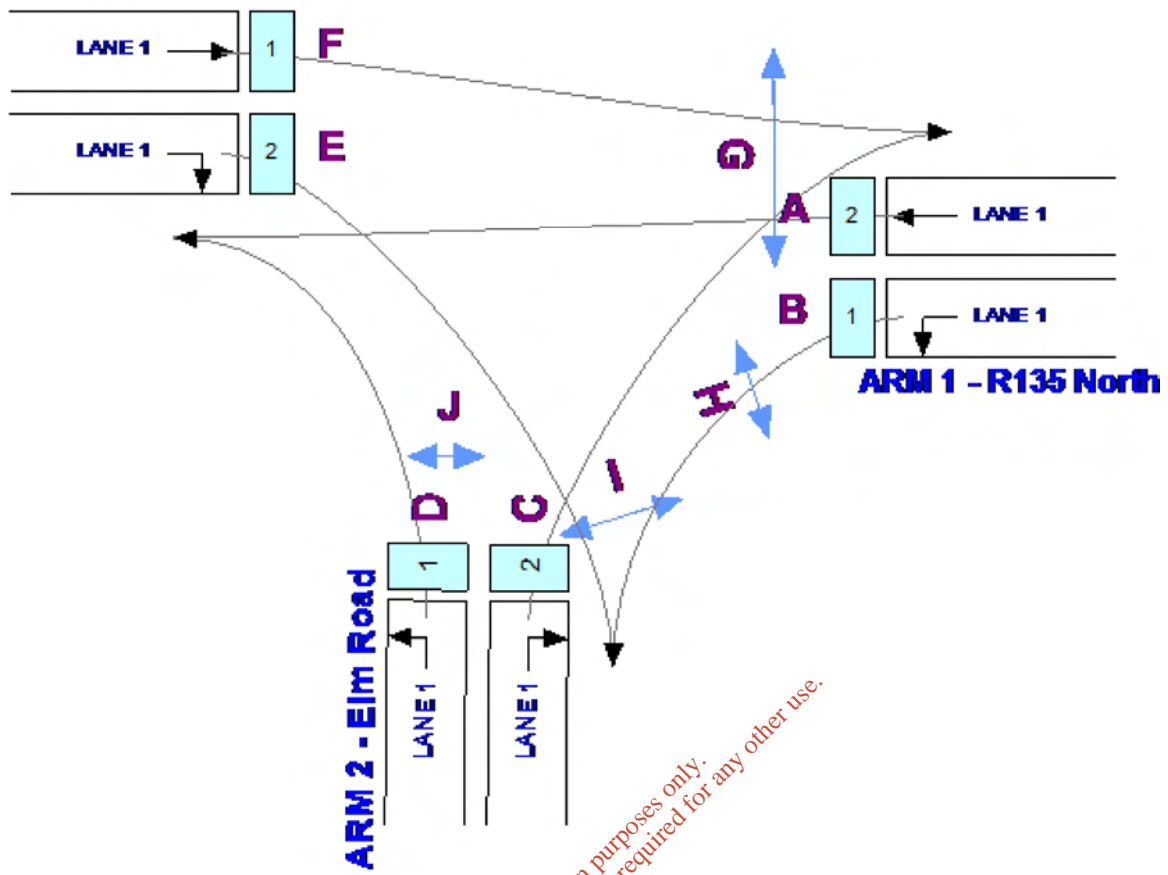
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
1	2	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
3	2	0.0	0.0	90	100	0	0.0	1.10	0
(Ped)	1	0.0	0.0	90	100	0	0.0	-	0
(Ped)	2	0.0	0.0	90	100	0	0.0	-	0
(Ped)	3	0.0	0.0	90	100	0	0.0	-	0
(Ped)	4	0.0	0.0	90	100	0	0.0	-	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2			1.00	100	Yes	3.50	0.0	0
1	2	1			3		0.00	10	No	3.50	0.0	0
2	1	1		3			1.00	12	Yes	3.50	0.0	0
2	2	1				1	1.00	12	No	3.50	0.0	0
3	1	1				1	0.00	10	Yes	3.50	0.0	0
3	2	1				2	1.00	10	No	3.50	0.0	0

Junction Diagram

ARM 3 - R135 South



Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No
G	(Name)	Pedestrian	-	7.0	0.0	No
H	(Name)	Pedestrian	-	7.0	0.0	No
I	(Name)	Pedestrian	-	7.0	0.0	No
J	(Name)	Pedestrian	-	7.0	0.0	No

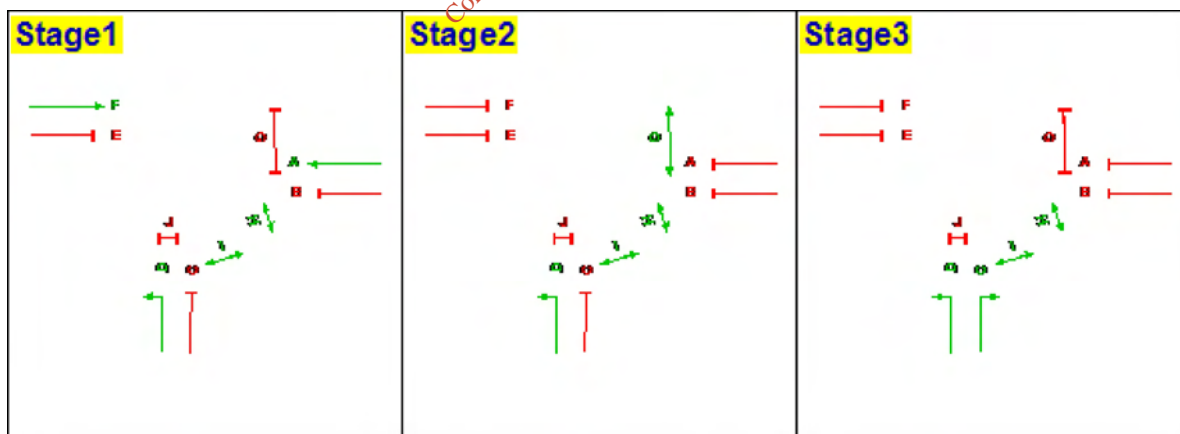
Intergreen Matrix

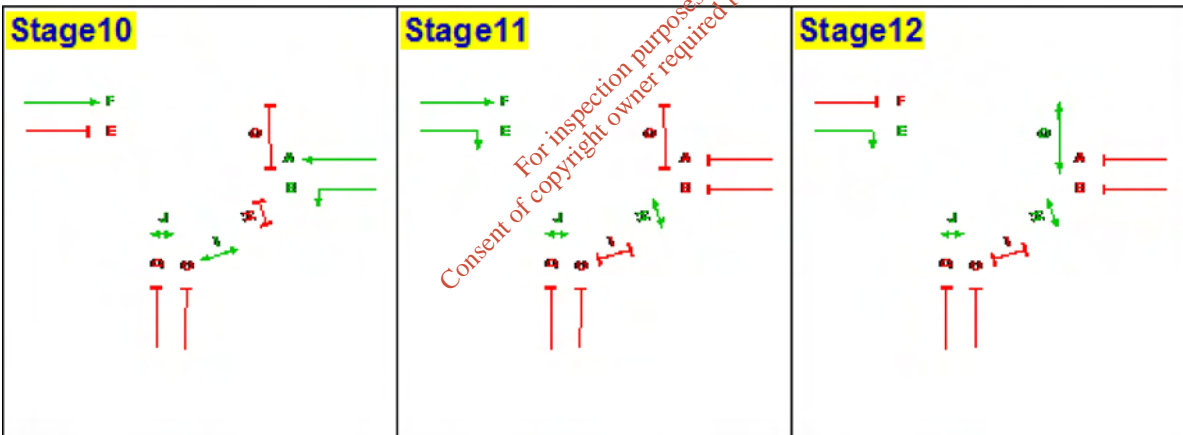
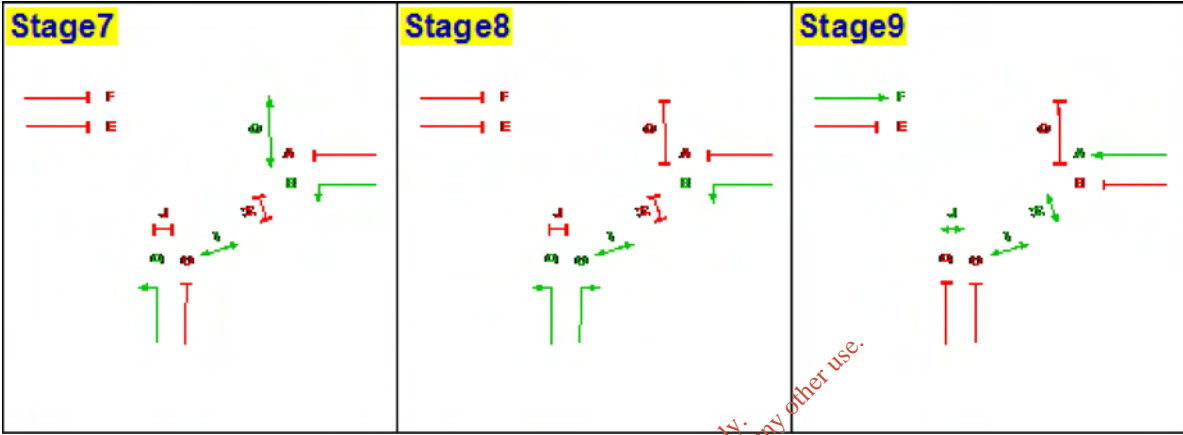
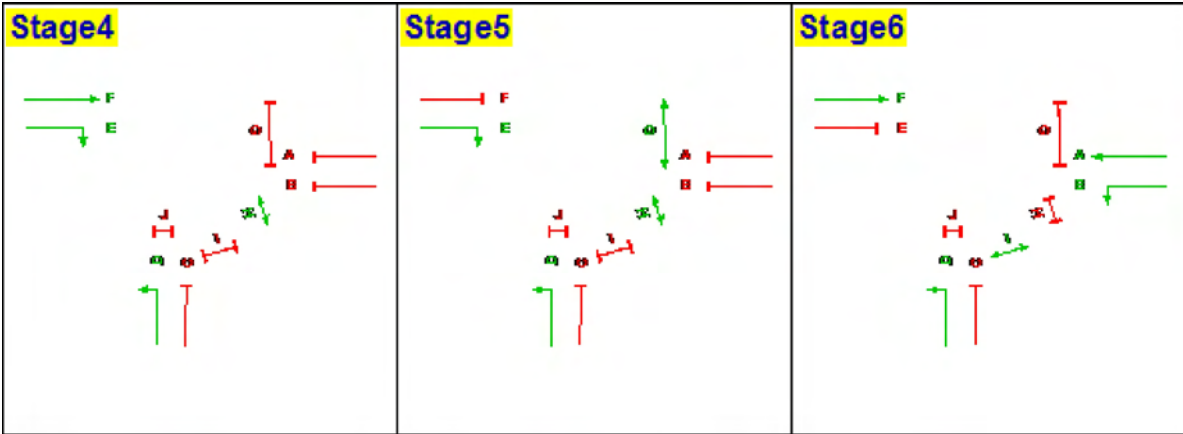
		To									
		A	B	C	D	E	F	G	H	I	J
From	A	-		5		5		5			
	B		-			5			5		
	C	5		-		5	5	8			5
	D				-						5
	E	5	6	5		-				7	
	F			5			-	8			
	G	5		5			5	-			
	H		6						-		
	I					5				-	
	J			5	7						-

Stages

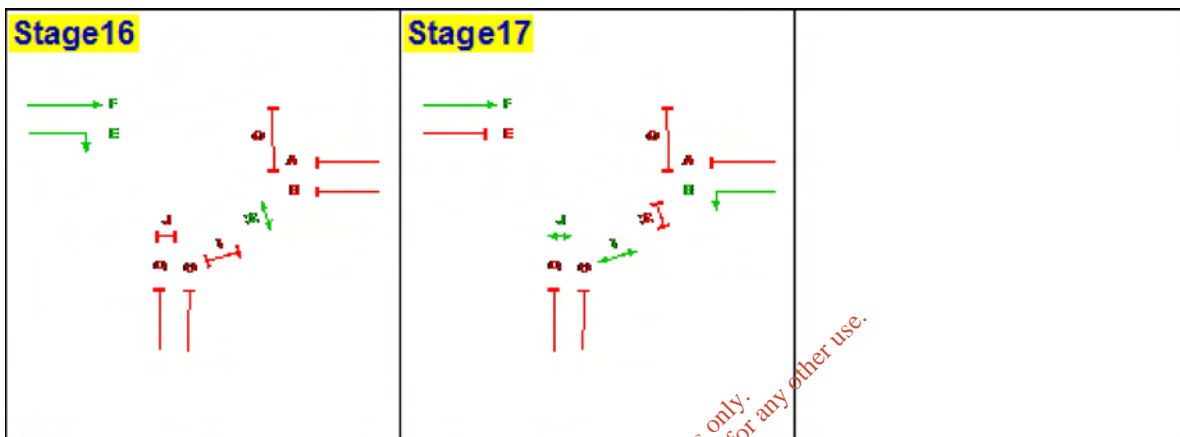
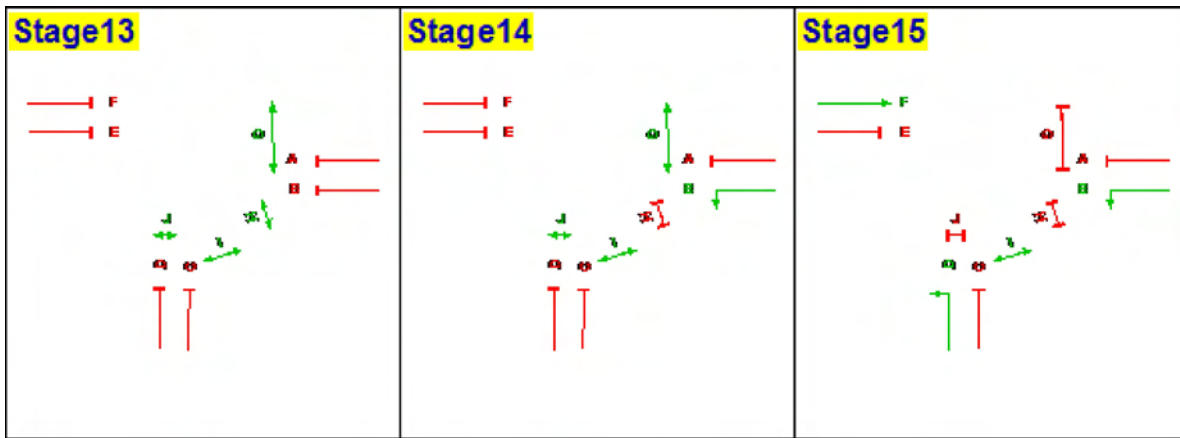
Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	A,D,F,H,I	Yes
2	-1	D,G,H,I	Yes
3	-1	C,D,H,I	Yes
4	-1	D,E,F,H	Yes
5	-1	D,E,G,H	Yes
6	-1	A,B,D,F,I	Yes
7	-1	B,D,G,I	Yes
8	-1	B,C,D,I	Yes
9	-1	A,F,H,I,J	Yes
10	-1	A,B,F,I,J	Yes
11	-1	E,F,H,J	Yes
12	-1	E,G,H,J	Yes
13	-1	G,H,I,J	Yes
14	-1	B,G,I,J	Yes
15	-1	B,D,F,I	Yes
16	-1	E,F,H	Yes
17	-1	B,F,I,J	Yes

Stage Diagrams





For inspection purposes only.
 Consent of copyright owner required for any other use.



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,13,12,13,14,7,8,6
2		1,6,8,7,14,13,12,13,2
3		1,2,13,14,13,2,3,4
4		1,4,3,2,13,14,13,2
5		1,2,13,12,13,14,13,2,3
6		1,3,2,13,14,13,12,13,2
7		1,2,13,14,13,12,13,2,3
8		1,3,2,13,12,13,14,13,2

Constraints

(No constraints)

For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	Yes	16:30	18:00	ODTAB	No	D1

Demand Set10 - 2023 PM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To		
		Arm 1	Arm 2	Arm 3
From	Arm 1	-	354	35
	Arm 2	32	-	2
	Arm 3	403	178	-

Average pedestrian flow on each pedestrian stream (if applicable): 1 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - R135 North	1	B	265	317	388	388	317	265
1 - R135 North	2	A	26	31	38	38	31	26
2 - Elm Road	1	D	2	2	2	2	2	2
2 - Elm Road	2	C	24	29	35	35	29	24
3 - R135 South	1	F	301	359	440	440	359	301
3 - R135 South	2	E	135	161	198	198	161	135
Pedestrians	1	J	1	1	1	1	1	1
Pedestrians	2	I	1	1	1	1	1	1
Pedestrians	3	H	1	1	1	1	1	1
Pedestrians	4	G	1	1	1	1	1	1

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - R135 North	91	9	-
2 - Elm Road	6	-	94
3 - R135 South	-	69	31

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	225.91	3.76	3.76	156.4

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	171.31	3.79	3.79	155.80

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
8	7.0	7.0	14.0
11	19.0	38.0	57.0
10	64.0	43.0	107.0
14	115.0	5.0	0.0

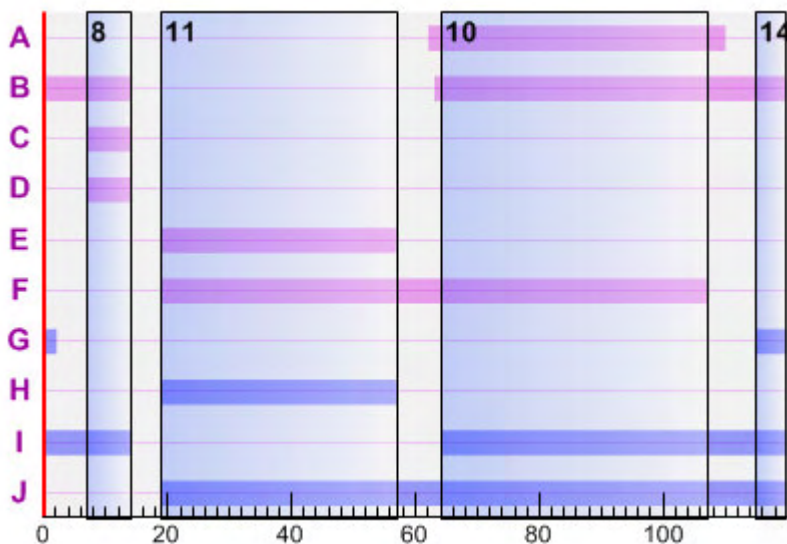
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	62	48.0	110						
B	63	71.0	14						
C	7	7.0	14						
D	7	7.0	14						
E	19	38.0	57						
F	19	88.0	107						
G	115	7.0	2						
H	19	38.0	57						
I	64	70.0	14						
J	19	101.0	0						

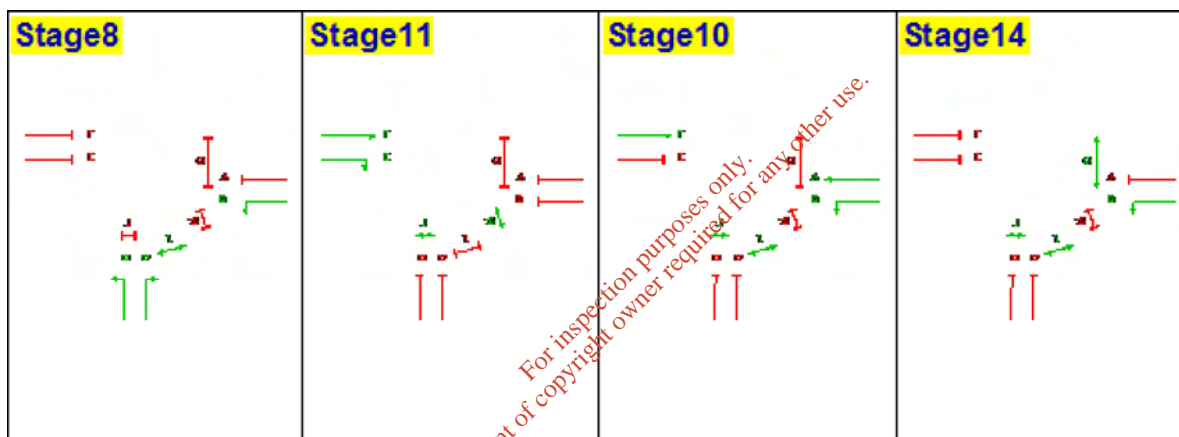
Phase Delays

Type	Phase	Terminating Stage	Starting Stage	Absolute Delay (s)	Relative Delay (s)
Losing	A	10	14	3.00	
Losing	G	14	8	2.00	

Phase Timings Diagram



Final Stage Sequence



For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	323	B	72.50	12.06	1.08	27.61	225.91	0.07	4.63	4.56	55.30
1	2	32	A	49.50	21.13	0.19	3.69	9999.00	0.00	0.63	0.63	4.30
2	1	2	D	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.10
2	2	29	C	8.50	57.62	0.46	21.88	311.30	0.04	0.94	0.90	0.90
3	1	367	F	89.50	5.31	0.54	25.04	259.40	0.06	3.42	3.36	80.70
3	2	165	E	39.50	31.16	1.43	27.39	228.57	0.07	3.89	3.82	15.10
Ped	1	1	J	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	I	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	H	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	G	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	265	2	72.50	11.49	0.85	22.66	297.25	0.05	3.74	3.69	7.80
1	2	26	1	49.50	21.05	0.15	2.99	9999.00	0.00	0.51	0.51	0.60
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	24	3	8.50	56.36	0.38	18.11	396.98	0.03	0.77	0.75	0.10
3	1	301	6	89.50	5.00	0.42	20.54	338.21	0.04	2.75	2.71	11.20
3	2	135	5	39.50	30.28	1.14	22.41	301.58	0.04	3.15	3.10	2.20
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	317	2	72.50	12.00	1.06	27.10	232.08	0.07	4.54	4.47	9.10
1	2	31	1	49.50	21.12	0.18	3.57	9999.00	0.00	0.61	0.61	0.70
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	29	3	8.50	57.58	0.46	21.88	311.30	0.04	0.94	0.90	0.10
3	1	359	6	89.50	5.27	0.53	24.50	267.41	0.06	3.33	3.28	13.20
3	2	161	5	39.50	31.04	1.39	26.73	236.73	0.07	3.79	3.72	2.50
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	388	2	72.50	12.76	1.38	33.17	171.31	0.12	5.67	5.55	10.70
1	2	38	1	49.50	21.22	0.22	4.38	9999.00	0.00	0.75	0.75	0.80
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	35	3	8.50	59.15	0.58	26.41	240.79	0.06	1.15	1.09	0.20
3	1	440	6	89.50	5.69	0.70	30.02	199.77	0.09	4.18	4.09	15.80
3	2	198	5	39.50	32.20	1.77	32.87	173.81	0.11	4.72	4.61	2.90
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	388	2	72.50	12.77	1.38	33.17	171.31	0.12	5.67	5.55	10.70
1	2	38	1	49.50	21.22	0.22	4.38	9999.00	0.00	0.75	0.75	0.80
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	35	3	8.50	59.21	0.58	26.41	240.79	0.06	1.15	1.09	0.20
3	1	440	6	89.50	5.69	0.70	30.02	199.77	0.09	4.18	4.09	15.80
3	2	198	5	39.50	32.21	1.77	32.87	173.81	0.11	4.72	4.61	2.90
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	317	2	72.50	12.00	1.06	27.10	232.08	0.07	4.54	4.47	9.10
1	2	31	1	49.50	21.12	0.18	3.57	9999.00	0.00	0.61	0.61	0.70
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	29	3	8.50	57.70	0.46	21.88	311.30	0.04	0.94	0.90	0.10
3	1	359	6	89.50	5.27	0.53	24.50	267.41	0.06	3.33	3.28	13.20
3	2	161	5	39.50	31.05	1.39	26.73	236.73	0.07	3.79	3.72	2.50
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	265	2	72.50	11.49	0.85	22.66	297.25	0.05	3.74	3.69	7.80
1	2	26	1	49.50	21.05	0.15	2.99	9999.00	0.00	0.51	0.51	0.60
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	24	3	8.50	56.48	0.38	18.11	396.98	0.03	0.77	0.75	0.10
3	1	301	6	89.50	5.00	0.42	20.54	338.21	0.04	2.75	2.71	11.20
3	2	135	5	39.50	30.29	1.14	22.41	301.58	0.04	3.15	3.10	2.20
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	71.50	9.80	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	39.50	27.00	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

OSCADY PRO

GUI Version: 1.3.1 [05/05/11]
Analysis Program Version: v1.3 23/03/2009

Copyright (c) 2006-09, by TRL and UCL, 2006-09. All rights reserved. All copyright, trade marks and other intellectual property rights subsisting in or used in connection with the Software (including but not limited to all images and other identifiable material relating to the Software) are and remain the sole property of the Licensor.

For sales and distribution information, program advice and maintenance, contact:

TRL Limited
Crowthorne House
Nine Mile Ride
Wokingham, Berks.
RG40 3GA, UK



Tel: +44 (0)1344 770758
Fax: +44 (0)1344 770864
E-mail: software@trl.co.uk
Web: www.trlsoftware.co.uk

The user of this computer program for the solution of an engineering problem is in no way relieved of their responsibility for the correctness of the solution

File: S:\Jobs\2016\16047 New Access at Huntstown Quarry, Co. Dublin\16047-02 North Quarry TIA\Reports\Appendices\T-Junction.osc

Report generation date: 04/08/2016 10:39:31

Summary

File Description

Title	(untitled)
Date	21/07/2016
Location	
Driving Side	Left
Identifier	
Client	
Jobnumber	
Enumerator	gfrisby [ROADPLAN-PC02]
Status	(new file)
Description	

Run Options

Run Evaluation Set	No
Evaluation Only	No
Optimise Critical Cycle TimeOnly	No
Use Horizontal Queues	Yes
Favour Continuous Green	No
Phase Timings Fuzziness (s)	0.5
Integer Phase Timings	Yes
Phase Snapping Distance (s)	0
Automatic Lane Turning Props	Yes
Automatic Vehicle Props	No

Geometry

Arms

Arm	Name	Exit Width (m)	Approach Speed (kph)	Exit Speed (kph)	Speed Limit (kph)	Stagger Distance (m)
1	R135 North	50.0	10	10	80	0
2	Elm Road	50.0	10	10	80	0
3	R135 South	50.0	10	10	80	0

Traffic Streams

Arm	Traffic Stream	Type	Name	Sat Flow (PCU/hr)	Estimate Sat Flow	Sat Flow 2 (PCU/hr)	Green Phase	Arrow Phase
1	1	Traffic		1936	Yes	0	B	-
1	2	Traffic		2105	Yes	0	A	-
2	1	Traffic		1747	Yes	0	D	-
2	2	Traffic		1871	Yes	0	C	-
3	1	Traffic		1965	Yes	0	F	-
3	2	Traffic		1830	Yes	0	E	-
(Ped)	1	Pedestrian		10000	Yes	0	J	-
(Ped)	2	Pedestrian		10000	Yes	0	I	-
(Ped)	3	Pedestrian		10000	Yes	0	H	-
(Ped)	4	Pedestrian		10000	Yes	0	G	-

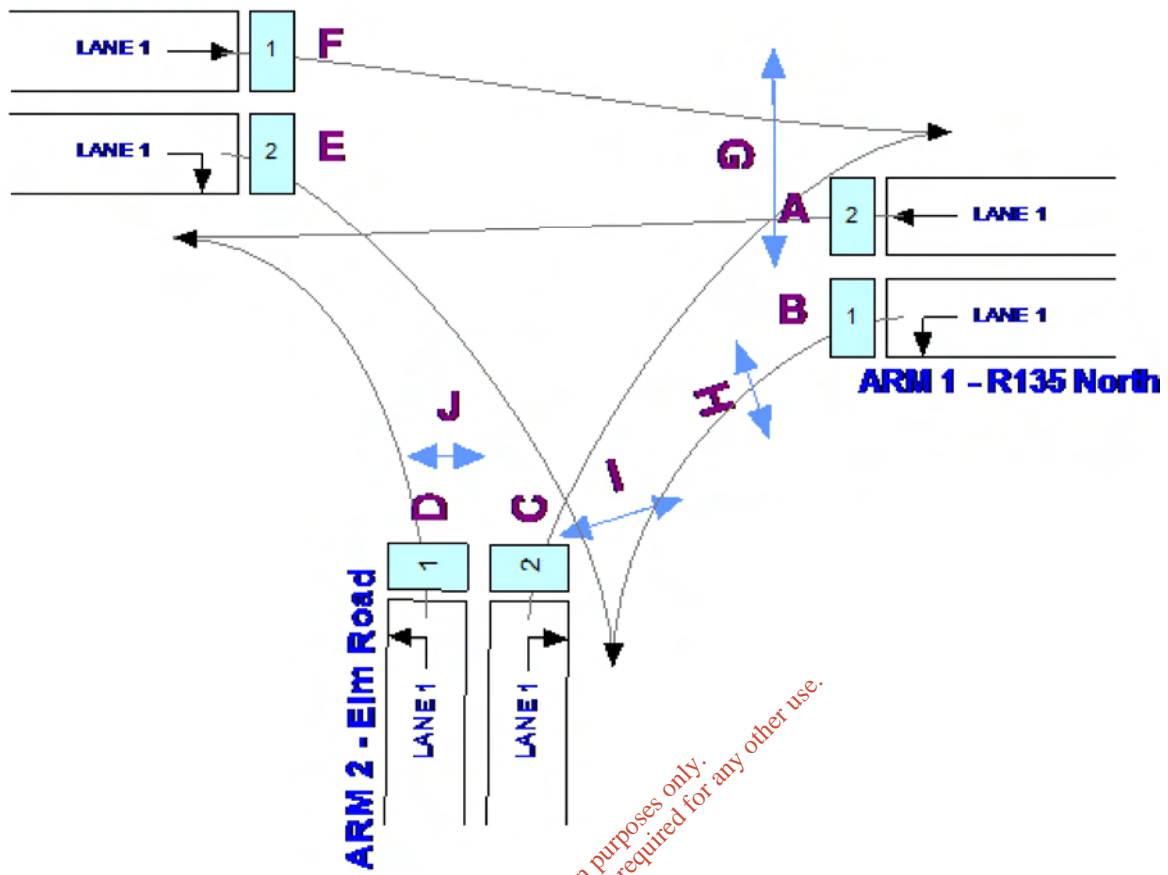
Arm	Traffic Stream	Relative Start Displacement (s)	Relative End Displacement (s)	Max Deg Sat (%)	Delay Weight (%)	Max Queue (PCU)	Initial Queue (PCU)	Average PCU Per Veh	Heavy Vehicles Percentage
1	1	0.0	0.0	90	100	0	0.0	1.10	0
1	2	0.0	0.0	90	100	0	0.0	1.10	0
2	1	0.0	0.0	90	100	0	0.0	1.10	0
2	2	0.0	0.0	90	100	0	0.0	1.10	0
3	1	0.0	0.0	90	100	0	0.0	1.10	0
3	2	0.0	0.0	90	100	0	0.0	1.10	0
(Ped)	1	0.0	0.0	90	100	0	0.0	-	0
(Ped)	2	0.0	0.0	90	100	0	0.0	-	0
(Ped)	3	0.0	0.0	90	100	0	0.0	-	0
(Ped)	4	0.0	0.0	90	100	0	0.0	-	0

Lanes

Arm	Traffic Stream	Lane	Name	Nearside Dest Arm	Straight Dest Arm	Offside Dest Arm	Proportion That Turn	Turning Radius (m)	IsNearside Lane	Width (m)	Gradient (%)	Short Lane Storage (PCU)
1	1	1		2			1.00	100	Yes	3.50	0.0	0
1	2	1			3		0.00	10	No	3.50	0.0	0
2	1	1		3			1.00	12	Yes	3.50	0.0	0
2	2	1				1	1.00	12	No	3.50	0.0	0
3	1	1				1	0.00	10	Yes	3.50	0.0	0
3	2	1				2	1.00	10	No	3.50	0.0	0

Junction Diagram

ARM 3 - R135 South



Signals

Signals

Max Cycle Time (s)	120
Fixed Cycle Time (s)	0
Evaluation Cycle Time (s)	0
Start Displacement (s)	1.4
End Displacement (s)	2.9

Phases

Phase	Name	Type	Associated Phase	Phase Min Green (s)	Phase Max Green (s)	Double Green
A	(Name)	Traffic	-	7.0	0.0	No
B	(Name)	Traffic	-	7.0	0.0	No
C	(Name)	Traffic	-	7.0	0.0	No
D	(Name)	Traffic	-	7.0	0.0	No
E	(Name)	Traffic	-	7.0	0.0	No
F	(Name)	Traffic	-	7.0	0.0	No
G	(Name)	Pedestrian	-	7.0	0.0	No
H	(Name)	Pedestrian	-	7.0	0.0	No
I	(Name)	Pedestrian	-	7.0	0.0	No
J	(Name)	Pedestrian	-	7.0	0.0	No

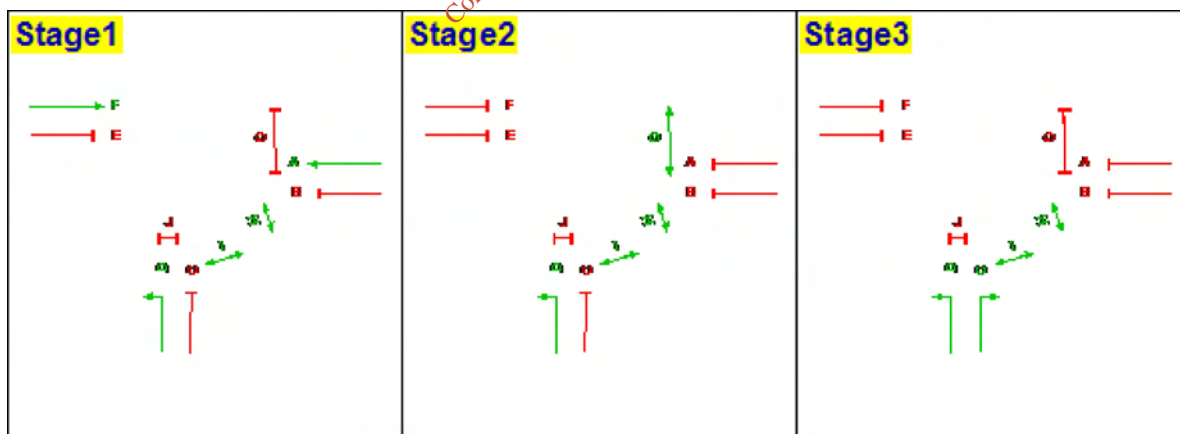
Intergreen Matrix

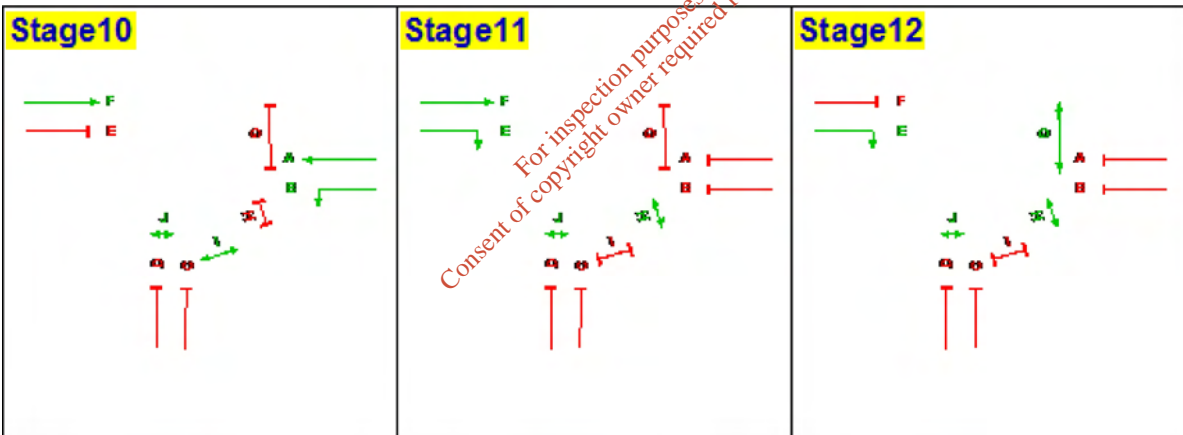
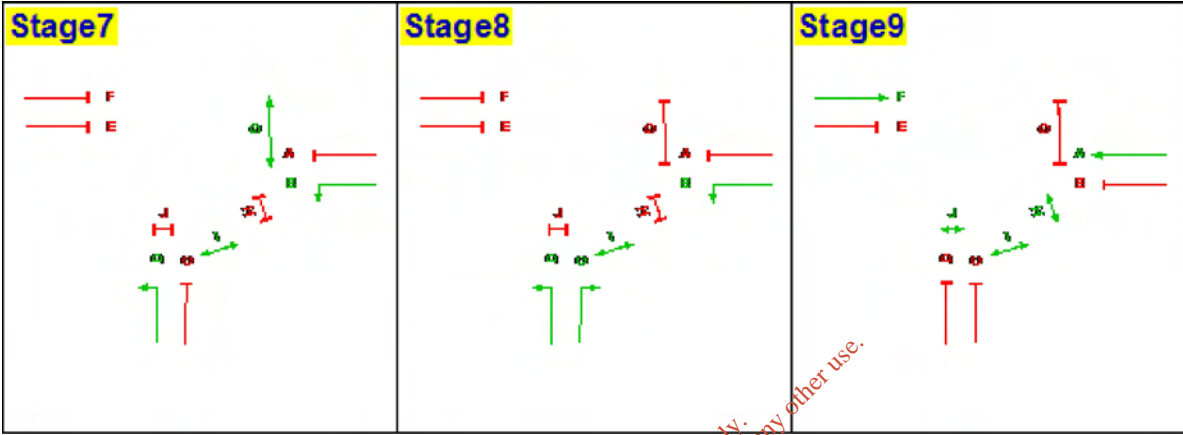
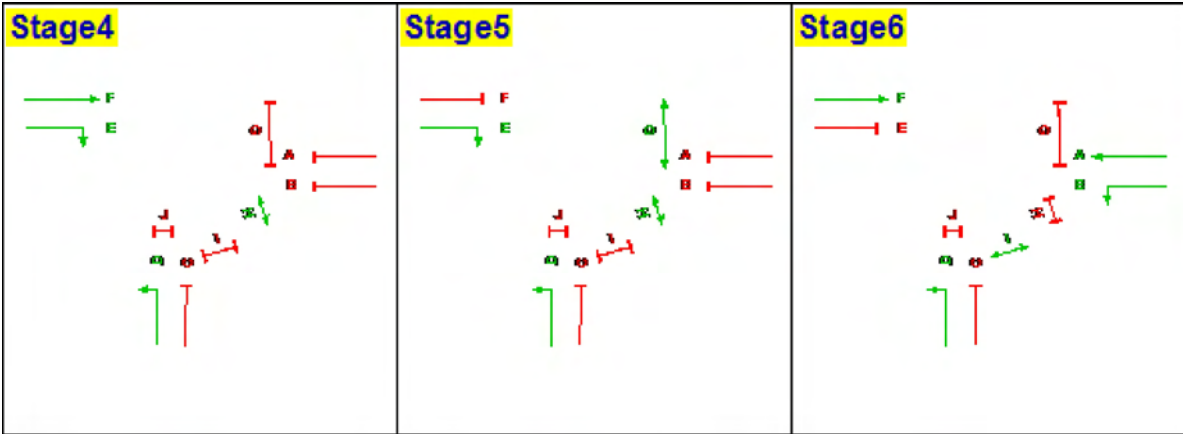
		To									
		A	B	C	D	E	F	G	H	I	J
From	A	-		5		5		5			
	B		-			5			5		
	C	5		-		5	5	8			5
	D				-						5
	E	5	6	5		-				7	
	F			5			-	8			
	G	5		5			5	-			
	H		6						-		
	I					5				-	
	J			5	7						-

Stages

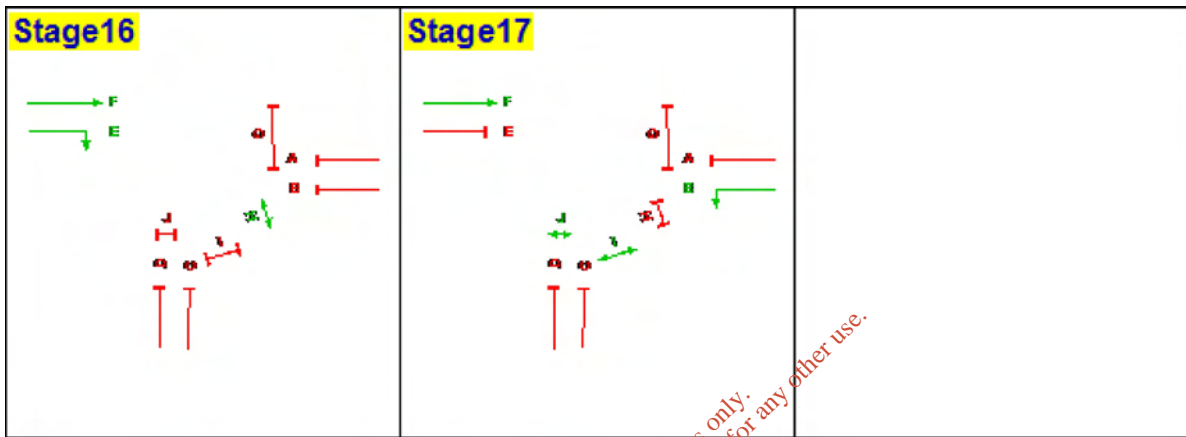
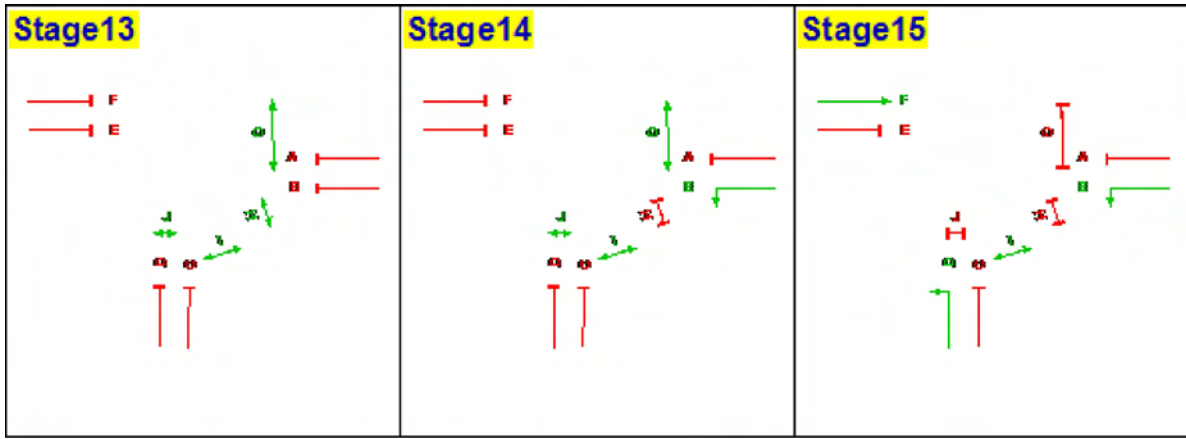
Stage	Stage Min Green (s)	Phases In This Stage	Use To Generate Sequences
1	-1	A,D,F,H,I	Yes
2	-1	D,G,H,I	Yes
3	-1	C,D,H,I	Yes
4	-1	D,E,F,H	Yes
5	-1	D,E,G,H	Yes
6	-1	A,B,D,F,I	Yes
7	-1	B,D,G,I	Yes
8	-1	B,C,D,I	Yes
9	-1	A,F,H,I,J	Yes
10	-1	A,B,F,I,J	Yes
11	-1	E,F,H,J	Yes
12	-1	E,G,H,J	Yes
13	-1	G,H,I,J	Yes
14	-1	B,G,I,J	Yes
15	-1	B,D,F,I	Yes
16	-1	E,F,H	Yes
17	-1	B,F,I,J	Yes

Stage Diagrams





For inspection purposes only.
 Consent of copyright owner required for any other use.



Sequences

Sequence	Name	Stages In This Sequence
1		1,2,13,12,13,14,7,8,6
2		1,6,8,7,14,13,12,13,2
3		1,2,13,14,13,2,3,4
4		1,4,3,2,13,14,13,2
5		1,2,13,12,13,14,13,2,3
6		1,3,2,13,14,13,12,13,2
7		1,2,13,14,13,12,13,2,3
8		1,3,2,13,12,13,14,13,2

Constraints

(No constraints)

For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic

Note: Traffic flows are only shown for selected demand sets. Resultant flows are the sums of the selected demand sets adjusted by the global traffic scaling factor, and are shown as the arrival rates in the final results tables.

Configuration

Traffic Scaling Factor	1.00
Time Period (min)	90
Time Segment Length (min)	15
Signal Optimiser Flows	Average
PCUs per Heavy Vehicle	2.00

Demand Sets

Name	Selected	Time Start	Time End	Profile Type	Use Relationship	Relationship
2016 AM Peak Existing	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2017 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak No Dev	No	07:15	08:45	ODTAB	No	D1
2023 AM Peak With Dev	No	07:15	08:45	ODTAB	No	D1
2016 PM Peak Existing	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2017 PM Peak With Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak No Dev	No	16:30	18:00	ODTAB	No	D1
2023 PM Peak With Dev	Yes	16:30	18:00	ODTAB	No	D1

Demand Set10 - 2023 PM Peak With Dev

ODTAB Data (PCU/hr during central 60 min peak period)

		To		
		Arm 1	Arm 2	Arm 3
From	Arm 1	-	354	51
	Arm 2	32	-	2
	Arm 3	419	194	-

Average pedestrian flow on each pedestrian stream (if applicable): 1 ped/hr

Traffic flows (PCU/hr)

Arm	Traffic Stream	Phase	16:30-16:45	16:45-17:00	17:00-17:15	17:15-17:30	17:30-17:45	17:45-18:00
1 - R135 North	1	B	264	316	386	386	316	264
1 - R135 North	2	A	39	47	58	58	47	39
2 - Elm Road	1	D	2	2	2	2	2	2
2 - Elm Road	2	C	24	29	35	35	29	24
3 - R135 South	1	F	313	373	457	457	373	313
3 - R135 South	2	E	147	176	215	215	176	147
Pedestrians	1	J	1	1	1	1	1	1
Pedestrians	2	I	1	1	1	1	1	1
Pedestrians	3	H	1	1	1	1	1	1
Pedestrians	4	G	1	1	1	1	1	1

Turning Proportions

Arm	Left Movement Percentage	Straight Movement Percentage	Right Movement Percentage
1 - R135 North	87	13	-
2 - Elm Road	6	-	94
3 - R135 South	-	68	32

For inspection purposes only.
Consent of copyright owner required for any other use.

Results

Note: Duplicate solutions are not shown.

For inspection purposes only.
Consent of copyright owner required for any other use.

Sequence3; Objective: MAXIMUM CAPACITY

Note: Individual time segment results are included for this sequence/objective. Results for the 'Signal Optimiser Run' tables are based on the signal optimiser traffic flows, rather than individual time segment flows.

Summary (Signal Optimiser Run)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
120.0	217.91	4.05	4.05	160.9

- Cycle Time is the minimum cycle time that meets all safety criteria whilst optimising this objective.
- PRC is the lowest value encountered over all streams.
- Rate of delay is the sum of each stream's rate of delay.

Summary (Time Segments)

Cycle Time (s)	Practical Reserve Capacity (%)	Rate of Delay (PCU)	Weighted Rate of Delay (PCU)	Geometric Delay (PCU-min)
-	164.92	4.07	4.07	159.80

- PRC is the lowest value encountered over all streams and time segments.
- Rate of delay is the sum of each stream's rate of delay, averaged over time segments.

Stage Timings

Stage	Start Time (s)	Duration (s)	End Time (s)
8	7.0	7.0	14.0
11	19.0	40.0	59.0
10	66.0	41.0	107.0
14	115.0	5.0	0.0

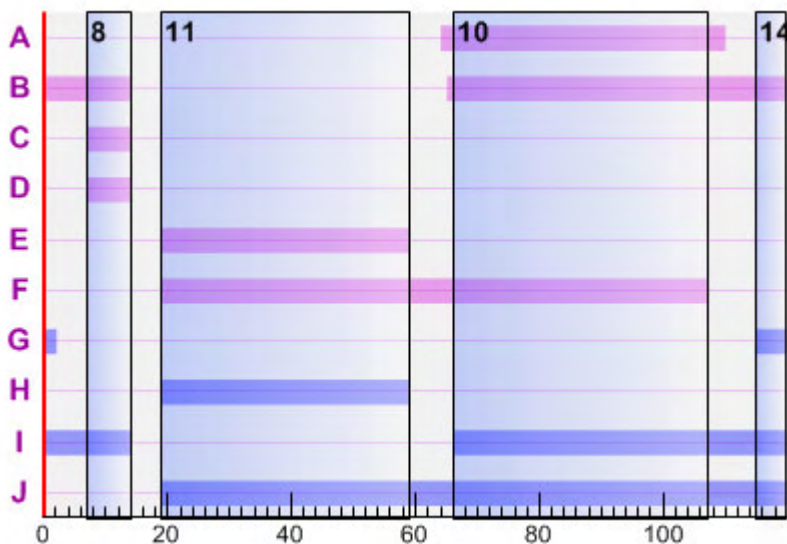
Phase Timings

Phase	Start Time (s)	Duration (s)	End Time (s)	Filter Arrow Time (s)	Indicative Arrow Start (s)	Indicative Arrow Duration (s)	Start Time (s) (2nd green)	Duration (s) (2nd green)	End Time (s) (2nd green)
A	64	46.0	110						
B	65	69.0	14						
C	7	7.0	14						
D	7	7.0	14						
E	19	40.0	59						
F	19	88.0	107						
G	115	7.0	2						
H	19	40.0	59						
I	66	68.0	14						
J	19	101.0	0						

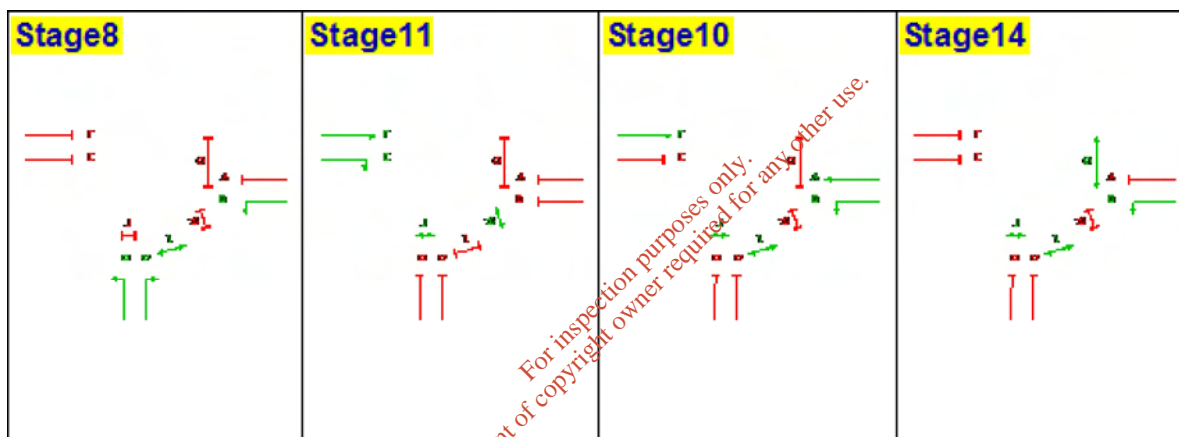
Phase Delays

Type	Phase	Terminating Stage	Starting Stage	Absolute Delay (s)	Relative Delay (s)
Losing	A	10	14	3.00	
Losing	G	14	8	2.00	

Phase Timings Diagram



Final Stage Sequence



For inspection purposes only.
Consent of copyright owner required for any other use.

Traffic Stream Details (Signal Optimiser Run)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	322	B	70.50	13.07	1.17	28.31	217.91	0.08	4.81	4.73	53.20
1	2	48	A	47.50	22.59	0.30	5.76	9999.00	0.00	0.98	0.98	6.10
2	1	2	D	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.10
2	2	29	C	8.50	57.62	0.46	21.88	311.30	0.04	0.94	0.90	0.90
3	1	381	F	89.50	5.38	0.57	26.00	246.20	0.07	3.56	3.49	83.50
3	2	179	E	41.50	29.94	1.49	28.28	218.21	0.08	4.12	4.05	17.10
Ped	1	1	J	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	I	69.50	10.63	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	H	41.50	25.68	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	G	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (16:30-16:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	264	2	70.50	12.45	0.91	23.21	287.75	0.05	3.88	3.83	7.50
1	2	39	1	47.50	22.45	0.24	4.68	9999.00	0.00	0.79	0.79	0.80
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	24	3	8.50	56.36	0.38	18.11	396.98	0.03	0.77	0.75	0.10
3	1	313	6	89.50	5.05	0.44	21.36	321.41	0.04	2.87	2.83	11.60
3	2	147	5	41.50	29.05	1.19	23.23	287.47	0.05	3.35	3.30	2.40
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	69.50	10.63	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	41.50	25.68	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (16:45-17:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	316	2	70.50	13.00	1.14	27.78	223.94	0.08	4.71	4.64	8.70
1	2	47	1	47.50	22.57	0.29	5.64	9999.00	0.00	0.96	0.96	1.00
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	29	3	8.50	57.58	0.46	21.88	311.30	0.04	0.94	0.90	0.10
3	1	373	6	89.50	5.34	0.55	25.45	253.62	0.06	3.48	3.41	13.70
3	2	176	5	41.50	29.85	1.46	27.81	223.63	0.07	4.05	3.98	2.80
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	69.50	10.63	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	41.50	25.68	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:00-17:15)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	386	2	70.50	13.82	1.48	33.94	165.20	0.12	5.88	5.75	10.20
1	2	58	1	47.50	22.73	0.37	6.96	9999.00	0.00	1.19	1.18	1.20
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	35	3	8.50	59.15	0.58	26.41	240.79	0.06	1.15	1.09	0.20
3	1	457	6	89.50	5.78	0.73	31.18	188.62	0.10	4.36	4.26	16.30
3	2	215	5	41.50	31.02	1.85	33.97	164.92	0.12	5.02	4.90	3.30
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	69.50	10.63	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	41.50	25.68	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:15-17:30)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	386	2	70.50	13.82	1.48	33.94	165.20	0.12	5.88	5.75	10.20
1	2	58	1	47.50	22.73	0.37	6.96	9999.00	0.00	1.19	1.18	1.20
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	35	3	8.50	59.21	0.58	26.41	240.79	0.06	1.15	1.09	0.20
3	1	457	6	89.50	5.78	0.73	31.18	188.62	0.10	4.36	4.26	16.30
3	2	215	5	41.50	31.02	1.85	33.97	164.92	0.12	5.02	4.90	3.30
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	69.50	10.63	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	41.50	25.68	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:30-17:45)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	316	2	70.50	13.01	1.14	27.78	223.94	0.08	4.71	4.64	8.70
1	2	47	1	47.50	22.57	0.29	5.64	9999.00	0.00	0.96	0.96	1.00
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	29	3	8.50	57.70	0.46	21.88	311.30	0.04	0.94	0.90	0.10
3	1	373	6	89.50	5.34	0.55	25.45	253.62	0.06	3.48	3.41	13.70
3	2	176	5	41.50	29.86	1.46	27.81	223.63	0.07	4.05	3.98	2.80
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	69.50	10.63	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	41.50	25.68	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00

Traffic Stream Details (17:45-18:00)

Arm	Traffic Stream	Arrival Rate (PCU/hr)	Controlling Phase	Effective Green (s)	Average Delay (s)	Rate of Delay (PCU)	Degree of Saturation (%)	Practical Reserve Capacity (%)	Queue at End of Green (PCU)	Queue at End of Red (PCU)	Uniform Queue (PCU)	Geometric Delay (PCU-min)
1	1	264	2	70.50	12.45	0.91	23.21	287.75	0.05	3.88	3.83	7.50
1	2	39	1	47.50	22.45	0.24	4.68	9999.00	0.00	0.79	0.79	0.80
2	1	2	4	8.50	52.18	0.03	1.62	9999.00	0.00	0.06	0.06	0.00
2	2	24	3	8.50	56.48	0.38	18.11	396.98	0.03	0.77	0.75	0.10
3	1	313	6	89.50	5.05	0.44	21.36	321.41	0.04	2.87	2.83	11.60
3	2	147	5	41.50	29.06	1.19	23.23	287.47	0.05	3.35	3.30	2.40
Ped	1	1	10	102.50	1.28	0.00	0.01	9999.00	0.00	0.00	0.00	0.00
Ped	2	1	9	69.50	10.63	0.00	0.02	9999.00	0.00	0.01	0.01	0.00
Ped	3	1	8	41.50	25.68	0.01	0.03	9999.00	0.00	0.02	0.02	0.00
Ped	4	1	7	8.50	51.81	0.01	0.14	9999.00	0.00	0.03	0.03	0.00