

Mr Mick Lorigan  
Director of Transportation & Water Services  
Fingal County Council  
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Fingal County Hall  
Swords  
County Dublin

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4<sup>th</sup> September 2013

Reg No: W0009-03

Dear Mr Lorigan

I am to advise you that the Agency has received an application for a Technical Amendment to a Waste Licence from Fingal County Council, for a facility located at Balleally Landfill, Balleally, Lusk, Co. Dublin.

The Licensee proposes, as part of this application, to provide for the discharge of process effluent to a sewer, which the Licensee has stated is vested in, or controlled by, your Council. Process effluent includes trade effluent or other matter (other than domestic sewage or storm water). I enclose a copy of the Technical Amendment request, which detail proposed discharges.

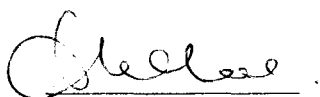
The provisions of Section 52 of the Waste Management Acts, 1996 to 2013, provides that the Agency shall obtain the consent of the sanitary authority to the proposed discharge from an activity which involves the discharge of trade effluent or other matter (other than domestic sewage or storm water), to a sewer vested in or controlled by a sanitary authority.

In order to expedite the Agency's consideration of this Technical Amendment, I am to request your authority's consent to the proposed discharge/s. It should be noted that, your authority's consent may be subject to such conditions as your authority considers appropriate as provided for in Section 52 of the Waste Management Acts, 1996 to 2013 and Section 99E(3) of the Environmental Protection Agency Acts, 1992 and 2013. Your attention is drawn to paragraphs (3) and (4) of the attached copy of the relevant section of the Act. For your convenience please find attached a reply form including a list of draft conditions compiled by the Agency.

In accordance with paragraph (2) of this section of the Act, you are requested to forward your response within 5 weeks of the date of this letter. Please note that any decision given after the expiry period shall be invalid and in those circumstances the Agency may proceed to determine the application concerned as if consent was obtained. Brian Meaney is dealing with this matter and can be contacted at the Licensing Unit, Office of Climate, Licensing & Resource Use (Tel. No. 0539160600), if you have any queries.

Your co-operation in this matter is appreciated.

Yours sincerely,



Ann Kehoe

**Programme Officer**

**Licensing Unit**

**Office of Climate, Licensing & Resource Use**

**Section 99E (3) & (4) of the Environmental Protection Agency Acts, 1992 and 2007**

- (3) Subject to subsection (4), a consent under subsection (1) may be granted subject to or without conditions and if it is granted subject to conditions the Agency shall include in the licence or revised licence concerned conditions corresponding to them or, as the Agency may think appropriate, conditions more strict than them.
- (4) The conditions that may be attached to a consent by a sanitary authority under this section are the following and no other conditions, namely conditions-
- (a) relating to-
    - (i) the nature, composition, temperature, volume, level, rate, and location of the discharge concerned and the period during which the discharge may, or may not, be made,
    - (ii) the provision, operation, maintenance and supervision of meters, gauges, manholes, inspection chambers and other apparatus and other means for monitoring the nature, extent and effect of emissions,
    - (iii) the taking and analysis of samples, the keeping of records and furnishing of information to the sanitary authority,
  - (b) providing for the payment by the licensee to the sanitary authority concerned of such amount or amounts as may be determined by the sanitary authority having regard to the expenditure incurred or to be incurred by it in monitoring, treating and disposing of discharges of trade effluent, sewage effluent and other matter to sewers in its functional area or a specified part of its functional area,
  - (c) specifying a date not later than which any conditions attached under this section shall be complied with,
  - (d) relating to, providing for or specifying such other matter as may be prescribed.

**SANITARY AUTHORITY RESPONSE**  
**re: SECTION 52 OF THE WASTE MANAGEMENT ACTS, 1996 to 2011**

**Name & Address of Sanitary Authority:** Fingal County Council, PO Box 174, Fingal County Hall, Swords, County Dublin.

**Waste Reg. No.** W0009-03

**Waste Facility:** Balleally Landfill, Balleally, Lusk, Co. Dublin,

**Waste Licence Applicant:** Fingal County Council

**Consent:** Indicate Yes to one of the following statements:

Consent granted subject to the consent conditions outlined below	
Consent granted without conditions	
Consent refused <sup>Note 1</sup>	

**Note 1** Where it is proposed to refuse permission the reasons for the refusal should be clearly outlined in the response.

GENERAL CONSENT CONDITIONS	Condition to be included (Yes/No)
1. No specified emission from the installation shall exceed the emission limit value set out in <i>Schedule B: Emissions Limits to Sewer</i> . There shall be no other emission to sewer of environmental significance.	
2. The licensee shall carry out such sampling, analyses, measurements, examinations, maintenance and calibrations as out in <i>Schedule C</i> .	
3. Monitoring and analytical equipment shall be operated and maintained as necessary so that monitoring accurately reflects the discharge or emission.	
4. The licensee shall permit authorised persons of the Agency and the Sanitary Authority to inspect, examine and test, at all reasonable times, any works and apparatus installed, in connection with the process effluent, and to take samples of the process effluent.	
5. All automatic monitors and samplers shall be functioning at all times (except during maintenance and calibration) when the activity is being carried on unless alternative sampling or monitoring has been agreed in writing by the Agency for a limited period. In the event of the malfunction of any continuous monitor, the licensee shall contact the Agency as soon as practicable, and alternative sampling and monitoring facilities shall be put in place. Prior written agreement for the use of alternative equipment, other than in emergency situations, shall be obtained from the Agency.	
6. The licensee shall record all sampling, analyses, measurements, examinations, calibrations and maintenance carried out in accordance with the requirements of this licence.	
7. The licensee shall provide safe and permanent access to all on-site sampling and monitoring points and to off-site points as required by the Agency.	
8. The licensee shall at no time discharge or permit to be discharged into the sewer any liquid matter or thing which is or may be liable to set or congeal at average sewer temperature or is capable of giving off any inflammable or explosive gas or any acid, alkali or other substance in sufficient concentration to cause corrosion to sewer pipes, penstock and sewer fittings or the general integrity of the sewer.	
9. In the event of any incident which relates to discharges to sewer, having taken place, the licensee shall notify the Agency, Local Authority and Sanitary Authority as soon as practicable after the incident.	





## Frequency of Monitoring Process Effluent to Sewer

### Schedule C

Waste Licence application Register No. **W0009-03**

Emission Point Reference No: \_\_\_\_\_

Parameter <i>(delete parameters which are not applicable)</i>	Monitoring Frequency <i>(e.g. monthly, quarterly, annually)</i>	Sampling Type <i>(grab, composite)</i>
Flow to sewer		
Temperature		
pH		
BOD		
COD		
Suspended Solids		
<b>ADDITIONAL PARAMETERS</b> <i>(if required)</i>		

SANITARY AUTHORITY CHARGES	
Charge per cubic metre of process effluent (per s52 of the Waste Management Acts, 1996 to 2011)	
Payment Frequency	
Annual Monitoring Costs	

Signed on behalf of Fingal County Council

\_\_\_\_\_ Date \_\_\_\_\_





Office of licensing and Guidance  
Environmental Protection Agency  
P Box 3000  
Johnstown Castle Estate  
Wexford

FAO Brian B Meaney

**27 June 2013**

**Re: Balleally Landfill Ref W009-03**

Dear Sirs

Fingal County Council hereby apply for a amendment to our waste licence for Balleally landfill under section 42B of the Waste Management Act 1996-2012

We are seeking a change to condition 5.12 & 6.12 and the associated schedule C.6 of the waste licence.

The original licence provided for the discharge of treated leachate to Rogerstown Estuary. To this purpose a treatment plant was constructed, however it was unable to meet the discharge limits in the licence, particularly in regard to COD and as a result partially treated leachate and untreated leachate has been tankered to Ringsend Wastewater Treatment Works.

In 2010 and 2011 hydrodynamic and water quality modeling was carried out in the estuary to see if a higher discharge standard which could be met by the existing plant and allow the estuary to meet the statutory water quality objectives for the estuary. The results of this modeling were reasonably positive, however in discussions with the National Parks and Wildlife service and the Agency it was felt that the imminent construction of the new Portrane wastewater treatment plant serving among other town Lusk would provide the opportunity to discharge the leachate into the sewer and with treatment in the plant at Portrane discharge to the Irish Sea a less sensitive water body than Rogerstown Estuary. We therefore did not proceed with the licence change at that time

**Environment Department**

Waste Management Division

P.O. Box 174,

County Hall,

Swords,

Fingal,

Co. Dublin

**An Roinn Seirbhísl Comshaoil**

Bosca 174,

Áras an Chontae,

Sord,

Fine Gall,

Contae Átha Cliath

The new wastewater treatment plant at Portrane was constructed in 2012 and has now been operating for a year. There is now the opportunity to connect to the Portrane plant. Tenders have been sought to construct a new pipeline to the Lusk Sewer Network which is 750m from the landfill site

This application for a technical amendment is to discharge untreated leachate to the sewer network as the wastewater treatment plant has capacity to treat the leachate at much lower cost than the Council running a separate ( and more difficult to operate) treatment plant at Balleally

There has been substantial testing of the leachate as part of its acceptance at Ringsend WWTP and a spreadsheet of the Ringsend results is attached. Separate monitoring has been carried out on site however the Ringsend testing is more extensive. However we have attached our testing of list I & II substances carried out in

The main consideration is the effect of the significant Ammonia load on the Portrane Plant. The proposed ammonia load from Balleally would amount to less 7% of the design load and 21 % of the current load.

Discussions have been had with the DBO contractor for the Portrane Plant regarding the ability of the plant to accept Leachate from Balleally. The contractor has confirmed that the plant currently has the capacity to accept the leachate. There main concern is that the flow of leachate would be kept relatively constant and the substantial storage provided by the tanks at Balleally will allow for this.

In the longer term it is possible that the Portrane plant will reach capacity due to development within its catchment area. However with the current outlook for development this is likely to be over 10 years away. In addition there will be no load coming from the cancelled Nevitt Landfill which was previously proposed and was permitted in the licence granted for that project (this was for partially treated leachate) in addition the strength and quantity of the leachate at Balleally landfill will reduce over time. However should the leachate from Balleally Landfill adversely affect the capacity of the Portrane plant to accept other permitted development over time it is open to the Water Services Authority to seek a review of the waste licence and to reallocate capacity at the Portrane plant. In that case Fingal County Council would be required to provide pretreatment.

The Council would be open to trialing the discharge of leachate to the sewer in advance of a final decision by the Agency to determine this application

The following tables detail the loadings to the Portrane plant and the discharge limits sought for the Balleally Leachate Treatment Plant

Table 1 Design and current load figures for the Portrane Plant

	Design Load	Current Load
Flow m <sup>3</sup> /d	14,625	4,510
P.E.	65,000	18,840
BOD kg/d	3,900	1,146
COD kg/d	8,965	2,765
SS kg/d	4,357	1,353
Ammonia kg/d	978	288

Table 2 Discharge limits sought by Fingal County Council for the discharge from Balleally

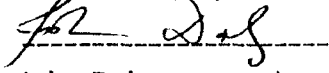
	Discharge Limits Sought based on 90% ile concentrations and 150m <sup>3</sup> /d flow			For Information Average Loadings (ie average flow by average concentration)		
	concentration	loading	as a % of current flow/load	concentration	loading	as a % of current flow/load
	mg/l or m <sup>3</sup> /d	kg/d		mg/l or m <sup>3</sup> /d	kg/d	
Daily flow m <sup>3</sup> /d	150		3%	130		2.9%
Ammonia mg/l	800	120	42%	485	63.1	21.9%
BOD mg/l	150	22.5	2%	68	8.8	0.8%
COD mg/l	1100	165	6%	740	96.2	3.5%

The following are appended to this letter

- Table 3 Appended to this application gives results of the leachate from Balleally accepted at Ringsend from August 2011 to May 2013
- Table 4,5 & 6 List I and II substances analysis of the leachate from samples taken at Balleally on Sept 2009
- Table 7 give flow, BOD, COD and SS data for the Portrane WWTP from

- Also enclosed are details of the design of the Portrane plant from AECOM the DBO plant operator and a AECOM design report DR2259/3 for the Portrane plant.
- Map showing proposed discharge location to the sewer on Rogerstown Lane

Yours sincerely



John Daly

Senior Engineer

Waste Management Division

Tel 087 283 9186



Table 6



**Additional Monitoring Results - Leachate cells and treated leachate plant**

Parameter	Unit	MAC	Cell 1 07/09/06	Cell 2 07/09/06	Cell 3 07/09/06	P1 a (Southern Boundary)	P1 b (Northern Boundary)	LTP
pH	pH Units	5.5-8.5	7.38	7.62	7.82	7.74	7.54	6.22
Conductivity	uS/cm	1000	11000	14000	18000	11000	11000	8000
Temperature	°C	-	-	-	-	-	-	-
Total Ammonia (as N)	mg/l	0.2	670.4	668.3	638.1	987.6	1002.6	47.3
Total Oxidised Nitrogen (as N)	mg/l	-	<0.3	<0.3	<0.3	<0.3	0.3	721.4
BOD	mg/l	5	1030	269	520	634	709	805
COD	mg/l	-	2000	1314	2128	1738	1178	1419
Total Suspended Solids	mg/l	50	155	30	108	41	101	65
Total Phosphorous	mg/l	No Abnormal Change	2.74	3.14	6.52	1.25	1.06	9.03
Sodium#	mg/l	150	950	1300	1700	1200	1150	1050
Chloride#	mg/l	30	959	1328	1760	1350	1341	1279

Shading = Value has exceeded MAC

European Communities Regulations SI 439 of 2000

Analysis conducted by Alcontrol Geochem Laboratories

# indicator parameters

NDP = No Determination Possible

Table 4

Additional Monitoring - Pesticides									
Sample Identity	Method Detection Limit	IGV	Cell 1	Cell 2	Cell 3	P1SB	P1NB	LTP	
Dichlorvos**	ug/l	<0.01ug/l	0.001ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Mevinphos**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
alpha-BHC**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
beta-BHC**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
gamma-BHC**	ug/l	<0.01ug/l	0.1ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Diazinon**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Methyl parathion**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Heptachlor**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Fenitrothion**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Malathion**	ug/l	<0.01ug/l	0.01ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Aldrin**	ug/l	<0.01ug/l	0.0ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Parathion**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Heptachlor epoxide**	ug/l	<0.01ug/l	0.001ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Endosulfan 1**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
p,p'-DDE**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Dieldrin**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Endrin**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Endosulfan 2**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
p,p'-DDD**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Ethion**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
p,p'-DDT**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Endosulfan sulphate**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
p,p'-Methoxychlor**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01
Azinphos methyl**	ug/l	<0.01ug/l	0.5ug/l	<0.02	<0.02	<0.05	<0.1	<0.2	<0.01

Table 5



**Additional Monitoring - Volatile Organic Compounds**

Trace Organics (VOCs)	Unit	Cell 1	Cell 2	Cell 3	P1 - SB	P1 - NB	LTP	EPA IGVs
		07/09/06	07/09/06	07/09/06	07/09/06	07/09/06	09/09/06	
Dichlorofluoromethane	µg/l	<1	<1	<1	<1	<1	<1	-
Chloromethane	µg/l	<1	<1	<1	<1	<1	<1	-
Vinyl chloride	µg/l	<1	<1	<1	<1	<1	<1	-
Bromomethane	µg/l	<1	<1	<1	<1	<1	<1	-
Chloroethane	µg/l	<1	<1	<1	<1	<1	<1	-
Trichlorofluoromethane	µg/l	<1	<1	<1	<1	<1	<1	-
1,1-Dichloroethene	µg/l	<1	<1	<1	<1	<1	<1	-
Dichloromethane	µg/l	<1	<1	<1	<1	<1	<1	10
trans-1,2-Dichloroethene	µg/l	<1	<1	<1	<1	<1	<1	-
1,1-Dichloroethane	µg/l	<1	<1	<1	<1	<1	<1	-
2,2-Dichloropropane	µg/l	<1	<1	<1	<1	<1	<1	-
tert-butyl methyl ether	µg/l	67	102	45	<1	<1	<1	-
cis-1,2-Dichloroethane	µg/l	<1	<1	<1	<1	<1	<1	-
Bromochloromethane	µg/l	<1	<1	<1	<1	<1	<1	-
Chloroform	µg/l	<1	<1	<1	<1	<1	<1	12
1,1,1-Trichloroethane	µg/l	<1	<1	<1	<1	<1	<1	500
1,1-Dichloropropene	µg/l	<1	<1	<1	<1	<1	<1	-
Carbontetrachloride	µg/l	<1	<1	<1	<1	<1	<1	-
1,2-Dichloroethane	µg/l	<1	<1	<1	<1	<1	<1	3
Benzene	µg/l	8	6	7	6	4	<1	10
Dibromomethane	µg/l	<1	<1	<1	<1	<1	<1	-
1,2-Dichloropropane	µg/l	<1	<1	<1	<1	<1	<1	-
Bromodichloromethane	µg/l	<1	<1	<1	<1	<1	<1	-
Trichloroethane	µg/l	<1	<1	<1	<1	<1	<1	70
cis-1,3-Dichloropropene	µg/l	<1	<1	<1	<1	<1	<1	-
trans-1,3-Dichloropropene	µg/l	<1	<1	<1	<1	<1	<1	-
1,1,2-Trichloroethane	µg/l	<1	<1	<1	<1	<1	<1	-
Toluene	µg/l	<b>20</b>	<b>69</b>	<b>34</b>	<1	<1	<1	10
Dibromochloromethane	µg/l	<1	<1	<1	<1	<1	<1	-
1,2-Dibromoethane	µg/l	<1	<1	<1	<1	<1	<1	-
Tetrachloroethane	µg/l	<1	<1	<1	<1	<1	<1	40
Chlorobenzene	µg/l	<1	<1	<1	6	<1	<1	1
Ethylbenzene	µg/l	<b>24</b>	<b>13</b>	<b>14</b>	6	<1	<1	10
Xylenes (Total O, M, and P)	µg/l	<b>33</b>	<b>24</b>	<b>20</b>	<b>5</b>	<1	<1	10
Bromoform	µg/l	<1	<1	<1	<1	<1	<1	-
Styrene	µg/l	<1	<1	<1	<1	<1	<1	-
1,1,2,2-Tetrachloroethane	µg/l	<1	<1	<1	<1	<1	<1	-
o-Xylene	µg/l	18	12	10	<1	4	<1	-
1,3-Dichloropropane	µg/l	<1	<1	<1	<1	<1	<1	-
Isopropylbenzene	µg/l	<1	<1	<1	<1	<1	<1	-
Bromobenzene	µg/l	<1	<1	<1	<1	<1	<1	-
2-Chlorotoluene	µg/l	<1	<1	<1	<1	<1	<1	-
n-Propylbenzene	µg/l	<1	<1	<1	<1	<1	<1	-
4-Chlorotoluene	µg/l	<1	<1	<1	<1	<1	<1	-
1,3,5-Trimethylbenzene	µg/l	13	8	9	2	5	<1	-
4-Isopropyltoluene	µg/l	<1	<1	<1	<1	<1	<1	-
1,2,4-Trimethylbenzene	µg/l	3	2	2	4	4	<1	-
1,2-Dichlorobenzene	µg/l	<1	<1	<1	<1	<1	<1	10
1,3-Dichlorobenzene	µg/l	<1	<1	<1	<1	<1	<1	-
sec-Butylbenzene	µg/l	<1	<1	<1	<1	<1	<1	-
tert-Butylbenzene	µg/l	8	7	28	<1	<1	<1	-
1,4-Dichlorobenzene	µg/l	<1	<1	<1	<1	<1	<1	-
n-Butylbenzene	µg/l	<1	<1	<1	<1	<1	<1	-
1,2,3-Trichloropropane	µg/l	<1	<1	<1	<1	<1	<1	-
1,2-Dibromo-3-Chloropropane	µg/l	<1	<1	<1	<1	<1	<1	-
1,2,4-Trichlorobenzene	µg/l	<1	<1	<1	<1	<1	<1	0.4
Hexachlorobutadiene	µg/l	<1	<1	<1	<1	<1	<1	0.1
Naphthalene	µg/l	<1	<b>5</b>	<b>8</b>	<b>10</b>	<b>8</b>	<1	1
1,2,3-Trichlorobenzene	µg/l	<1	<1	<1	<1	<1	<1	-

LEGEND  
 EPA IGVs = Environmental Protection Agency (EPA) Guidelines ( 2003)  
 Results are shaded in bold where they exceed the EPA Guideline Value

White Young Green Ireland  
 Environmental Consultants  
 Job No. CE05022





Table 7

Portrane, Doi Portrane, Donabate, Rush & Lusk WWTs - DBO Contract  
Liquid Stream Plant Flows and Sampling Results - O&M Phase

Updated on: 18/02/2013



Date	WWT Inflow		Influent Concentration		Loading		WWT Effluent Flow		Effluent Concentration		SBR 1	SBR 2	SBR 1	SBR 2	Notes	
15/11/2012	4,606	15	4,621	462	210		4,239	0	10	2	42					
16/11/2012	4,353	0	4,353	455	221		3,926	0	14	2	56					
17/11/2012	3,496	0	3,496				3,225	0								
18/11/2012	5,174	0	5,174				4,601	0								
19/11/2012	8,905	302	9,207	426	203		8,367	0	10	1	19					
20/11/2012	6,249	0	6,249	322	161		5,696	0	19	4	700					
21/11/2012	5,226	0	5,226	163.00	342	0.48	852	4,564	0	5.00	14	1	5,100		UV plant malfunction experienced.	
22/11/2012	6,350	0	6,350	405	169		5,664	0	7	4	210					
23/11/2012	5,075	0	5,075	333	144		4,867	0	21	5	110					
24/11/2012	4,350	0	4,350				3,972	0								
25/11/2012	8,365	87	8,452				7,786	0								
26/11/2012	13,516	6,109	19,625	166	66		17,791	2,781	14	4	260					
27/11/2012	10,706	2,040	12,746	106	50		12,595	0	16	8	300					
28/11/2012	7,850	0	7,850	105.00	242	0.43	88	824	7,450	0	5.00	10	5	930		
29/11/2012	6,700	25	6,725	161	32		6,080	0	12	2	70					
30/11/2012	6,191	0	6,191	387	136		5,623	0	9	3	25					
01/12/2012	5,189	0	5,189				4,494	0								
02/12/2012	5,921	0	5,921				5,282	0								
03/12/2012	7,248	0	7,248	312	125		6,601	0	10	1	21					
04/12/2012	5,790	0	5,790	364	139		5,389	0	18	7	6					
05/12/2012	6,085	0	6,085	354	127		5,444	0	14	6	1					
06/12/2012	5,885	21	5,906	350	153		5,205	0	7	3	14					
07/12/2012	7,236	0	7,236	314	170		6,807	0	6	5	98					
08/12/2012	5,488	0	5,488				4,979	0								
09/12/2012	6,156	0	6,156				5,505	0								
10/12/2012	6,311	0	6,311	240	114		5,920	0	8	2	12					
11/12/2012	5,254	224	5,478	440	234		4,624	0	18	4						
12/12/2012	5,226	16	5,242	183.00	354	0.52	159	959	5,011	0	5.00	16	6	37		
13/12/2012	5,436	56	5,492	363	179		5,135	0	16	3	57					
14/12/2012	6,482	16	6,498	333	156		5,795	0	25	3	25					
15/12/2012	4,474	21	4,495				3,617	0								
16/12/2012	6,544	39	6,583				5,738	0								
17/12/2012	6,447	17	6,464	367	187		6,068	0	13	4	36					
18/12/2012	5,469	23	5,492	380	144		4,967	0	22	3	15	4,038	2,918	2,970	2,147	Estimated sampling water depth = 3.45m.
19/12/2012	6,623	18	6,641	406	184		5,940	0	17	2	34					
20/12/2012	7,058	319	7,377	304	157		6,885	0	25	8	27					
21/12/2012	6,566	0	6,566	168	50		6,016	0	20	2	120					
22/12/2012	6,224	0	6,224				5,041	0								
23/12/2012	7,653	20	7,673				7,151	0								
24/12/2012	6,752	0	6,752				6,020	0								
25/12/2012	7,302	11	7,313				6,586	0								
26/12/2012	7,302	11	7,313				6,586	0								
27/12/2012	4,175	80	4,255				3,951	0								
28/12/2012	6,486	0	6,486				5,780	0								
29/12/2012	6,834	0	6,834				5,995	0								
30/12/2012	4,285	0	4,285				4,069	0								
31/12/2012	6,100	17	6,117	115	95		5,433	0	20	11						
01/01/2013	5,542	0	5,542				4,922	0								
02/01/2013	5,049	0	5,049	87.00	176	0.49	65	4,583	0	10.00	24	15				
03/01/2013	6,248	0	6,248	298.00	419	0.71	131	1,862	5,554	0	2.00	31	18	57		
04/01/2013	4,465	0	4,465	517	208		3,911	0	42	10	31					
05/01/2013	3,566	0	3,566				3,360	0								

Table 7

Portrane, Doi Portrane, Donabate, Rush & Lusk WWTS - DBO Contract  
Liquid Stream Plant Flows and Sampling Results - O&M Phase

Updated on: 18/02/2013



Date	WWTW Influent Flows		Influent Concentration		Loading		WWTW Effluent Flows		Effluent Concentration		SBR 1	SBR 2	SBR 1	SBR 2	Notes
06/01/2013	6,178	0	6,178				5,258	0							
07/01/2013	5,583	0	5,583	636		332	4,944	0	19	8					
08/01/2013	4,923	0	4,923	519		195	4,743	0	22	7	30				
09/01/2013	4,370	0	4,370	366.00	978	0.37	483	1,599	4,109	0	6.00	28	5	80	
10/01/2013	4,686	0	4,686	984		618	4,303	0	17	9	85				
11/01/2013	4,775	0	4,775	641		318	4,276	0	15	2	47				
12/01/2013	3,894	19	3,913				3,122	0							
13/01/2013	4,724	0	4,724				4,374	0							
14/01/2013	6,394	0	6,394	791		408	5,474	0	44	13	100				
15/01/2013	4,693	0	4,693	550		316	4,379	0	36	7	92				
16/01/2013	4,828	0	4,828	227.00	616	0.37	323	1,096	4,282	0	5.00	23	3	85	4.092 4.872 3.010 3.584 Estimated sampling water depth = 3.45m.
17/01/2013	5,223	0	5,223	846		510	4,282	0	22	4	31				
18/01/2013	9,438	1,785	11,223				9,492	0							
19/01/2013	11,273	2,053	13,326	194		130	13,393	0	15	4					
20/01/2013	13,616	670	14,286	169		71	13,105	0	15	9	29				
21/01/2013	10,118	22	10,140	286		109	9,786	0	14	6		4,350	4,886	3,200	3,584 Estimated sampling water depth = 3.45m.
22/01/2013	8,044	0	8,044	126		78	6,649	0	10	2	1				
23/01/2013	7,190	0	7,190	118.00	286	0.41	134	848	6,771	0	7.00	7	6	65	
24/01/2013	6,595	0	6,595	309		161	5,877	0	15	5	48	3,710	4,420	2,729	3,251 Estimated sampling water depth = 3.45m.
25/01/2013	6,629	0	6,629	377		181	5,748	0	12	7	37				
26/01/2013	11,106	5,175	16,281	215		121	14,977	0	10	4					
27/01/2013	15,953	113	16,066	134		59	14,563	0	13	9					
28/01/2013	6,847	0	6,847	242		120	6,186	0	79	20	970				
29/01/2013	8,783	21	8,804	250		116	8,108	0	27	8	85	3,820	4,744	2,810	3,490
30/01/2013	8,341	0	8,341	118.00	258	0.46	133	984	7,412	0	6.00	16	6	31	
31/01/2013	8,225	0	8,225	252		119	7,252	0	14	5	51	4,344	4,732	3,195	3,481
01/02/2013	7,267	0	7,267	102		53	6,473	0	7	4	29				
02/02/2013	6,536	0	6,536	289		155	5,875	0	9	2					
03/02/2013	6,777	0	6,777	285		124	6,195	0	13	3	9				
04/02/2013	6,782	0	6,782	195		77	5,956	0	14	3	5				
05/02/2013	6,318	36	6,354	652		176	5,321	0	22	6	30				
06/02/2013	5,720	0	5,720	154.00	366	0.42	191	881	5,226	0	5.00	11	10	10	
07/02/2013	6,632	0	6,632	464		246	5,787	0	21	2	8				
08/02/2013	6,118	0	6,118	250		89	5,719	0	14	2	30				
09/02/2013	7,127	0	7,127				6,095	0							
10/02/2013	12,647	3,784	16,431				15,225	0							
11/02/2013	11,255	51	11,306	119		57	10,765	0	10	9	111				
12/02/2013	8,686	21	8,707	211		89	7,656	0	29	4	26				
13/02/2013	9,865	22	9,886	118.00	272	0.43	105	1,167	8,665	0	9.00	14	8	30	
14/02/2013	8,373	0	8,373	126		35	7,379	0	24	7	16	4,370	4,394	3,215	3,232
15/02/2013	7,155	0	7,155	280		116	6,565	0	27	6	64				
16/02/2013	6,799	0	6,799				5,707	0							
17/02/2013	6,324	0	6,324				5,760	0							
18/02/2013	5,895	0	5,895	461		271	5,346	0	21	7	3				
19/02/2013	5,559	0	5,559	430		235	4,592	0	23	6	61				
20/02/2013	5,444	0	5,444	175.00	301	0.58	201	953	4,997	0	9.00	27	10	910	3,832 4,420 2,819 3,251
21/02/2013	5,150	0	5,150	361		184	4,597	0	19	5	47				
22/02/2013	4,957	0	4,957	797		367	4,548	0	29	7	148				
23/02/2013	4,932	0	4,932				4,664	0							
24/02/2013	4,917	0	4,917				4,080	0							
25/02/2013	4,575	0	4,575	358		93	4,246	0	20	7	230				
26/02/2013	4,531	0	4,531	690		325	3,935	0	27	5	193				





Table 7

Portrane, Doi Portrane, Donabate, Rush & Lusk WWTS - DBO Contract  
Liquid Stream Plant Flows and Sampling Results - O&M Phase

Updated on: 18/02/2013



Date	WWTP Influent Flows		Influent Concentration		Loading		WWTP Effluent Flows		Effluent Concentration			SBR 1	SBR 2	SBR 1	SBR 2	Notes		
11/06/2013	5,626	0	5,626	270.06	643	350	1,519	5,295	0	22	9	1,000						
12/06/2013	5,109	0	5,109	199.50	475	299	1,019	5,133	0	48	9	900						
13/06/2013	6,046	0	6,046	162.00	412	269	979	5,487	0	3.80	30	12	300	3,658	3,840			
14/06/2013	5,225	0	5,225	182.28	434	237	952	5,060	0		27	10	78					
15/06/2013	6,362	0	6,362	189.42	451	299	1,205	5,890	0		23	6	56					
16/06/2013	5,071	157	5,228	235.62	561	343	1,232	5,417	0		40	18	298					
17/06/2013	6,518	0	6,518	238.98	569	392	1,568	5,728	0		40	19	200					
18/06/2013	5,062	0	5,062	352.80	840	583	1,786	4,893	0		24	7	44	3,406	3,442			
19/06/2013	4,656	0	4,656	221.34	527	239	1,031	4,494	0		24	6	284					
20/06/2013	4,433	0	4,433	207.00	676	397	918	3,854	0	5.20	34	16	156					
21/06/2013	4,051	263	4,314	230.58	549	340	995	3,990	0		26	7	79					
22/06/2013	4,388	54	4,442	261.66	623	259	1,162	4,423	0		42	14	203					
23/06/2013	4,229	0	4,229	198.24	472	210	838	3,703	0		27	10	80					
24/06/2013	4,121	0	4,121	246.12	586	297	1,014	3,989	0		26	6	142					
25/06/2013	3,922	0	3,922	248.64	592	320	975	3,605	0		34	8	101	2,762	2,708			
26/06/2013	3,993	0	3,993	222.18	529	259	887	3,845	0		16	2	41					
27/06/2013	4,110	0	4,110	292.40	567	259	1,202	3,941	0	9.74	39	13	78					
28/06/2013	3,879	0	3,879	270.48	644	293	1,049	3,386	0		36	2	1,800					
29/06/2013	3,828	0	3,828	304.08	724	393	1,164	3,672	0		45	13	15					
30/06/2013	3,961	0	3,961	271.74	647	311	1,076	3,764	0		55	11	13					
Totals	134,814	474	135,288					126,284	0									
Averages	4,494	16	4,510	254	613	0	326	1,130	4,209	0	5	30	8	269	3,422	3,588	#DIV/0!	#DIV/0!

Average Influent BOD:COD Ratio

Average PE Loading (based on sampled BOD)

Average PE Loading (based on estimated BOD from COD)

Average Daily Flow to Treatment (m<sup>3</sup>)

Average Daily Solids (kg)

# AECOM DESIGN BUILD IRELAND LTD.

DESIGN REPORT No. DR2259/3

Rev A

DATE: 30/03/10 SUBJECT: Portrane, Donbate, Rush & Lusk WWTS SHEET NO: 1 of 19

BY: MOB CHKD: Portrane WWTW SBR Design JOB NO: J2259

## Introduction

This report reviews the design for the secondary treatment system proposed for Portrane WWTW. This report has been revised as the fill decant cycle will be used for 3 basin maintenance operation (rather than no fill during the decant phase of the cycle).

## Design Standards, Internal/External

## Calculations

The treatment process is required to contain sufficient flexibility to allow the maximum treatment capacity to be gradually increased from 50% of the current capacity up to the design capacity over the Contract Period. The current required capacity is estimated as having a PE of 25,000.

The following table gives a summary of Performance Requirements (as per specification clause 9.6.3.7):

Design Parameter	Current	Design	
Design Population, PE	25,000	65,000	
Loading to WWTP (kg BOD/day)	1,500	3,900	
Expected maximum fluctuation in daily BOD load, relative to observed average	250%	200%	
Dry weather flow (m <sup>3</sup> /day)	5625	14625	
Expected Maximum fluctuation in daily flow relative to observed average	200%	150%	
Peak Flow to WWTP, l/s	225	770	
Final Effluent Standards:	BOD (mg/l)	25	25
	COD (mg/l)	125	125
	SS (mg/l)	35	35

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# AECOM DESIGN BUILD IRELAND LTD.

**DESIGN REPORT No. DR2259/3**

**Rev A**

**DATE: 30/03/10      SUBJECT: Portrane, Donbate, Rush & Lusk WWTS      SHEET NO: 2 of 19**

**BY: MOB      CHKD:      Portrane WWTW SBR Design      JOB NO: J2259**

The maximum daily BOD load that the Contractor is required to handle is:

$3900 \times 200\% = 7800 \text{ kg/day}$  (this will be catered for in the aeration requirements which will be the subject of a separate report).

The maximum daily flow that the Contractor is required to handle is:

$14625 \times 150\% = 21937.5 \text{ m}^3/\text{day}$  (responses to Tender queries Item 66)

The influent suspended solids, TN and ammonia loadings have not been given in the Employer's Requirements. The suspended solids loading is taken as 300mg/l which is typical for domestic sewage (though slightly higher than the analytical results given in Section 21 of the specification). The Total Nitrogen and Ammonia loads are based on the average TN:BOD and Ammonia:BOD ratios calculated from the analytical results given in Section 21 of the specification. These are 0.375 and 0.251, respectively (which is higher than typical for domestic sewage).

Therefore the design influent parameters used for Portrane WWTW are as follows:

DWF	14625m <sup>3</sup> /day
FFT	43875m <sup>3</sup> /day (3 DWF)
BOD	3900kg/day
SS	4387.5kg/day
TN	1462.5kg/day
Ammonia	978.9kg/day

It is assumed that the load includes the contribution from any waste liquids received at the site at the tanker reception facilities.

### *Return Liquors*

Allowance has to be made in the design calculations for the additional loads due to return liquors. These are primarily from the thickeners, dewatering unit and preliminary treatment washwater. The maximum hourly supernatant flow is estimated as 142m<sup>3</sup>/h (see Appendix 1) and is based on dewatering for 10 hour/day, 5 days/week.

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# AECOM DESIGN BUILD IRELAND LTD.

DESIGN REPORT No. DR2259/3

Rev A

DATE: 30/03/10      SUBJECT: Portrane, Donbate, Rush & Lusk WWTS      SHEET NO: 3 of 19

BY: MOB      CHKD:      Portrane WWTW SBR Design      JOB NO: J2259

The BOD and SS loading contribution from the return liquors is taken from the tender stage mass balance (to be reviewed). These loadings are 279.8kg/day and 290.7kg/day, respectively. These values are based on BOD and SS concentrations of 500mg/l in the return liquors from the thickeners and dewatering unit. The TN and Ammonia values for the return liquors are taken as 5% of the influent loadings.

## *Total Loads*

Therefore the total flows and loads used for the SBR design are as follows:

DWF	18033m <sup>3</sup> /day (includes return liquors – the SBR design uses the maximum hourly flow over 24 hours)
FFT	47283m <sup>3</sup> /day (3.23 DWF) – 547.3 l/s
BOD	4179.8kg/day
SS	4678.2kg/day
TN	1535.6kg/day
Ammonia	1027.8kg/day

## *Basin Sizing*

The activated sludge system proposed is based on the use of sequencing batch reactors. 4 No. SBR tanks are proposed. Flow to the SBR tanks is distributed in a flow distribution chamber using weirs with actuated penstocks for isolation purposes.

Influent to each SBR basin is mixed with returned activated sludge (RAS) biomass from Zone 3 of the basin and passes initially through a captive selector reactor (Zone 1). This zone provides the appropriate floc loading rate to optimize sludge settleability and is not aerated during normal operation. Zone 1 is fitted with a series of baffles to prevent short circuiting of the influent and the RAS.

The mixed wastewater and RAS then enters Zone 2 from Zone 1 via a submerged overflow weir and then continues to Zone 3, via an underflow baffle. Zone 2 acts as an area of high loading in case of overloading of Zone 1 in peak conditions and also prevents disturbance of the sludge blanket during settling, while the tanks are filling.

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# AECOM DESIGN BUILD IRELAND LTD.

DESIGN REPORT No. DR2259/3

Rev A

DATE: 30/03/10      SUBJECT: Portrane, Donbate, Rush & Lusk WWTS      SHEET NO: 4 of 19

BY: MOB      CHKD:      Portrane WWTW SBR Design      JOB NO: J2259

After a period of aeration followed by settling, clear effluent is removed from the surface of the basin by using a decanter and settled sludge is removed from the bottom using the surplus activated sludge (SAS) pump. Surplus sludge is pumped to the SAS Storage Tank.

A decanter length of 14.5m is provided for each basin. The decant period will be on a time basis. There are two decant sequences: 1 hour during the standard 4 hour cycle and 37.5 minutes during the shorter 2.5 hour high flow cycle. At the end of the cycle the decanter is driven to the park position above top water level, where it operates as an emergency overflow.

The size of the SBR tanks for Portrane is based on the sizes of the Phase 2 tanks at Balbriggan (where the 2 No. Phase II tanks were sized for 38182 PE). This uses a fill-settle time of 58 minutes for the 2.5 hour high flow cycle.

The basin dimensions are 42.6m long (allowing for a 300mm wide baffle wall between Zones 2 and 3) × 18m wide, with a TWL of 5.01m (see Appendix 2 for calculation results sheet). The BWL is 3.39m.

The proposed dimensions yield a Sludge Age of 13.7 days and an F/M ratio of 0.1 for four basin operation at design loadings.

The zone dimensions are as for the Phase 2 Balbriggan Skerries tanks, giving the percentage volume in Zones 1, 2 and 3 as 6.3%, 17.3% and 76.4%, respectively.

Specification clause 15.3.3.2 (as per amendment No. 1.3) requires design calculations also to be based on operating scenarios whereby tanks are taken out of service for maintenance purposes.

**It is now proposed to use a fill decant cycle for three basin maintenance operation. This will allow the use of the same TWL and BWL as for 4 basin operation, rather than no filling during the decant phase of the cycle which would require an increase in depth between TWL and BWL to accommodate the maximum flow to full treatment. Filling during decant has been carried out on other sites though it is not the preferred treatment option, however taking a basin out for maintenance is expected on rare occasions and would be a planned operation.**

The Sludge Age is 10 days and F/M ratio is 0.14 in this case (see Appendix 3 for results sheet).

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# AECOM DESIGN BUILD IRELAND LTD.

DESIGN REPORT No. DR2259/3

Rev A

DATE: 30/03/10 SUBJECT: Portrane, Donbate, Rush & Lusk WWTS SHEET NO: 5 of 19

BY: MOB CHKD: Portrane WWTW SBR Design JOB NO: J2259

**Therefore the same decanter level settings can be retained for 3 basin operation as for 4 basin operation.**

The decanter length will be as required for the 4 basin design, i.e. 14.5m. This was sized based on a removal rate of approximately 38 l/s/m weir length. For 3 basin operation at the maximum flow the removal rate with a 14.5m long decanter will be approximately 50.3 l/s/m weir length which may have the potential to disturb the sludge blanket. However, taking a basin out of service should be for planned maintenance which should be carried out during dry periods and every effort should be made to limit return liquor flows.

The current treatment capacity (CTC) is the capacity of the WWTP as determined by the Liaison Monitoring Committee (LMC). The LMC will review the status of the CTC at various times during the contract period and will vary the CTC as required by the catchment served (specification clause 12.5.3). 2 No. SBR basins will be in operation to provide the current BOD treatment capacity (at the current design flowrates). See Appendix 4 for results sheet.

## *Levels*

At tender stage the following levels were proposed within the tank:

TWL	11.443mOD (corresponding to a TWL of 5.01m)
BWL	9.823mOD (corresponding to a BWL of 3.39m)
Floor level	6.433mOD

Based on the finished ground level of 9.35mOD proposed at tender stage, the floor level is 2.917m below ground.

These levels will be confirmed on completion of the Plant Hydraulics report.

**The above levels will also apply for three basin maintenance operation using the fill decant cycle.**

# AECOM DESIGN BUILD IRELAND LTD.

**DESIGN REPORT No. DR2259/3**

**Rev A**

**DATE: 30/03/10 SUBJECT: Portrane, Donbate, Rush & Lusk WWTS SHEET NO: 6 of 19**

**BY: MOB CHKD: Portrane WWTW SBR Design JOB NO: J2259**

## *Sludge Production Formula*

The maximum sludge production formula given in our tender submission was as follows (based on influent loadings):

If  $SS > BOD$  then

$$0.918 \times BOD + [SS \times 1.066 - BOD \times 1.072] \times 0.625$$

If  $SS \leq BOD$  then

$$0.918 \times BOD$$

For the design influent loadings the following maximum sludge production is calculated:

$$0.918 \times 3900 + [4387.5 \times 1.066 - 3900 \times 1.072] \times 0.625 = 3890\text{kg/day}$$

From the SBR design spreadsheet a value of 3886kg/day is calculated therefore the maximum sludge production formula given is valid.

## *Ancillary Equipment*

The SAS pumps, RAS pumps and air blowers will be the subject of separate reports.

## *Other issues*

The SBR tanks at Portrane WWTW will be covered as odour removal is a specified requirement. GRP housing will be provided at the flow splitter chamber and at each of the decanters for ease of access to equipment. 2 No. further access opes (1200mm x 1200mm) are proposed at each of Zones 2 and 3 for each tank, as was agreed with the operations department at tender stage. Therefore visual inspection of the process will be more difficult than at other sites with open tanks.

Other smaller opes will be required in the roof for pipework, ductwork and instrumentation.

# AECOM DESIGN BUILD IRELAND LTD.

DESIGN REPORT No. DR2259/3

Rev A

DATE: 30/03/10      SUBJECT: Portrane, Donbate, Rush & Lusk WWTS      SHEET NO: 7 of 19

BY: MOB      CHKD:      Portrane WWTW SBR Design      JOB NO: J2259

## Significant Environmental Impacts/Effects on Environmental Review

The impacts/effects will be reviewed when the Environmental Review has been completed.

## Appendices

1. Return Liquor Estimate
2. SBR design results for 4 basin operation
3. SBR design results for 3 basin operation – **fill during decant**
4. SBR design results for 50% of initial loading
5. Hazard Identification/ Risk Assessment

## Conclusions

1. The influent suspended solids, TN and ammonia loadings have not been given in the Employer's Requirements. The suspended solids loading is taken as 300mg/l which is typical for domestic sewage. The Total Nitrogen and Ammonia loads are based on the average TN:BOD and Ammonia:BOD ratios calculated from the analytical results given in Section 21 of the specification.
2. The activated sludge system proposed is based on the use of sequencing batch reactors. 4 No. SBR tanks are proposed. Flow to the SBR tanks is distributed in a flow distribution chamber using weirs with actuated penstocks for isolation purposes.
3. The size of the SBR tanks for Portrane is based on the sizes of the Phase 2 tanks at Balbriggan (where the 2 No. Phase II tanks were sized for 38182 PE). This uses a fill-settle time of 58 minutes for the 2.5 hour high flow cycle.
4. The basin dimensions are 42.6m long (allowing for a 300mm wide baffle wall between Zones 2 and 3) × 18m wide, with a TWL of 5.01m. The BWL is 3.39m. The proposed dimensions yield a Sludge Age of 13.7 days and an F/M ratio of 0.1 for four basin operation at design loadings.
5. **It is now proposed to use a fill decant cycle for three basin maintenance operation which will allow the use of the same TWL and BWL as for 4 basin operation.**

The information contained in this report is confidential and must not be divulged to any third party without the written consent of AECOM Design Build Ireland Ltd.

# AECOM DESIGN BUILD IRELAND LTD.

**DESIGN REPORT No. DR2259/3**

**Rev A**

**DATE: 30/03/10 SUBJECT: Portrane, Donbate, Rush & Lusk WWTS SHEET NO: 8 of 19**

**BY: MOB CHKD: Portrane WWTW SBR Design JOB NO: J2259**

6. The Sludge Age is 10 days and F/M ratio is 0.14 for three basin maintenance operation using a fill decant cycle.
7. Therefore the same decanter level settings can be retained for 3 basin (maintenance) operation as for 4 basin operation.
8. The decanter length will be as required for the 4 basin design, i.e. 14.5m. This was sized based on a removal rate of approximately 38 l/s/m weir length. For 3 basin operation at the maximum flow the removal rate with a 14.5m long decanter will be approximately 50.3 l/s/m weir length which may have the potential to disturb the sludge blanket.
9. 2 No. SBR basins will be in operation to provide the current BOD treatment capacity (at the current design flowrates).
10. At tender stage the following levels were proposed within the tank:

TWL	11.443mOD (corresponding to a TWL of 5.01m)
BWL	9.823mOD (corresponding to a BWL of 3.39m)
Floor level	6.433mOD

Based on the finished ground level of 9.35mOD proposed at tender stage, the floor level is 2.917m below ground. These levels will be confirmed on completion of the Plant Hydraulics report.

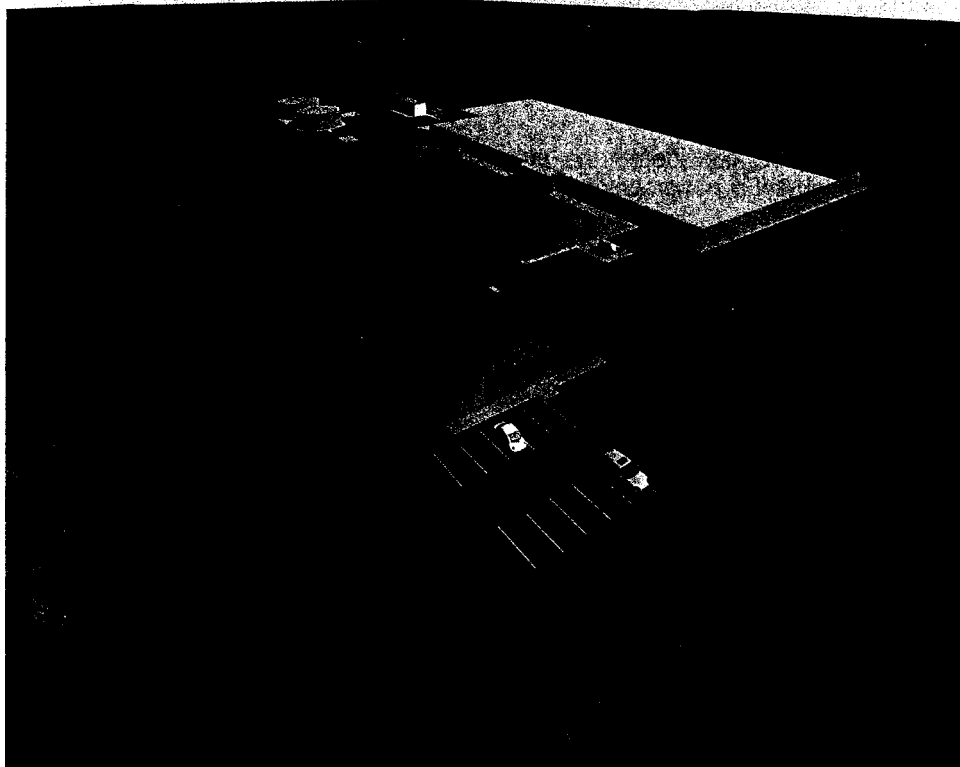
**These levels will also apply for the three basin maintenance operation using the fill decant cycle.**

## **Hazard Identification / Risk Assessment**

See Appendix 5.

Volume 3 - Section 1 – Portrane Waste Water Treatment Plant

**Item 1 – Design Capacity**



**JUSTIFICATION OF TENDER DESIGN**

Our design not only offers excellent value for money, but also an effective wastewater treatment solution that minimises odour and noise emissions, uses energy efficient processes, has a compact footprint and is visually unobtrusive.

Our selections for processes and equipment were driven by a commitment to provide a good quality operational plant at low cost, taking into consideration the Employers Requirements (ER) and Planning Constraints arising from the EIS.

We have chosen processes that Earth Tech Ireland Ltd. are experienced in operating to ensure the smooth and efficient operation of the plant during the 20 year operations period.

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**Tender**

All of our selected processes and equipment have also taken into consideration the Operation and Maintenance Requirements and indeed the Capital Replacement Frequency of the plant to ensure that the whole life cost of the project is optimised for the Client.

The main drivers in our design are:

**ODOUR**

Process selection considered the impact on odour generation. For this reason primary settlement is not proposed, as primary sludge has significant odour potential.

Our design contains all odours and extracts foul air for subsequent treatment thereby removing the potential for odour nuisance to the area and ensuring the boundary odour limits are adhered to at all times.

More details on our proposed Odour Abatement Measures are included in Item 6.4 of this section of our tender submission.

**NOISE**

All major plant items are housed in process buildings and further acoustically enclosed where necessary to ensure acceptable noise limits prevail at the boundary as prescribed by the Client.

**ENERGY EFFICIENCY**

Inherent in our proposals is the selection of drive motors and process plant based on efficiency and reduced kW ratings. Energy efficient processes have been selected to not only offer value to the Employer, but also to reduce the carbon footprint of the proposed WWTP.



## **JUSTIFICATION OF PROPOSED PROCESSES**

### **INLET WORKS**

We propose the Eimco Combined Inlet System (CIS) which combines screening and grit removal in a single tank. This leads to savings in both space requirements and civil costs. Our proposal includes for screening and grit removal of all influent flows as this reduces the potential for solids deposition in the storm tank.

### **STORMWATER TREATMENT**

Flows in excess of FFT are diverted to the storm tank.

Cleaning of the storm tanks is by tipping buckets. This is a reliable and effective method of tank cleaning, especially in tanks of this size. It has the added advantages of low power usage and the minimum of operation and maintenance input.

### **PRIMARY TREATMENT**

It is not proposed to provide primary settlement for the works. Primary sludge is by its nature unstable. The movement and treatment of primary sludge tends to generate odours. On plants of this size, the costs benefits achieved in the secondary treatment are not sufficient to outweigh the increased building costs, operational costs and the odour nuisance potential associated with the handling of primary sludge.

### **SECONDARY TREATMENT**

A number of options were considered for the Biological Treatment Process in order to meet the specified final effluent requirements:

- Membrane Plant
- Standard Activated Sludge Plant
- Sequencing Batch Reactor Plant

A membrane plant has the advantage of producing a very high effluent quality, however, there are significant capital costs. The operating costs are also high due to the high cost of replacement membranes, the large power usage for membrane air scour and the requirement to screen the influent to the membrane plant to 3mm.

Conventional Treatment using aeration and final settlement tanks was also considered. This has high capital costs due to the construction of a larger number of tanks and associated sludge pumping stations.

Sequencing Batch Reactors (SBRs) provide the optimum solution for this application as the requirement for final settlement tanks is eliminated.

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**Tender**

The SBR system proposed is our own Earth Tech CASS (Cyclic Activated Sludge System) plant. This system has been used extensively on similar applications and details of reference plants are given below. It also gives us the benefit of substantial in-house expertise and experience on the plant proposed. This will ensure that both the design and operation of the plant proceed smoothly, without depending on third party expertise.

### **UV TREATMENT**

Both in-line and open channel UV treatment units were considered for Portrane WWTW. Although an in-line unit has a lower capital cost, an open channel unit is proposed here as it has a lower pressure drop and provides for easy access to the UV lamps for cleaning and replacement.

Though medium pressure lamps were also considered, low pressure lamps are proposed due to the lower power consumption.

Vertical and horizontal UV treatment modules were examined during the tendering period. Vertical modules are proposed as these allow maintenance without lifting the modules out of the water, thus allowing simpler and safer lamp replacement. The staggered lamp arrays also assure the disinfection path even in the case of lamp failure.

A serpentine weir was selected to maintain the level in the channel due to the low headloss and the absence of moving parts. This type of weir is currently operating successfully at Wexford and Skerries WWTWs.

### **SLUDGE TREATMENT**

A number of options were considered for sludge thickening. A drum thickener or belt thickener has relatively high maintenance and operator input, a high washwater requirement and it does not perform well with variable feed. It also requires a separate poly make up system.

Picket Fence gravity Thickeners are proposed for waste activated sludge thickening. These are a simple, reliable thickening device requiring minimal operator input. The Picket Fence Thickener also has low power consumption, requires no poly or washwater and provide integral sludge storage potential.

For sludge dewatering, both decanter centrifuges and belt presses were considered. A decanter centrifuge was chosen as this has a smaller footprint, provides a more consistent dry solids product in a clean environment, with the minimum of operator input. ETIL have extensive experience of this equipment and general maintenance can be carried out by our own staff.

**DESIGN CALCULATIONS**

A Mass Balance for the proposed process at design loading is also attached.

**MAIN TREATMENT PROCESS**

<b>Preliminary Treatment</b>		
Peak Inlet Flowrate	770	l/s
No. of fine screens	2	
No. of duty screens (for max. flow)	2	
Screen capacity/ screen (to allow for washwater)	404	l/s
Grit capacity	808	l/s
Storm flow	262.19	l/s

<b>Storm Tank</b>		
Maximum storm flows	262.19	l/s
No. of storm tanks	1	
Total tank volume	2268	m <sup>3</sup>
Total Retention time @ max. flow	2.4	hours

<b>Activated Sludge Plant - Design Basis</b>		
<b>FLOWS</b>		
Total Dry Weather flow	17783	m <sup>3</sup> /d
Peak diurnal flow at DWF	22171	m <sup>3</sup> /d
Total flow to full treatment	47033	m <sup>3</sup> /d
Return Liquors (max. instantaneous over 24 hours)	3158	m <sup>3</sup> /d
<b>INFLUENT (at dry weather flow including return liquors)</b>		
BOD concentration	235	mg/l
	4180	kg/day
SS concentration	263	mg/l
	4678	kg/day
TKN concentration	88	mg/l
	1566	kg/day
<b>EFFLUENT (at dry weather flow)</b>		
BOD concentration	25	mg/l
SS concentration	35	mg/l
<b>BASIN DESIGN</b>		
No. of basins	4	
Effective basin length	42.3	m
Basin width	18	m
Basin depth	5.5	m
Normal cycle time	4	hours
High flow cycle time	2.5	hours
High flow cycle decant time	37.5	minutes
TWL	5.01	m
BWL	3.39	m
MLSS at TWL	3493	mg/l
Total Waste Sludge	3891	kg/day
Skimmer weir length	14.5	m
SOR per basin	294	kg O <sub>2</sub> /h
SOR per basin - 200% BOD peak	445	kg O <sub>2</sub> /h

Activated Sludge Plant - Design Loading		
No. of basins operating	4	
Sludge Age	14	days
F/M	0.1	

Activated Sludge Plant - Current Loading		
No. of basins operating	4	
Sludge Age	35	days
F/M	0.04	

Activated Sludge Plant - Basin out of service for scheduled maintenance		
No. of basins operating	3	
Sludge Age	10	days
F/M	0.143	

UV Treatment		
Maximum flow	2765	m <sup>3</sup> /h

**SLUDGE TREATMENT PROCESS**

<b>Sludge Production</b>		
Waste Activated Sludge Production	3891	kg/day
Average dry solids content	0.80%	
WAS volume	486.4	m <sup>3</sup> /day

<b>SAS Storage</b>		
No. of tanks	1	
Liquid volume	364	m <sup>3</sup>
Retention	0.75	days

<b>Waste Activated Sludge Thickening</b>		
No. of Thickeners	2	
No. of duty Thickeners	2	
Diameter	10.1	m
Solids loading rate	1.0	kgDS/m <sup>2</sup> h
Minimum dry solids content	2.50%	
Side wall liquid depth	4	m
Floor slope	7.5	°
Hopper slope	60	°
Hopper top diameter	1.4	m
Hopper base diameter	0.7	m
Tank volume	677.5	m <sup>3</sup>
Sludge thickened to	2.5%	
Sludge volume	155.6	m <sup>3</sup> /day
Assumed supernatant depth	1.5	m
Sludge storage volume available	437.1	m <sup>3</sup>
Actual sludge retention available in PFT	2.8	days

Waste Activated Sludge Thickening - current loading		
Solids production	1496	kg/day
No. of Thickeners	2	
No. of duty Thickeners	2	
Diameter	10.1	m
Solids loading rate	0.4	kgDS/m <sup>2</sup> h

Sludge Dewatering		
Minimum cake solids	22%	
No. of hours dewatering per week	100	
Solids loading required	272.4	kg/h
Diameter	10.1	m
No. of dewatering units	2.0	
No. of standby dewatering units	1	
Maximum dewatering unit capacity	500	kg/h

<b>Polyelectrolyte dosing</b>		
Total solids	272.4	kg/h
Poly dosing rate	8	kg/h
Poly make-up concentration	0.5%	
Poly required	2.2	kg/h
Poly make-up unit flow	435.8	l/h
Poly make-up unit size	1000.0	l/h

<b>Dewatered Sludge Quantities</b>		
% Dry solids	22.0%	
Volume of sludge per week	123.8	m <sup>3</sup> /week

## **REFERENCE SITES**

### **INLET WORKS**

The combined inlet works system is designed and supplied by Eimco (Jones and Attwood), with whom Earth Tech Ireland have worked closely for many years. The components of the combined inlet works are standard Eimco (Jones & Attwood) equipment, i.e. screens, grit pumps, grit classifier etc. Earth Tech Ireland have successfully installed combined inlet works systems at Moycullen, Wexford, South Tipperary Grouped Wastewater Schemes and Balbriggan Skerries.

- Moycullen STW: CIS system for 4,000 P.E.  
Commissioned: 2003  
Client: Galway County Council
- Wexford STW: CIS system for 30,000 P.E.  
Commissioned: 2003  
Client: Wexford County Council
- Carrick-on-Suir STW: CIS system for 11,000 P.E.  
Commissioned: 2004  
Client: Tipperary County Council



- Ballbriggan Skerries STW: CIS system for 70,000 P.E.  
Commissioned: 2007  
Client: Fingal County Council

### STORM TANKS

Earth Tech have used tipping buckets for storm tank cleaning at:

- Greystones STW: Storm tank 44 m x 25 m x 5.623 m.
- Wexford STW: Storm tank 45 m x 12.3 m x 6.5 m.
- Skerries WWTW: Storm tank 25.325m x 6m x 5.3m.
- Tralee STW: Storm tank 26.7 m x 16.5 m x 6.2 m.
- Moycullen STW: Storm tank 14.5 m x 6.0 m x 5.3 m.
- Navan STW: Storm tank No. 1 21.3 m x 10.5 m x 3.88 m.  
Storm tank No. 2&3 26 m x 10.2 m x 5.313 m.

### SBR

The secondary treatment system proposed is the Earth Tech "CASS" system. This system has been developed by our sister company. There are over 150 installations world-wide including Osberstown STW. Earth Tech Ireland have installed CASS systems at Balbriggan Skerries, Headford, Ballymoney and Rosslare. We are also currently constructing the WWTW for New Ross, where a two basin CASS SBR system will be provided.

- Ballbriggan Skerries STW (Phase 1): CASS SBR for 30,000 P.E.  
Commissioned: 2007  
Client: Fingal County Council
- Ballbriggan Skerries STW (Phase 2): CASS SBR for 40,000 P.E.  
Commissioned: 2008  
Client: Fingal County Council
- Headford STW: CASS SBR for 3,500 P.E.  
Commissioned: 2007  
Client: Galway County Council
- Ballymoney WWTW: CASS SBR for 40,650 P.E.  
Commissioned: 2008  
Client: Water Services N.I.
- Rosslare STW: CASS SBR for 7,700 P.E.  
Commissioned: 2008  
Client: Wexford County Council

## **UV TREATMENT**

The proposed UV Treatment units are manufactured and supplied by LIT. A reference list of previous LIT installations is attached.

## **SLUDGE DEWATERING**

The proposed dewatering units at Portrane are Noxon centrifuges (supplied by Solids Technology). A reference list of previous Noxon installations for the proposed machine is attached. We are currently commissioning Noxon centrifuges at Portlaoise STW.

## **ADEQUACY OF PROCESSES TO ACHIEVE REQUIRED STANDARDS**

We have chosen processes with which we have a wealth of experience from an operational and maintenance perspective. All processes & equipment proposed by Earth Tech for use in the Portrane WWTW have been used successfully on other plants (reference site information given above and attached).

## **ABILITY OF PROCESSES TO COPE WITH FLOW AND LOAD FLUCTUATIONS**

The Activated Sludge Process selected for the site is a particularly robust and stable process. It is easy to operate and is resilient to shock loads. The plant as designed is sufficiently flexible to cater for under and over-loading. The 4 No. duty blowers are rated to provide sufficient aeration for short term BOD load peaks of up to +200%. Turndown has been provided on critical plant such as pumps and aeration systems. The sludge age in the plant will be in excess of 10 days and this biomass provides excellent resilience to shock loads. The CASS SBR activated sludge process has a long track record and has proved itself as a most reliable method of consistently meeting effluent standards.

## **ABILITY OF PROCESSES TO COPE WITH GROWTH IN LOAD**

The design calculations (given above) show operating parameters at both current and design flows.

It is proposed to operate the SBR plant in four basin mode for the current capacity. 2 No. duty blowers have been provided for each pair of basins. This will allow additional aeration turndown at low influent loads. For underloading conditions, the

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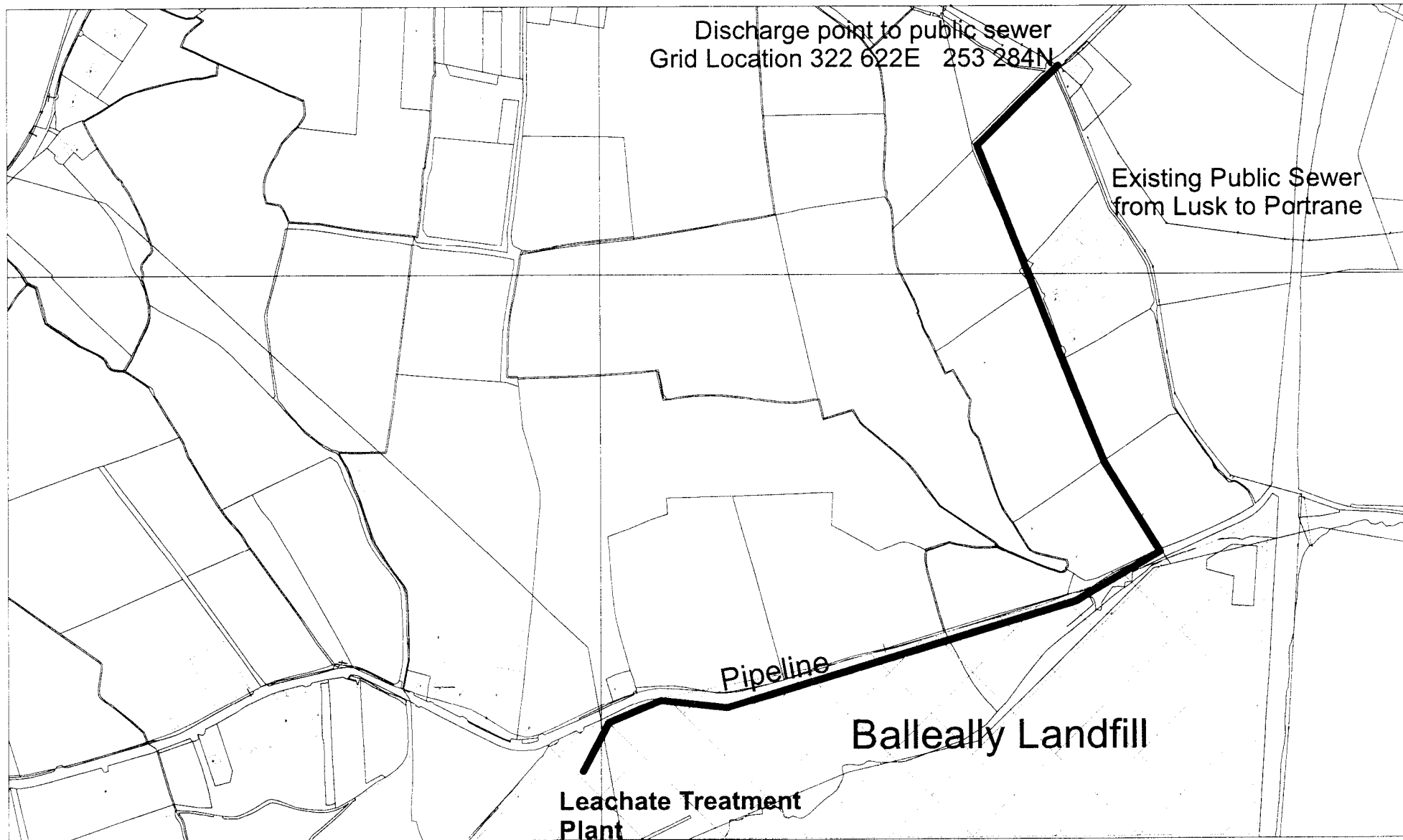
Tender

process control systems will react by reducing the blower output. The oxygen uptake rate in the process is measured by means of a dissolved oxygen probe in each basin. If there is a reduction in load to the basin, the oxygen uptake of the mixed liquor organisms will be less, hence for a given air flow rate, the recorded process dissolved oxygen concentration will start to increase. The control system responds to the increase in dissolved oxygen by reducing the output of the blower system until the dissolved oxygen concentration falls to the desired level. In this way energy efficient operation is maintained under varying load conditions.

If long term influent loads are less than the minimum output of the aeration blowers then the control system can be adapted to operate the blowers on a run/dwell basis.

Therefore the plant is adequately configured to treat from 50% of current capacity to design load.

The SBR plant at Skerries is currently operating at loads less than design but consistently achieving the final effluent discharge standards (results attached).



Sept 2013

Scale 1:5,000

**Fingal County Council  
Balleally Landfill  
Licence W009-03  
Proposed Leachate Pipeline**