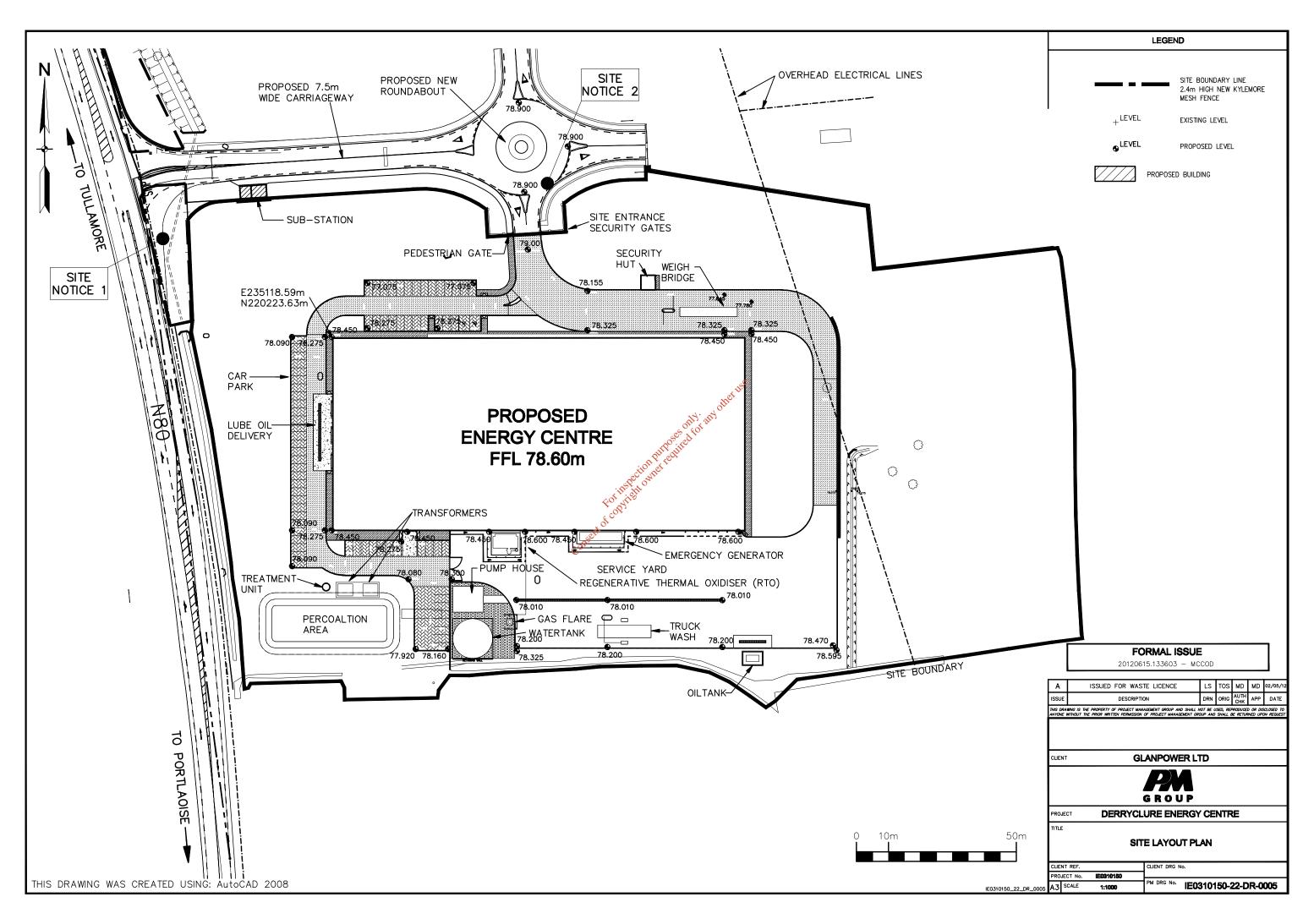
INFRASTRUCTURE & OPERATION

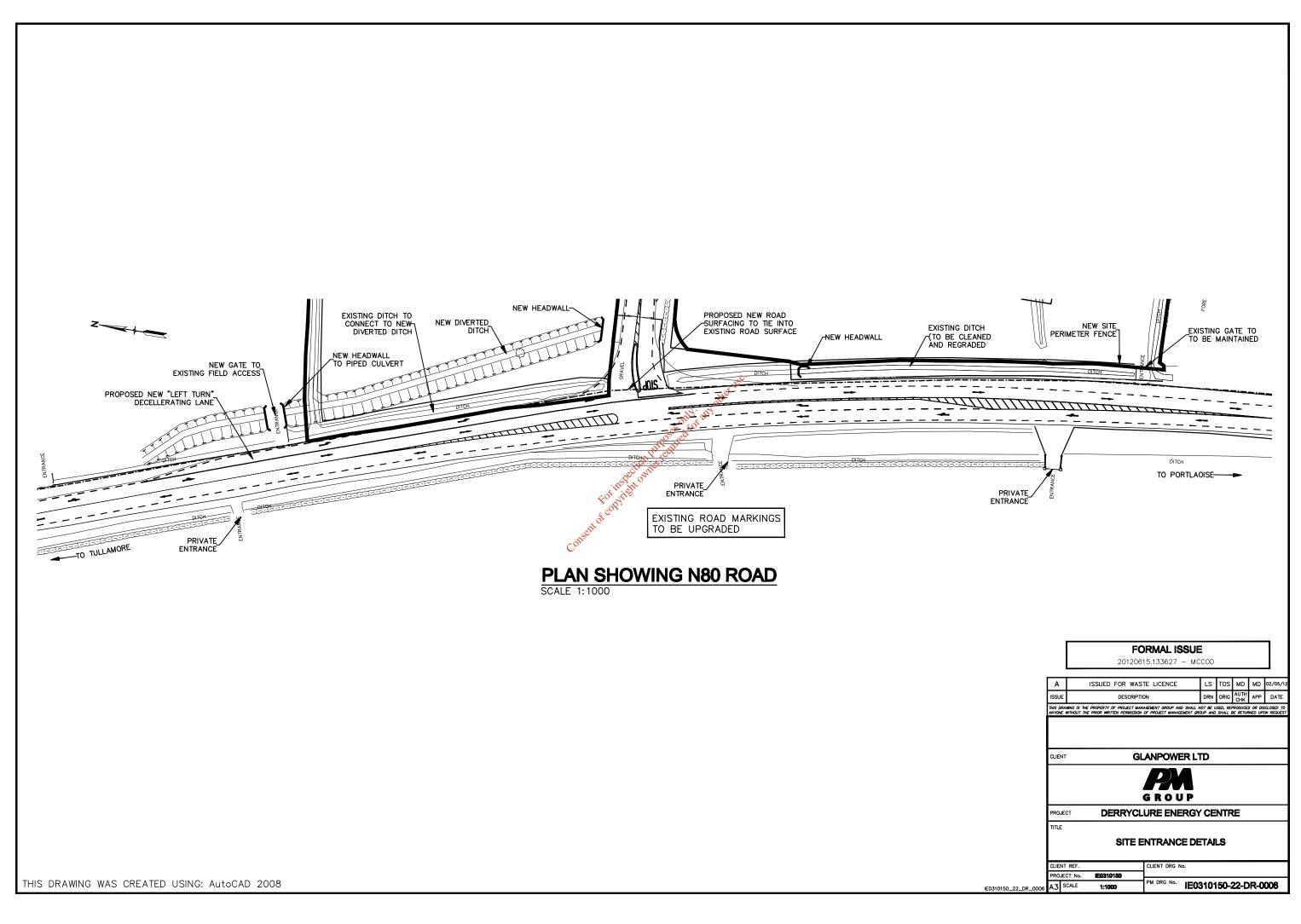
FURTHER INFORMATION

		net "
•	D.1:	Site Layout
•	D.2:	Site Entrance Details
•	D.3:	Site Layout Site Entrance Details Site Paving Layout
•	D.4	Plant Lavour
•	D.5	WWTP Design Report
•	D.6	Site Drainage Layout
•	D.7	Site Drainage Detail
•	D.8	Internal Layout of Plant
•	D.9	Office & Staff Accommodation
•	D.10	Cross Sections A-A & B-B
•	D.11	Cross Sections C-C & D-D
•	D.12	Pre-Shredder
•	D.13	Drum Separator
•	D.14	Eddy Separator Unit
•	D.15	Wind Separator Unit
•	D.16	Final Shredder
•	D.17	Baling Press
•	D.18	Dryer
•	D.19	Regenerative Thermal Oxidiser
•	D.20	Laboratory Analysis Results - Char
•	D.21	Laboratory Analysis Results - Slag

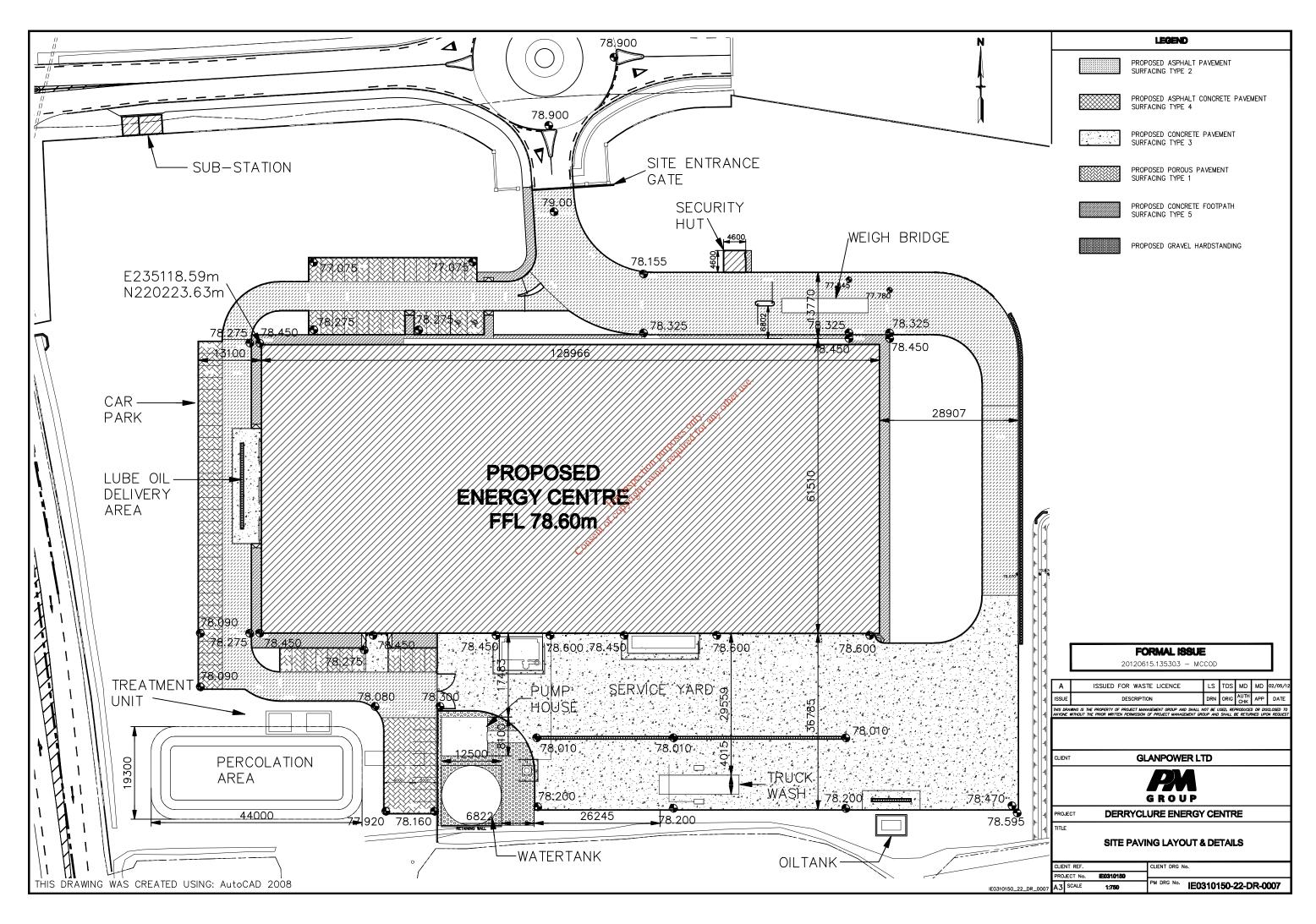
SITE LAYOUT



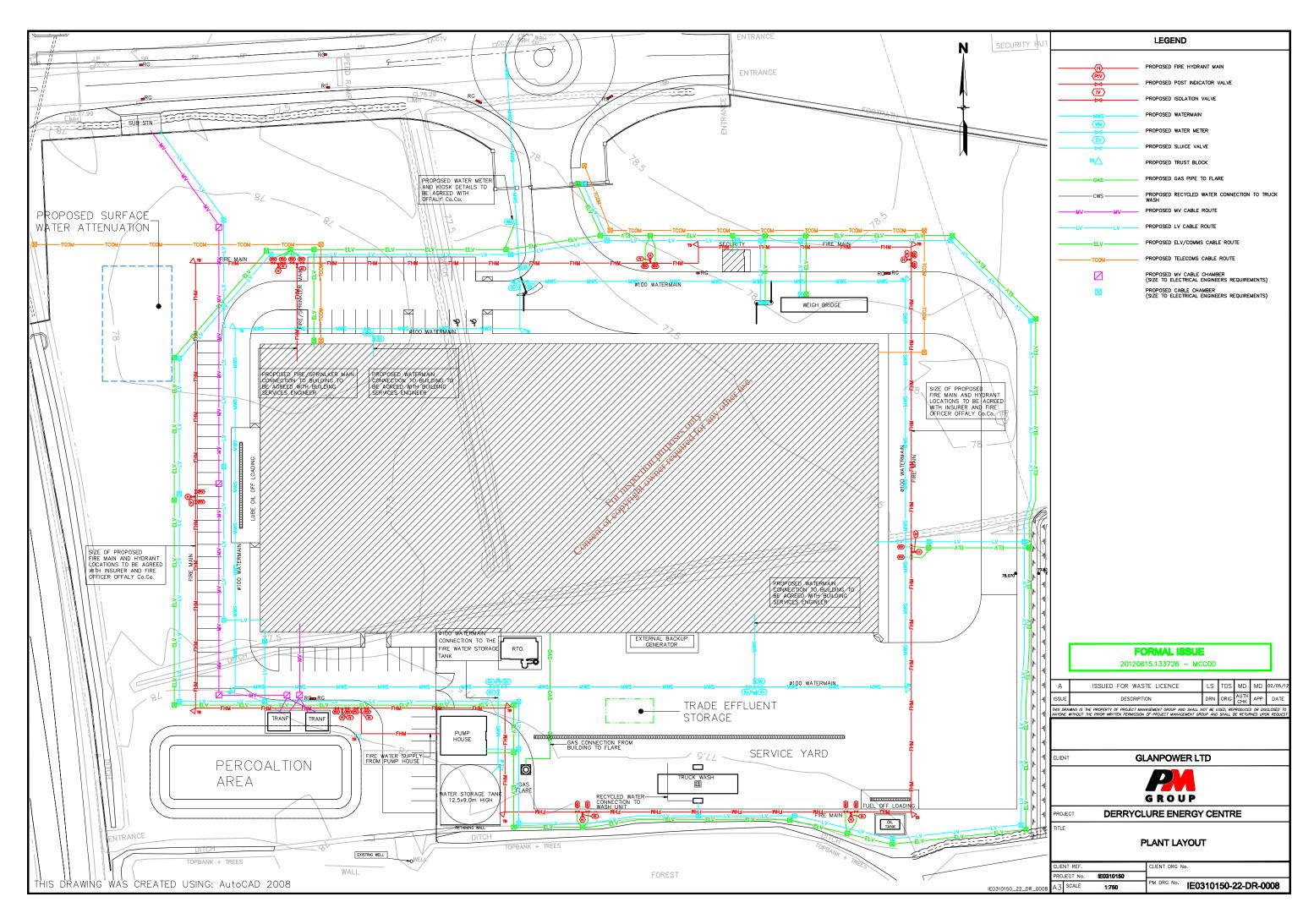
SITE ENTRANCE DETAILS



SITE PAVING LAYOUT



PLANT LAYOUT



WWTP DESIGN REPORT

(21 pages)

(22 pages)

(23 pages)

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(25 pages)

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Waste Water Division

Coleraine, Clara Road, Tullamore, Co. Offaly.

Tel: 057 9326000 Fax: 057 9326060 Email: info@molloyprecast.com Web:www.molloyprecast.com

Date: 8th November 2010

Glanpower, Energy Park,

c/o AXIS: architecture

Penthouse, Castle Buildings, Tara St, Tullamore, Co. Offaly

T: 05793 29628 |F: 05793 29638 | M: 087 1271367

RE: Site specific Information: Planning permission for an industrial unit and proprietary

treatment system at: Derryclure, Tullamore, Co. Offaly. File ref: PTT 1526

Dated: 27/04/2010

1.0 Site relevant parameters

The report presents an opinion on the on-site percolation construction based on the percolation test results/relevant information supplied by Offaly County Council. The assumptions though reasonable, are given for guidance only and no liability can be accepted for changes in conditions not revealed by the percolation test results or by any other information provided. The recommendations in this report are based on the following parameters:

Percolation values: P: 15.46 mon. T 53.93 min. (Mottling at 1m BGL)

Water table: at 1.8 meters

Bedrock: n/a

Proposed total number of workers in the unit: 50 which is equal to 8.33 P.E. Hydraulic load = 1500 lit/d (1000gr Organics)

(Refer to the attached calculation sheet based on Table 3. page 8 from the EPA Waste Water Treatment Manual for Small Communities, Business, Leisure Centres and Hotels.)

Required minimum EPA required percolation area is $500m^2$ of soil polishing filter at a dosing rate of 3 lit/day/ m^2

(Ref. EPA Code of Practice (CoP) (2009), Wastewater Treatment and Disposal System Serving Single Houses $(p.e. \le 10)$ - Table 10.1, p44.) [T values above 51 require a dosing rate of 3 lit/m²/day]

Aquifer category: L1 Vulnerability rating: H Vulnerability response: R2²

Acceptable subject to normal good practice. Where domestic water supplies are located nearby, particular attention should be given to the depth of subsoil over bedrock such that the minimum depths required in Section 6 are met and that the likelihood of microbial pollution is minimised.

The location of the percolation area should be in accordance with the planning permission granted. The following table (EPA 2009 CoP, Section 6, Table 6.1, p14) below details the on the minimum separation distances in meters for the Aswaflow Effluent Treatment System and the percolation area. For minimum separation distances in meters for wells and for Ground Water Protection Response, refer to EPA, 2009 CoP, p. 57 - 62, 2009. In addition, for Heritage features (NHA/SAC), the distances required are dependent on the importance of the feature (contact Local Authorities or Dept. of the Environment and Local Government).

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Table: 6.1 page 14 EPA CoP 2009 minimum separation distances in meters

System	Surface water soakaway	Water- course/ stream	Open drain	Lake or foreshore	Dwelling	Site boundary	Trees	Road	Slope breaks
Aswaflow WWTS	5	10	10	50	7	3	3	4	4
Percolation area	5	10	10	50	10	3	3	4	4

Note: The soakaway for surface water drainage should be located down gradient of the percolation area or polishing filter and that this distance is maintained from neighbouring storm water disposal areas soakaways.

2.0 Secondary treatment system

The proposed *Aswaflow Wastewater Treatment System* (NSAI Agrément (IAB) Approval 02/149, EN 12566-3, No. B 31.06.015.01) is a SBR (Sequencing Batch Reactor) mechanical aeration system designed to cater **for 4 – 12 P.E.** in a 7,000 lit. tank *(population equivalent)*. The system is inclusive, amongst others, of:

- Dual chambered pre-cast concrete tank for primary treatment (septic tank).
- A single chamber pre-cast concrete tank for secondary treatment [Sequential Batch Reactor (SBR)] tank, (Ref: EPA, CoP, 2009, Fig 9.3, page 39)
- Delivery, installation and commissioning by employees of MOLLOY PRECAST LTD.
- The Aswaflow Effluent Treatment System is fitted with a comprehensive alarm system. Alarming will occur with overloading or under-loading of the system and if there is a failure/blockage of the pumps or aerator device. In addition, the system has a high water alarm system.

3.0 Recommendations

According to the EPA's Code of Practice (2009) - Wastewater Treatment and Disposal System Serving Single Houses (p.e. \leq 100 (Ref: EPA, CoP, 2009, Section 9.0, p.37) a packaged wastewater system uses mechanical parts (e.g. an aeration system) to treat wastewater from a dwelling house where a site is unsuitable for a conventional septic tank system. The resultant treated wastewater requires a polishing filter to allow for further treatment of the wastewater and to convey the treated wastewater to groundwater. The polishing filter should be designed in accordance with the procedures outlined in the EPA, CoP, 2009, Section 10.1 p43.

EPA, CoP, 2009: The mean influent concentration and minimum performance standards. [EPA CoP 2009 Table 5.1 p 9]

Characteristics	Typical mean influent concentration (mg/l) (p58)	On-site domestic treatment minimum effluent performance standards EPA CoP 2009 (p9)
$B.O.D_5$	318	<20
C.O.D.	956	N.A.
Total Solids	200	<30
Ammonia (NH ₃ -N)	70	<20

Ref: EPA's Code of Practice - Wastewater Treatment and Disposal System Serving Single Houses $(p.e. \le 10)$ (2009) page 58 and page 9, respectively.

A correctly constructed polishing filter will give final results as follows:

Characteristics	Prior to Treatment	Treated (SBR)	After Polishing
Characteristics	riioi to Treatilielit	Healed (SBK)	Filter (percolation)

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Ph	6-8	7 – 7.5	7
B.O.D ₅	>300	< 15	< 5
C.O.D.	>900	< 75	< 35
Total Solids	>200	< 30	< 4
Ammonia (NH ₃ -N)	>60	< 10	< 1

The achievement of these results is not guaranteed as the treatment achieved will depend on the system being properly operated and maintained.

MOLLOY PRECAST LTD. fully guarantees the *Aswaflow Wastewater Treatment System* as suitable for the development in question when installed according to the manufacturer's specific instructions and commissioned by trained personnel. It is the responsibility of the client to ensure that the percolation area is construction properly in order for the system to operate. To ensure the efficient treatment and disposal of the domestic wastewater, we recommend the following:

- NO RAINWATER, SURFACE WATER etc., should be discharged to the treatment system or percolation area.
- Only domestic wastewater, which includes all effluent from the kitchen, toilets, bathroom, showers, appliances, should be discharged to the treatment system.
- A correctly vented percolation area must be constructed to facilitate effective filtration of the treated effluent into the subsoil.

3.1 Polishing Filters

The Aswaflow System (secondary treatment system) requires a polishing filter. According to the EPA's 2009 Code of Practice (Section 8 and P) a soil filter system may be used in situations where difficult site conditions are encountered, such as a shallow water table, insufficient subsoil depth or insufficient percolation characteristics of native subsoil. A soil filter system may be developed through the use of imported soil with favorable characteristics or may be developed through the use of in situ soil where the upper layer has been removed and replaced by a gravel distribution layer or improved soil. These soils should have percolation values in the range of 3–75 for in situ material and a P/T-value of 3–30 for imported material. Effluent may be loaded onto a soil-polishing filter by any one of three arrangements (direct discharge, pumped discharge or gravity pipe discharge). All polishing filters should have a minimum thickness of 0.9 m of free-draining unsaturated soil or sand between the point of infiltration of effluent and the water table and bedrock. However, greater depths/thicknesses may be required depending on the ground water protection responses (Ref: EPA CoP, Section 6 p14, Annex B). Polishing filters maybe below, at ground surface or partially or totally above ground surface. It must be constructed to facilitate effective filtration of the treated effluent into the subsoil.

3.2 Sizing of the Polishing Filter

The size of the percolation area depends on the hydraulic loading rate and on the treatment that precedes the percolation area. The EPA's *Code of Practice (Section 10.1 Polishing Filters p. 43)*, recommends a shorter trench length of 10m for gravity fed systems for effluent from a secondary treatment system due to its high quality. This allows for effluent from a polishing filter to discharge to ground provide that the subsoil has a T-value <90 and a P-value between 3 and 75.

3.3 Type of Polishing Filter - Intermittent Soil Filter

Where the underlying subsoil are within the acceptable range of $3 \le T \le 75$, an intermittent soil filter (**Ref: EPA CoP, Fig. 8.7, p31-32**) may provide the required solution to the on-site treatment of wastewater. Furthermore, where soil (10 < T < 30) has to be imported, it should be placed in lifts in the proposed percolation area such that there is a minimum thickness of 0.9 m of unsaturated permeable subsoil below the base of the polishing filter for secondary treatment systems i.e. minimum depth of unsaturated subsoil to bedrock and the water table (Ref: EPA, CoP, 2009, Table 6.2, p14). The fill should be placed in layers not exceeding 300-mm thick and lightly compacted. Care should be taken not to over-compact the soil as this will lead to ponding. After each lift is placed, percolation tests should be carried out. In the case of a gravity system, it is recommended to pump the effluent to a stilling chamber from where the effluent flows by gravity to a distribution device (Ref: EPA, CoP, 2009, Section 11, p46-50). In this case, the length of gravity pipe from the stilling chamber to the box should be greater than 3 m. Pumping to a sump/stilling chamber, which then discharges to a distribution device allows for even distribution of the effluent.

3.4 Construction of the Soil Polishing Filters

The treated wastewater from the secondary treatment unit is pumped to using typically 32mm over a 250-mm layer of gravel. The detail design should conform to best practice as outlined in design manuals. The loading rates should conform to those listed in EPA CoP 2009 (Sectio 10, Table 10.1, p. 44.).

- To construct the percolation area, clear the vegetation from an area of at least **40m** by **15.3m**. Fill this area (see profile, allow for banking and grading of the sides of the high raised mound) with soil to a depth of at least **400mm** (percolation T value 7-20) and ensure that the surface is perfectly level. This will insure a percolation depth of at least 1.2m of unsaturated soil below the invert of the distribution stones and above the Mottling.
- Raising an area with imported soil for percolation requires the services of a competent and experienced drainage contractor. The soil should be layered and tested as it is laid down as recommended by EPA 2009 CoP part 8.4 page 31:

In the case of a soil filter, the following procedure should be followed:

- Where soil (10 < T < 30) has to be imported, it should be placed in lifts in the proposed percolation area such that there is a minimum thickness of 1.2 m of unsaturated soil with drainage over the bedrock. The fill should be placed in layers not exceeding 300-mm thick and lightly compacted. Care should be taken not to over-compact the soil as this will lead to ponding.
- After each lift is placed, percolation tests should be carried out. A 150-mm square hole is excavated to a depth of 150 mm in the placed soil. After presoaking to completely wet the soil, 0.5 I of water is poured into the hole and the time in minutes for the water to soak away is recorded. This time should be between 10 min and 2 h.]
- In the center of this area, place a pad, **38m by 13.3m**, of **very clean** broken stones (sized 15mm to 30mm i.e.1" chippings) to a thickness of **250mm** and ensure that it is level.
- Place the 32mm diameter pipes on the stones as indicated on the accompanying drawing and ensure that they are at a gradient of no more than 1:200 away from the manifold. Secure the pipes with more stone as indicated in the drawing.
- The stone pad must then be covered with a geo-textile membrane before the final covering of soil is applied. A minimum recommended cover depth of soil is 300mm.
- A percolation area constructed in this manner is equivalent to an area of **500m**² allowing a dosing rate of **3 liters** of effluent per m² per day for **8.33 P.E**. which complies with EPA CoP (Sectio 10, Table 10.1, p. 44.).

- Constructed mounds can be surrounded and protected by interceptor drains. These interceptor drains should be constructed at a distance of 2 m from the percolation pad (Ref: EPA, CoP, 2009, Section 11.2, p48.)
- A minimum recommended cover depth of soil is 100-300 mm. The quality of the soil used for the percolation area is crucial in order to achieve good water percolation and tertiary treatment. However, the quality of the covering soil is only a consideration for planting purposes i.e. grass and/or shrubs. Planting of large trees is not recommended.
- It is essential that the mound is sealed around the edges with at least 1 m of soil to prevent side bleeding.
- During construction, compaction of any kind is to be avoided by ensuring machinery does not cross the area.
- Note: Not withstanding the percolation test results, or the recommendations based on the percolation test results given above, it is essential that the client, or prospective site occupier, is fully satisfied that the position chosen on the site, for percolation, is capable of soaking away the full amount of treated water that the household will produce each day and insure that any imported soil is suitable.
- **Note:** The distance between the Aswaflow Systems secondary tank and the periphery of the raised percolation area should be a minimum of 5m.
- Soakaways for rain water and surface water should not be located in the vicinity of the treatment system or percolation area (min. 10 m), if possible.
- It is essential that the Aswaflow wastewater tanks are properly located on site so as to ensure no ingression of rain or surface water to the tanks. Tanks should not be installed on sites where the highest wet weather water table reaches more than 1m above the base of the tanks. If there is a high water table, precautions need to be taken so as to ensure that the tanks are anchored and sealed. It the client's responsibility to contact Molloy Precast Ltd for details of remedial action that maybe required.
- Backfilling of the tanks should be done with soft. It is the responsibility of the client to ensure that risers are installed where required and manhole covers are properly sealed. Molloy Precast Products Ltd. cannot take responsibility for incorrectly located tanks or deep installations that Molloy Precast Ltd have not recommended and failure to backfill appropriately.
- It is essential that the drainage contractor constructs the percolation area according to the recommendations cited in this report as Molloy Precast Products Ltd. cannot take responsibility for incorrectly constructed percolation areas.
- See attached cross section drawing for recommended installation and percolation construction.
- Refer to included details for specifications and operation of the treatment system.

Please contact me should you require any additional information.

3 Bahill

Yours sincerely,

Michael F. Cahill.

Technical manager.

Wastewater Treatment Products.

Waste Water Division

Coleraine, Clara Road, Tullamore, Co. Offaly.

Tel: 057 9326000 Fax: 057 9326060 Email: <u>info@molloyprecast.com</u> Web:www.molloyprecast.com

Maintenance Contract Agreement for Site ID: t.b.a.

<u>Aswaflow</u> Wastewater Treatment System for Single Houses

For: Glanpower, Energy Park	
At: Derryclure, Tullamore, Co. Offaly	(Check Correct Postal Address)
Contact phone numbers:	
MOLLOY PRECAST LTD. undertakes to service	ce the system as follows:
	ving core parameters: nk (to determine de-sludging frequency) blume check (Settled Sludge Volume – SSV ₃₀) el and check alarm log ntrol panel where necessary
Annual charge: € 190.00 per annum including Payable in advance. Renewable yearly. Gall ou and mileage. Equipment under warranty is deposited as advised. As a minimum the system shows that the second Service Call due after:	at charges are at our standard rates plus parts bendent on Molloy Precast Ltd. servicing the
Optional Laboratory testing of effluent on rec	quest.
It is advisable to avoid excessive use of detergethe phosphate free alternative products.	nts and household cleaning agents and to use
Disclaimer: Molloy Precast Ltd. cannot take treatment achieved will depend on household in Molloy Precast Ltd cannot take responsi flooding/unfavourable site conditions. Signed: ————————————————————————————————————	nput, good system operation and maintenance.
Position: Tec. Mgr	
Date: 08/11/2010	Date:

(Prices Valid 2009 – VAT @ 13.5% - See terms & conditions)

MOLLOY PRECAST PRODUCTS LTD. 057 9326000

Waste Water Division

Coleraine, Clara Road, Tullamore, Co. Offaly.

Tel: 05793 26000 Fax: 05793 26060 Email: info@molloyprecast.com

Maintenance Contract Terms & Conditions

- Molloy Precast Products Ltd. trained service personnel will conduct the servicing of the designated Sewage Treatment System in a safe and efficient manner in accordance with Molloy Precast Products Ltd. Safety Statement.
- The site Occupier / Owner will insure that the site is accessible and safe.
- This contract does not bind Molloy Precast Products Ltd. to effluent quality, however every effort will be made to advise the occupier and to make any adjustments to the plant that will ensure the best possible effluent quality.
- 1. The occupier should insure that all manholes pertaining to the sewage treatment system are accessible.
- 2. Access to the control panel is also required. (If access to the control panel is not available at the time of a service call, the service will proceed without that component of the service.)
- 3. Access to a water tap is needed to clean components during servicing.
- 4. It is advisable that children and pets are not present in the vicinity of the sewage treatment tanks during servicing. (Open manholes present a significant risk of injury or drowning in the deep tanks.)
- The Service visit includes the items and visits that are paid for and indicated on the maintenance contract only.
- Replacement, broken or unservice parts will be charged for at cost.
- Breakdowns, alarms, etc. requiring call out, are subject to the charges below.
- Note: This maintenance contract only covers the treatment system. It does not extend to any fowl sewer pipe work or blockages or house fittings external to the treatment system. It does not cover any problems relating to percolation or soak-aways.
- A call out request, that occurs close to a due service call date, may be combined with the service by mutual agreement.

Failure to have the system serviced, on an annual basis, will invalidate any current warranty after the first 12 months of operation.

Call out costs are: €100 per visit plus mileage @ €1.20 per mile from Tullamore and €40 per hour for every hour after the first hour, all plus VAT at 13.5%.

Access and mutually acceptable times must be arranged beforehand. No access at the time of an arranged call will incur the full callout charge.

Please mark the services required on the signed document and return to Molloy Precast with payment & VAT for the required services. The counter-signed document will be returned with a receipt.

(Prices Valid 2009- VAT @ 13.5%)

Waste Water Division

Coleraine, Clara Road, Tullamore, Co. Offaly.

Tel: 057 9326000 Fax: 057 9326060 Email: info@molloyprecast.com Web:www.molloyprecast.com

Thank you for your request for Site Specific Information with regard to planning permission for dwelling and proprietary treatment system at **Site Location.**

A full system for a single house, inclusive of all tanks, delivery and commissioning costs €4,140 (Plus VAT at 13.5 %). Price is valid for 3 months.

However, this price does not include the civil works i.e. digging the hole for the tanks, construction of the percolation area or percolation pipes. However, Molloy Precast Product Ltd can provide percolation kits (Wavin pipes split 5 and 7 o'clock, T pipes, elbows, connecting pipes, vent riser, air vent, distribution box and geotextile membrane).

Please contact me should you require any further information.

Yours sincerely, Michael F. Cahill, Technical manager,

Sewage Treatment Systems

Single house Commercial - small Communities.

Interceptors

A range of sizes for Oil & for Grease.

All systems based on Pre-cast Concrete Tanks

For all construction pre-cast products Ring Molloy Precast 057 9326000

MOLLOY PRECAST PRODUCTS LTD. 057 9326000

MOLLOY PER CAST PRODUCTS LTD.,

Coleraine, Clara Road, Tullamore, Co. Offaly. Tel: 05793 26000 Fax: 05793 26060 Email: info@molloyprecast.com

Commercial, Industrial & Leisure Premises (wastewater loading rates)

Industrial C (Urinals must C be economy type) C Schools N	Normal residental Office and/or factory without canteen Office and/or factory with canteen Open industrial site, e.g. quarry - canteen Non-residential with cooking on-site. Students	180 30 60	60	1.00					
Industrial C (Urinals must C be economy type) C Schools N	Office and/or factory without canteen Office and/or factory with canteen Open industrial site, e.g. quarry - canteen	30		1.00					
(Urinals must C be economy type) C Schools	Office and/or factory with canteen Open industrial site, e.g. quarry - canteen		20			0.00	0.00	0.00	0.00
be economy type) C Schools N	Open industrial site, e.g. quarry - canteen	60		0.33	50	16.67	1500.00	1000.00	8.33
Schools N			30	0.50		0.00	0.00	0.00	0.00
	Non-residential with cooking on-site. Students	40	25	0.42		0.00	0.00	0.00	0.00
(6Hr. shock load)	9	60	30	0.50		0.00	0.00	0.00	0.00
·	Non-residential with no canteen. Students	35	20	0.33		0.00	0.00	0.00	0.00
(Urinals must B	Boarding school residents	180	60	1.00		0.00	0.00	0.00	0.00
be economy type)	School day staff	60	30	0.50		0.00	0.00	0.00	0.00
Hotels	Guests (including meals)	250	75	1.25		0.00	0.00	0.00	0.00
(Urinals must	Guests (no meals)	180	45	0.75		0.00	0.00	0.00	0.00
be economy type)	Resident staff	180	60	1.00		0.00	0.00	0.00	0.00
Г	Day staff	60	30	0.50		0.00	0.00	0.00	0.00
	Conference	40	20			0.00	0.00	0.00	0.00
	luxury catering (Meals)	25	25			0.00	0.00	0.00	0.00
	prepared catering (Meals)	15	15	0.25		0.00	0.00	0.00	0.00
	snack bar (Meals)	10	10	0.17		0.00	0.00	0.00	0.00
7 71 7	Function rooms incl.buffets	10	10			0.00	0.00	0.00	0.00
	fast food (Meals)	10	10			0.00	0.00	0.00	0.00
	Residents	200	60			0.00	0.00	0.00	0.00
	Day staff	60	30			0.00	0.00	0.00	0.00
,	Bar drinkers	10	10			0.00	0.00	0.00	0.00
7 7 7	Bar meals	10	10			0.00	0.00	0.00	0.00
	Players inc showers	30	20		,	0.00	0.00	0.00	0.00
	Spectators - Toilet blocks (per use)	8	10		net	0.00	0.00	0.00	0.00
` ,	Restaurants	15	15		1. 4	0.00	0.00	0.00	0.00
	Function rooms	10	10		only, any	0.00	0.00	0.00	0.00
	Toilet blocks (per use)	8	10	Ć-	740	0.00	0.00	0.00	0.00
,	Toilet blocks (long stay car park)	10	15	0.00	25	0.00	0.00	0.00	0.00
	Golf clubs (Players inc. showers)	25	15	37 30		0.00	0.00	0.00	0.00
	Squash, with club house	25	15	A 13		0.00	0.00	0.00	0.00
	Swimming	10	S 0	0.20		0.00	0.00	0.00	0.00
	Touring	50	:111.36			0.00	0.00	0.00	0.00
	Static not serviced	75	₹0° ₹1°35			0.00	0.00	0.00	0.00
		150	55			0.00	0.00	0.00	0.00
	Static fully serviced		~~						
	Tent sites	50 2 50	35 60			0.00	0.00	0.00	0.00
	Residential elderly people	200				0.00	0.00	0.00	0.00
	Residential elderly people plus nursing Nursing homes (convalescent)	350	65 75			0.00	0.00	0.00	0.00

Planned commercial peak usage calculation for -Glanpower Energy Park Industrisl Unit 50 Workers.

Recommended System :

Aswaflow 4-12PE in 7,000 lit S+ tank.

Total organic loading per day in gr.

Total Hydraulic load, litres per day

1500.00

Total PE at normal organic load BOD₅

16.67

Total PE at normal hydraulic output

8.33

Percolation area required @ 50 lit. per m². T <20 Percolation area required @ 20 lit. per m². T >20

Stone & pipe trench, 450mm wide (T <20) Stone & pipe trench, 450mm wide (T >20)

Number of rows 18m long (T <20) Number of rows 18m long (T >20)

Direct discharge Polishing Filters

Stone Pads percoalrion area design, size each pad. Dosing rate < 20 lit. pre $\,$ m² per 24 hours.

Dosing rate $\,$ 10 lit. pre $\,$ m 2 per 24 hours.

Recommended minimum \ldots pads for this site.

Open Sand filter @ 3m² per PE Open Sand filter @ 5m² per PE 25.00 m² 41.67 m²

30 m²

75 m²

67 m

167 m

1 of 14m X 6.25

1 of 18m X 4.86

2 of 18m X 2.43

1 of 14m X 10.71

1 of 18m X 8.33

4 of 18m X

4 Rows

9 Rows

m³ stone

26

26

26

45

45

Offaly County Council Environment Section

Áras an Chontae, Charleville Road, Tullamore, Co. Offaly

Tel: 057 9357403 Fax: 057 9329230 www.offaly.ie



SITE CH	ARACTERI	SATION FOR	RM					
File Ref	PTT1526	Date Rea	dy	27/04/2010)	GPS Co- ordinates	235103 220148	
1.0 GENER	RAL DETAILS	8				Ĺ		
Applicant		Glanpower Ltd	•					
Applicant A	Address	19 Uisneach C	ourt, Mi	ullingar, Co	. Westn	neath		
Site Location Townland	on and	Derryclure, Tul	amore			Site Area	10.05 acres	3
Phone No		044 9333831						
Max no. Residents	N/A		ouble rooms	N/A	TIEC.	No. Sing bedroon		
Proposed V	Vater Supply:	Mains X		Private 🕅			Group Well/ Borehole	
Soil Type	Soils: Cut ove	r peat/made gro	und Thir	oses edited to				
Aquifer Cat		egionally mportant	de Contrer	Loc import	* 1	X	Poor	
Vulnerabilit	y High GW vi	ulnerability of						
Bedrock Type	Dinantian U	Ipper Impure Lim	nestone	S				
Public/Grou	ıp water scher	ne within 1km	Non	e within 1k	m			
Groundwate Protection S		es Groundw		rotection [esponse	R1	Source	Protection Area	N/A
Flood Risk		d in flood risk are ood hazard mapp		benefiting	lands, c	Irainage distr	ct as per OP	W
	Significant Si al, Natural and	F	None p	oresent.				
Previous Ex	perience in th	e Area	Modera	ate percola	tion cha	racteristics.		
Comments	appropriate pe	hlighted during d ercolation on this may cause unad	site.					_

2.0 ON SITE ASSESSMENT

Landscape Position	Relatively flat t area.	opography. Prop	osed treatme	ent area lo	ocated in slightly elev	/ated
Slope	Steep (>1:5)	Shallow	/ (1:5 – 1:20)		Relatively Flat (<1	1:20) X
Surface Fea	atures within a min	imum of 250m (distance to t	features	should be noted in	metres)
Houses	No dwelling wit of proposed site	hin 250m of prop e.	osed site. La	ındfill is lo	ocated approx 220m	to the NE
Existing Land use	Pasture Land.					
Vegetation Indicators	Vegetative indic	cators in certain a however this is n	areas of the 1 ot the case ir	0.05 acre	es suggest poor perc of proposed percolati	olation ion area.
Groundwate	er Flow Direction	Westerly direct	ion across th	e site giv	en topography.	
Ground Cor	ndition	Dry and firm ur	nderfoot durir	ng testing	period.	
Site Boundaries		t. Woodland bou ogland to the Eas		Roads	N80 runs beside the proposed site to the	_
Outcrops (B	edrock and/or sub	citon	sible within 2	50m.		
Surface wat ponding	er None visible wit	hin 250m. Hill	Lakes	None vi	isible within 250m.	
Beaches /Shellfish	N/A	Consentor	Areas/ Wetlands	None vi	sible within 250m.	
Karst Features	None visible wit	hin 250m.	Water- courses	No majo	or water courses.	
Drainage ditches	Drainage ditche locality.	s prevalent in	Springs /Wells	None vi	sible within 250m.	
Comments	percolation through also by density of d Extensive drainage	visiting . This is rains in the area. network has bee therefore the mo	indicated by ven carried out ottled layer wi	vegetation on the si ill not be	to high water table/s n in some parts of fie ite/field and may hav the true winter high v ceptor in this case.	eld and e

3.1 TRIAL HOLE (Should be a minimum of 2.1m deep (3m for regionally important aquifers)

To avoid any accidental damage, a trial hole assessment or percolation tests should not be undertaken in areas which are at or adjacent to significant sites (e.g. NHA's SAC's SPA's and/or Archaeological etc.) without prior advice from National Parks and Wildlife Service or the Heritage Service.

Depth (of Trial ho	ole (m)	1.8m		of water ingress:	1.1	m	Rock	Type	Ilmestone
	Depth from				Surfa			· om Ground er table (m)		1
our.		d Time o	,	ours]			nd Time of		5.2010
		xcavatio	į.					kamination		
	Depth	Soil/Sub	soil	Plasticity	Soil		Dens		Colou	
	of P/T	texture :		and	Structur	e e	/Con	npactness	****	Flowpaths
0.1 m	Test*	Classific Black CL/		dilatancy*** Dilatant	Crumb		Uncor	mpact	Black	N/A
0.1 m	P test	Topsoil		4 ribbons						
0.2 m										
0.4 m										
0.5 m		Boulder C	LAY	4 ribbons					, , ,	
0.6 m				Dilatant	Structure	less	Soft		Brown	N/A
0.7 m							se.			
0.8 m						dile	۶,			
0.9 m					ally a	MA				
1.0 m		Mottling -			- 200 SO FOI					
1.1 m		OH T		Dilatont	Touries only of the structure	loco				
1.2 m	T test	SILT		2 ribbone citor	Massive	1622	Firm		Grey	N/A
1.3 m				2 linguist of	Massive		1 11111		Oley	1777
1.4 m 1.5 m				Dilatant in a constitution of the convirgation						
1.6 m]			dolo						
1.7 m			උර	ser						
1.8 m	Water tabl	e present h	- ,		ı	1		ľ		ι
1.9 m										
2.0 m										
2.1 m										
2.2 m										
2.3 m										
2.4 m										
2.5 m	_									
2.6 m			:							
2.7 m 2.8 m	-									i
2.9 m	-									
3.0 m	1									
<u> </u>	tion [Носии	ubecile n	noane noecih	le elow ne	arcol	ation	rates Drain	ane im	provements will
Evaluat	แอก			ter table.	ne slow pe	o COR	auvii i	iaico. Didili	aye iii	proveniente wiii
Likaly T	י בייובע T Value	>50m			MINISTER AND THE STREET AND THE STRE					
Likely 1	ı valut	~JUIII								
	1									

3.2 (a) PERCOLATION ("T") TEST FOR DEEP SUBSOILS AND/OR WATER TABLE

STEP 1: Test Hole Prepar			
Percolation Test Hole	1	2	3
Depth from ground surface to top of hole (mm) (A)	900	900	900
Depth from ground surface to base of hole (mm) (B)	1300	1300	1300
Depth of hole (mm) [B-A]	400	400	400
Dimensions of hole [length x breadth (mm)]	300 x 300	300 x 300	300 x 300
STEP 2: Pre-soaking Test	holes		
Date and Time Pre-soaking Started	5/2010 13.00	12/05/2010 13.00	12/05/2010 13.00
Each hole should be pre-soake Each hole should be empty be	ed twice before the tes fore re-filling.	st is carried out.	
STEP 3: Measuring T ₁₀₀	2 Bird	ose did	
Percolation Test Hole	1 stagestonier	2	3
Date of Test	13/05/2010	13/05/2010	13/05/2010
Time filled to 400mm	Course, 0	0	0
Time water level at 300mm	231	230	245
Time to drop 100mm (T ₁₀₀)	231	230	245

If T_{100} > 300 minutes then T value >90 – site is unsuitable for discharge to ground If $T_{100} \le 210$ minutes then go to STEP 4 If $T_{100} \ge 210$ minutes then go to STEP 5

Average T₁₀₀

235.3

STEP 4: Standard method where $T_{100} \le 210$ minutes

Percolation Test Hole	1				2				3			
No Fill No.	Start Time	1	nish ime	∆t (min)	Start Time		inish ime	∆t (min)	Start Time		nish ime	Δt (min)
1												
2												-
3												
Average ∆t Value												
	Averaç Hole N	ge ∆t/4= Io. 1	=			rage ∆t ole No.				rage ∆t ble No.		
Result of Te	est T =			(miı	n/25mm)						
Comments												
STEP 5: M	odified	Metho	od whe	re T ₁₀₀	> 210 r	ninutes	other use.					
Percolation Test Hole			4	İ		as only.	and S				3	
			1		OUT	diffed						1
No Fall of water in hole (mm)	Time Factor =T _F	Time of Fall (mins) = T _m	K _{f8} = T _F / T _m	K _{f8} o	Time of Factor	Time of Fall (mins)	K _{f8} = T _F / T _m	T- Value =4.45 K _{f8}	Time Factor	Time of Fall (mins) = T _m	K _{f8} = T _F / T _m	T- Value =4.45 K _{f8}
No Fall of water in	Factor	Time of Fall (mins)	K _{f8} = T _F / T _m	\$ \tag{\tau}	Factor		K _{f8} = T _F / T _m	1		Time of Fall (mins)	K _{f8} =	Value =4.45
No Fall of water in hole (mm)	Factor =T _F	Time of Fall (mins) = T _m	K _{f8} = T _F / T _m	K _{f8} og		= T _m	l m	K _{f8}	Factor	Time of Fall (mins) = T _m	K _{f8} = T _F / T _m	Value =4.45 K _{f8}
Fall of water in hole (mm)	Factor =T _F	Time of Fall (mins) = T _m	K _{f8} = T _F / T _m	K _{f8}	8.1	= T _m	0.0648	K _{f8} 68.67	Factor 8.1	Time of Fall (mins) = T _m	K _{f8} = T _F / T _m	Value =4.45 K _{f8} 69.9
No Fall of water in hole (mm) 300 – 250 250 – 200	Factor = T _F 8.1 9.7	Time of Fall (mins) = T _m 120 125	K _{f8} = T _F / T _m 0.0675 0.0776	K _{f8} .00 65.9 57.34	8.1 9.7	= T _m 125 127	0.0648 0.0763	K _{f8} 68.67 58.32	8.1 9.7	Time of Fall (mins) = T _m 127	$K_{f8} = T_F / T_m$ 0.0637	Value =4.45 K _{f8} 69.9 58.25
No Fall of water in hole (mm) 300 – 250 250 – 200 200 – 150	Factor =T _F 8.1 9.7 11.9	Time of Fall (mins) = T _m 120 125	K _{f8} = T _F / T _m 0.0675 0.0776 0.0937 0.1093	K ₁₈ 0 65.9 57.34 47.49	8.1 9.7 11.9 14.1	125 127 129	0.0648 0.0763 0.0922 0.1076	K _{f8} 68.67 58.32 48.26	8.1 9.7 11.9	Time of Fall (mins) = T _m 127 127	K _{f8} = T _F / T _m 0.0637 0.0764 0.0908 0.1060	Value =4.45 K _{f8} 69.9 58.25 49.00
No Fall of water in hole (mm) 300 – 250 250 – 200 200 – 150 150 - 100 Average T	8.1 9.7 11.9 14.1 T – Va	Time of Fall (mins) = T _m 120 125 127	K _{f8} = T _F / T _m 0.0675 0.0776 0.0937 0.1093	K ₁₈ 65.9 57.34 47.49 40.71 52.86	8.1 9.7 11.9 14.1	= T _m 125 127 129 131 lue Hole	0.0648 0.0763 0.0922 0.1076	K _{f8} 68.67 58.32 48.26 41.35	8.1 9.7 11.9	Time of Fall (mins) = T _m 127 127 131	K _{f8} = T _F / T _m 0.0637 0.0764 0.0908 0.1060	Value =4.45 K _{f8} 69.9 58.25 49.00 41.98
No Fall of water in hole (mm) 300 – 250 250 – 200 200 – 150 150 - 100 Average T Value	Factor =T _F 8.1 9.7 11.9 14.1 T – Valest T =	Time of Fall (mins) = T _m 120 125 127 129 lue Hole	K _{f8} = T _F / T _m 0.0675 0.0776 0.0937 0.1093	K _{f8} 65.9 57.34 47.49 40.71 52.86	8.1 9.7 11.9 14.1 T – Va	= T _m 125 127 129 131 lue Hole	0.0648 0.0763 0.0922 0.1076	K _{f8} 68.67 58.32 48.26 41.35	8.1 9.7 11.9	Time of Fall (mins) = T _m 127 127 131	K _{f8} = T _F / T _m 0.0637 0.0764 0.0908 0.1060	Value =4.45 K _{f8} 69.9 58.25 49.00 41.98

3.2 (b) PERCOLATION ("P") TEST FOR SHALLOW SUBSOILS AND/OR WATER TABLE

STEP 1: Test Hole Prepa	aration		
Percolation Test Hole	1	2	2
Depth from ground surfac to top of hole (mm) (A)	e 0	0	0
Depth from ground surfact to base of hole (mm) (B)	e 400	400	400
Depth of hole (mm) [B-A]	400	400	400
Dimensions of hole [length x breadth (mm)]	300 x 320	310 x 310	350 x 340
STEP 2: Pre-soaking Tes	t holes		
Date and Time Pre-soaking Started	/05/2010 13.00	12/05/2010 13.00	12/05/2010 13.00
Each hole should be pre-soal Each hole should be empty be	ked twice before the telefore re-filling.	st is carried out.	
STEP 3: Measuring P ₁₀₀	out	opses of for	
Percolation Test Hole	1 specificante	2	3
Date of Test	13/05/2010	13/05/2010	13/05/2010
Time filled to 400mm	Coursely 0	0	0
Time water level at 300mm	55	50	49
Time to drop 100mm (P ₁₀₀)	55	50	49
Average P ₁₀₀			51.3

If $P_{100}>300$ minutes then T value >90 – site is unsuitable for discharge to ground If $P_{100}\le210$ minutes then go to STEP 4 If $P_{100}\ge210$ minutes then go to STEP 5

STEP 4: Standard	l method whe	re P ₁₀₀ ≤ 210 minutes
------------------	--------------	-----------------------------------

STEP 4: Sta	ındard ı	metho	d whe	re P ₁₀₀ :	≤ 210 m	inutes						
Percolation Test Hole	1		2			3						
No Fill No.	Start Time	Fini Tin		Δp (min)	Start Time	Ti	nish me	Δp (min)	Start Time	Ti	nish me	Δp (min)
1	0	60)	60	0	6	60	60	0		19	49
2	0	68	В	68	0	6	51	61	0		55	55
3	0	72	2	72	0	6	88	68	0		33	63
Average ∆p Value									13.93			
	Average ∆p/4= 16.7 Hole No. 1		16.7	Average ∆p/4= 15.75 Hole No. 2		Average Δp/4= Hole No. 3			13.93			
Result of Te	st P =	15.46	3	(min	/25mm)							
Comments	Relative	ely fast	percola	ation rate b structu	e throug	h uppe	r layer	s as evid	lenced t	y P va	lue ach	ieved.
	POSSIDI	y ade t	y Grustii		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	(0 0 0 0	other use.	•				
					وا	es off of any	3					
					OU STIPLE	iiie						
STEP 5: Mo	odified	Metho	d whe	re P ₁₀₀	≯ 210 m	ninutes						
Percolation Test Hole No	Relatively fast percolation rate Possibly due to crumb structure odified Method where Programme Time Time Factor of Fall Transfer Value			2			3					
Fall of water in hole (mm)	Time Factor =T _F	Time of Fall (mins) = T _m	K _{f8} = T _F / T _m	P- Value =4.45 K _{f8}	Time Factor	Time of Fall (mins) = T _m	K _{f8} = T _F / T _m	P- Value =4.45 K _{f8}	Time Factor	Time of Fall (mins) = T _m	K _{f8} = T _F / T _m	P- Value =4.45 K _{f8}
300 – 250	8.1											
250 – 200	9.7											
200 – 150	11.9											
150 - 100	14.1											
Average P Value	P-Value Hole 1= (p ₁) P-Value Hole 2= (pt ₂) P-Value Hole 3= (p ₃)											
Result of Test P = (min/25mm)												
Comments												
Commence												

Offaly County Council Environment Section

Áras an Chontae, Charleville Road, Tullamore, Co. Offaly

Tel: 057 9357403 Fax: 057 9329230 www.offaly.ie



4. CONCLUSION OF SITE CHARACTERISATION

Integrate the information from the desk study and on site assessment above and conclude the type of system(s) that is (are) appropriate. The information to choose the optimum final disposal route of the treated wastewater.

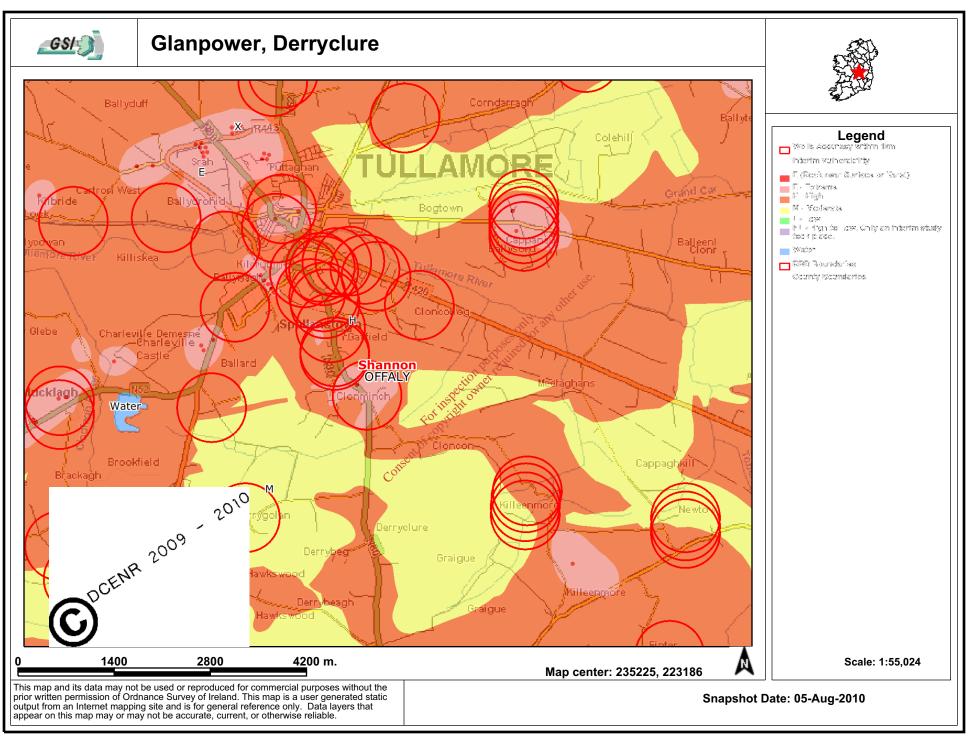
Not suitable for development	
Suitable for ¹	Discharge Route
1. Septic tank system (septic tan	, I I I I I I I I I I I I I I I I I I I
2. Secondary Treatment System	
 a. septic tank and filter sy and polishing filter; or 	/stem constructed on-site
 b. packaged wastewater polishing filter 	treatment system on site X d effluent treatment system followed by an interesting to the system of the system followed by an interesting to the system of t
5. RECOMMENDATION	or its got own
Propose to install Packaged ground so	d effluent treatment system followed by an intermittent above
and discharge to Ground.	Carte
Trench Invert level Above gro	ound level as per fig. 8.7 Page 32 EPA CoP 2009. Trench invert Il be 0.5m above ground level.
Site specific conditions	
Packaged effluent treatment syste Intermittent soil filter page 32 EPA imported material to be of appropri as EPA CoP 2009. Setback distand	em followed by intermittent soil filter above ground as per fig 8.7. CoP 2009 having trench invert level 0.5m above ground level. All interest in the last of the
3M 13/05/2010	

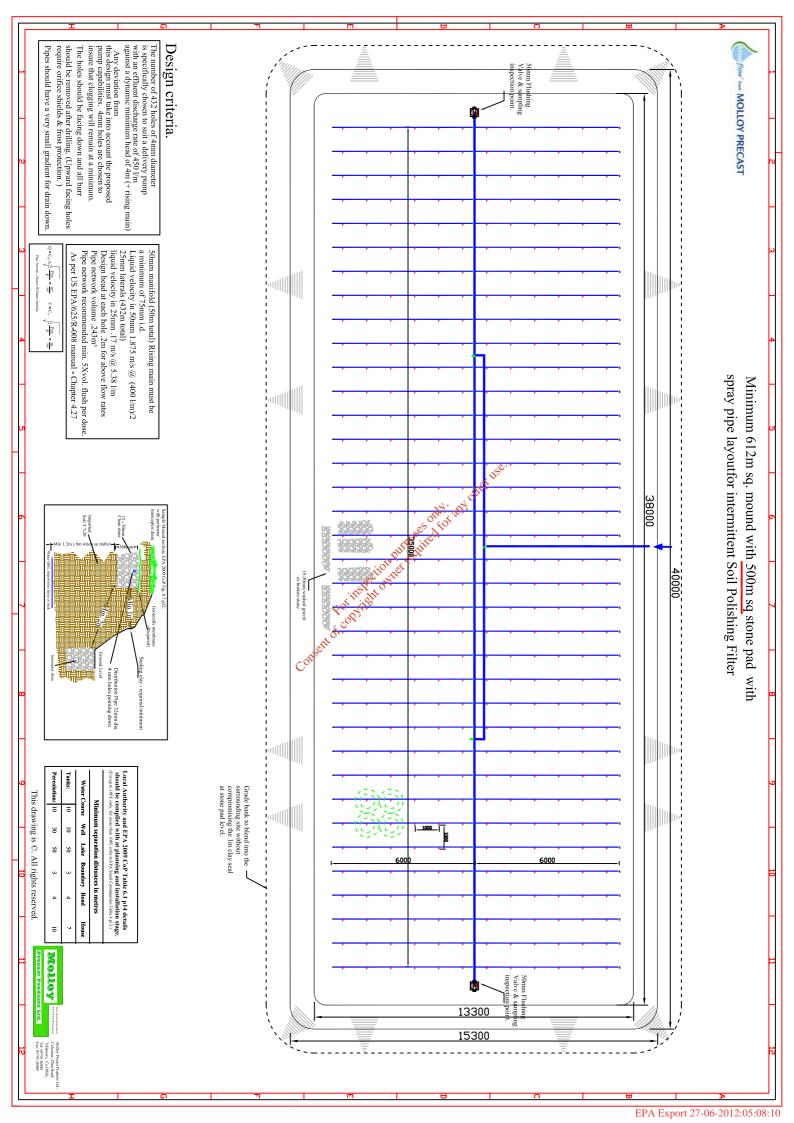
1 note: more than one option may be suitable for a site and this should be recorded

6. TREATMENT SYSTEM DETAILS

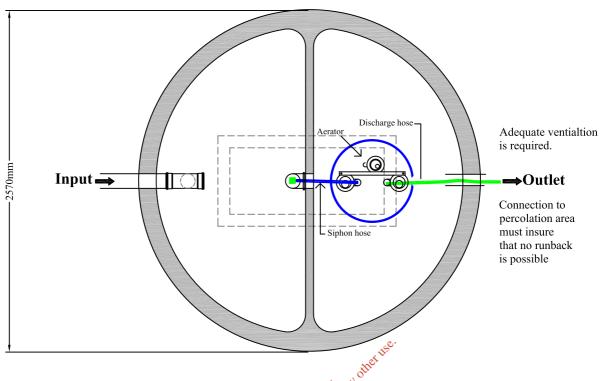
PRIOR TO SUBMISSION TO THE PLANNING SECTION OF OFFALY COUNTY COUNCIL THE TREATMENT SYSTEM DESIGN DETAILS, INCLUDING DESIGN DETAILS OF THE INFILTRATION SYSTEM PROPOSED FOR THE SITE, MUST BE COMPLETED BY A COMPETENT, SUITABLY QUALIFIED AND PROFESSIONALLY INDEMNIFIED PERSON AND IN ACCORDANCE WITH THE CODE OF PRACTICE: WASTEWATER TREATMENT & DISPOSAL SYSTEMS SERVING SINGLE HOUSES (P.E. < 10).

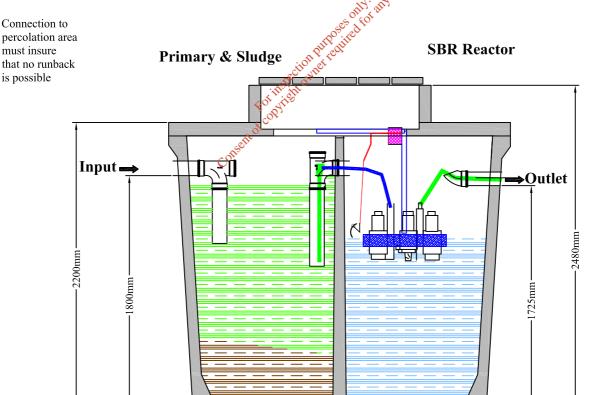
System Type: Secondary Treatment System	
Filter Systems	Package Treatment Systems
Media Area (m²) Depth of Invert Type Filter Level	Туре
Sand/soil	Aswaflow 4-12 PE S+
	Capacity PE 4 – 12 PE
	Sizing of Primary Compartment (m³)
W. and offi	3m ³
System Type: Tertiary Treatment System	
Polishing Filter: Surface Area (m²)* 500 Night Package	treatment System: Capacity (pe)
Or Gravity Fed:	
System Type: Tertiary Treatment System Polishing Filter: Surface Area (m²)* Or Gravity Fed: No. of Trenches Length of Trenches Constru	ucted Wetland: Surface Area (m²)
Length of Trenches	
Invert Level Ground	
DISCHARGE ROUTE	
	urface Discharge Rate (m³/hr)
TREATMENT STANDARDS	
Treatment System Performance Standards BOD SS	S NH ₃ Total N Total P
EPA CoP 2009 Table 5.1 page 9 20mg/ 30)mg/l 20mg/l
QUALITY ASSURANCE	
Installation and Commissioning One	going Maintenance
Molloy Precast Products Ltd., Coleraine, Clara Rd., Tullamore, Co	olloy Precast Products Ltd., bleraine, Clara Rd., Tullamore, o. Offaly. Tel : 05793 26000





Aswaflow[®] 4-12 PE, S+ tank, (7,000 Lit).





A good firm, rock free, level base is required.

This drawing is ©. All rights reserved.

E-mail: info@molloyprecast.com Web: www.molloyprecast.com Aswa flow from MOLLOY PRECAST

Coleraine, Clara Road, Tullamore, Co.Offaly. Tel: 05793 26000 Fax:05793 26060

Molloy Precast Products Ltd. Tank Type Aswaflow 4-12 PE,S tank Tank Size: 2570 Ø (Overall) Height: 2480mm (Overall) Height: 2200mm Volume: 7000 liters Weight: 6400 Kg.

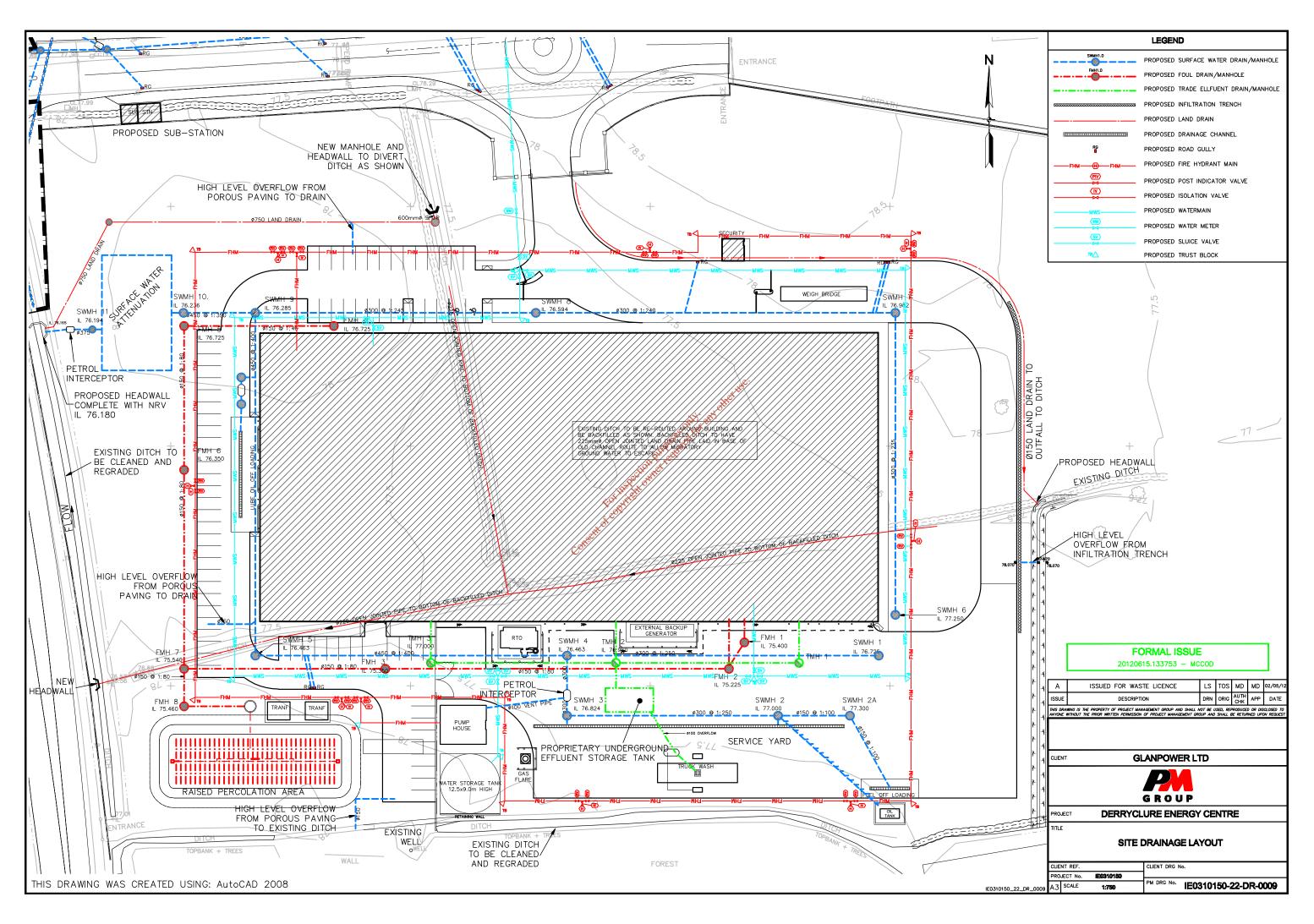
(Tank Dim: ± 25mm. Weight: ± 30Kg.)

Note: Observe all safety regulations in regard to excavation and lifting requirements. Never leave opening uncovered or unattended at any time.

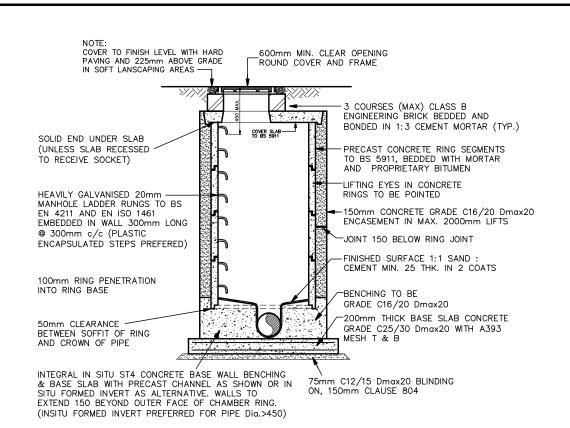
Note: Specify any specific requirements prior to ordering. All civil works by customer.

Note: Do NOT scale from drawings. Drawings for illustration purposes only.

SITE DRAINAGE LAYOUT

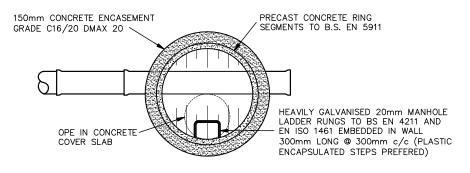


SITE DRAINAGE DETAIL

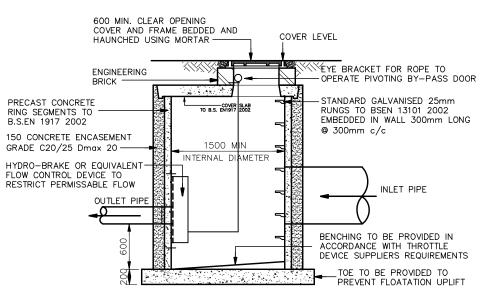


NOTE: IF BENCHING NOT FORMED WITH INTEGRAL IN — SITU CONCRETE IT WILL BE FORMED USING GRANOLITHIC RENDER

PRECAST CONCRETE MANHOLE < 3000 DEEP. < 525mm PIPES DIAMETER

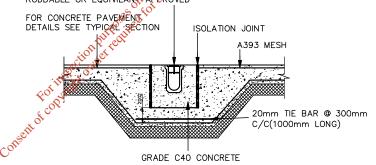


TYPICAL STRAIGHT THROUGH PRECAST CONCRETE MANHOLE

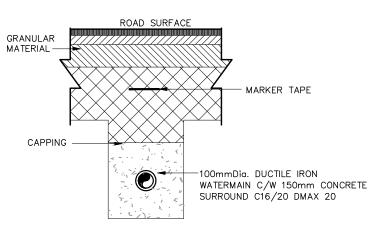


MANHOLE CHAMBER FITTED WITH HYDRO-BRAKE

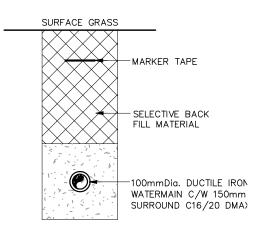
200mm WIDE POLYMER HEAVY DUTY CONCRETE DRAINAGE CHANNEL TO BS EN 1433. CONSTANT
DEPTH TYPE C/W LOCKABLE CAST DUCTILE IRON
GRATINGS TO LOAD CLASS E 600 TO BS EN 124
PROPRIETARY SUMP OUTLET TO BE TRAPPED AND
RODDABLE OR EQUIVILANT APPROVED



TYPICAL SECTION THROUGH ACO **CHANNEL AT CONCRETE YARD**



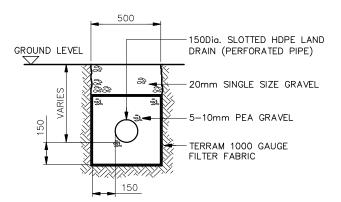
TYPICAL WATER MAIN TRENCH IN ROAD DETAIL



TYPICAL WATER MAIN TRENCH IN GRASS DETAIL

-CAST IRON COVER AND FRAME TO BS EN 124 CLASS D400 POLYPROPYLENE 150 CONCRETE INSPECTION CHAMBER ENCASEMENT BY MARLEY OR GRADE C16/20 450 Did SIMILAR APPROVED 150Dia. PIPEWORK

AJ TYPICAL DETAIL



TYPICAL INFILTRATION TRENCH/FILTER DRAIN

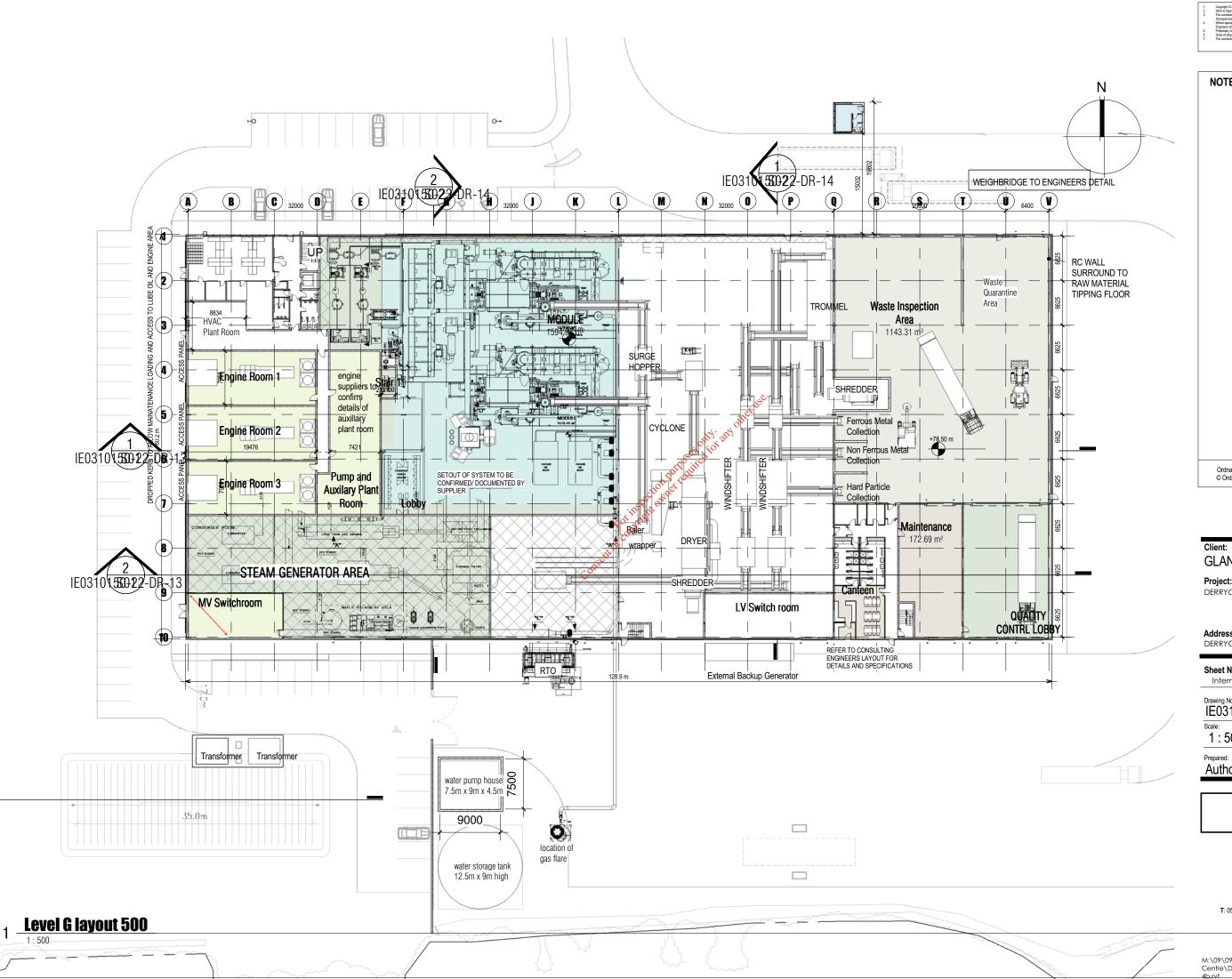
FORMAL ISSUE 20120615.133818 - MCCOD

ISSUED FOR WASTE LICENCE

LS TOS MD MD 02/05/ DESCRIPTION **GLANPOWER LTD** CLIENT GROUP **DERRYCLURE ENERGY CENTRE** ROJECT TITLE SITE DRAINAGE DETAIL CLIENT REF. PROJECT No. IE0310150 A3 SCALE IE0310150-22-DR-0010 As Shown

THIS DRAWING WAS CREATED USING: AutoCAD 2008

INTERNAL LAYOUT OF PLANT



NOTES:

Ordnance survey Ireland Licence No AR 0045810

GLANPOWER LIMITED

Project:

DERRYCLURE ENERGY PARK

Address:

DERRYCLURE, TULLAMORE, CO OFFALY.

Sheet Name:

Internal Layout of Plant

IE0310150-22-DR-0011

1:500

Date: Feb 2011

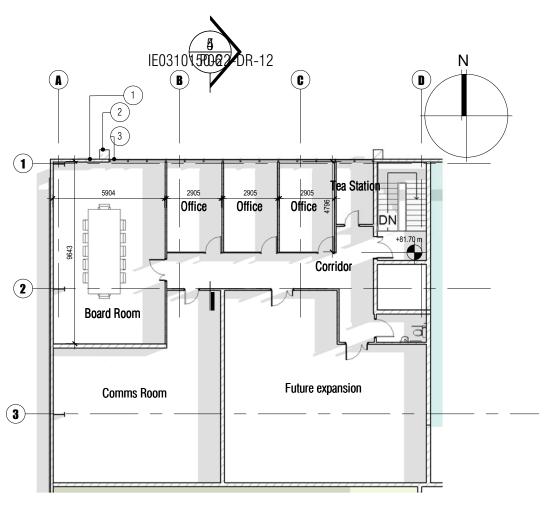
Author

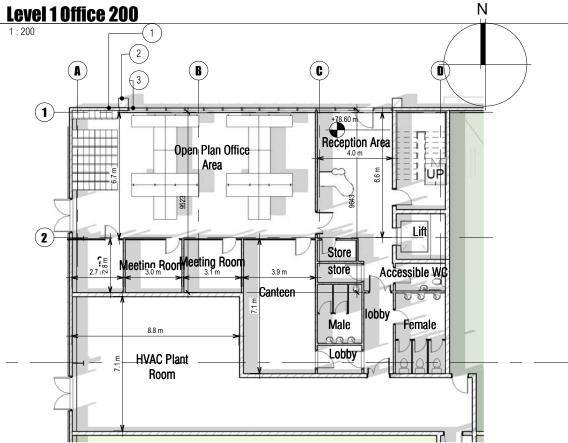
Job No: 0906

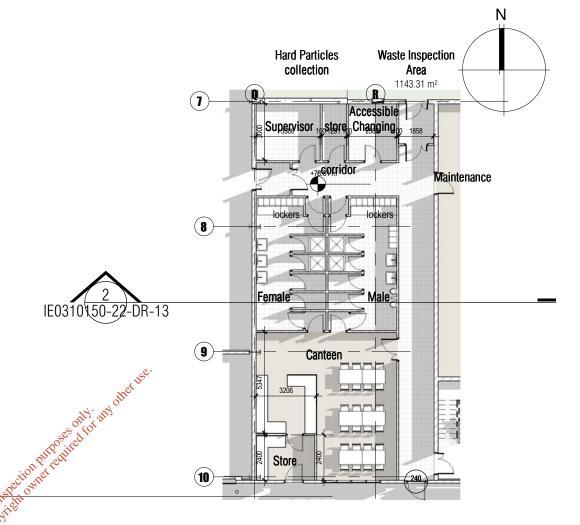
architecture
19 High Street, Tullamore, Co. Offaly.
T: 057 9329628 F: 057 9329638 E: info@axis



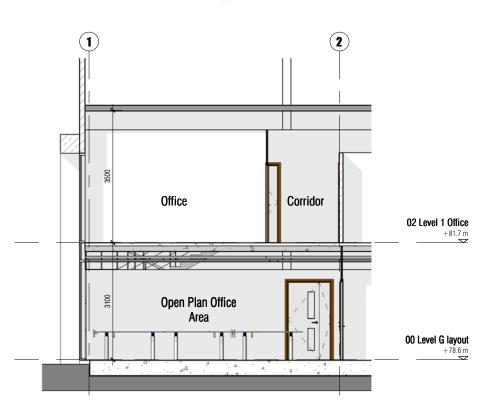
OFFICE & STAFF ACCOMMODATION







Staff Facilites (200)



Building Section E (100)

Finishes Legend

NOTES: Selected coloured microline surface with polyester coated 150mm BRUCHA insulated cladding panel laid vertically installed to manufacturers instructions and details.

- Selected colour BRUCHA architectural panel to dims indicated installed to manufacturers instructions and details.
- Aluminium framed thermally broken curtain walling with powder coated finish to selected colour.
- Selected approved signage.
- Opaque polycarbonate sheeting to create daylight provision to main areas.
- Selected coloured microline surface with polyester coated 150mm BRUCHA insulated cladding panel laid horizontally installed to manufacturers instructions and details.
- Steel doors powdercoated to colours to match cladding.
- Insulated roller shutter door.
- 10 Rendered blockwork.
- 11 Extruded aluminium blade louvre system to selected colour.
- 12 Selected colour BRUCHA roof sheeting installed to manufacturers instructions and details.
- 14 Cast insitu concrete roof to substation. 15 Substation steel doors (Refer to ESB
- specification and details)

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Client:

GLANPOWER LIMITED

Project:

DERRYCLURE ENERGY PARK

Address: DERRYCLURE, TULLAMORE, CO OFFALY.

Sheet Name:

Office & Staff accommodation

Drawing No: IE0310150-22-DR-12

As indicated

Author

architecture





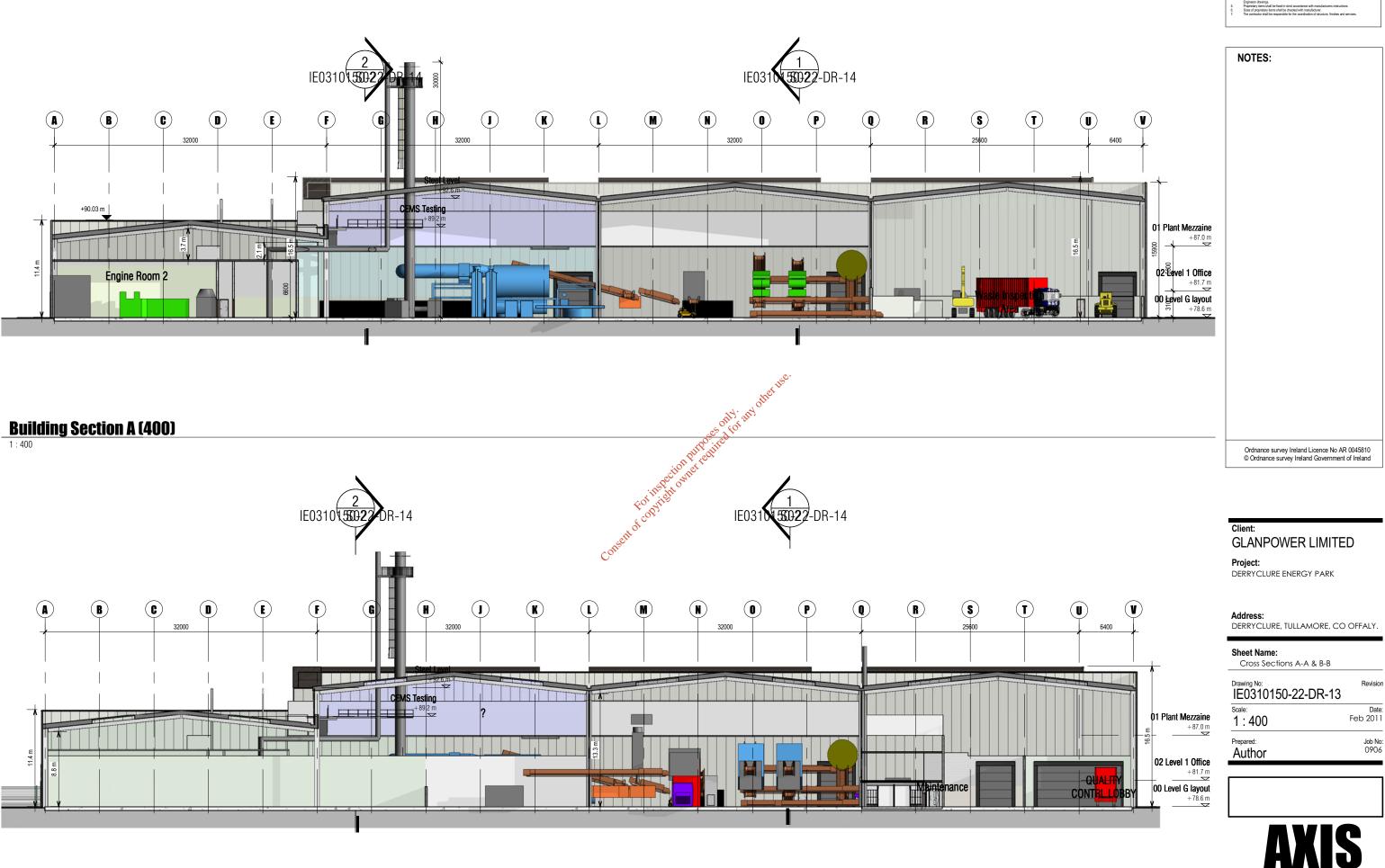
Date: Feb 2011

Job No: 0906

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2 Level G Office 200

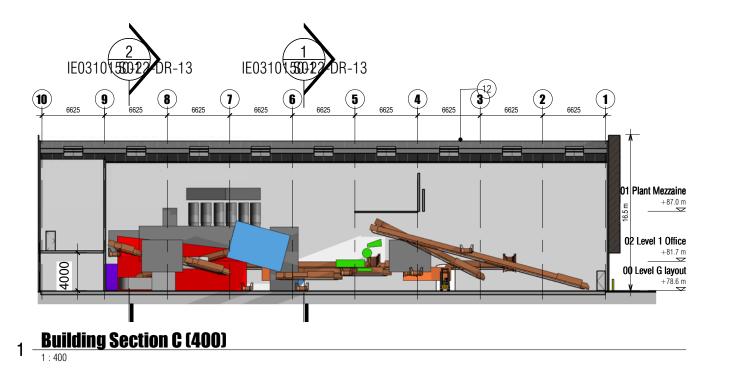
CROSS SECTIONS (A-A & B-B)

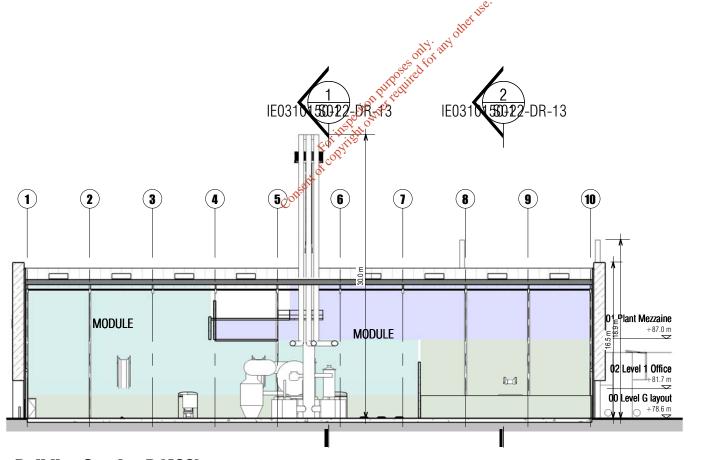






CROSS SECTIONS (C-C & D-D)





2 Building Section D (400)

NOTES:

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Client:

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Project:

DERRYCLURE ENERGY PARK

Address:

DERRYCLURE, TULLAMORE, CO OFFALY.

Sheet Name:

Cross Sections C-C & D-D

Drawing No: IE0310150-22-DR-14

1:400

Author







Date: Feb 2011

Job No: 0906