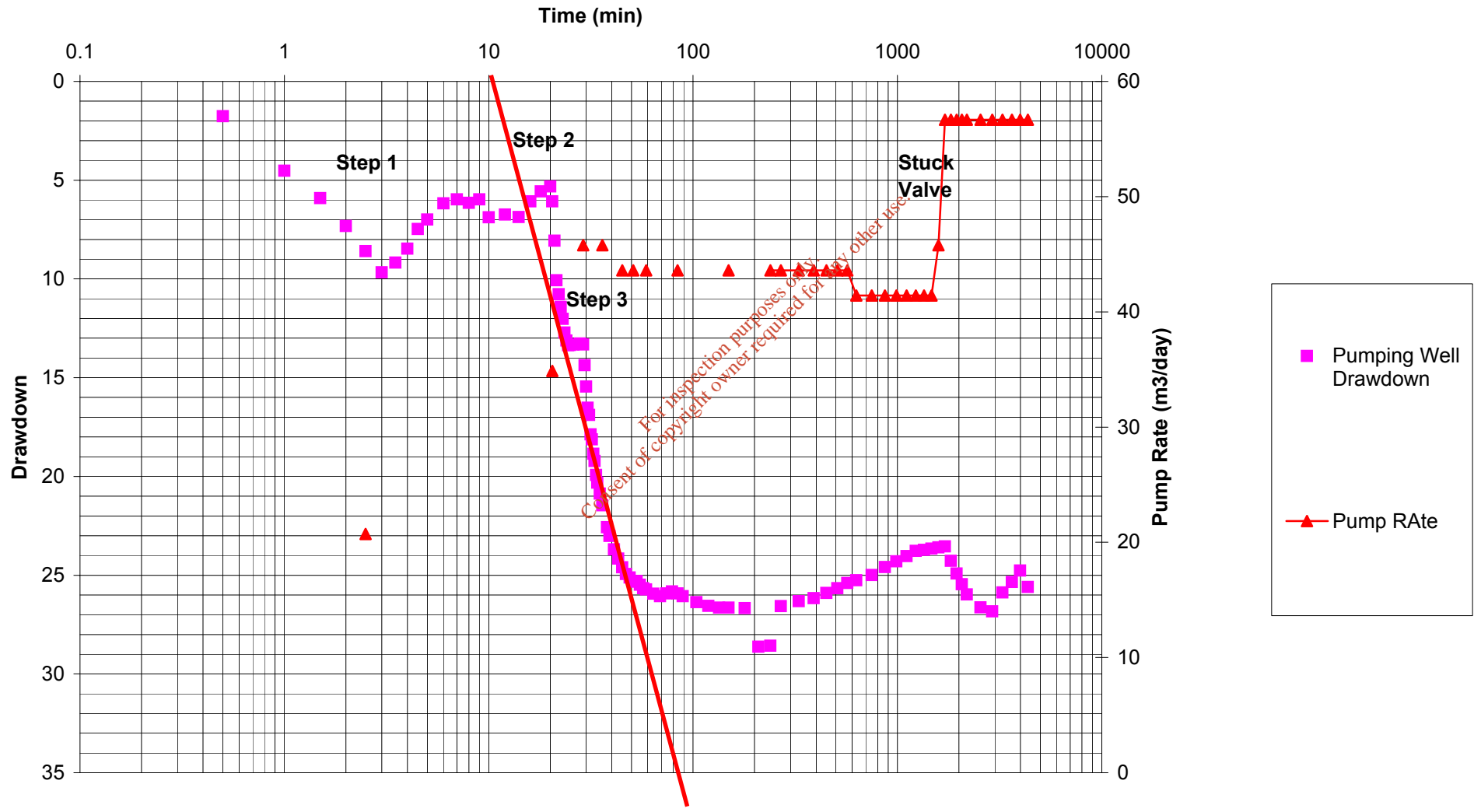


# **APPENDIX 2.4.7**

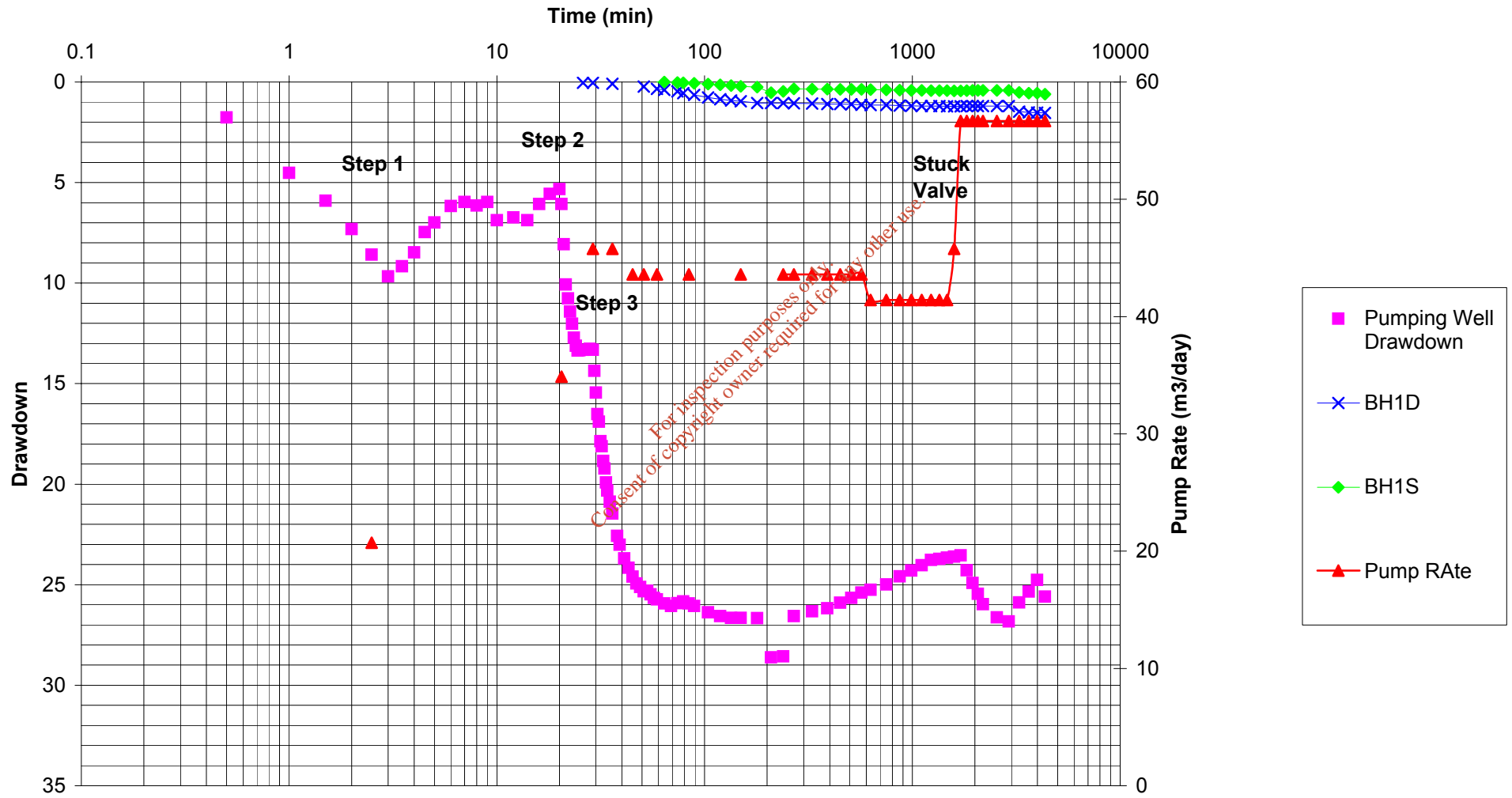
## **Pump Test Data**

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# Pumping



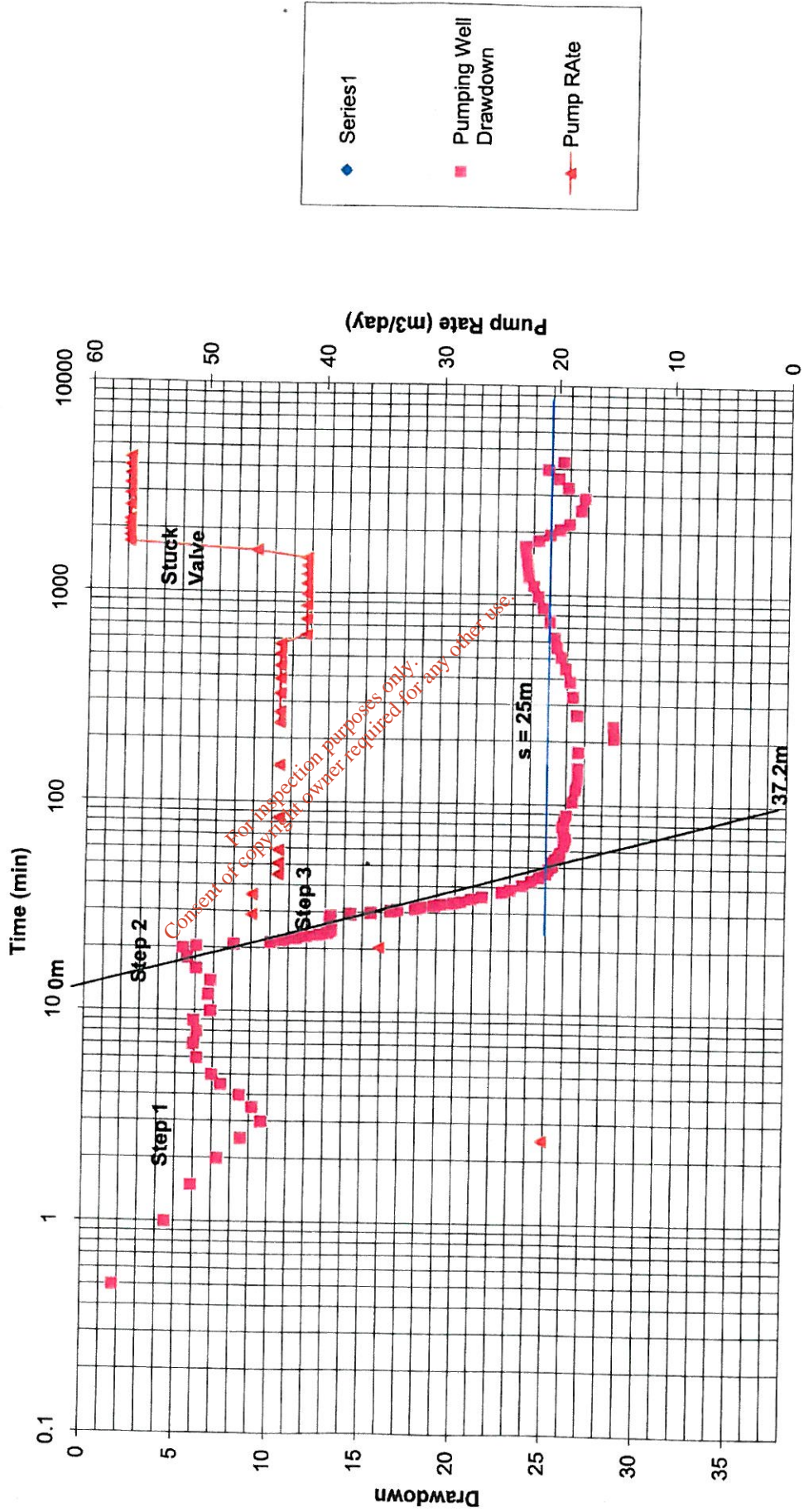
# Pumping



**APPENDIX 2.4.8**  
**Pump Test Calculation Sheets and Graphs**

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# Drawdown and Pumping Rate at Pumping Well GW6 Drehid



TES

CONSULTING ENGINEERS

Project

BNM - DREHID LANDFILL

Design

Test

Sheet No.

Revision No.

Monitor

Date

Event: TES - GW6 PUMP TEST

Interpreting Log

Prepared

Checked

Project No.

1131

DRAWDOWN T-CALCS

Other Reports

KM

Reference

JACOB STRAIGHT LINE FORMULA

$$T = \frac{2.3Q}{4\pi \Delta s}$$

Q = PUMPING RATE (m<sup>3</sup>/d)

Δs = DRAWDOWN PER LOG CYCLE (m)

$$Q = 43.6 \text{ m}^3/\text{d}$$

$$\Delta s_{10-100} = 37.2 \text{ m}$$

10-100  
MINUTES

$$T = \frac{2.3 \times 43.6 \text{ m}^3/\text{d}}{4\pi \times 37.2 \text{ m}} = 0.215 \text{ m}^2/\text{d}$$

LOGAN APPROXIMATION CALCULATION

$$T = \frac{1.22 Q}{s}$$

Q = PUMPING RATE (m<sup>3</sup>/d)

s = DRAWDOWN IN WELL (m)

s varies between 23m and 27m but averages at 25m s = 25m

$$Q = 48.5 \text{ m}^3/\text{d}$$

$$T = \frac{1.22 \times 48.5 \text{ m}^3/\text{d}}{25 \text{ m}} = 2.37 \text{ m}^2/\text{d}$$

RESULTS:

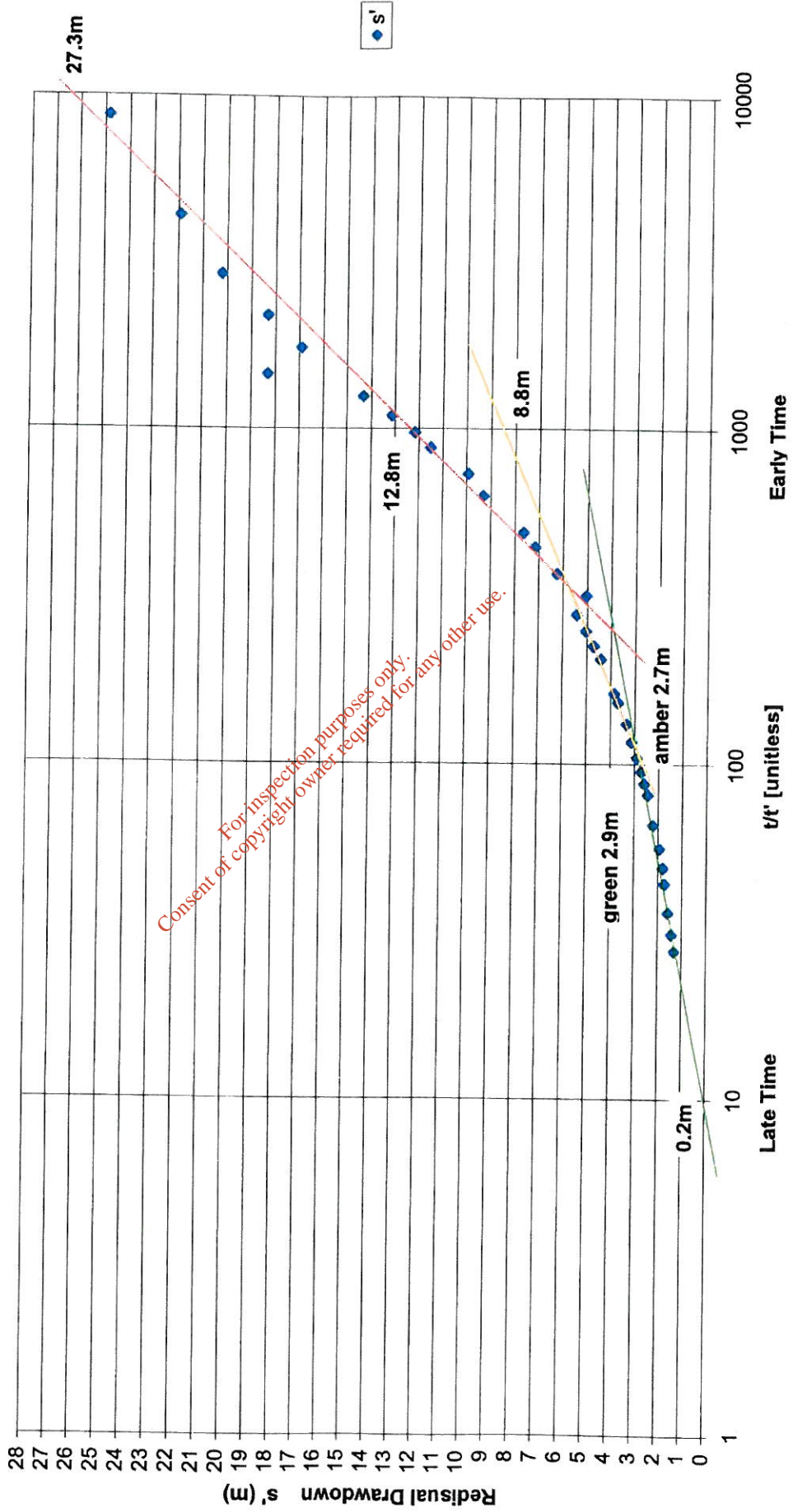
JACOB TRANSMISSIVITY 10-100 minutes

$$T = 0.215 \text{ m}^2/\text{d}$$

LOGAN APPROXIMATION TRANSMISSIVITY 100-4349 minutes

$$T = 2.37 \text{ m}^2/\text{d}$$

Recovery Data plot for GW6PW Drehid  
 Showing Jacob Slopes used for Calculations





TES

CONSULTING ENGINEERS

Project

BNN - DREHID LANDFILL

Design

Title

Sheet No. 1 of 2

Minutes

✓

Revision No.

Client: TES - GWS PUMP TEST

Telephone Log

Date

Project No.

1131

RECOVERY T-CALCS

Other Record

Prepared

KM

Checked

Reference

Output/Action

JACOB STRAIGHT LINE RECOVERY FORMULA

$$T = \frac{2.3Q}{4\pi \times \Delta s'}$$

 $\Delta s' =$  CHANGE IN RESIDUAL DRAWDOWN PER LOG CYCLE (m)

 $Q = \bar{Q}$  AVERAGE PUMP RATE FOR TEST ( $m^3/d$ )

$$= 148.40 m^3 \text{ PUMPED OVER 4349 MINUTES}$$

$$= 49.14 m^3/d \quad (72 \text{ hours} = 4,320 \text{ minutes})$$

LATE TIME

$$\Delta s' = 2.9m - 0.2m = 2.7m$$

$$T = \frac{2.3 \times 49.14 m^3/d}{4\pi \times 2.7m} = 3.33 m^2/d$$

MID TIME

$$\Delta s' = 8.8m - 2.7m = 6.1m$$

$$T = \frac{2.3 \times 49.14 m^3/d}{4\pi \times 6.1m} = 1.47 m^2/d$$

EARLY TIME

$$\Delta s' = 27.3m - 12.8m = 14.5m$$

$$T = \frac{2.3 \times 49.14 m^3/d}{4\pi \times 14.5m} = 0.62 m^2/d$$

RESULTS USING  $\bar{Q} = 49.14 m^3/d$ 

$$\text{LATE TIME} \quad T = 3.33 m^2/d$$

$$\text{MID TIME} \quad T = 1.47 m^2/d$$

$$\text{EARLY TIME} \quad T = 0.62 m^2/d$$

$$\text{MEAN} \quad T = 1.81 m^2/d$$



TES

CONSULTING ENGINEERS

Project

BNM - DREHID LANDFILL

Design

Minutes

Tick

Reason for

Client

TES - GWS PUMP TEST

Telephone Log

Date

Prepared

Checked

Time Log

1131

RECOVERY T-CALLS

Date Record

HAM

Description

$$\text{IF } Q_{\text{MAX}} = 56.68 \text{ m}^3/\text{d}$$

LATE  
TIME

$$\Delta S' = 2.7 \text{ m}$$

$$T = \frac{2.3 \times 56.68 \text{ m}^3/\text{d}}{4\pi \times 2.7 \text{ m}} = 3.84 \text{ m}^2/\text{d}$$

MID  
TIME

$$\Delta S' = 6.1 \text{ m}$$

$$T = \frac{2.3 \times 56.68 \text{ m}^3/\text{d}}{4\pi \times 6.1 \text{ m}} = 1.70 \text{ m}^2/\text{d}$$

EARLY  
TIME

$$\Delta S' = 14.5 \text{ m}$$

$$T = \frac{2.3 \times 56.68 \text{ m}^3/\text{d}}{4\pi \times 14.5 \text{ m}} = 0.72 \text{ m}^2/\text{d}$$

RESULTS USING  $Q_{\text{MAX}} = 56.68 \text{ m}^3/\text{d}$

LATE TIME  $T = 3.84 \text{ m}^2/\text{d}$

MID TIME  $T = 1.70 \text{ m}^2/\text{d}$

EARLY TIME  $T = 0.72 \text{ m}^2/\text{d}$

MEAN  $T = 2.09 \text{ m}^2/\text{d}$

**TES**

CONSULTING ENGINEERS

Project No: 1131

Project

BNM - DREHID LANDFILL

Element

ASLIBEKIAN T-CALC

Design

Minutes

Telephone Log

Other Record

Task

✓

Sheet No:

Revision No:

Date:

Project:

KM

Reference:

$$T = \frac{q \times \ln \left( \frac{1}{r} \times \sqrt{\frac{Q}{\pi E}} \right)}{2\pi}$$

$$q = \text{SPECIFIC CAPACITY (m}^3/\text{d/m)} = \frac{48.5 \text{ m}^3/\text{d}}{25 \text{ m}} = 1.94 \text{ m}^3/\text{d/m}$$

$$r = \text{RADIUS OF BOREHOLE TUBE (m)} = 0.100 \text{ m}$$

$$Q = \text{PUMPING RATE (m}^3/\text{d)} = 48.5 \text{ m}^3/\text{d}$$

$$E = \text{RECHARGE RATE (m/d/m}^2) = 5 \times 10^{-4} \text{ m/d/m}^2$$

\* stated as ESTIMATED RECHARGE RATE FOR MIDLAND AQUIFER  
IN PAPER - TAKEN FROM SEVERAL STUDIES.

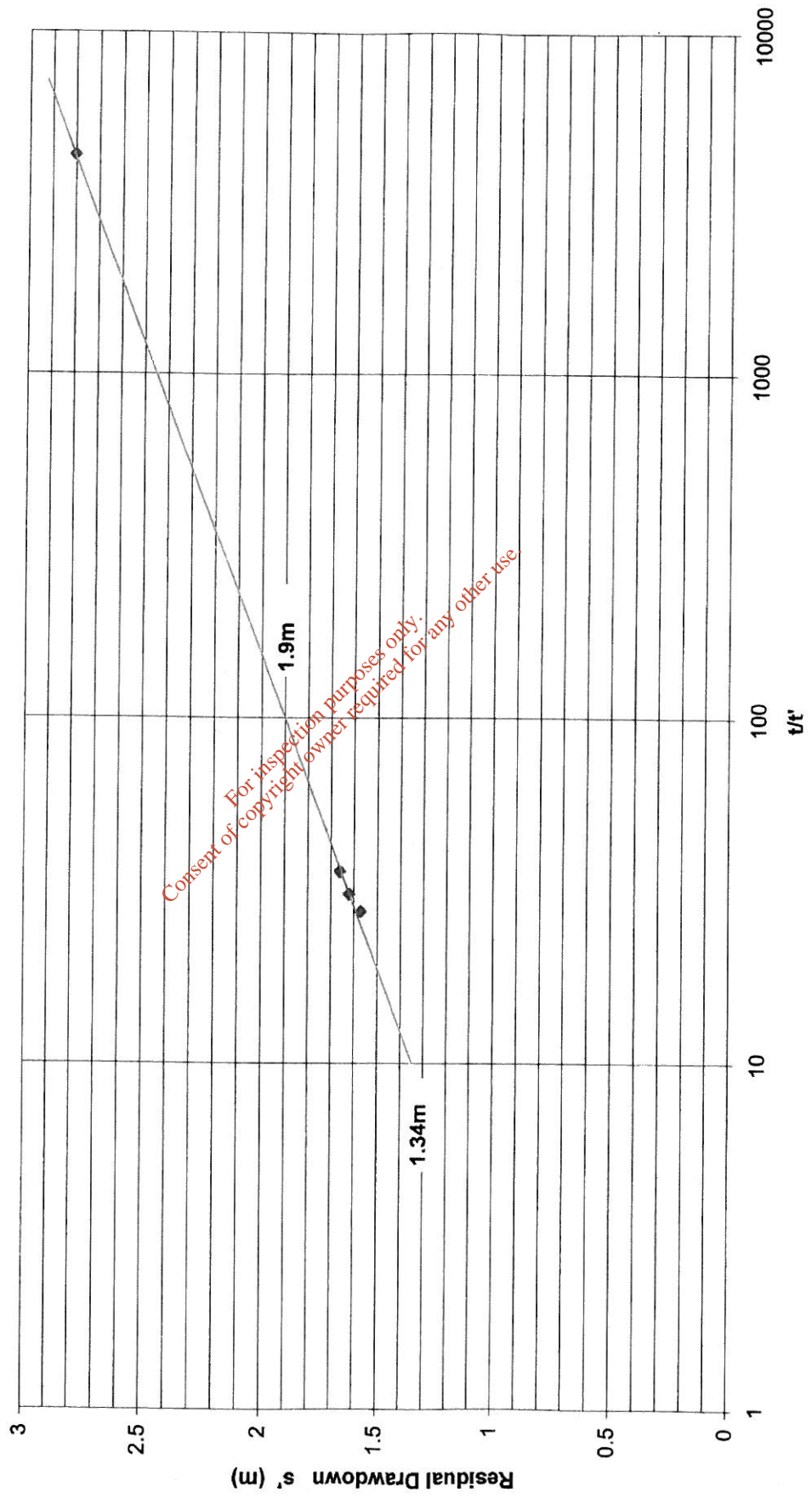
$$T = \frac{1.94 \text{ m}^3/\text{d/m} \times \ln \left( \frac{1}{0.1} \times \sqrt{\frac{48.5 \text{ m}^3/\text{d}}{\pi \times 5 \times 10^{-4} \text{ m/d/m}^2}} \right)}{2\pi}$$

$$= \frac{1.94 \text{ m}^3/\text{d/m} \times \ln (1.7572 \times 10^3)}{2\pi}$$

$$= \frac{1.94 \text{ m}^3/\text{d/m} \times 7.472}{2\pi}$$

$$T = 2.31 \text{ m}^2/\text{d}$$

**Recovery Test data at BH1D  
Showing Jacob Slope used for Calculations**



**TES**

CONSULTING ENGINEERS

Project No: 1131

Project: BNM - DREKID LANDFILL  
 Method: TES - GWS PUMP TEST  
 File No: SW10 RECOVERY T-CALC

Design	✓	Sheet No.	1 of 1
Minutes		Revised to	
Telephone Log		Date	
Other Record		Prepared	Checked

Reference

JACOBS STRAIGHT LINE RECOVERY FORMULA

$$T = \frac{2.3Q}{4\pi \Delta s'}$$

$\Delta s'$  = CHANGE IN RESIDUAL DRAWDOWN PER LOG CYCLE (m)

$Q = \bar{Q}$ , AVERAGE PUMPING RATE FOR TEST ( $m^3/d$ )  
 148.4  $m^3$  OVER 4349 MINUTES  
 (72 hours = 4,320 minutes)  
 = 49.14  $m^3/d$

$Q_{max}$  = MAXIMUM PUMPING RATE DURING TEST ( $m^3/d$ )

$Q_{max} = 56.68 m^3/d$   $\Delta s (9m - 1.34m) = 0.56m$

$\bar{Q}$   $T = \frac{2.3 \times 49.14 m^3/d}{4\pi \times 0.56m} = 16.06 m^2/d$

$Q_{max}$   $T = \frac{2.3 \times 56.68 m^3/d}{4\pi \times 0.56} = 18.53 m^2/d$

RESULTS

$\bar{Q}$   $T = 16.06 m^2/d$

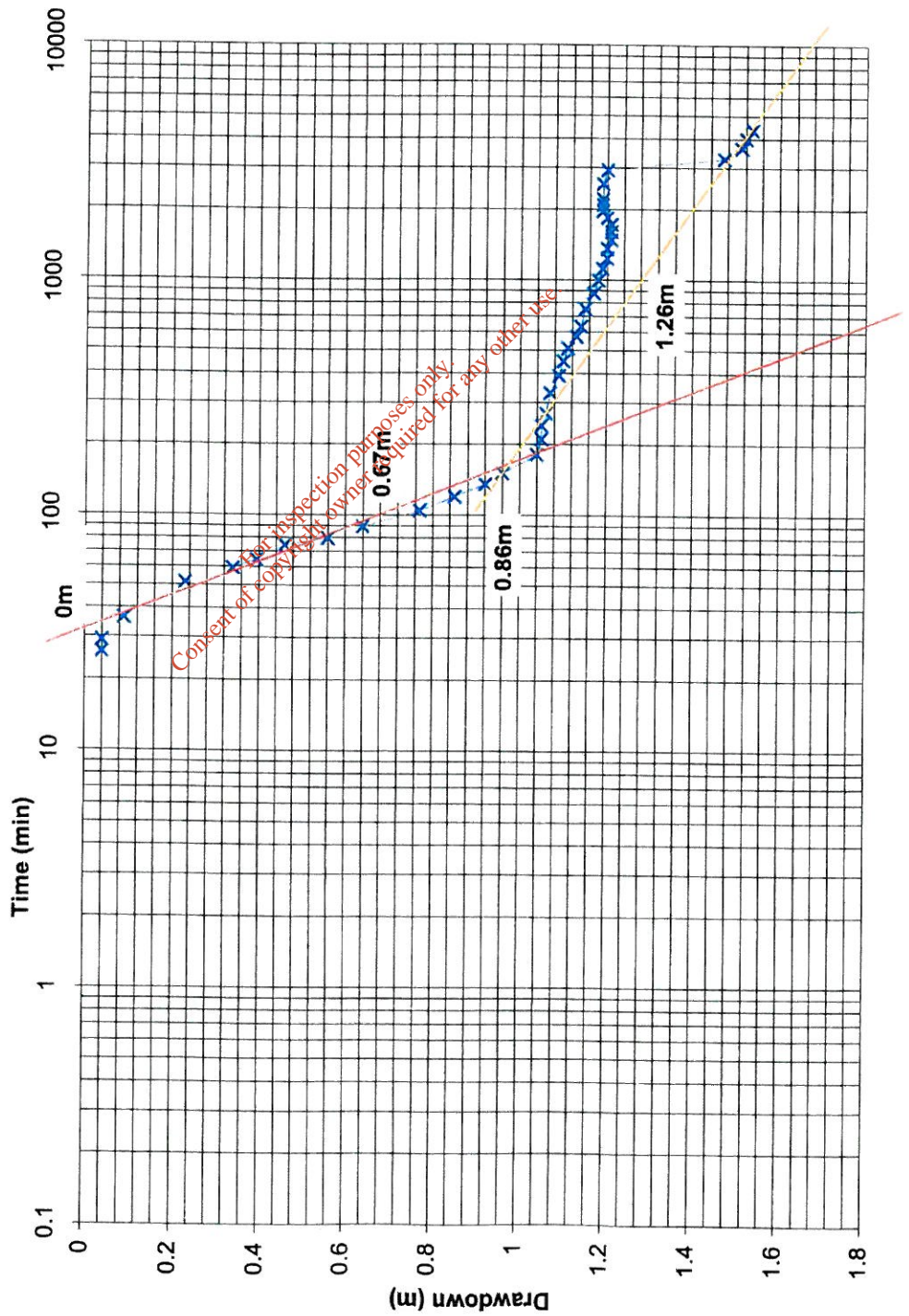
$Q_{max}$   $T = 18.53 m^2/d$

MEAN  $T = 17.3 m^2/d$

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# Drawdown Data at GW1D Showing Jacob Slopes used for Calculations



**TES**

CONSULTING ENGINEERS

Project No

1131

Project

TES-GW Pump Test  
GWID-DRAWDOWN T-TEST

Design

Minutes

Telephone Log

Other Record

101

Sheet No

Revision No

Date

Prepared

Checked

Reference

JACOBS STRAIGHT LINE FORMULA

$$T = \frac{2.3Q}{4\pi \Delta s}$$

Q = PUMPING RATE (m<sup>3</sup>/d)  
Δs = DRAWDOWN PER LOG CYCLE

EARLY TIME

10-100 minutes

$$Q = 43.6 \text{ m}^3/\text{d}$$

$$\Delta s = 0\text{m} - 0.67\text{m} = 0.67\text{m}$$

(slope line intersects zero drawdown at 21 minutes, use s=0 as negative value would not make sense)

$$T = \frac{2.3 \times 43.6 \text{ m}^3/\text{d}}{4\pi \times 0.67\text{m}} = 11.9 \text{ m}^2/\text{d}$$

LATE TIME

$$Q = 48.5 \text{ m}^3/\text{d}$$

$$\Delta s = 1.26\text{m} - 0.86\text{m} = 0.4\text{m}$$

$$T = \frac{2.3 \times 48.5 \text{ m}^3/\text{d}}{4\pi \times 0.4} = 22.19 \text{ m}^2/\text{d}$$

RESULTS

EARLY TIME  $T = 11.9 \text{ m}^2/\text{d}$

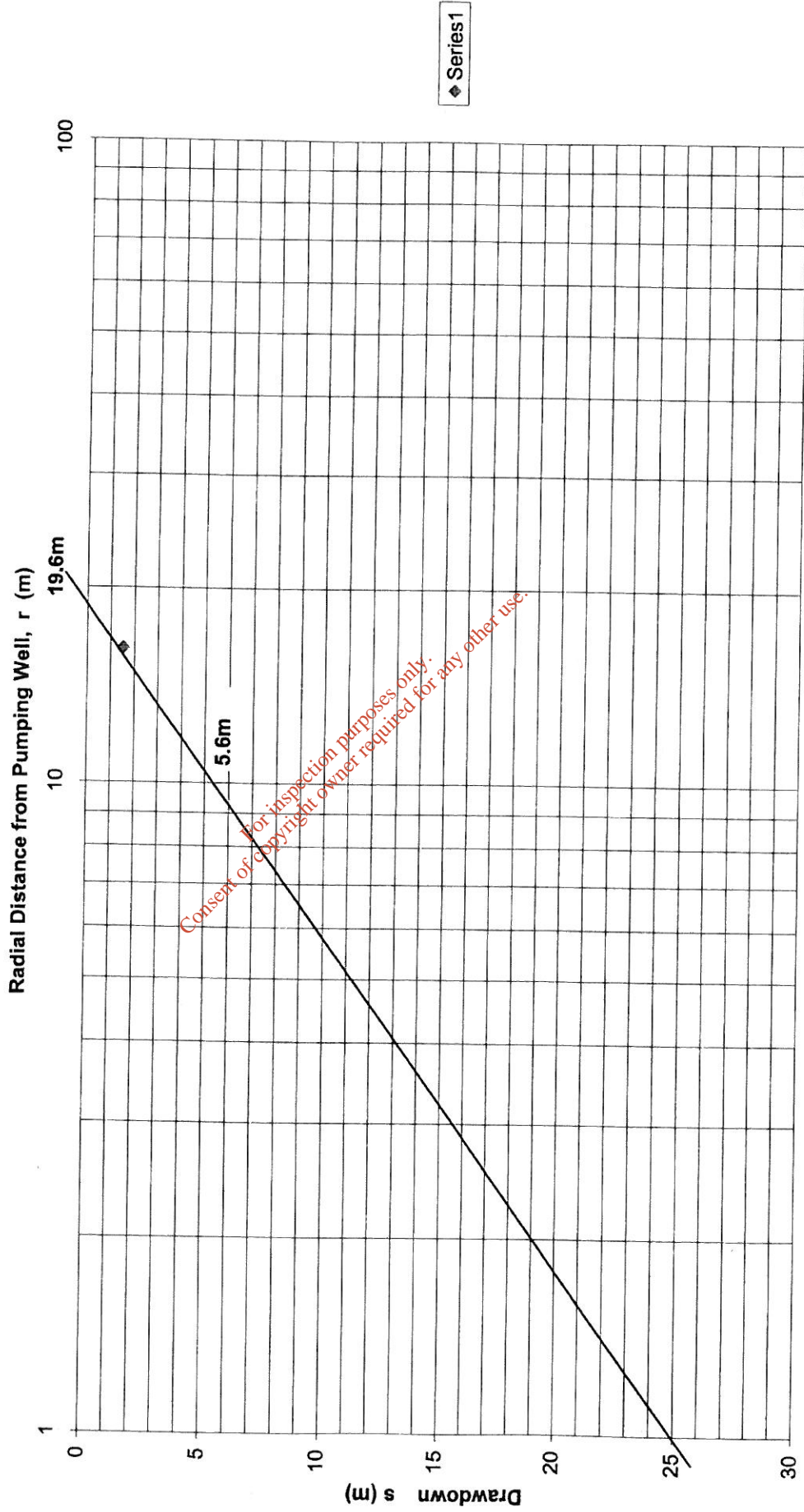
LATE TIME  $T = 22.19 \text{ m}^2/\text{d}$

MEAN  $T = 17.05 \text{ m}^2/\text{d}$

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# Zero Drawdown analysis using GW6 & GW1D



Project Sheet:

<b>TES</b> CONSULTING ENGINEERS	Project	Design	Sheet No.	
	BVM - DREHIO LANDFILL	Monitor	Revision No.	
Project No.	Comment	Telephone Eng.	Prepared	Checked
1131	DISTANCE DRAWDOWN ANALYSIS GW6 + GW1D	Other Person		

JACOBS DISTANCE DRAWDOWN

STEADY S IN GW6 = 25m

$\bar{Q} = 49.14 \text{ m}^3/\text{d}$

$Q_{\text{MAX}} = 56.68 \text{ m}^3/\text{d}$

$T = \frac{2.3Q}{2\pi \Delta s}$  DISTANCE DRAWDOWN FORMULA

$\Delta s$  = DRAWDOWN PER LOG CYCLE

$\Delta s_{-10} = 25.0 \text{ m} - 5.6 \text{ m} = 19.4 \text{ m}$

$\bar{Q} \quad T = \frac{2.3 \times 49.14 \text{ m}^3/\text{d}}{2\pi \times 19.4 \text{ m}} = 0.927 \text{ m}^2/\text{d}$

$Q_{\text{MAX}} \quad T = \frac{2.3 \times 56.68 \text{ m}^3/\text{d}}{2\pi \times 19.4} = 1.069 \text{ m}^2/\text{d}$

RESULTS

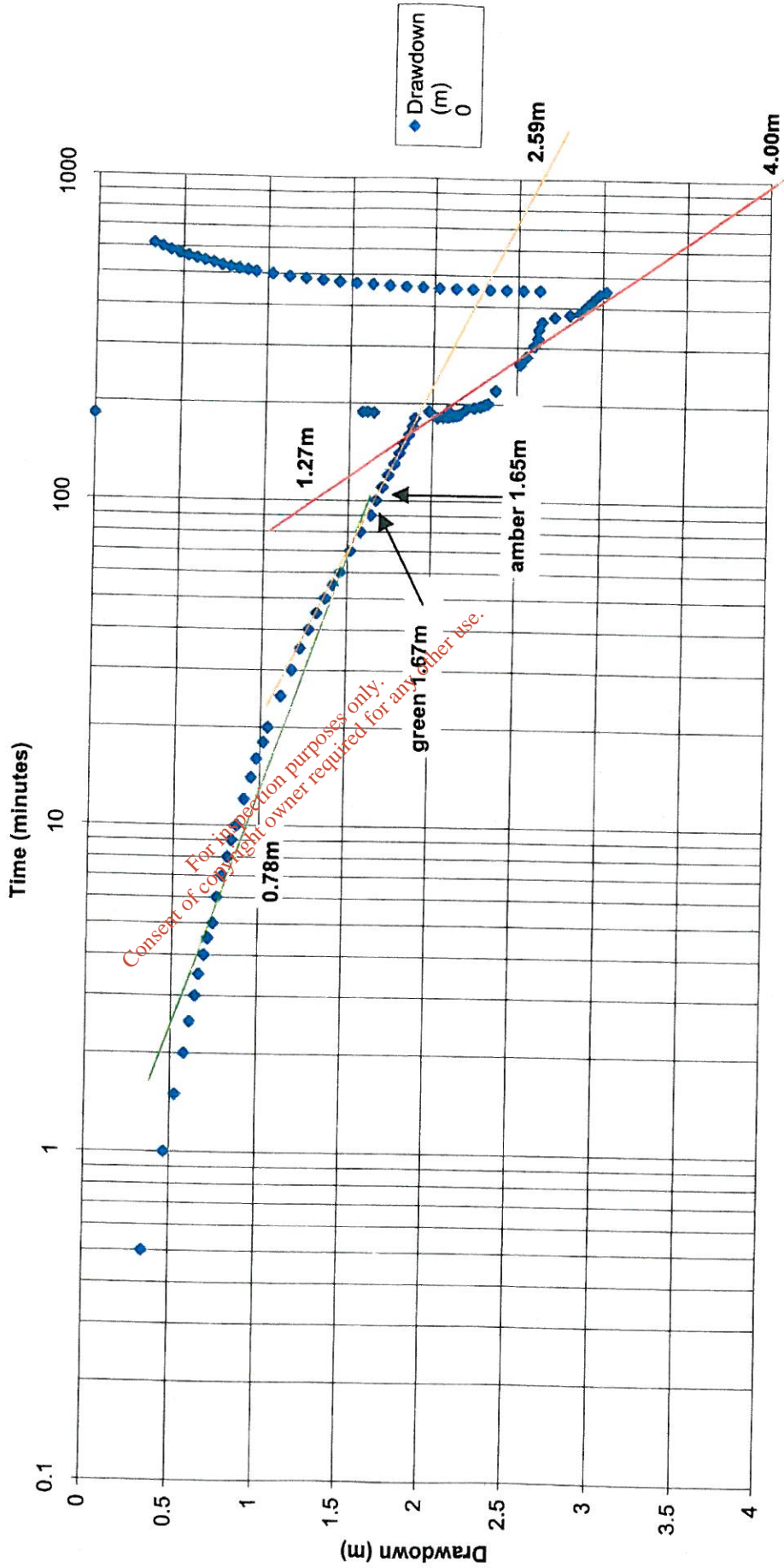
$T = 0.927 \text{ m}^2/\text{d}$

$T = 1.069 \text{ m}^2/\text{d}$

MEAN  $T = 0.998 \text{ m}^2/\text{d}$

$T = 1 \text{ m}^2/\text{d}$

Pump Test Drawdown & Recovery Plot FTC Borehole BH9 Drehid  
TES PLOT



Project Sheet:

**TES**

CONSULTING ENGINEERS

Project No: 1131

Project: BNM - DREHID LANDFILL

Discipline:

Electrical

Task	Sheet No. ( of )
✓	Revision No.
	Date
Completed Log	Prepared
Other Record	Checked
	KM

Element: FTC - BH9 PUMP TEST  
DRAWDOWN DATA T-CALCS

JACOB STRAIGHT LINE FORMULA

$$T = \frac{2.3Q}{4\pi \Delta S} \quad Q = \text{PUMPING RATE (m}^3/\text{d)}$$

$$\Delta S = \text{DRAWDOWN PER LOG CYCLE (m)}$$

①  
EARLY  
TIME

$$Q = 136.12 \text{ m}^3/\text{d}$$

$$\Delta S_{10-100} = 1.57 \text{ m} - 0.78 \text{ m} = 0.87 \text{ m} \quad (\text{FROM GRAPH})$$

$$T = \frac{2.3 \times 136.12 \text{ m}^3/\text{d}}{4\pi \times 0.87 \text{ m}} = 28.64 \text{ m}^2/\text{d}$$

②  
MID  
TIME

$$Q = 170.24 \text{ m}^3/\text{d}$$

$$\Delta S_{10-100} = 2.59 \text{ m} - 1.65 \text{ m} = 0.94 \text{ m} \quad (\text{FROM GRAPH})$$

$$T = \frac{2.3 \times 170.24 \text{ m}^3/\text{d}}{4\pi \times 0.94} = 33.15 \text{ m}^2/\text{d}$$

③  
LATE  
TIME

$$Q = 170.24 \text{ m}^3/\text{d}$$

$$\Delta S_{100-1000} = 4.00 \text{ m} - 1.27 \text{ m} = 2.73 \text{ m} \quad (\text{FROM GRAPH})$$

$$T = \frac{2.3 \times 170.24 \text{ m}^3/\text{d}}{4\pi \times 2.73} = 11.41 \text{ m}^2/\text{d}$$

RESULTS:

EARLY TIME  $T = 28.64 \text{ m}^2/\text{d}$

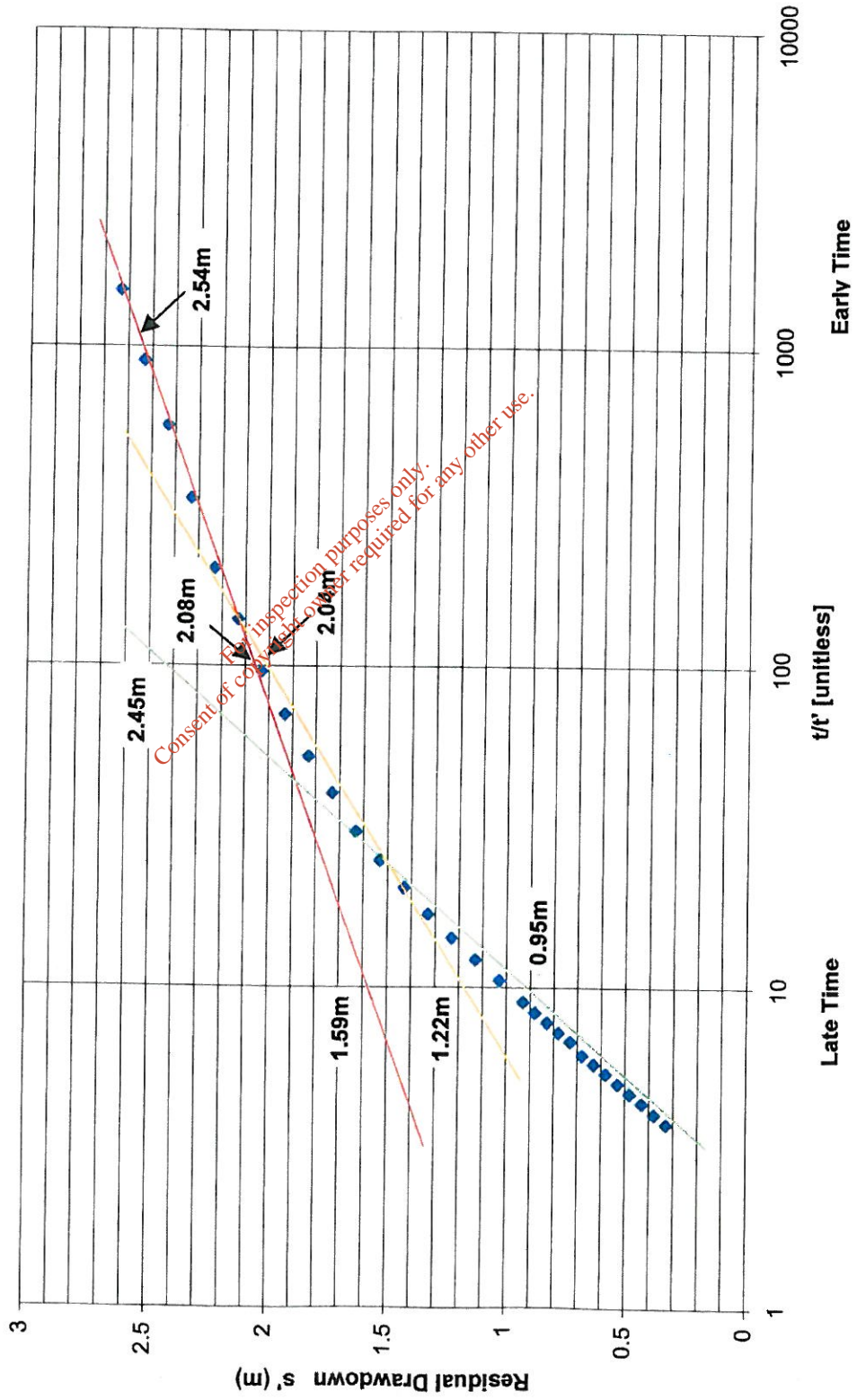
MID TIME  $T = 33.15 \text{ m}^2/\text{d}$

LATE TIME  $T = 11.41 \text{ m}^2/\text{d}$

MEAN  $T = 24.40 \text{ m}^2/\text{d}$



Recovery Data Plot FTC Borehole BH9 Drehid Showing Jacon Slopes used for Calculations  
TES PLOT



6.19  
93.81

**TES**

CONSULTING ENGINEERS

Project No: 1131

Project: BNM-DRENID LANDFILL

Elaboration: FTC-B49 PUMPTEST  
RECOVERY DATA T-CALC

Design	Sheet No. of
Minutes	Revision No.
Telephone Log	Date
Other Record	Proposed
	Checked

KM

JACOBS STRAIGHT LINE RECOVERY FORMULA

$$T = \frac{2.3Q}{4\pi \times \Delta s'}$$

$\Delta s'$  = change in residual drawdown (s') per log cycle (m)

$Q$  =  $\bar{Q}$  (AVERAGE PUMP RATE FOR TEST) ( $m^3/d$ )

48,145L over 150 minutes pumping

$$\frac{48,145L}{150min} = 106.99 \text{ l/min} = 154.06 \text{ m}^3/d$$

(FTC DATA SHEET)

LATE TIME

$$\Delta s' = 2.45m - 0.95m = 1.5m$$

$$T = \frac{2.3 \times 154.06 \text{ m}^3/d}{4\pi \times 1.5m} = 18.79 \text{ m}^2/d$$

MID TIME

$$\Delta s' = 2.04m - 1.22m = 0.82m$$

$$T = \frac{2.3 \times 154.06 \text{ m}^3/d}{4\pi \times 0.82} = 34.40 \text{ m}^2/d$$

EARLY TIME

$$\Delta s' = 2.64m - 2.08m = 0.46m$$

$$T = \frac{2.3 \times 154.06}{4\pi \times 0.46} = 61.3 \text{ m}^2/d$$

RESULTS:

EARLY TIME  $T = 61.3 \text{ m}^2/d$

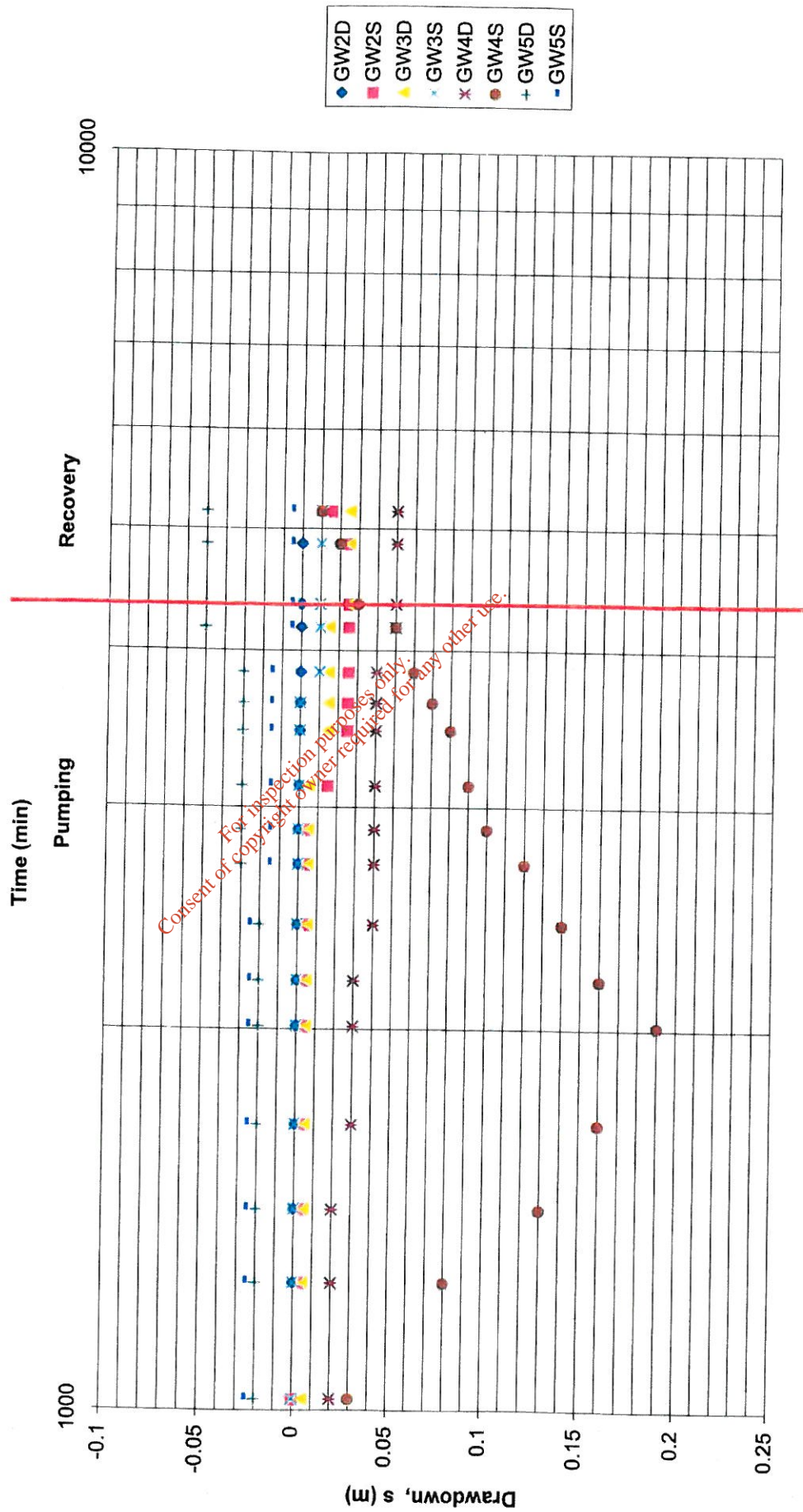
MID TIME  $T = 34.40 \text{ m}^2/d$

LATE TIME  $T = 18.79 \text{ m}^2/d$

MEAN  $T = 38.16$



# Drawdown at Piezometers Drehhid Pump Test



**APPENDIX 2.4.9**  
**Calculation Sheets for estimation of Bedrock**  
**Permeability**

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Project Sheet:

**TES**

CONSULTING ENGINEERS

Project No: 1131

Client: BNM - DRENID LANDFILL

Project: BEPROCK K-ESTIMATION

Sheet No: 1 of 2

Revision No:

Date:

Prepared:

KM

Checked:

Reference:

Suppl/Action:

ESTIMATION OF PERMEABILITY FROM TRANSMISSIVITY VALUES.

TRANSMISSIVITY RANGE 2 - 16 m<sup>2</sup>/d

$$T = KD \quad \text{OR} \quad K = \frac{T}{D}$$

K = PERMEABILITY (m/day)

D = THICKNESS OF THE AQUIFER (m)

LENGTH OF WELL SCREEN = 8.8m

DEPTH OF WATER IN BOREHOLE

GROUND LEVEL = 84.737 MOD

BASE OF BOREHOLE (84.737 - 36.8) = 47.94 MOD

SWL 14/103 = 83.782 MOD

BASE OF BOREHOLE = 47.94 MOD

WATER COLUMN = 35.84m (SATURATED THICKNESS)

POSSIBLE VALUES FOR D

Length of Screen = 8.8m

WATER COLUMN IN BOREHOLE = 35.84m

OR

UPPER PORTION OF WA AQUIFER WHERE K IS HIGHEST

THE TOP 8.8m IS A REASONABLE ESTIMATE.

Project Sheet:		Task	Sheet No. 2 of 2
<b>TES</b> CONSULTING ENGINEERS	Project	Design	Revision No.
	Project No. 1131	BNU - DREHID LANDFILL	Date
	Location	Telephone Log	Prepared
	BECKOCK K-ESTIMATION	Other Records	Checked
Reference			Output/Notes
	$T = 2 \text{ m}^2/\text{d}$ SCREEN LENGTH $K = 2 \text{ m}^2/\text{d} \div 88 \text{ m} = 227 \times 10^{-1} \text{ m/d}$ or $2.6 \times 10^{-6} \text{ m/s}$ SATURATED THICKNESS $K = 2 \text{ m}^2/\text{d} \div 3584 \text{ m} = 5.58 \times 10^{-2} \text{ m/d}$ or $6.45 \times 10^{-7} \text{ m/s}$ TOP 50m $K = 2 \text{ m}^2/\text{d} \div 50 \text{ m} = 4.0 \times 10^{-2} \text{ m/d}$ or $4.63 \times 10^{-7} \text{ m/s}$ ESTIMATED BEDROCK $K = 4.63 \times 10^{-7}$ to $2.6 \times 10^{-6} \text{ m/s}$ IF $T = 2 \text{ m}^2/\text{d}$		
	<hr/> $T = 16 \text{ m}^2/\text{d}$ SCREEN LENGTH $K = 16 \text{ m}^2/\text{d} \div 88 \text{ m} = 182 \text{ m/d}$ or $2.1 \times 10^{-5} \text{ m/s}$ SATURATED THICKNESS $K = 16 \text{ m}^2/\text{d} \div 3584 \text{ m} = 4.46 \times 10^{-3} \text{ m/d}$ or $5.2 \times 10^{-6} \text{ m/s}$ TOP 50m $K = 16 \text{ m}^2/\text{d} \div 50 \text{ m} = 3.2 \times 10^{-1} \text{ m/d}$ or $3.7 \times 10^{-6} \text{ m/s}$ ESTIMATED BEDROCK $K = 3.7 \times 10^{-6}$ to $2.1 \times 10^{-5} \text{ m/s}$ IF $T = 16 \text{ m}^2/\text{d}$		

**APPENDIX 2.4.10**  
**Aquifer Throughput Calculations and**  
**Methodology**

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Project Sheet

**TES**

CONSULTING ENGINEERS

Project: **BMM DUBLIN LANDFILL**

Subject: **AQUIFER THROUGHPUT CALC**

Sheet No: 1 of 2

Revised No:

Date:

Prepared:

Checked:

Project No: 1131

Reference:

FLOW PER UNIT WIDTH  $Q = Tl$   
 FLOW BENEATH FOOTPRINT  $Q = WTL$

WHERE

- $Q$  = FLOW IN ( $M^3/d$ )
- $T$  = TRANSMISSIVITY IN ( $M^2/d$ )
- $l$  = HYDRAULIC GRADIENT - (UNITLESS)
- $W$  = WIDTH OF AQUIFER CONSIDERED IN ( $M$ )

$T$  FOR SITE SHOWN TO BE IN APPROXIMATE RANGE OF 2 - 16  $M^2/d$  WITH LOWER VALUES PROBABLY MORE REPRESENTATIVE

$l$  HYDRAULIC GRADIENTS FROM FIGURE 2.4.7 (AFTER PAGE 70 IN EIS) RANGE FROM 0.0012 TO 0.00391, WITH  $l$  MORE LIKELY CLOSER TO 0.0012 IN FOOTPRINT FLAT AREA OF BOB, RATHER THAN FRINGES WHERE  $l$  SLIGHTLY HIGHER. PRECAUTIONARY VALUE TAKEN AS 0.0025 (HIGHER THAN EXPECTED, FACTOR SAFETY = 2)

$W$  FOR LANDFILL FOOTPRINT IS THE MAXIMUM LENGTH SECTION ACROSS THE FOOTPRINT  
 462m x 460m DIMENSIONS  
 PYTHAGORAS  $\Rightarrow$  DIAGONAL SECTION ACROSS FOOTPRINT = 652m

Output/Action:



Project Sheet:		Task	Sheet No. 2 of 2
<b>TES</b> CONSULTING ENGINEERS	Project	Design	Revision No.
	BNM - DREHID LANDFILL Element AQUIFER THROUGHPUT CALC.	Site Plan	Date
Project No. 1131	Telephone Log	Prepared	Checked
	Other Record		
Reference	$Q = wTc$ ① Let $T = 2 \text{ m}^2/\text{d}$ $c = 0.0025$ $w = 652 \text{ m}$ $Q = 652 \text{ m} \times 2 \text{ m}^2/\text{d} \times 0.0025$ $= 3.26 \text{ m}^3/\text{d}$		Output/Action
	② Let $T = 16 \text{ m}^2/\text{d}$ $c = 0.0025$ $w = 652 \text{ m}$ $Q = 652 \text{ m} \times 16 \text{ m}^2/\text{d} \times 0.0025$ $= 26.08 \text{ m}^3/\text{d}$		
	AQUIFER THROUGHPUT BENEATH FOOTPRINT RANGES 3.26 to 26.08 m <sup>3</sup> /d WITH LOWER VALUE OF 3.26 m <sup>3</sup> /d MORE PROBABLE		