

Golder Associates Ireland

**Proposed Little Island Waste
Recovery Facility**

Traffic Impact Assessment

PMCE

March 2008

Golder Associates Ireland

Proposed Little Island Waste Recovery Facility

Traffic Impact Assessment

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7 th February 2008	D1	First Draft
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20 th March 2008	F	Final



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1 Introduction

1.1 General

PMCE Ltd was commissioned in November 2007 by Golder Associates Ireland to undertake a review of the likely traffic impacts at the location of the proposed Construction, Demolition and Excavation Waste Recovery Facility ('CD&E Facility') on a 22Ha site, in the townland of Wallingstown, Little Island in Co. Cork (Figure 1.1).

The proposed development consists of a Construction, Demolition and Excavation Waste Recovery Facility, the end product of which is to be used in the rock-filling of adjacent lands around the site. The development lands are located on the western side of Little Island. The site is to be accessed directly from the R623 Regional Road.

1.2 Information Reviewed

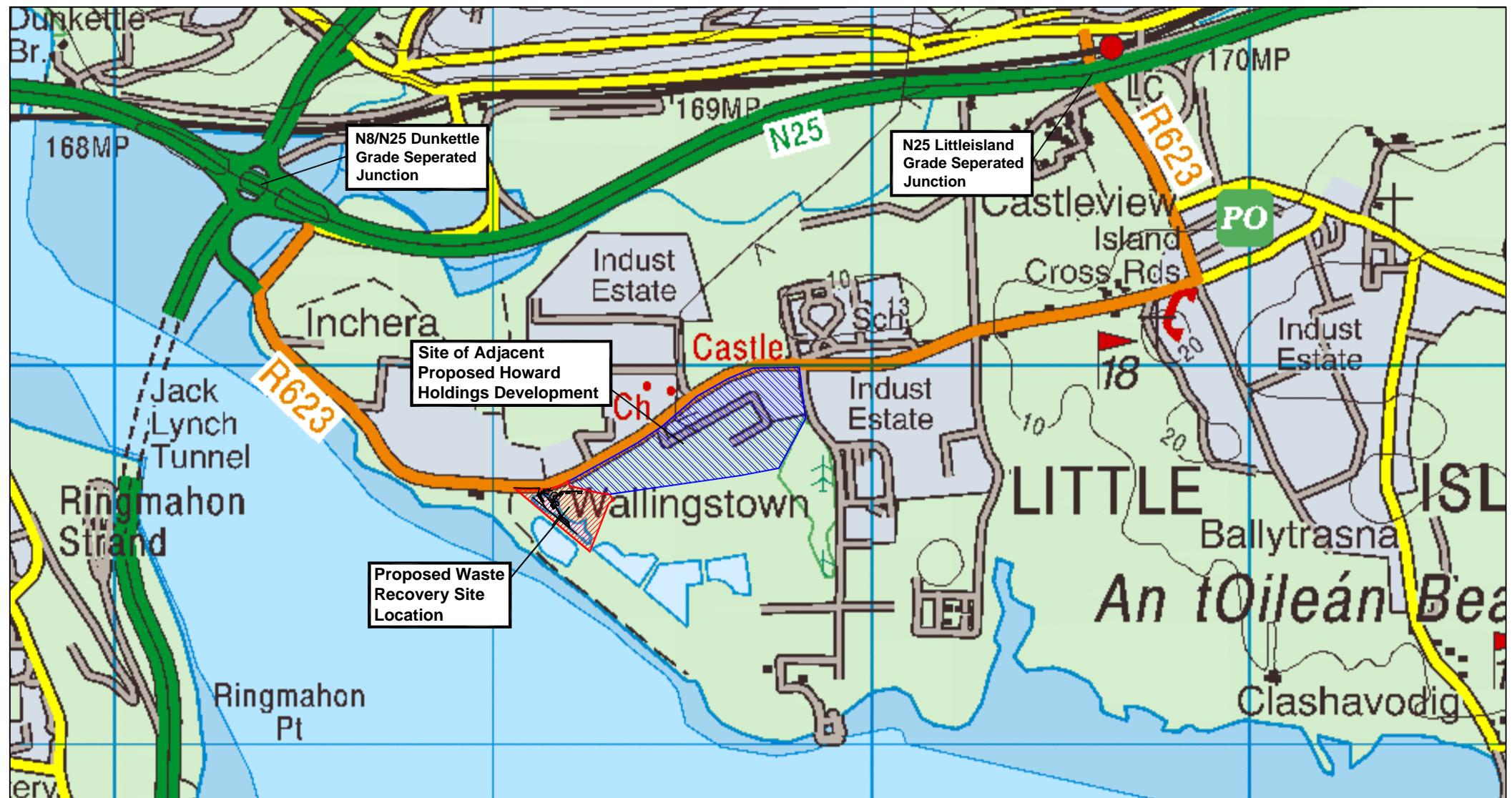
In preparing this report, reference has been made to the following documents:-

- "Traffic and Transport Assessment Guidelines" published by the National Roads Authority;
- "Guidelines for Traffic Impact Assessment" published by the Institution of Highways and Transportation;
- "Future Traffic Forecasts 2002 to 2040" published by the National Roads Authority;
- Traffic Count Survey Data, collected by Abacus Transportation Surveys Ltd (Appendices A and B); and
- Topographical Survey Data and Ordnance Survey Mapping provided by Golder Associates Ireland.

1.3 Scope

The objective of this report is to examine the traffic implications associated with the proposed development in terms of how it can integrate with existing traffic in the area.

The report will determine and quantify the extent of additional trips generated by the development, and the impact on operational performance of such trips on the local road network.



Rev.	Comment	Date

**Proposed Little Island
Waste Recovery
Facility**
Traffic Report

**Location of Proposed
CD&E Facility**

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1.4 Methodology

The methodology adopted for this appraisal and report involved, in brief:-

- A site visit;
- Classified Traffic Counts undertaken on the 16th January 2008 at two locations as detailed in section 2.3;
- Existing Traffic Assessment – The traffic count data was used to develop a PICADY model for the proposed access; and
- Future Year Assessments – The estimated future year volumes on the study area network, as a result of the increase in background traffic and the additional development related traffic was used to assess the the future operational performance of the R623 & the access junction both at the year of opening of the development and at an assessment year.

2 Existing Conditions

2.1 The Site

The site is located adjacent to the R623 in the townland of Wallingstown. No access currently exists onto the R623 from the site at this location. The site is bounded to the north by the R623 and to the south and east by a waste lagoon site. To the east of the waste lagoon site is the Waterfront Business Park Road.

The site is traversed by overhead power lines close to both the southern and western boundaries. As part of the development it is proposed to construct an access from the site directly onto the R623.

Another site, subject to a recent planning application, is located to the north-east.

2.2 Existing Road Network

2.2.1 N25 National Primary Route

The N25 is the primary Cork-Rosslare route which also serves the east and south of Cork city. In the vicinity of the proposed site the N25 is a dual carriageway with grade separated interchanges. Little Island can be accessed from the N25 by one of two interchanges, to the west the Dunkettle Interchange and from the east the Little Island Interchange.

The National Roads Authority estimate the Annual Average Daily Traffic (AADT) for the N25 to be 51,516 with HGV's accounting for 6.5% of road traffic. These figures are derived from traffic counter data for 2007 and are based on 245 days of recorded data, as published on the NRA website for the traffic counter located at Little Island, on the western side of the Little Island Interchange.

2.2.2 R623 Regional Road

The R623 is a single carriageway route, with footpaths/verges along either one or both sides of the road, that runs from the N25 at the Dunkettle Interchange at its western end to the N25 at the Little Island Interchange at its eastern end. It runs through Little Island village and is the primary route serving the majority of employment centres in the area and is busy during peak periods. It is intended to develop an access onto the R623 from the proposed development site.

2.3 Traffic Volumes

Two classified traffic counts were carried out on the R623 on the 16th January 2008 at the junction of the R623 and the Waterfront Business Park Road to the east of the proposed site, and on the R623 at the three arm roundabout to the north west of the proposed site (Figure 2.1). The counts were carried out between 07:00 and 19:00. This time period encompasses the proposed main operating hours of the construction and demolition waste facility and also includes the peak hours on the R623. Surveyed vehicles were broken down into five categories as follows: -

- Cars;
- LGV's (Light Goods Vehicles);
- OGV1 (Two and three axle goods vehicles);
- OGV2 (Four and five axle goods vehicles); and,
- Buses.

The detailed results of the traffic surveys are given in Appendices A & B and are summarised in Tables 2.1 and 2.2. At the junction of the R623 and the local road servicing the Waterfront Business Park and other business facilities, the morning and evening peak hours have been established as 07:45 to 08:45 and 16:45 to 17:45. The morning and evening peak hours on the R623 at the roundabout junction with the Richmond and Inchera roads have been established as 07:45 to 08:45 and 16:30 to 17:30.

The count data has been converted to Annual Average Daily Traffic (AADT) values, as given in Tables 2.1 and 2.2, using the methods described in "Expansion Factors For Short Period Traffic Counts" (John Devlin/National Roads Authority, 1978, RT 201). Table 1B of the document relating to Urban Commuter Routes was used in the expansion of traffic counts to AADT's.

A combined urban commuter factor of 1.437 was derived by combining the individual hourly factors for the count duration. This factor was then used to determine AADT's for each arm surveyed.

The count data in Tables 2.1 and 2.2 is derived from the survey data in Appendices A and B. The tables in the appendices give the count data in 15 minute intervals. The count data presented in Tables 2.1 and 2.2 is the sum of the two-way approach flows for each arm of each junction. For instance in Table 2.1, column 2, the total traffic on the R623 west of the junction is the sum of movements 1, 2, 3 and 6 (Refer to diagram in Appendix A).



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**Proposed Little Island
Waste Recovery
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Traffic Report

**Traffic Count
Locations**

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Table 2.1: Surveyed Flows and AADT for R623/Waterfront Business Park Junction

Hour Ending	R623 West	Local Road	R623 East
08:00	635	305	604
09:00	913	308	871
10:00	428	223	441
11:00	307	173	314
12:00	325	175	320
13:00	363	227	416
14:00	349	209	390
15:00	404	247	443
16:00	400	254	398
17:00	589	336	553
18:00	683	221	666
19:00	369	105	326
Period Total	5765	2783	5742
Period Total HGV's	497	389	492
% HGV's	8.62%	13.98%	8.57%
Total AADT	8283	3998	8250

Table 2.2: Surveyed Flows and AADT for R623 Roundabout at Richmond Road/Inchera Road

Hour Ending	Richmond Road	R623	Inchera Road
08:00	494	711	221
09:00	661	1012	361
10:00	291	449	164
11:00	146	298	162
12:00	143	313	174
13:00	118	351	247
14:00	153	334	183
15:00	169	379	216
16:00	136	384	254
17:00	106	633	535
18:00	57	769	714
19:00	80	379	307
Period Total	2554	6012	3532
Period Total HGV's	198	483	299
% HGV's	7.75%	8.03%	8.47%
Total AADT	3669	8638	5074

Table 2.3 gives the turning movements at each surveyed junction at both the AM and PM peak hour for each junction.

Table 2.3: Peak Hour Turning Movements at Survey Locations

R623/Waterfront Business Park Road Junction			
Travelling From	Travelling To	AM Peak	PM Peak
R623 West	R623 East	346	123
	Business Park	147	12
Business Park	R623 West	39	132
	R623 East	21	109
To R623 East	Business Park	127	23
	R623 West	421	467
R623 at Roundabout Junction with Richmond Road and Inchera Road			
Travelling From	Travelling To	AM Peak	PM Peak
Richmond Road	Inchera Road	4	3
	R623	675	83
R623	Richmond Road	0	0
	Inchera Road	374	698
Inchera Road	Richmond Road	1	1
	R623	0	0

3 Proposed Development

3.1 General

The proposed CD&E Facility is located in the north-western corner of a c. 24Ha brownfield site comprising waste lagoons ('the Thornbush Site'). Its development in the 1970s was undertaken by Mitsui Denman and involved the excavation of large artificial lagoons to facilitate waste material from an adjacent industrial plant. It was subsequently bought by Thornbush Holdings, who are obliged to restore the site under the existing planning permission (Planning Ref: 1466/73) and the IPPC licence (No. 389). The proposed CD&E waste recovery facility will facilitate the restoration of the Thornbush Site.

Approximately 1,500,000 tonnes of material will be required for the capping and restoration of the Thornbush site. Annual volumes processed will vary, as the source material will come from construction activities and the maximum volume in any given year is estimated to be 250,000 tonnes. A 10 year duration for the operation has been assumed to account for the sporadic nature of obtaining suitable material.

Construction, Demolition and Excavation material will be primarily sourced locally and all incoming material will be controlled to ensure that it produces a high quality product using screening and segregation processes. To ensure this all material entering the site will be subject to strict controls on site or before arrival on site. On site the primary operations will include:-

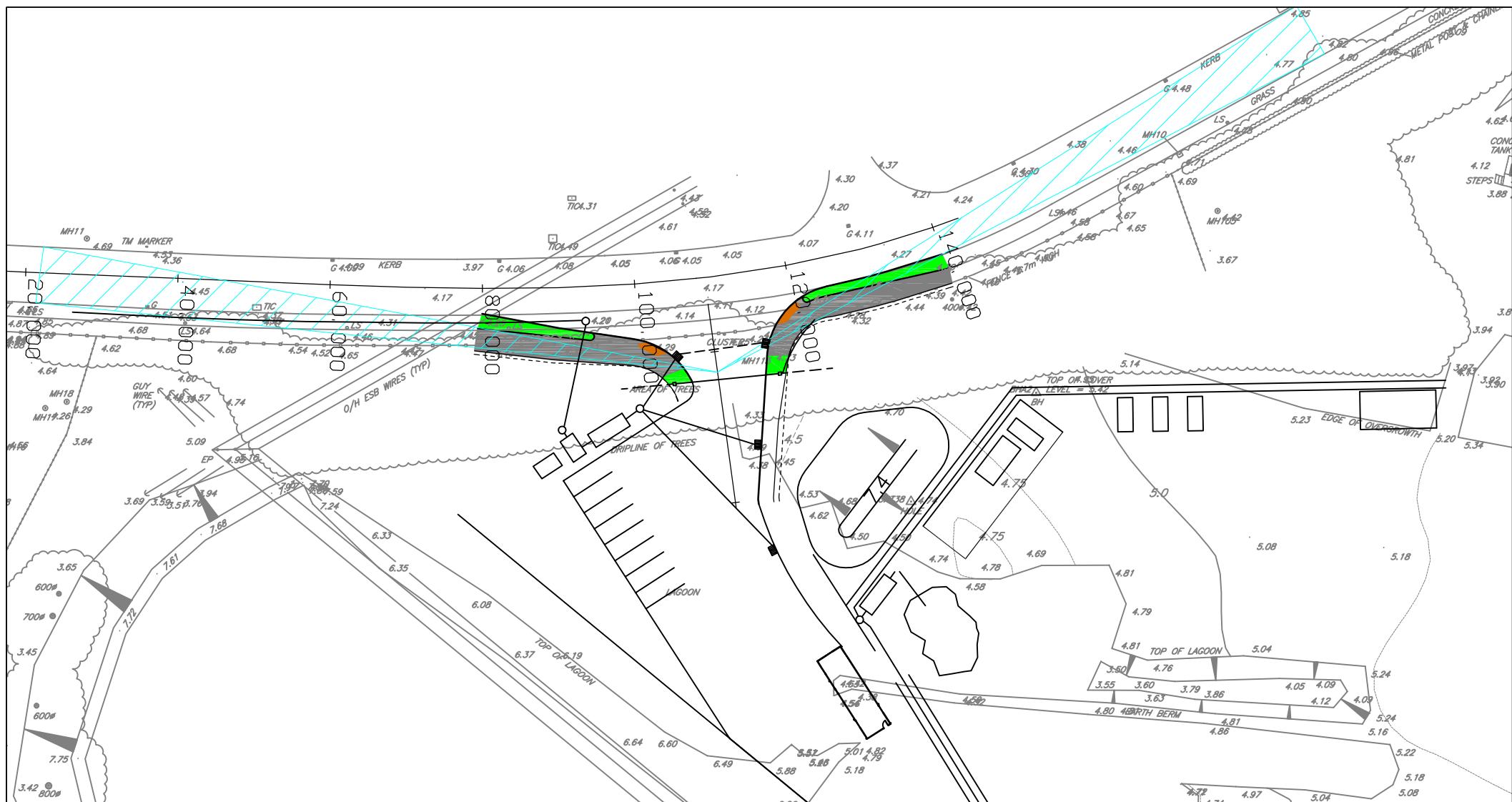
- Acceptance and Receipt;
- Primary Sorting;
- Crushing and Screening;
- Finished Product Stockpiling; and
- Transport to Final Application.

The incoming Construction, Demolition and Excavation waste may contain a number of waste products which are unsuitable for recovery. These materials are likely to include, timber, plastics, metals, cardboard, plasterboard and aggregate from the processed waste, in particular the finer crushed aggregate which when placed in the existing lagoons may aid and induce the raising of the existing water table level. Recyclable materials will be segregated out and distributed to licensed recovery agents. It has been estimated that these materials will constitute between 10-20% of the total incoming waste. For the purpose of this assessment it is assumed, conservatively, that 20% of the total imported waste will not be suitable for recovery.

3.2 Proposed Access

It is proposed to provide an access from the site onto the R623 at the location shown in Figure 3.1. The proposed access road will be 6.0 metres in width, and the junction with the R623 will comply with the requirements of the Road Geometry Handbook (NRA). The corner radii will be 10m, and the arrangement will incorporate exit tapers over 30m at 1:5.

Provisions for pedestrians at the proposed access will include tie-ins with the footways to the east and west, and dropped kerbs with associated tactile paving to facilitate mobility and visually impaired pedestrians crossing. The location of the access has been chosen to maximise available sightlines for approaching and exiting vehicles, and at a location not directly below existing overhead electricity cables so as to minimise risks to operatives during construction and following opening.



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Notes:

- Do Not Scale - use figured dimensions only.
- Drawing is the property of PMCE Ltd.

Legend

- Footpath
- Verge
- 90 metre Sightline Envelope for 9m Setback
- Dropped Kerb Locations with 'Buff' Tactile Paving

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Proposed Little Island Waste Recovery Facility

Traffic Report

Proposed Access from Site onto R623

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3.3 Trip Generation

3.3.1 General

At present the site generates no traffic. In determining the trip generation by the proposed development a number of assumptions have made, as follows:-

- The facility will operate for 50 weeks per annum;
- The facility will operate for six days per week (Monday to Saturday inclusive);
- The facility opening times will be 08:00 to 18:00 on Monday to Friday and 08:00 to 16:00 on Saturday; and
- Material will be imported in twenty tonne trucks;

3.3.2 Forecast Development Traffic

With 20% of the total imported material assumed to be unsuitable for recovery as Construction, Demolition and Excavation waste, to attain the target for usable material of 250,000 tonnes per annum a total of 300,000 tonnes of imported material will be required annually. Working on the assumptions outlined above an average of 50 incoming waste deliveries daily are estimated. However the availability of source material will vary over time, and short term peaking associated with construction and development activities are likely. To account for this a peak factor of 2.0 is assumed. This represents a conservative approach, and would give 100 waste deliveries per day during peak times.

Assuming a nine hour working day (operations ceasing between 13:00 and 14:00), an average of six deliveries will arrive on site each hour or 11 per hour for the conservative scenario. For the purposes of this report the conservative scenario has been assessed.

In determining the trip generation for the unsuitable material that is to be transported away from the site, a number of assumptions have been made. These assumptions are considered to be conservative as the transport of the material is unlikely to occur daily, but periodically when sufficient unsuitable material has accumulated. The assumptions are:-

- Material will be incrementally exported when approximately 4,200 tonnes have built up on site in a stockpile; and
- Material will be exported by up to four trucks from the site, resulting in 36 trips in and 36 trips out (1 load per truck per working hour).

Therefore Peak Hour Trips associated with disposal of unsuitable material are 4 in and 4 out.

3.3.3 Staff Trips

It has been assumed that five staff would be based at the site during operations, giving five additional peak hour trips to account for staff movements. It is assumed that 60% of staff car trips will enter/exit to/from the west and that 40% will enter/exit to/from the east.

3.3.4 Miscellaneous Trips

An additional number of four trips (two inbound and two outbound) has been assumed during the AM and PM Peaks to cater for possible miscellaneous trips associated with the site. These miscellaneous trips allow for site supervision meetings, HSA site inspections, maintenance operations for plant and machinery, etc. It is assumed that: -

- 50% of miscellaneous trips enter/exit to/from the west; and
- 50% of miscellaneous trips enter/exit to/from the east.

3.4 Trip Assignment

Tow scenarios have been assessed in relation to the trip assignment. Scenario A assumes that 100% of the HGV's importing material into the site would travel to/from the R623 west of the site access. In this case all of the HGV's travelling to/from the site would do so via the Dunkettle Interchange.

Scenario B assumes an 80/20 directional split between HGV's travelling to/from West/East. Therefore 80% would travel to/from the Dunkettle Interchange and 20% would travel to/from the Little Island Interchange.

It is considered that the most (close to 100%) trips will travel to/from the Dunkettle Interchange to avoid delays associated with travelling via the Little Island village area and it represents the shortest route to/from Cork City where a high percentage of the imported material is likely to be sourced. However both scenarios have been assessed in order to provide complete data on the traffic generated by the proposed development.

4 Proposed Adjacent Development

4.1 General

A search of planned future developments which may have an impact on future traffic flows in the vicinity of the proposed development was undertaken. A significant scheme which is currently at planning stage was identified as being potentially significant in relation to traffic flows in the vicinity of the proposed development and its access. This site is located to the east of the proposed development (Refer to Figure 1.1).

The site in question is being developed by Howard Holdings plc and the proposed development is to consist of office and light industrial units of approximately 47,681m² Gross Floor Area. Howard Holdings plc propose to develop 42,123m² of office floor space and 5,579m² of warehouse/light industry floor space.

4.2 Trip Generation

An examination of predicted traffic flows associated with this development was undertaken. Predicted traffic flows are outlined in a report produced by Boreham Consulting Engineers dated November 2007. These were compared to expected trip rates produced using the Trip Rate Information Computer System (TRICS) programme. Table 4.1 compares the peak hour trip rates produced by each source. The TRICS figures in Table 4.1 contain the trips predicted for the office and warehousing elements of the development for the peak hours.

Table 4.1: Trip Rate Comparison for Proposed Adjacent Development

Peak Hour	Howard Holdings PLC		TRICS Output	
	Arrivals	Departures	Arrivals	Departures
AM	381	51	354	49
PM	42	254	55	290

The predicted flows from TRICS are broadly in line with the Boreham Report and for the purposes of this assessment the Boreham flows are used where the traffic from the adjacent development is included in the assessment.

4.3 Derived Trip Rate

The average total daily trips associated with the Waste Recovery Facility is 186, with HGV's accounting for 92% of these. These are arrived at as follows:-

- 50 daily truck deliveries to the site importing material giving 100 trips (Section 3.3.2);
- 36 HGV trips daily when unusable material is being exported from site (Section 3.3.2);
- 10 staff trips daily (Section 3.3.3); and,
- 4 miscellaneous trips daily (Section 3.3.4).

The conservative number of trips, using the short term peaking factor, is 286 with HGV's 95% of these. These are arrived at as follows:-

- 100 (50×2.0) daily truck deliveries to the site importing material giving 200 trips (Section 3.3.2);
- 36 HGV trips daily when unusable material being exported from site (Section 3.3.2);
- 10 staff trips daily (Section 3.3.3); and,
- 4 miscellaneous trips daily (Section 3.3.4).

An assessment of the junction capacity analysis was undertaken based the short term peaking scenario, which represents the worst case.

4.4 Committed Development

Under the existing planning permission (Planning Ref: 1466/73) and IPPC license (No. 389), Thornbush Holdings are obliged to restore the waste lagoons on site. To undertake this work, 50 incoming waste daily deliveries are estimated (Refer to 3.3.2). These are trips which will occur on the R623 in the future due to the obligations arising from the existing planning permissions.

Therefore the forecast average 50 HGV deliveries to the site will arise in the future as part of the site remediation without the proposed development. The resulting additional traffic, for the short term peak scenario will be an additional 50 HGV deliveries daily.

5 Traffic Impacts

5.1 Assessment Years

The National Roads Authority's publication "Future Traffic Forecasts 2002-2040" has been used to determine future year traffic flows on the R623 from the 2008 flows. For this assessment the Non-National indices have been used for the R623 traffic forecasts.

The Institution of Highways and Transportation's (IHT) "Guidelines for Traffic Impact Assessment" recommends that traffic impacts be assessed for a period of 10 years beyond the opening year of the development unless a new or modified road infrastructure is required. The "Traffic and Transport Assessment Guidelines" published by the National Roads Authority recommend assessment of traffic in the Opening Year, for the Opening Year +5 Years and the Opening Year +15.

In recognition of the limited lifespan of the proposed development, and of the effects of proposed adjacent developments, it is proposed to carry out assessment for: -

- 2008 - the Opening Year of the Waste Recovery Facility;
- 2009 - the possible Opening Year of the adjacent development; and
- 2018 - the year when operations at the Waste Recovery Facility are expected to cease.

5.2 Link Capacity Assessment

The National Roads Authority's document "Traffic and Transport Assessment Guidelines" (September 2007) states that a full Traffic and Transport Assessment is not required if the development traffic does not exceed 5% of the two way flow on the adjoining road where congestion exists or in sensitive locations. Table 5.1 gives the development traffic as a percentage of the traffic on the R623.

The level of development trips associated with the proposed scheme does not constitute a traffic impact on the basis of the National Roads Authority's criteria.

Table 5.1: Peak Hour Link Capacity Assessment

	R623 Existing Traffic (Vehicles per Hour)			Development Traffic (Vehicles per Hour)			Development Traffic as a % of existing Traffic		
	2008	2009	2018	2008	2009	2018	2008	2009	2018
Excluding Adjacent Development									
AM	1050	1069	1190	37	37	37	3.5%	3.5%	3.1%
PM	782	785	886	37	37	37	4.7%	4.7%	4.2%
Including Adjacent Development									
AM	1050	1232	1356	37	37	37	3.5%	3.0%	2.7%
PM	782	944	1036	37	37	37	4.7%	3.9%	3.6%

5.3 Junction Capacity Analysis

The capacity of the proposed access onto the R623 was assessed using the Transport Research Laboratory's (TRL) computer programme PICADY (**P**riority **I**ntersection **C**apacity and **D**ela**Y**). Junction performance is measured as a ratio between the flow and capacity (RFC). The capacity analysis has been carried out for both the AM and PM Peaks for each of the assessment years (2008, 2009 and 2018). An urban junction with an RFC below 0.85 is considered to be operating within capacity, with an RFC of 0.85 indicating a junction operating at capacity.

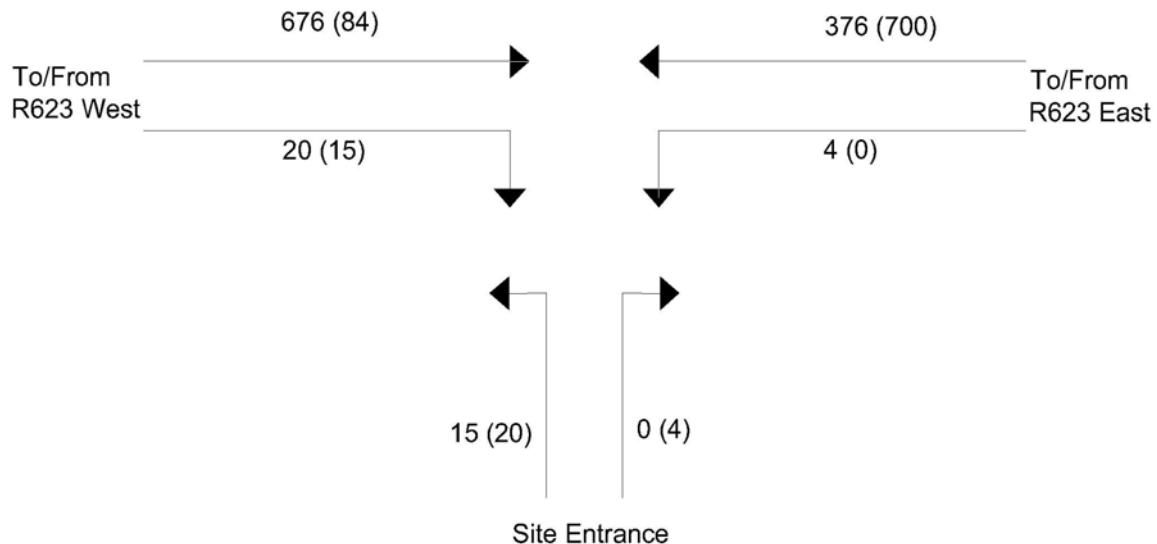
The PICADY analysis outputs are included in Appendices C, D and E to this report. Appendix C contains the output for the 2008 AM and PM Peaks. Appendix D contains the output from the analysis for the 2009 AM and PM peaks. Appendix E contains the output from the analysis for the 2018 AM and PM peaks. The results are summarised in Tables 5.2 to 5.6.

The junction capacity was assessed under the two assignment scenarios:-

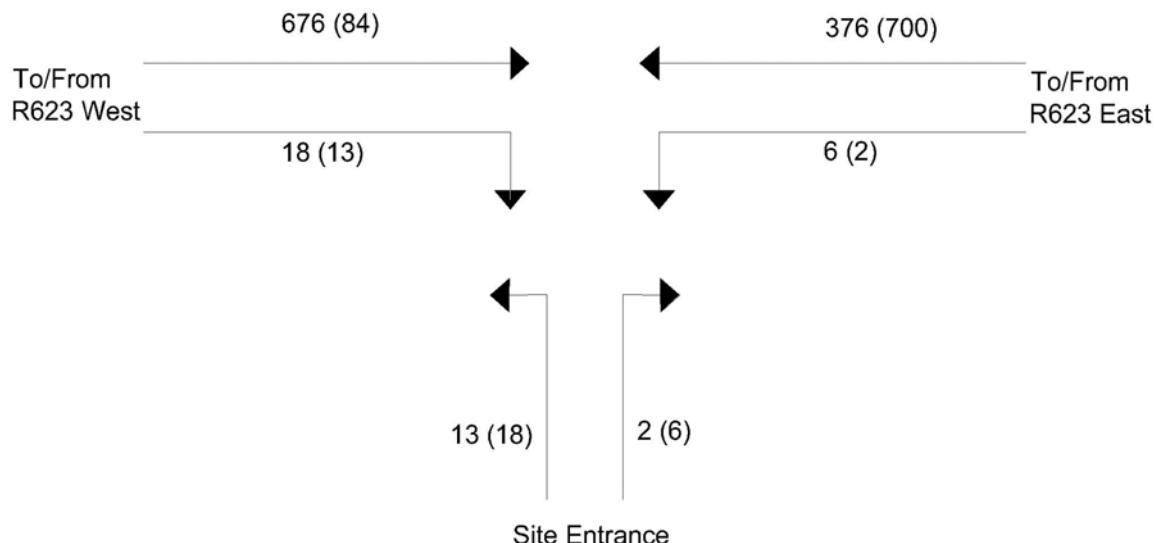
- Scenario A: 100% of trips to/from R623 west; and
- Scenario B: 80/20 West/East directional split.

Figures 5.1 to 5.5 illustrate the predicted peak hour turning movements at the proposed access and correspond to the analysis summaries given in Tables 5.2 to 5.6. The turning movements for both the AM and PM peaks are shown.

The PICADY analysis indicates that the proposed junction will operate within capacity for each of the Assessment Years for all scenarios assessed. Capacity for Scenario A is greater than for Scenario B, although the difference is marginal.

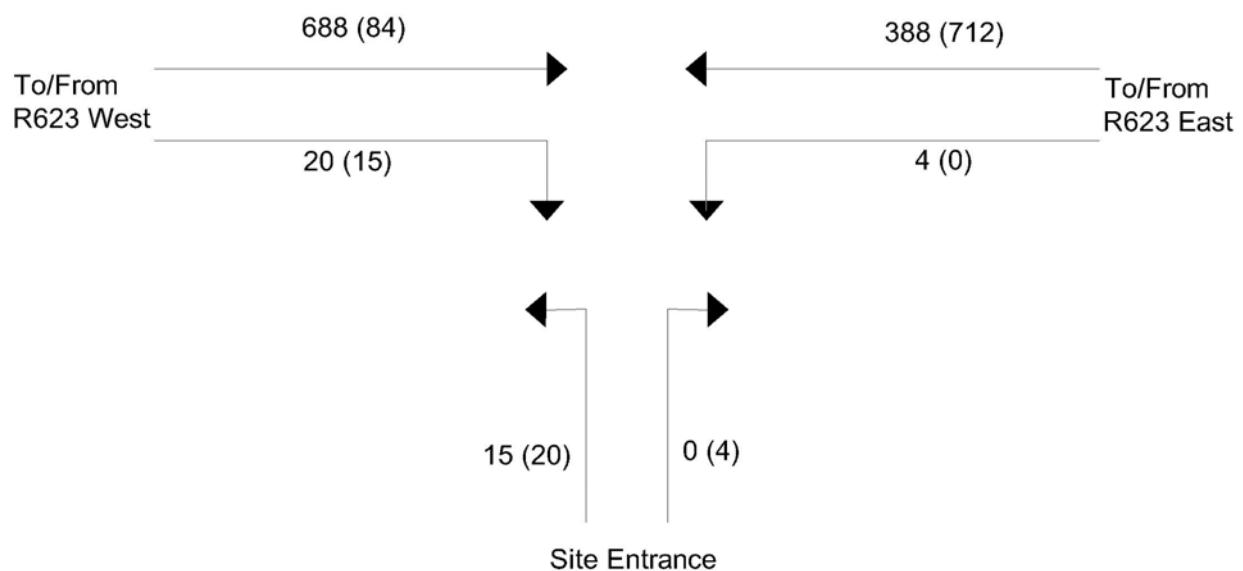


Forecast Turning Movements For Scenario A

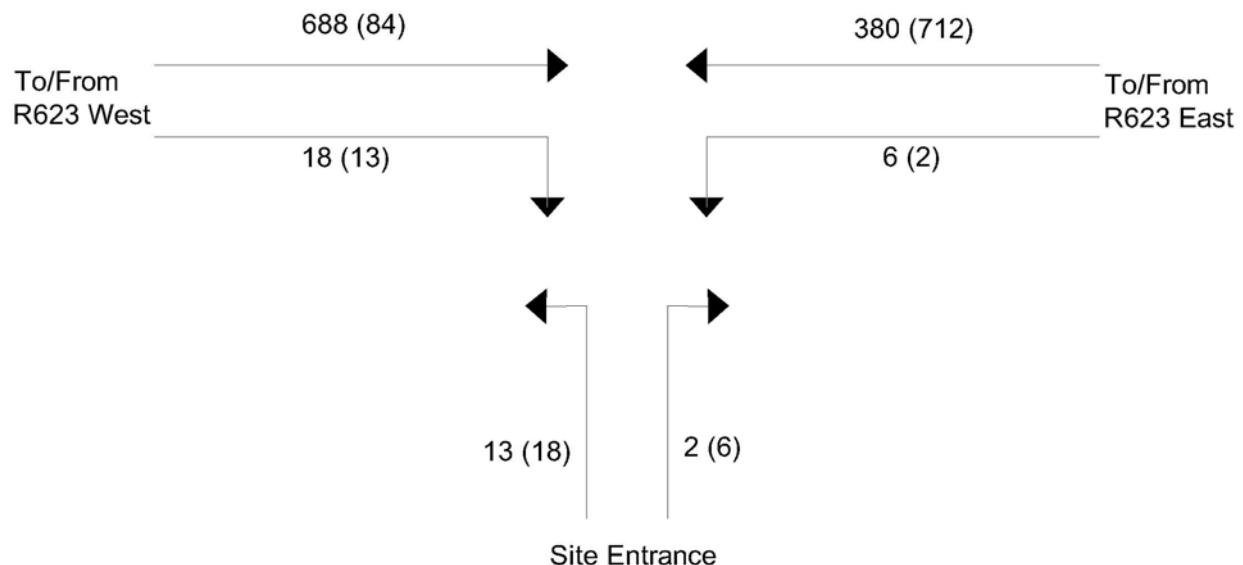


Forecast Turning Movements For Scenario B

Figure 5.1: 2008 Forecast Traffic Movements At Proposed Access



Forecast Turning Movements For Scenario A



Forecast Turning Movements For Scenario B

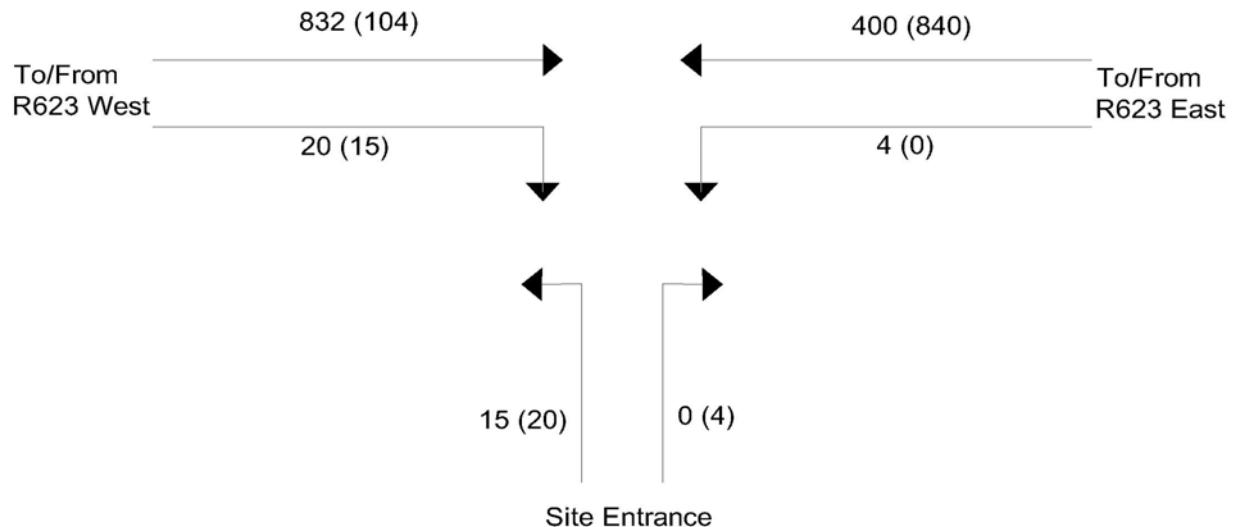
Figure 5.2: 2009 Turning Movements At Proposed Access (Excl. Adjacent Development)

Table 5.2: Summary of PICADY Junction Capacity Analysis for 2008

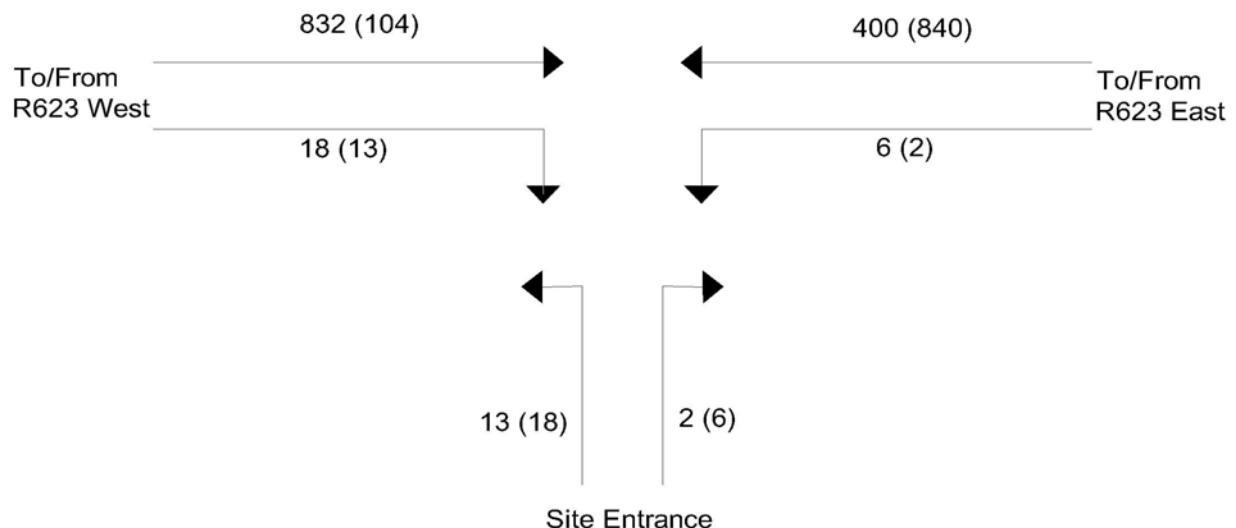
	Maximum RFC	Maximum Queue (vehicles)	Queuing Delay (Min/Veh)
Scenario A – AM Peak			
Site Access	0.056	0.1	0.22
R623	0.068	0.1	0.22
Scenario A – PM Peak			
Site Access	0.082	0.1	0.22
R623	0.067	0.1	0.29
Scenario B – AM Peak			
Site Access	0.062	0.1	0.25
R623	0.060	0.1	0.21
Scenario B – PM Peak			
Site Access	0.105	0.1	0.25
R623	0.232	0.4	0.34

Table 5.3: 2009 PICADY Analysis (Excl. Adjacent Development)

	Maximum RFC	Maximum Queue (vehicles)	Queuing Delay (Min/Veh)
Scenario A – AM Peak			
Site Access	0.060	0.1	0.24
R623	0.068	0.1	0.22
Scenario A – PM Peak			
Site Access	0.088	0.1	0.24
R623	0.068	0.1	0.29
Scenario B – AM Peak			
Site Access	0.067	0.1	0.27
R623	0.060	0.1	0.21
Scenario B – PM Peak			
Site Access	0.108	0.1	0.26
R623	0.058	0.1	0.29

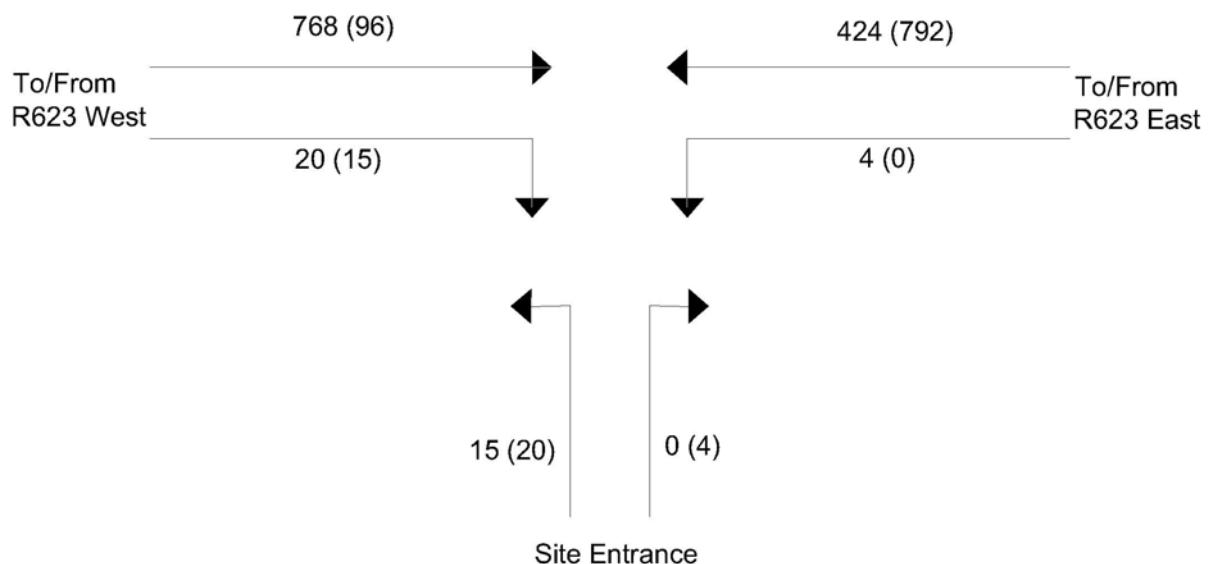


Forecast Turning Movements For Scenario A

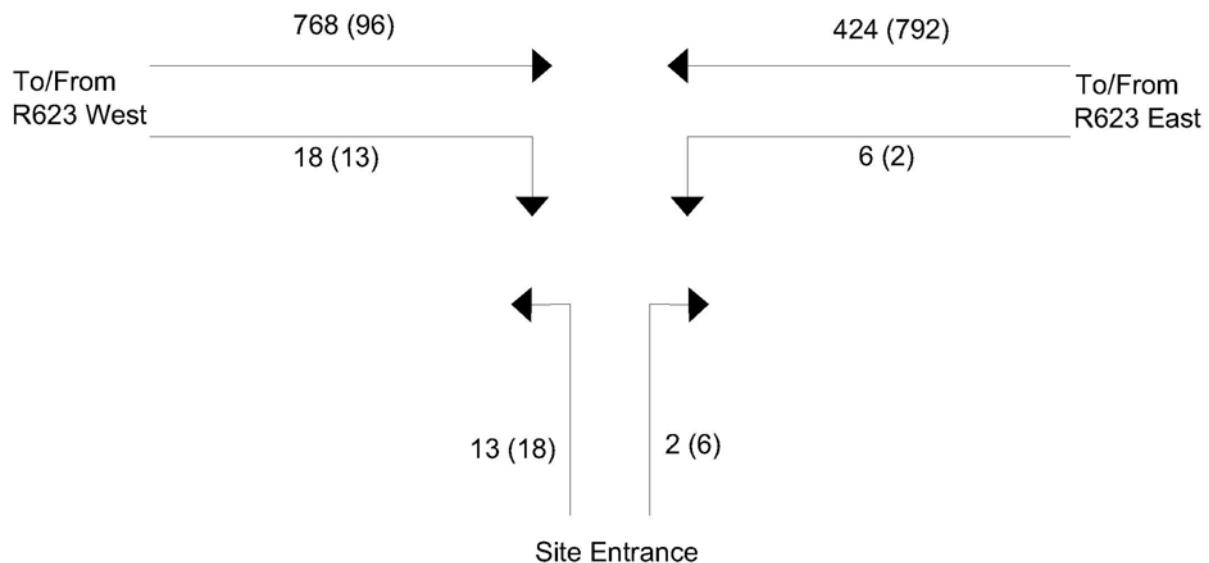


Forecast Turning Movements For Scenario B

Figure 5.3: 2009 Turning Movements At Proposed Access (Incl. Adjacent Development)



Forecast Turning Movements For Scenario A



Forecast Turning Movements For Scenario B

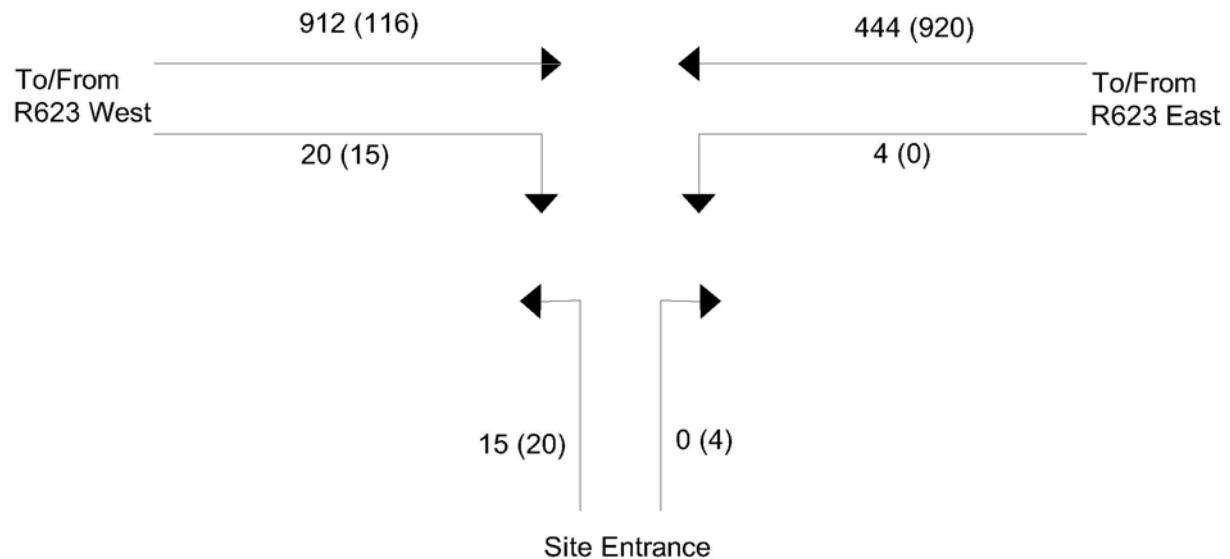
Figure 5.4: 2018 Turning Movements At Proposed Access (Excl. Adjacent Development)

Table 5.4: 2009 PICADY Analysis (Incl. Adjacent Development)

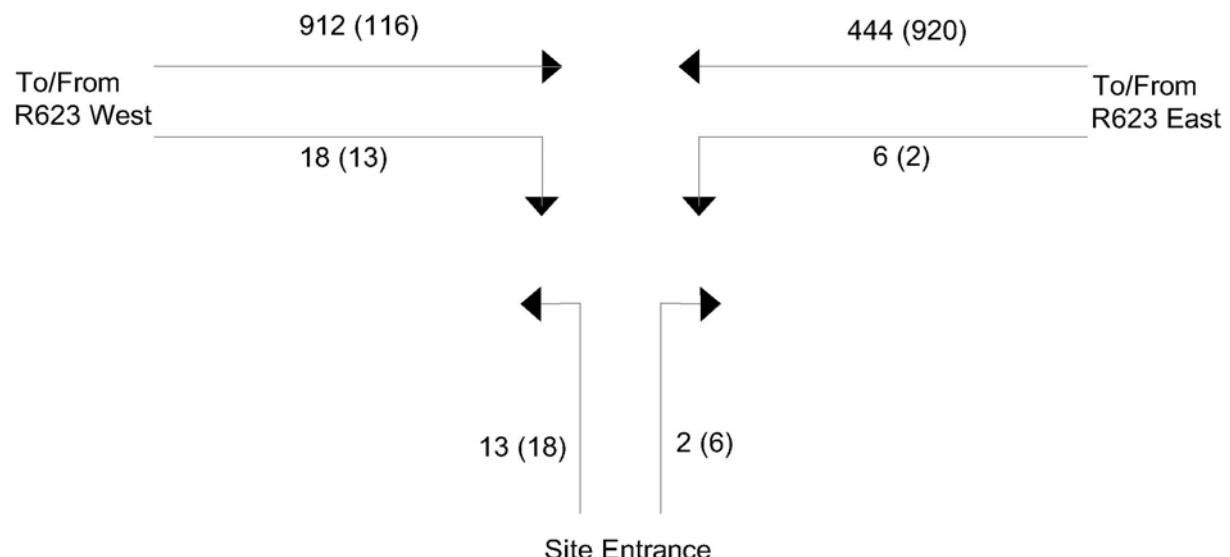
	Maximum RFC	Maximum Queue (vehicles)	Queuing Delay (Min/Veh)
Scenario A – AM Peak			
Site Access	0.060	0.1	0.24
R623	0.069	0.1	0.22
Scenario A – PM Peak			
Site Access	0.095	0.1	0.26
R623	0.072	0.1	0.31
Scenario B – AM Peak			
Site Access	0.068	0.1	0.27
R623	0.061	0.1	0.22
Scenario B – PM Peak			
Site Access	0.117	0.1	0.28
R623	0.062	0.1	0.30

Table 5.5: 2018PICADY Analysis (Excl. Adjacent Development)

	Maximum RFC	Maximum Queue (vehicles)	Queuing Delay (Min/Veh)
Scenario A – AM Peak			
Site Access	0.061	0.1	0.24
R623	0.069	0.1	0.22
Scenario A – PM Peak			
Site Access	0.092	0.1	0.25
R623	0.071	0.1	0.30
Scenario B – AM Peak			
Site Access	0.069	0.1	0.28
R623	0.062	0.1	0.22
Scenario B – PM Peak			
Site Access	0.113	0.1	0.27
R623	0.060	0.1	0.30



Forecast Turning Movements For Scenario A



Forecast Turning Movements For Scenario B

Figure 5.5: 2018 Turning Movements At Proposed Access (Incl. Adjacent Development)

Table 5.6: 2018 PICADY Analysis (Incl. Adjacent Development)

	Maximum RFC	Maximum Queue (vehicles)	Queuing Delay (Min/Veh)
Scenario A – AM Peak			
Site Access	0.062	0.1	0.25
R623	0.070	0.1	0.23
Scenario A – PM Peak			
Site Access	0.064	0.1	0.17
R623	0.075	0.1	0.32
Scenario B – AM Peak			
Site Access	0.071	0.1	0.29
R623	0.062	0.1	0.22
Scenario B – PM Peak			
Site Access	0.123	0.1	0.30
R623	0.065	0.1	0.32

6 Conclusions

The analysis of the impact of the traffic to be generated by the proposed development on the adjacent R623 Regional Road, under the two scenarios outlined in Section 5, indicates that the link capacity and the access junction capacity will operate within capacity.

The assessment indicates that the development of the Construction, Demolition and Excavation Waste Recovery Facility and the rock-filling of the adjacent lands will have a small impact on traffic flows on the existing road network due to the low volumes of the traffic to be generated by the proposed development.

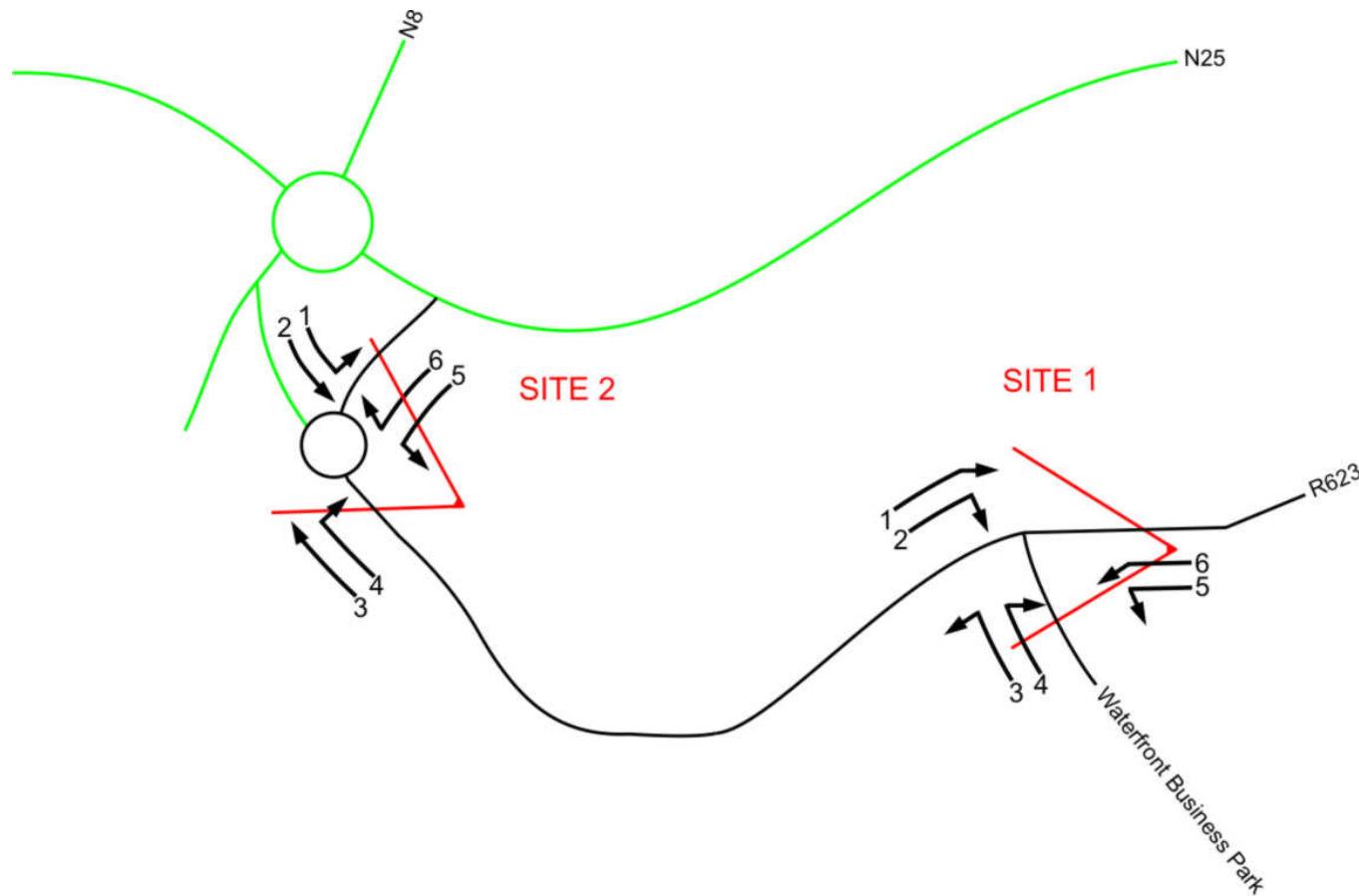
The volume of traffic generated is below the thresholds set out in the “Traffic and Transport Assessment Guidelines” published by the National Roads Authority, and the differences between Scenario A and Scenario B on these thresholds is insignificant. In light of the negative effects that HGV traffic could have on Little Island village, it is recommended that a Operational Traffic Management Plan be developed to encourage heavy vehicles to travel to/from the site via the Dunkettle Interchange.

The priority junction formed by the proposed access to the site with the adjacent R623 Regional Road will be capable of accommodating the generated traffic in both the opening and assessment years.

In addition, an assessment of the proposed access including traffic flows from the possible future planned adjacent development, the access junction will operate within capacity over the proposed lifespan of the Waste Recovery Facility.

Appendix A – Traffic Count Data for R623/Local Road Junction

Movement Numbers & Directions



	Job number: ATH/08/006	Job date: 16 th January 2008	Drawing No: ATH/08/006-2	
Client: PMCE Ltd	Job day Wednesday		Author: BCK	Transportation Surveys

ABACUS TRANSPORTATION SURVEYS

LITTLE ISLAND TRAFFIC COUNTS
MANUAL CLASSIFIED JUNCTION COUNTS

JANAUARY 2008
ATH/08/006

SITE: 02

DATE: 16th January 2008

LOCATION: R623/Waterfront Business Park

DAY: Wednesday

TIME	MOVEMENT 1					TOT	MOVEMENT 2					TOT	MOVEMENT 3					TOT
	CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS	
07:00	0	0	0	0	0	0	62	2	0	0	0	64	0	0	0	0	0	0
07:15	2	0	0	0	0	2	139	10	1	0	0	150	0	0	0	0	0	0
07:30	0	0	0	0	0	0	103	9	1	1	0	114	0	0	0	0	0	0
07:45	0	0	0	0	0	0	138	16	6	4	0	164	0	0	0	0	0	0
H/TOT	2	0	0	0	0	2	442	37	8	5	0	492	0	0	0	0	0	0
08:00	0	0	0	0	0	0	156	13	3	1	0	173	0	0	0	0	0	0
08:15	0	0	0	0	0	0	150	14	2	3	0	169	0	0	0	0	0	0
08:30	3	0	1	0	0	4	152	12	4	1	0	169	0	0	0	0	0	0
08:45	0	1	0	0	0	1	128	8	8	1	0	145	0	0	0	0	0	0
H/TOT	3	1	1	0	0	5	586	47	17	6	0	656	0	0	0	0	0	0
09:00	0	0	0	0	0	0	106	10	2	1	0	119	0	0	0	0	0	0
09:15	0	1	0	0	0	1	59	8	1	0	0	68	0	0	0	0	0	0
09:30	0	0	0	0	0	0	32	10	6	2	0	50	0	0	0	0	0	0
09:45	0	1	0	0	0	1	33	9	8	1	0	51	0	0	0	0	0	0
H/TOT	0	2	0	0	0	2	230	37	17	4	0	288	0	0	0	0	0	0
10:00	1	0	0	0	0	1	26	4	8	2	0	40	0	0	0	0	0	0
10:15	0	0	0	0	0	0	25	5	4	2	0	36	0	0	0	0	0	0
10:30	2	0	0	0	0	2	21	7	3	0	0	31	0	0	0	0	0	0
10:45	1	0	0	0	0	1	21	8	3	1	0	33	1	0	0	0	0	1
H/TOT	4	0	0	0	0	4	93	24	18	5	0	140	1	0	0	0	0	1
11:00	0	0	0	0	0	0	20	8	2	3	0	33	0	0	0	0	0	0
11:15	1	0	0	0	0	1	22	8	4	2	0	36	0	0	0	0	0	0
11:30	0	0	0	0	0	0	23	8	6	2	0	39	1	0	0	0	0	1
11:45	1	0	0	0	0	1	26	3	1	2	0	32	0	0	0	0	0	0
H/TOT	2	0	0	0	0	2	91	27	13	9	0	140	1	0	0	0	0	1
12:00	2	0	1	0	0	3	15	6	7	1	0	29	1	0	0	0	0	1
12:15	0	0	0	0	0	0	25	11	2	0	0	38	0	0	0	0	0	0
12:30	2	0	0	0	0	2	14	5	3	0	0	22	0	0	0	0	0	0
12:45	0	0	0	0	0	0	13	6	1	1	0	21	0	0	0	0	0	0
H/TOT	4	0	1	0	0	5	67	28	13	2	0	110	1	0	0	0	0	1

ABACUS TRANSPORTATION SURVEYS

LITTLE ISLAND TRAFFIC COUNTS
MANUAL CLASSIFIED JUNCTION COUNTS

JANAUARY 2008
ATH/08/006

SITE: 02

DATE: 16th January 2008

LOCATION: R623/Waterfront Business Park

DAY: Wednesday

TIME	MOVEMENT 1					TOT	MOVEMENT 2					TOT	MOVEMENT 3					TOT
	CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS	
13:00	0	1	0	0	0	1	25	5	2	1	0	33	0	0	0	0	0	0
13:15	0	0	0	0	0	0	19	4	1	2	0	26	0	0	0	0	0	0
13:30	0	0	0	0	0	0	30	11	3	1	0	45	0	0	0	0	0	0
13:45	0	0	0	0	0	0	27	18	3	0	0	48	0	0	0	0	0	0
H/TOT	0	1	0	0	0	1	101	38	9	4	0	152	0	0	0	0	0	0
14:00	0	0	0	0	0	0	23	12	3	2	0	40	0	0	0	0	0	0
14:15	0	0	0	0	0	0	33	9	1	1	0	44	0	0	0	0	0	0
14:30	0	0	0	0	0	0	28	13	4	1	0	46	0	0	0	0	0	0
14:45	1	1	0	0	0	2	22	9	5	0	0	36	0	0	0	0	0	0
H/TOT	1	1	0	0	0	2	106	43	13	4	0	166	0	0	0	0	0	0
15:00	0	0	0	0	0	0	9	15	4	1	0	29	0	0	0	0	0	0
15:15	0	0	0	0	0	0	18	8	5	2	0	33	0	0	0	0	0	0
15:30	0	0	1	0	0	1	19	13	7	0	0	39	0	0	0	0	0	0
15:45	1	0	1	0	0	2	19	6	5	0	1	31	1	0	0	0	0	1
H/TOT	1	0	2	0	0	3	65	42	21	3	1	132	1	0	0	0	0	1
16:00	0	0	0	0	0	0	16	8	2	2	0	28	0	0	0	0	0	0
16:15	1	0	0	0	0	1	18	7	4	0	0	29	0	0	0	0	0	0
16:30	3	0	0	0	0	3	14	10	1	1	0	26	0	0	0	0	0	0
16:45	0	0	0	0	0	0	10	7	2	0	0	19	0	0	0	0	0	0
H/TOT	4	0	0	0	0	4	58	32	9	3	0	102	0	0	0	0	0	0
17:00	0	0	0	0	0	0	14	7	2	0	0	23	0	0	0	0	0	0
17:15	0	0	0	0	0	0	12	3	0	0	0	15	0	0	0	0	0	0
17:30	1	0	0	0	0	1	5	0	0	1	0	6	0	0	0	0	0	0
17:45	0	0	0	0	0	0	9	2	0	1	0	12	0	0	0	0	0	0
H/TOT	1	0	0	0	0	1	40	12	2	2	0	56	0	0	0	0	0	0
18:00	0	0	0	0	0	0	12	1	2	0	0	15	0	0	0	0	0	0
18:15	0	0	0	0	0	0	23	3	1	0	0	27	0	0	0	0	0	0
18:30	1	0	0	0	0	1	22	2	2	0	0	26	0	0	0	0	0	0
18:45	0	0	0	0	0	0	8	2	1	0	0	11	0	0	0	0	0	0
H/TOT	1	0	0	0	0	1	65	8	6	0	0	79	0	0	0	0	0	0
P/TOT	23	5	4	0	0	32	1944	375	146	47	1	2513	4	0	0	0	0	4

ABACUS TRANSPORTATION SURVEYS

LITTLE ISLAND TRAFFIC COUNTS
MANUAL CLASSIFIED JUNCTION COUNTS

JANAUARY 2008
ATH/08/006

SITE: 02

DATE: 16th January 2008

LOCATION: R623/Waterfront Business Park

DAY: Wednesday

TIME	MOVEMENT 4					TOT	MOVEMENT 5					TOT	MOVEMENT 6					TOT
	CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS	
07:00	12	2	1	1	0	16	4	1	0	0	0	5	0	0	0	0	0	0
07:15	33	5	4	2	0	44	2	2	0	0	0	4	0	0	0	0	0	0
07:30	56	12	3	3	0	74	0	0	0	1	0	1	0	0	0	0	0	0
07:45	54	8	8	4	0	74	1	0	0	0	0	1	0	0	0	0	0	0
H/TOT	155	27	16	10	0	208	7	3	0	1	0	11	0	0	0	0	0	0
08:00	95	12	6	3	0	116	0	0	0	0	0	0	0	0	0	0	0	0
08:15	73	20	8	2	0	103	0	0	0	0	0	0	0	0	0	0	0	0
08:30	65	8	7	1	0	81	0	0	0	0	0	0	0	0	0	0	0	0
08:45	36	10	5	5	0	56	0	0	0	0	0	0	0	0	0	0	0	0
H/TOT	269	50	26	11	0	356	0	0	0	0	0	0	0	0	0	0	0	0
09:00	35	9	7	3	0	54	1	0	0	0	0	1	0	0	0	0	0	0
09:15	14	9	1	2	0	26	0	0	0	0	0	0	0	0	0	0	0	0
09:30	19	8	4	4	0	35	0	0	0	0	0	0	0	0	0	0	0	0
09:45	28	9	5	1	0	43	1	0	0	1	0	2	1	0	0	0	0	1
H/TOT	96	35	17	10	0	158	2	0	0	1	0	3	1	0	0	0	0	1
10:00	27	6	3	2	0	38	0	0	0	0	0	0	0	0	0	0	0	0
10:15	27	7	7	4	0	45	0	0	0	0	0	0	0	0	0	0	0	0
10:30	23	13	5	2	0	43	0	0	0	0	0	0	1	0	0	0	0	1
10:45	16	9	3	2	0	30	1	0	0	0	0	1	0	0	0	0	0	0
H/TOT	93	35	18	10	0	156	1	0	0	0	0	1	1	0	0	0	0	1
11:00	21	11	5	2	0	39	1	0	0	0	0	1	0	0	0	0	0	0
11:15	20	14	7	2	0	43	1	0	0	0	0	1	0	0	0	0	0	0
11:30	25	12	4	2	0	43	2	1	0	0	0	3	0	0	0	0	0	0
11:45	29	5	5	1	0	40	1	0	1	0	0	2	0	0	0	0	0	0
H/TOT	95	42	21	7	0	165	5	1	1	0	0	7	0	0	0	0	0	0
12:00	27	19	5	0	0	51	3	0	3	1	0	7	0	0	0	0	0	0
12:15	26	6	5	5	0	42	0	0	0	0	0	0	0	0	0	0	0	0
12:30	55	14	4	2	0	75	1	0	0	0	0	1	0	0	0	0	0	0
12:45	45	13	3	2	0	63	1	0	0	0	0	1	1	0	1	0	0	2
H/TOT	153	52	17	9	0	231	5	0	3	1	0	9	1	0	1	0	0	2

ABACUS TRANSPORTATION SURVEYS

LITTLE ISLAND TRAFFIC COUNTS
MANUAL CLASSIFIED JUNCTION COUNTS

JANAUARY 2008
ATH/08/006

SITE: 02

DATE: 16th January 2008

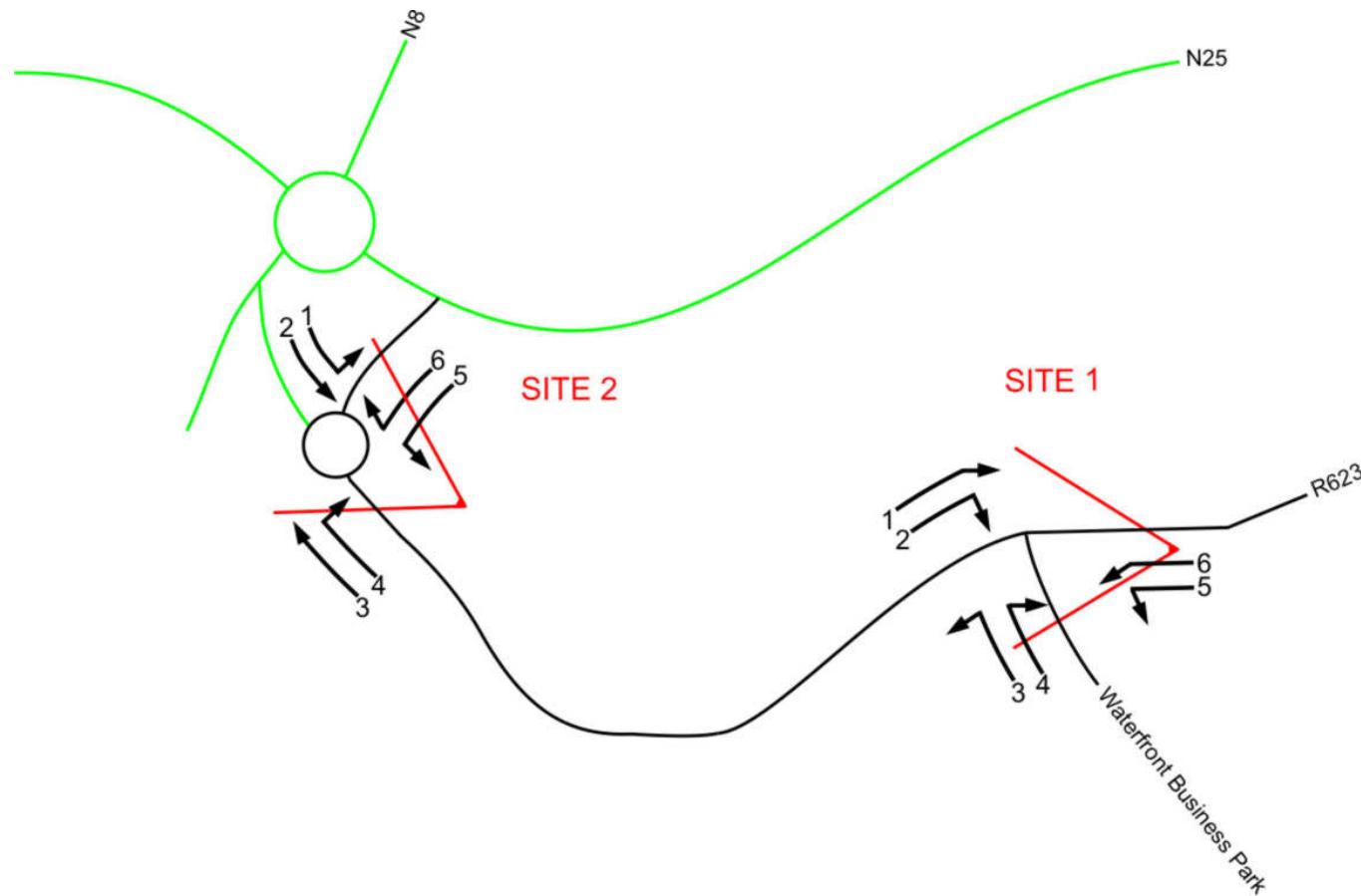
LOCATION: R623/Waterfront Business Park

DAY: Wednesday

TIME	MOVEMENT 4					TOT	MOVEMENT 5					TOT	MOVEMENT 6					TOT
	CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS	
13:00	66	6	5	0	0	77	1	0	0	0	0	1	0	0	0	0	0	
13:15	32	2	2	0	0	36	1	0	0	0	0	1	0	0	0	0	0	
13:30	22	1	4	1	0	28	0	0	0	0	0	0	0	0	0	0	0	
13:45	32	3	2	1	0	38	0	1	0	0	0	1	0	0	0	0	0	
H/TOT	152	12	13	2	0	179	2	1	0	0	0	3	0	0	0	0	0	
14:00	39	4	5	2	0	50	2	0	0	0	0	2	0	0	0	0	0	
14:15	36	4	7	0	0	47	1	0	1	0	0	2	0	0	0	0	0	
14:30	43	12	9	3	0	67	2	0	0	0	0	2	1	0	0	0	1	
14:45	28	9	5	1	0	43	0	0	0	0	0	0	0	0	0	0	0	
H/TOT	146	29	26	6	0	207	5	0	1	0	0	6	1	0	0	0	1	
15:00	46	11	2	1	0	60	0	0	0	0	0	0	0	0	0	0	0	
15:15	38	11	2	4	0	55	0	0	0	0	0	0	0	0	0	0	0	
15:30	53	16	4	3	0	76	0	0	0	0	0	0	0	0	0	0	0	
15:45	41	13	1	4	0	59	1	0	0	0	0	1	0	0	0	0	0	
H/TOT	178	51	9	12	0	250	1	0	0	0	0	1	0	0	0	0	0	
16:00	86	15	6	1	0	108	2	0	0	0	0	2	0	0	0	0	0	
16:15	85	20	1	1	0	107	1	0	0	0	0	1	0	0	0	0	0	
16:30	144	26	2	0	0	172	0	0	0	0	0	0	0	0	0	0	0	
16:45	113	21	2	4	0	140	1	0	0	0	0	1	0	0	0	0	0	
H/TOT	428	82	11	6	0	527	4	0	0	0	0	4	0	0	0	0	0	
17:00	196	23	2	0	1	222	0	0	0	0	0	0	0	0	0	0	0	
17:15	141	18	3	2	0	164	0	0	0	0	0	0	0	0	0	0	0	
17:30	168	16	5	1	1	191	0	0	0	0	0	0	0	0	0	0	0	
17:45	127	9	0	0	0	136	0	0	0	0	0	0	0	0	0	0	0	
H/TOT	632	66	10	3	2	713	0	0	0	0	0	0	0	0	0	0	0	
18:00	98	4	5	1	0	108	0	0	0	0	0	0	0	0	0	0	0	
18:15	80	6	3	2	0	91	2	0	0	0	0	2	0	0	0	0	0	
18:30	45	3	3	1	0	52	1	0	0	0	0	1	0	0	0	0	0	
18:45	40	3	1	0	0	44	2	0	0	0	0	2	0	0	0	0	0	
H/TOT	263	16	12	4	0	295	5	0	0	0	0	5	0	0	0	0	0	
P/TOT	2660	497	196	90	2	3445	37	5	5	3	0	50	4	0	1	0	5	

Appendix B – Traffic Count Data for R623 at Roundabout

Movement Numbers & Directions



	Job number: ATH/08/006	Job date: 16 th January 2008	Drawing No: ATH/08/006-2	
Client: PMCE Ltd	Job day Wednesday		Author: BCK	Transportation Surveys

ABACUS TRANSPORTATION SURVEYS

LITTLE ISLAND TRAFFIC COUNTS
MANUAL CLASSIFIED JUNCTION COUNTS

JANAUARY 2008
ATH/08/006

SITE: 01

DATE: 16th January 2008

LOCATION: Link to Jack Lynch Tunnel/Link to N25

DAY: Wednesday

TIME	MOVEMENT 1					TOT	MOVEMENT 2					TOT	MOVEMENT 3					TOT
	CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS	
07:00	14	3	0	0	0	17	23	2	0	0	0	25	0	0	1	1	0	2
07:15	42	8	1	0	0	51	39	3	0	0	0	42	7	0	2	0	0	9
07:30	47	6	1	2	0	56	22	3	0	0	0	25	2	1	1	0	0	4
07:45	59	9	3	4	0	75	44	5	3	0	0	52	5	2	1	1	0	9
H/TOT	162	26	5	6	0	199	128	13	3	0	0	144	14	3	5	2	0	24
08:00	66	11	1	0	0	78	33	2	2	0	0	37	3	3	4	2	0	12
08:15	81	10	2	3	0	96	24	6	0	0	0	30	4	6	4	0	0	14
08:30	91	5	1	0	0	97	20	5	3	0	0	28	2	2	0	0	0	4
08:45	85	11	4	0	0	100	32	3	5	1	0	41	5	4	0	0	0	9
H/TOT	323	37	8	3	0	371	109	16	10	1	0	136	14	15	8	2	0	39
09:00	53	6	2	0	0	61	23	4	0	1	0	28	4	4	2	1	0	11
09:15	40	5	0	0	0	45	8	3	1	0	0	12	0	3	0	1	0	4
09:30	22	5	4	1	0	32	8	2	2	1	0	13	6	4	2	2	0	14
09:45	31	3	6	1	0	41	9	4	2	1	0	16	3	4	0	0	0	7
H/TOT	146	19	12	2	0	179	48	13	5	3	0	69	13	15	4	4	0	36
10:00	9	3	7	0	0	19	2	3	2	2	0	9	4	3	2	2	0	11
10:15	17	3	3	0	0	23	11	1	3	2	0	17	3	1	2	2	0	8
10:30	17	3	1	0	0	21	7	0	1	0	0	8	6	5	3	1	0	15
10:45	22	6	1	0	0	29	4	2	2	1	0	9	2	1	1	2	0	6
H/TOT	65	15	12	0	0	92	24	6	8	5	0	43	15	10	8	7	0	40
11:00	19	3	2	1	0	25	4	3	0	2	0	9	4	2	1	2	0	9
11:15	18	6	2	1	0	27	3	3	2	1	0	9	8	6	3	2	0	19
11:30	19	4	3	2	0	28	6	3	3	0	0	12	2	2	1	2	0	7
11:45	21	1	1	0	0	23	4	2	1	2	0	9	5	8	2	1	0	16
H/TOT	77	14	8	4	0	103	17	11	6	5	0	39	19	18	7	7	0	51
12:00	15	4	6	0	0	25	6	4	1	2	0	13	6	5	2	0	0	13
12:15	18	5	2	0	0	25	6	5	0	0	0	11	8	2	1	3	0	14
12:30	20	6	2	0	0	28	0	0	1	0	0	1	8	4	1	1	0	14
12:45	25	3	1	0	0	29	1	1	0	1	0	3	11	5	1	1	0	18
H/TOT	78	18	11	0	0	107	13	10	2	3	0	28	33	16	5	5	0	59

ABACUS TRANSPORTATION SURVEYS

LITTLE ISLAND TRAFFIC COUNTS
MANUAL CLASSIFIED JUNCTION COUNTS

JANAUARY 2008
ATH/08/006

SITE: 01

DATE: 16th January 2008

LOCATION: Link to Jack Lynch Tunnel/Link to N25

DAY: Wednesday

TIME	MOVEMENT 1					TOT	MOVEMENT 2					TOT	MOVEMENT 3					TOT
	CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS	
13:00	30	4	2	0	0	36	6	2	1	1	0	10	10	5	1	0	0	16
13:15	19	2	1	2	0	24	7	1	0	0	0	8	7	2	2	0	0	11
13:30	21	5	0	0	0	26	10	0	3	1	0	14	3	1	1	0	0	5
13:45	24	10	0	0	0	34	7	4	3	0	0	14	3	1	1	1	0	6
H/TOT	94	21	3	2	0	120	30	7	7	2	0	46	23	9	5	1	0	38
14:00	21	3	2	1	0	27	9	6	1	1	0	17	5	2	3	1	0	11
14:15	31	8	1	1	0	41	8	4	1	0	0	13	5	2	3	0	0	10
14:30	24	9	4	1	0	38	3	5	0	0	0	8	9	2	4	1	0	16
14:45	17	6	3	0	0	26	6	7	2	0	0	15	9	3	2	0	0	14
H/TOT	93	26	10	3	0	132	26	22	4	1	0	53	28	9	12	2	0	51
15:00	8	3	0	0	0	11	8	5	4	1	0	18	15	1	0	0	0	16
15:15	22	5	2	0	0	29	4	3	3	2	0	12	13	3	0	1	0	17
15:30	18	6	4	0	0	28	5	4	3	0	0	12	17	3	1	0	0	21
15:45	29	5	3	0	1	38	3	0	2	0	0	5	23	3	0	1	0	27
H/TOT	77	19	9	0	1	106	20	12	12	3	0	47	68	10	1	2	0	81
16:00	25	7	2	2	0	36	5	5	0	0	0	10	25	2	3	0	0	30
16:15	31	7	3	0	0	41	4	5	1	0	0	10	37	2	0	1	0	40
16:30	36	8	0	0	0	44	7	0	1	1	0	9	41	3	1	0	0	45
16:45	37	3	1	0	0	41	0	2	1	0	0	3	31	3	1	4	0	39
H/TOT	129	25	6	2	0	162	16	12	3	1	0	32	134	10	5	5	0	154
17:00	29	2	1	0	0	32	1	3	1	0	0	5	35	5	0	0	0	40
17:15	28	4	0	0	0	32	1	0	0	0	0	1	25	0	1	1	0	27
17:30	16	2	0	0	0	18	2	0	0	1	0	3	25	0	0	1	0	26
17:45	18	2	0	1	0	21	0	1	0	0	0	1	16	0	0	0	0	16
H/TOT	91	10	1	1	0	103	4	4	1	1	0	10	101	5	1	2	0	109
18:00	22	5	1	0	0	28	1	0	1	0	0	2	13	3	2	1	0	19
18:15	21	3	0	0	0	24	1	2	1	0	0	4	10	2	1	1	0	14
18:30	14	2	0	0	0	16	5	1	2	0	0	8	10	1	1	0	0	12
18:45	12	2	0	0	0	14	3	1	1	0	0	5	9	1	0	0	0	10
H/TOT	69	12	1	0	0	82	10	4	5	0	0	19	42	7	4	2	0	55
P/TOT	1404	242	86	23	1	1756	445	130	66	25	0	666	504	127	65	41	0	737

ABACUS TRANSPORTATION SURVEYS

LITTLE ISLAND TRAFFIC COUNTS
MANUAL CLASSIFIED JUNCTION COUNTS

JANAUARY 2008
ATH/08/006

SITE: 01

DATE: 16th January 2008

LOCATION: Link to Jack Lynch Tunnel/Link to N25

DAY: Wednesday

TIME	MOVEMENT 4					TOT	MOVEMENT 5					TOT	MOVEMENT 6					TOT
	CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS	
07:00	0	1	2	1	0	4	8	0	0	0	0	8	19	5	0	0	0	24
07:15	7	0	0	0	1	8	25	2	0	0	0	27	48	6	2	3	0	59
07:30	5	0	0	0	0	5	30	4	0	0	0	34	63	8	2	3	0	76
07:45	4	0	1	0	0	5	41	3	2	0	0	46	88	9	8	4	0	109
H/TOT	16	1	3	1	1	22	104	9	2	0	0	115	218	28	12	10	0	268
08:00	5	1	2	0	0	8	27	6	1	0	0	34	97	10	4	1	0	112
08:15	2	3	1	0	0	6	30	1	1	0	0	32	91	20	6	1	0	118
08:30	2	0	0	0	0	2	13	1	0	1	0	15	58	17	6	1	0	82
08:45	4	2	1	0	0	7	26	1	0	2	0	29	34	9	7	5	0	55
H/TOT	13	6	4	0	0	23	96	9	2	3	0	110	280	56	23	8	0	367
09:00	1	5	3	1	0	10	15	5	1	1	0	22	27	10	2	1	0	40
09:15	3	5	1	0	0	9	10	3	2	0	0	15	21	12	2	1	0	36
09:30	5	2	1	0	0	8	7	1	7	1	0	16	15	12	1	2	0	30
09:45	7	3	7	1	0	18	11	5	4	0	0	20	25	8	5	0	0	38
H/TOT	16	15	12	2	0	45	43	14	14	2	0	73	88	42	10	4	0	144
10:00	5	1	2	0	0	8	7	4	5	3	0	19	23	10	7	2	0	42
10:15	5	0	4	0	0	9	10	3	3	0	0	16	12	8	6	1	0	27
10:30	2	6	4	0	0	12	2	2	0	0	0	4	24	12	3	1	0	40
10:45	3	2	2	2	0	9	7	1	5	0	0	13	15	7	1	0	0	23
H/TOT	15	9	12	2	0	38	26	10	13	3	0	52	74	37	17	4	0	132
11:00	9	1	2	1	0	13	6	5	1	1	0	13	26	6	4	1	0	37
11:15	5	1	2	1	0	9	7	5	2	0	0	14	22	5	3	0	0	30
11:30	4	2	1	0	0	7	1	1	3	1	0	6	22	4	3	0	0	29
11:45	4	1	1	2	0	8	11	2	2	0	0	15	25	9	2	0	0	36
H/TOT	22	5	6	4	0	37	25	13	8	2	0	48	95	24	12	1	0	132
12:00	6	7	2	0	0	15	6	8	2	0	0	16	17	7	3	0	0	27
12:15	11	3	1	0	1	16	4	2	3	0	0	9	33	5	4	2	0	44
12:30	8	5	2	0	0	15	9	8	2	1	0	20	39	8	4	1	0	52
12:45	29	7	1	1	0	38	5	4	2	0	0	11	41	2	2	1	0	46
H/TOT	54	22	6	1	1	84	24	22	9	1	0	56	130	22	13	4	0	169

ABACUS TRANSPORTATION SURVEYS

LITTLE ISLAND TRAFFIC COUNTS
MANUAL CLASSIFIED JUNCTION COUNTS

JANAUARY 2008
ATH/08/006

SITE: 01

DATE: 16th January 2008

LOCATION: Link to Jack Lynch Tunnel/Link to N25

DAY: Wednesday

TIME	MOVEMENT 4					TOT	MOVEMENT 5					TOT	MOVEMENT 6					TOT
	CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS		CAR	LGV	OGV1	OGV2	BUS	
13:00	16	3	1	0	0	20	18	1	0	1	0	20	39	8	4	0	0	51
13:15	8	3	0	0	0	11	14	5	0	0	0	19	22	2	0	0	0	24
13:30	6	1	0	0	0	7	10	3	1	0	0	14	23	7	3	1	0	34
13:45	8	2	1	0	0	11	15	4	1	3	0	23	31	5	0	0	0	36
H/TOT	38	9	2	0	0	49	57	13	2	4	0	76	115	22	7	1	0	145
14:00	10	4	4	0	0	18	22	3	5	1	0	31	26	6	3	1	0	36
14:15	9	3	2	2	0	16	9	1	4	1	0	15	29	8	4	0	0	41
14:30	10	4	2	0	0	16	9	5	6	0	0	20	33	9	5	2	0	49
14:45	7	3	1	0	0	11	7	6	2	1	0	16	31	7	3	1	0	42
H/TOT	36	14	9	2	0	61	47	15	17	3	0	82	119	30	15	4	0	168
15:00	8	4	2	1	0	15	11	2	1	0	0	14	19	7	2	0	0	28
15:15	7	8	3	1	0	19	8	3	1	1	0	13	29	10	2	4	0	45
15:30	11	5	1	1	0	18	11	2	2	0	0	15	27	4	3	3	0	37
15:45	11	3	2	0	0	16	7	5	3	1	0	16	39	13	1	3	0	56
H/TOT	37	20	8	3	0	68	37	12	7	2	0	58	114	34	8	10	0	166
16:00	8	3	1	1	0	13	9	2	3	0	0	14	37	6	3	1	0	47
16:15	19	5	0	0	0	24	10	5	1	0	0	16	43	9	1	0	0	53
16:30	24	3	0	1	0	28	6	2	0	0	0	8	55	7	1	0	0	63
16:45	27	7	2	0	0	36	5	3	3	0	0	11	69	6	2	0	1	78
H/TOT	78	18	3	2	0	101	30	12	7	0	0	49	204	28	7	1	1	241
17:00	26	5	0	1	0	32	2	0	0	0	0	2	107	10	2	0	0	119
17:15	10	5	0	0	0	15	3	1	2	1	0	7	108	10	2	1	0	121
17:30	20	4	2	0	0	26	2	0	0	1	0	3	134	10	5	0	0	149
17:45	12	3	0	0	0	15	2	0	0	0	0	2	65	7	0	0	0	72
H/TOT	68	17	2	1	0	88	9	1	2	2	0	14	414	37	9	1	0	461
18:00	7	0	0	0	0	7	4	2	0	0	0	6	58	8	3	0	0	69
18:15	1	1	0	0	0	2	3	1	1	0	0	5	49	6	2	1	0	58
18:30	5	1	0	0	0	6	1	0	1	0	0	2	46	9	2	1	0	58
18:45	2	1	0	0	0	3	0	0	0	0	0	0	22	5	1	0	0	28
H/TOT	15	3	0	0	0	18	8	3	2	0	0	13	175	28	8	2	0	213
P/TOT	408	139	67	18	2	634	506	133	85	22	0	746	2026	388	141	50	1	2606

Appendix C – PICADY Outputs for 2008

TRL LIMITED

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CAPACITIES, QUEUES, AND DELAYS AT 3 OR 4-ARM MAJOR/MINOR PRIORITY JUNCTIONS

PICADY 5.0 ANALYSIS PROGRAM
RELEASE 3.0 (JUNE 2006)

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TRL SOFTWARE BUREAU
TEL: CROWTHORNE (01344) 770758, FAX: 770864
EMAIL: SoftwareBureau@trl.co.uk

THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF HIS RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

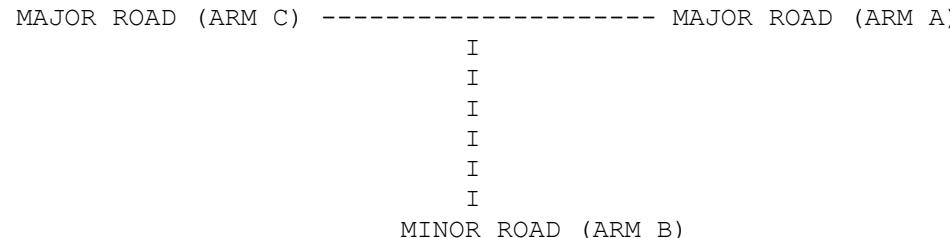
Run with file:-
"\\Pmcesrv1\\project\\2007\\0710\\PICADY\\0710_PICADY_2008 Assessments.vpi"
(drive-on-the-left) at 11:06:27 on Friday, 29 February 2008

.RUN INFORMATION

RUN TITLE: Little Island Traffic Assessment
LOCATION: Little Island, Co. Cork
DATE: 28/02/08
CLIENT: Golder Associates Ireland
ENUMERATOR: DGF
JOB NUMBER: 0710
STATUS: Draft 2
DESCRIPTION: 2008 Assessments

.MAJOR/MINOR JUNCTION CAPACITY AND DELAY

INPUT DATA



ARM A IS R623 East
ARM B IS Site Access
ARM C IS R623 West

STREAM LABELLING CONVENTION

STREAM A-B CONTAINS TRAFFIC GOING FROM ARM A TO ARM B

STREAM B-AC CONTAINS TRAFFIC GOING FROM ARM B TO ARM A AND TO ARM C

ETC

GEOMETRIC DATA

I	DATA ITEM	I	MINOR ROAD B	I
I	TOTAL MAJOR ROAD CARRIAGeway WIDTH	I (W)	7.30 M.	I
I	CENTRAL RESERVE WIDTH	I (WCR)	0.00 M.	I
I		I		I
I	MAJOR ROAD RIGHT TURN - WIDTH	I (WC-B)	2.20 M.	I
I	- VISIBILITY	I (VC-B)	60.0 M.	I
I	- BLOCKS TRAFFIC	I	YES	I
I		I		I
I	MINOR ROAD - VISIBILITY TO LEFT	I (VB-C)	80.0 M.	I
I	- VISIBILITY TO RIGHT	I (VB-A)	80.0 M.	I
I	- LANE 1 WIDTH	I (WB-C)	3.00 M.	I
I	- LANE 2 WIDTH	I (WB-A)	0.00 M.	I

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	674.30	0.25	0.10

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	543.66	0.24	0.09	0.15	0.34	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	I
I	608.71	0.22	0.22	I

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2008 AM Peak Traffic Flows (50 loads) (100% West) (Ex HH dev)

TIME PERIOD BEGINS 07.45 AND ENDS 08.45

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 07.45 - 08.45 I I I I I I
I I ARM A I 0.000 I 0.010 I 0.990 I
I I 0.0 I 4.0 I 380.0 I
I I (0.0)I (0.0)I (10.0)I
I I I I I I
I I ARM B I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 8.0 I
I I (0.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.977 I 0.023 I 0.000 I
I I 676.0 I 16.0 I 0.0 I
I I (4.0)I (73.0)I (0.0)I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2008 AM Peak Traffic Flows (50 loads) (100% West) (Ex HH dev)
AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-AC	0.13	4.77	0.028		0.00	0.03	0.4		0.22	I
I	C-AB	0.27	4.97	0.054		0.00	0.06	0.9		0.21	I
I	A-B	0.07									I
I	A-C	6.27									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-AC	0.13	4.77	0.028		0.03	0.03	0.4		0.22	I
I	C-AB	0.27	4.97	0.054		0.06	0.06	0.9		0.21	I
I	A-B	0.07									I
I	A-C	6.27									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-AC	0.13	4.77	0.028		0.03	0.03	0.4		0.22	I
I	C-AB	0.27	4.97	0.054		0.06	0.06	1.0		0.21	I
I	A-B	0.07									I
I	A-C	6.27									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-AC	0.13	4.77	0.028		0.03	0.03	0.4		0.22	I
I	C-AB	0.27	4.97	0.054		0.06	0.06	1.0		0.21	I
I	A-B	0.07									I
I	A-C	6.27									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(MIN)	I	(MIN/VEH)	I						
I	B-AC	I	8.0	I	8.0	I	1.7	I	0.21	I	1.7	I	0.21	I
I	C-AB	I	16.0	I	16.0	I	3.8	I	0.24	I	3.8	I	0.24	I
I	A-B	I	4.0	I	4.0	I		I		I		I		I
I	A-C	I	376.0	I	376.0	I		I		I		I		I
I	ALL	I	1080.0	I	1080.0	I	5.5	I	0.01	I	5.5	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I Intercept For Stream B-C	Slope For Stream A-C	Slope For Stream A-B	I
I 674.30	0.25	0.10	I

I Intercept For Stream B-A	Slope For Stream A-C	Slope For Stream A-B	Slope For Stream C-A	Slope For Stream C-B	I
I 543.66	0.24	0.09	0.15	0.34	I

I Intercept For Stream C-B	Slope For Stream A-C	Slope For Stream A-B	I
I 608.71	0.22	0.22	I

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2008 PM Peak Traffic Flows Demand Set (50 loads) (100% West) (Ex HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 700.0 I
I I (0.0) I (0.0) I (2.0) I
I I I I I I
I I ARM B I 0.200 I 0.000 I 0.800 I
I I 4.0 I 0.0 I 16.0 I
I I (0.0) I (0.0) I (73.0) I
I I I I I I
I I ARM C I 0.913 I 0.087 I 0.000 I
I I 84.0 I 8.0 I 0.0 I
I I (7.0) I (100.0) I (0.0) I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2008 PM Peak Traffic Flows Demand Set (50 loads) (100% West) (Ex HH dev)
AND FOR TIME PERIOD 2

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.33	4.99	0.067		0.00	0.07	1.0		0.21	I
I	C-AB	0.13	3.75	0.036		0.00	0.04	0.5		0.28	I
I	A-B	0.00									I
I	A-C	11.67									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.33	4.99	0.067		0.07	0.07	1.1		0.21	I
I	C-AB	0.13	3.75	0.036		0.04	0.04	0.6		0.28	I
I	A-B	0.00									I
I	A-C	11.67									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.33	4.99	0.067		0.07	0.07	1.1		0.21	I
I	C-AB	0.13	3.75	0.036		0.04	0.04	0.6		0.28	I
I	A-B	0.00									I
I	A-C	11.67									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START (VEHS)	END (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30					0.07	0.07	1.1		0.21	I
I	B-AC	0.33	4.99	0.067							I
I	C-AB	0.13	3.75	0.036		0.04	0.04	0.6		0.28	I
I	A-B	0.00									I
I	A-C	11.67									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
16.45	0.0
17.00	0.0
17.15	0.0
17.30	0.0

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I				
I		I		I		I		I		I				
I	B-AC	I	20.0	I	20.0	I	4.2	I	0.21	I	4.2	I	0.21	I
I	C-AB	I	8.0	I	8.0	I	2.2	I	0.28	I	2.2	I	0.28	I
I	A-B	I	0.0	I	0.0	I		I		I		I		I
I	A-C	I	700.0	I	700.0	I		I		I		I		I
I	ALL	I	812.0	I	812.0	I	6.4	I	0.01	I	6.4	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I Intercept For Stream B-C	Slope For Stream A-C	Slope For Stream A-B	I
I 674.30	0.25	0.10	I

I Intercept For Stream B-A	Slope For Stream A-C	Slope For Stream A-B	Slope For Stream C-A	Slope For Stream C-B	I
I 543.66	0.24	0.09	0.15	0.34	I

I Intercept For Stream C-B	Slope For Stream A-C	Slope For Stream A-B	I
I 608.71	0.22	0.22	I

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2008 AM Peak Traffic Flows (100 loads) (80/20 West/East) (Ex HH dev)

TIME PERIOD BEGINS 07.45 AND ENDS 08.45

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 07.45 - 08.45 I I I I I
I I ARM A I 0.000 I 0.016 I 0.984 I
I I 0.0 I 6.0 I 376.0 I
I I (0.0)I (33.0)I (10.0)I
I I I I I I
I I ARM B I 0.133 I 0.000 I 0.867 I
I I 2.0 I 0.0 I 13.0 I
I I (100.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.974 I 0.026 I 0.000 I
I I 676.0 I 18.0 I 0.0 I
I I (4.0)I (72.0)I (0.0)I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2008 AM Peak Traffic Flows (100 loads) (80/20 West/East) (Ex HH dev)
AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-AC	0.27	4.34	0.062		0.00	0.06	0.9		0.25	I
I	C-AB	0.30	4.98	0.060		0.00	0.07	1.1		0.21	I
I	A-B	0.10									I
I	A-C	6.30									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-AC	0.27	4.33	0.062		0.06	0.06	1.0		0.25	I
I	C-AB	0.30	4.98	0.060		0.07	0.07	1.1		0.21	I
I	A-B	0.10									I
I	A-C	6.30									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-AC	0.27	4.33	0.062		0.06	0.07	1.0		0.25	I
I	C-AB	0.30	4.98	0.060		0.07	0.07	1.1		0.21	I
I	A-B	0.10									I
I	A-C	6.30									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-AC	0.27	4.33	0.062		0.07	0.07	1.0		0.25	I
I	C-AB	0.30	4.98	0.060		0.07	0.07	1.1		0.21	I
I	A-B	0.10									I
I	A-C	6.30									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	16.0	I	16.0	I	3.8	I	0.24	I	3.8	I	0.24	I
I	C-AB	I	18.1	I	18.1	I	4.3	I	0.24	I	4.3	I	0.24	I
I	A-B	I	6.0	I	6.0	I		I		I		I		I
I	A-C	I	378.0	I	378.0	I		I		I		I		I
I	ALL	I	1096.0	I	1096.0	I	8.2	I	0.01	I	8.2	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I Intercept For Stream B-C	Slope For Stream A-C	Slope For Stream A-B	I
I 674.30	0.25	0.10	I

I Intercept For Stream B-A	Slope For Stream A-C	Slope For Stream A-B	Slope For Stream C-A	Slope For Stream C-B	I
I 543.66	0.24	0.09	0.15	0.34	I

I Intercept For Stream C-B	Slope For Stream A-C	Slope For Stream A-B	I
I 608.71	0.22	0.22	I

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2008 PM Peak Traffic Flows Demand Set (100 loads) (80/20 West/East) (Ex HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.003 I 0.997 I
I I 0.0 I 2.0 I 700.0 I
I I (0.0)I (100.0)I (2.0)I
I I I I I I
I I ARM B I 0.250 I 0.000 I 0.750 I
I I 6.0 I 0.0 I 18.0 I
I I (33.0)I (0.0)I (72.0)I
I I I I I I
I I ARM C I 0.866 I 0.134 I 0.000 I
I I 84.0 I 13.0 I 0.0 I
I I (7.0)I (100.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2008 PM Peak Traffic Flows Demand Set (100 loads) (80/20 West/East) (Ex HH dev)
AND FOR TIME PERIOD 2

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.47	4.44	0.105		0.00	0.12	1.6		0.25	I
I	C-AB	0.87	3.74	0.232		0.00	0.39	5.5		0.34	I
I	A-B	0.03									I
I	A-C	11.70									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.47	4.43	0.105		0.12	0.12	1.7		0.25	I
I	C-AB	0.87	3.74	0.232		0.39	0.40	6.1		0.35	I
I	A-B	0.03									I
I	A-C	11.70									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.47	4.43	0.105		0.12	0.12	1.8		0.25	I
I	C-AB	0.87	3.74	0.232		0.40	0.40	6.1		0.35	I
I	A-B	0.03									I
I	A-C	11.70									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.47	4.43	0.105		0.12	0.12	1.8		0.25	I
I	C-AB	0.87	3.74	0.232		0.40	0.40	6.1		0.35	I
I	A-B	0.03									I
I	A-C	11.70									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.4
17.00	0.4
17.15	0.4
17.30	0.4

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I				
I		I		I		I		I		I				
I	B-AC	I	28.0	I	28.0	I	6.9	I	0.25	I	6.9	I	0.25	I
I	C-AB	I	52.0	I	52.0	I	23.7	I	0.46	I	23.8	I	0.46	I
I	A-B	I	2.0	I	2.0	I		I		I		I		I
I	A-C	I	702.0	I	702.0	I		I		I		I		I
I	ALL	I	1120.0	I	1120.0	I	30.6	I	0.03	I	30.6	I	0.03	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	674.30	0.25	0.10

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	543.66	0.24	0.09	0.15	0.34	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2008 AM Peak Traffic Flows (100 loads) (100% West) (Ex HH dev)

TIME PERIOD BEGINS 07.45 AND ENDS 08.45

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 07.45 - 08.45 I I I I I
I I ARM A I 0.000 I 0.011 I 0.989 I
I I 0.0 I 4.0 I 376.0 I
I I (0.0)I (0.0)I (10.0)I
I I I I I I
I I ARM B I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 15.0 I
I I (0.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.971 I 0.029 I 0.000 I
I I 676.0 I 20.0 I 0.0 I
I I (4.0)I (75.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2008 AM Peak Traffic Flows (100 loads) (100% West) (Ex HH dev)
AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-AC	0.27	4.77	0.056		0.00	0.06	0.8		0.22	I
I	C-AB	0.33	4.91	0.068		0.00	0.08	1.2		0.22	I
I	A-B	0.07									I
I	A-C	6.27									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-AC	0.27	4.77	0.056		0.06	0.06	0.9		0.22	I
I	C-AB	0.33	4.91	0.068		0.08	0.08	1.3		0.22	I
I	A-B	0.07									I
I	A-C	6.27									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-AC	0.27	4.77	0.056		0.06	0.06	0.9		0.22	I
I	C-AB	0.33	4.91	0.068		0.08	0.08	1.3		0.22	I
I	A-B	0.07									I
I	A-C	6.27									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-AC	0.27	4.77	0.056		0.06	0.06	0.9		0.22	I
I	C-AB	0.33	4.91	0.068		0.08	0.08	1.3		0.22	I
I	A-B	0.07									I
I	A-C	6.27									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I				
I		I		I		I		I		I				
I	B-AC	I	16.0	I	16.0	I	3.5	I	0.22	I	3.5	I	0.22	I
I	C-AB	I	20.0	I	20.0	I	5.0	I	0.25	I	5.0	I	0.25	I
I	A-B	I	4.0	I	4.0	I		I		I		I		I
I	A-C	I	376.0	I	376.0	I		I		I		I		I
I	ALL	I	1092.0	I	1092.0	I	8.5	I	0.01	I	8.5	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I Intercept For Stream B-C	Slope For Stream A-C	Slope For Stream A-B	I
I 674.30	0.25	0.10	I

I Intercept For Stream B-A	Slope For Stream A-C	Slope For Stream A-B	Slope For Stream C-A	Slope For Stream C-B	I
I 543.66	0.24	0.09	0.15	0.34	I

I Intercept For Stream C-B	Slope For Stream A-C	Slope For Stream A-B	I
I 608.71	0.22	0.22	I

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2008 PM Peak Traffic Flows Demand Set (100 loads) (100% West) (Ex HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 700.0 I
I I (0.0)I (0.0)I (2.0)I
I I I I I I
I I ARM B I 0.167 I 0.000 I 0.833 I
I I 4.0 I 0.0 I 20.0 I
I I (0.0)I (0.0)I (75.0)I
I I I I I I
I I ARM C I 0.848 I 0.152 I 0.000 I
I I 84.0 I 15.0 I 0.0 I
I I (7.0)I (100.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2008 PM Peak Traffic Flows Demand Set (100 loads) (100% West) (Ex HH dev)
AND FOR TIME PERIOD 2

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.40	4.90	0.082		0.00	0.09	1.2		0.22	I
I	C-AB	0.25	3.75	0.067		0.00	0.07	1.1		0.29	I
I	A-B	0.00									I
I	A-C	11.67									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.40	4.90	0.082		0.09	0.09	1.3		0.22	I
I	C-AB	0.25	3.75	0.067		0.07	0.07	1.1		0.29	I
I	A-B	0.00									I
I	A-C	11.67									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.40	4.90	0.082		0.09	0.09	1.3		0.22	I
I	C-AB	0.25	3.75	0.067		0.07	0.07	1.1		0.29	I
I	A-B	0.00									I
I	A-C	11.67									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.40	4.90	0.082		0.09	0.09	1.3		0.22	I
I	C-AB	0.25	3.75	0.067		0.07	0.07	1.1		0.29	I
I	A-B	0.00									I
I	A-C	11.67									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	24.0	I	24.0	I	5.2	I	0.22	I	5.2	I	0.22	I
I	C-AB	I	15.2	I	15.2	I	4.4	I	0.29	I	4.4	I	0.29	I
I	A-B	I	0.0	I	0.0	I		I		I		I		I
I	A-C	I	700.0	I	700.0	I		I		I		I		I
I	ALL	I	824.0	I	824.0	I	9.6	I	0.01	I	9.6	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

Appendix D – PICADY Outputs For 2009

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CAPACITIES, QUEUES, AND DELAYS AT 3 OR 4-ARM MAJOR/MINOR PRIORITY JUNCTIONS

PICADY 5.0 ANALYSIS PROGRAM
RELEASE 3.0 (JUNE 2006)

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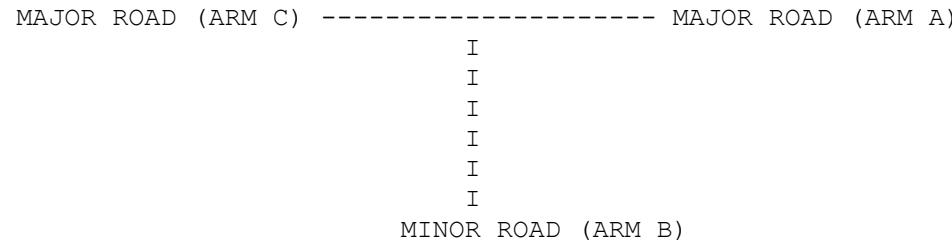
THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF HIS RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"\\"Pmcesrv1\project\2007\0710\PICADY\0710_PICADY_2009 Assessments.vpi"
(drive-on-the-left) at 11:04:23 on Friday, 29 February 2008

.RUN INFORMATION

RUN TITLE: Little Island Traffic Assessment
LOCATION: Little Island, Co. Cork
DATE: 28/02/08
CLIENT: Golder Associates Ireland
ENUMERATOR: DGF
JOB NUMBER: 0710
STATUS: Draft 2
DESCRIPTION: 2009 Assessments
MAJOR/MINOR JUNCTION CAPACITY AND DELAY

INPUT DATA



ARM A IS R623 East
ARM B IS Site Access
ARM C IS R623 West

STREAM LABELLING CONVENTION

STREAM A-B CONTAINS TRAFFIC GOING FROM ARM A TO ARM B

STREAM B-AC CONTAINS TRAFFIC GOING FROM ARM B TO ARM A AND TO ARM C

.GEOMETRIC DATA

I	DATA ITEM	I	MINOR ROAD B	I
I	TOTAL MAJOR ROAD CARRIAGEWAY WIDTH	I	(W) 7.30 M.	I
I	CENTRAL RESERVE WIDTH	I	(WCR) 0.00 M.	I
I		I		I
I	MAJOR ROAD RIGHT TURN - WIDTH	I	(WC-B) 2.20 M.	I
I	- VISIBILITY	I	(VC-B) 60.0 M.	I
I	- BLOCKS TRAFFIC	I	YES	I
I		I		I
I	MINOR ROAD - VISIBILITY TO LEFT	I	(VB-C) 10.0 M.	I
I	- VISIBILITY TO RIGHT	I	(VB-A) 11.0 M.	I
I	- LANE 1 WIDTH	I	(WB-C) 3.00 M.	I
I	- LANE 2 WIDTH	I	(WB-A) 0.00 M.	I

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2009 AM Peak Traffic Flows (50 loads) (100% West) (Ex HH dev)

TIME PERIOD BEGINS 07.45 AND ENDS 08.45

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 07.45 - 08.45 I I I I I
I I ARM A I 0.000 I 0.010 I 0.990 I
I I 0.0 I 4.0 I 380.0 I
I I (0.0)I (0.0)I (10.0)I
I I I I I I
I I ARM B I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 12.0 I
I I (0.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.976 I 0.024 I 0.000 I
I I 688.0 I 17.0 I 0.0 I
I I (3.0)I (73.0)I (0.0)I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2009 AM Peak Traffic Flows (50 loads) (100% West) (Ex HH dev)
AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-AC	0.20	4.45	0.045		0.00	0.05	0.7		0.23	I
I	C-AB	0.28	4.96	0.057		0.00	0.07	1.0		0.21	I
I	A-B	0.07									I
I	A-C	6.33									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-AC	0.20	4.45	0.045		0.05	0.05	0.7		0.24	I
I	C-AB	0.28	4.96	0.057		0.07	0.07	1.0		0.21	I
I	A-B	0.07									I
I	A-C	6.33									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-AC	0.20	4.45	0.045		0.05	0.05	0.7		0.24	I
I	C-AB	0.28	4.96	0.057		0.07	0.07	1.0		0.21	I
I	A-B	0.07									I
I	A-C	6.33									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-AC	0.20	4.45	0.045		0.05	0.05	0.7		0.24	I
I	C-AB	0.28	4.96	0.057		0.07	0.07	1.0		0.21	I
I	A-B	0.07									I
I	A-C	6.33									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	12.0	I	12.0	I	2.8	I	0.23	I	2.8	I	0.23	I
I	C-AB	I	17.0	I	17.0	I	4.1	I	0.24	I	4.1	I	0.24	I
I	A-B	I	4.0	I	4.0	I		I		I		I		I
I	A-C	I	380.0	I	380.0	I		I		I		I		I
I	ALL	I	1100.0	I	1100.0	I	6.8	I	0.01	I	6.8	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2009 PM Peak Traffic Flows Demand Set (50 loads) (100% West) (Ex HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 712.0 I
I I (0.0)I (0.0)I (2.0)I
I I I I I I
I I ARM B I 0.190 I 0.000 I 0.810 I
I I 4.0 I 0.0 I 17.0 I
I I (0.0)I (0.0)I (73.0)I
I I I I I I
I I ARM C I 0.875 I 0.125 I 0.000 I
I I 84.0 I 12.0 I 0.0 I
I I (7.0)I (100.0)I (0.0)I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2009 PM Peak Traffic Flows Demand Set (50 loads) (100% West) (Ex HH dev)
AND FOR TIME PERIOD 2

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.33	4.59	0.073		0.00	0.08	1.1		0.23	I
I	C-AB	0.20	3.73	0.054		0.00	0.06	0.8		0.28	I
I	A-B	0.00									I
I	A-C	11.87									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.33	4.59	0.073		0.08	0.08	1.2		0.24	I
I	C-AB	0.20	3.73	0.054		0.06	0.06	0.9		0.28	I
I	A-B	0.00									I
I	A-C	11.87									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.33	4.59	0.073		0.08	0.08	1.2		0.24	I
I	C-AB	0.20	3.73	0.054		0.06	0.06	0.9		0.28	I
I	A-B	0.00									I
I	A-C	11.87									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.33	4.59	0.073		0.08	0.08	1.2		0.24	I
I	C-AB	0.20	3.73	0.054		0.06	0.06	0.9		0.28	I
I	A-B	0.00									I
I	A-C	11.87									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	20.0	I	20.0	I	4.6	I	0.23	I	4.6	I	0.23	I
I	C-AB	I	12.0	I	12.0	I	3.4	I	0.29	I	3.4	I	0.29	I
I	A-B	I	0.0	I	0.0	I		I		I		I		I
I	A-C	I	712.0	I	712.0	I		I		I		I		I
I	ALL	I	828.0	I	828.0	I	8.0	I	0.01	I	8.0	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2009 AM Peak Traffic Flows (50 loads) (100% West) (Incl HH dev)

TIME PERIOD BEGINS 07.45 AND ENDS 08.45

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 07.45 - 08.45 I I I I I
I I ARM A I 0.000 I 0.010 I 0.990 I
I I 0.0 I 4.0 I 400.0 I
I I (0.0) I (0.0) I (10.0) I
I I I I I I
I I ARM B I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 12.0 I
I I (0.0) I (0.0) I (100.0) I
I I I I I I
I I ARM C I 0.980 I 0.020 I 0.000 I
I I 832.0 I 17.0 I 0.0 I
I I (4.0) I (73.0) I (0.0) I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2009 AM Peak Traffic Flows (50 loads) (100% West) (Incl HH dev)
AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-AC	0.20	4.41	0.045		0.00	0.05	0.7		0.24	I
I	C-AB	0.28	4.91	0.058		0.00	0.07	1.0		0.22	I
I	A-B	0.07									I
I	A-C	6.67									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-AC	0.20	4.41	0.045		0.05	0.05	0.7		0.24	I
I	C-AB	0.28	4.91	0.058		0.07	0.07	1.1		0.22	I
I	A-B	0.07									I
I	A-C	6.67									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-AC	0.20	4.41	0.045		0.05	0.05	0.7		0.24	I
I	C-AB	0.28	4.91	0.058		0.07	0.07	1.1		0.22	I
I	A-B	0.07									I
I	A-C	6.67									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-AC	0.20	4.41	0.045		0.05	0.05	0.7		0.24	I
I	C-AB	0.28	4.91	0.058		0.07	0.07	1.1		0.22	I
I	A-B	0.07									I
I	A-C	6.67									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	12.0	I	12.0	I	2.8	I	0.23	I	2.8	I	0.23	I
I	C-AB	I	17.0	I	17.0	I	4.2	I	0.25	I	4.2	I	0.25	I
I	A-B	I	4.0	I	4.0	I		I		I		I		I
I	A-C	I	400.0	I	400.0	I		I		I		I		I
I	ALL	I	1264.0	I	1264.0	I	7.0	I	0.01	I	7.0	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2009 PM Peak Traffic Flows Demand Set (50 loads) (100% West) (Incl HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 840.0 I
I I (0.0)I (0.0)I (2.0)I
I I I I I I
I I ARM B I 0.190 I 0.000 I 0.810 I
I I 4.0 I 0.0 I 17.0 I
I I (0.0)I (0.0)I (73.0)I
I I I I I I
I I ARM C I 0.897 I 0.103 I 0.000 I
I I 104.0 I 12.0 I 0.0 I
I I (7.0)I (100.0)I (0.0)I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2009 PM Peak Traffic Flows Demand Set (50 loads) (100% West) (Incl HH dev)
AND FOR TIME PERIOD 2

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.33	4.27	0.078		0.00	0.08	1.2		0.25	I
I	C-AB	0.20	3.48	0.057		0.00	0.06	0.9		0.30	I
I	A-B	0.00									I
I	A-C	14.00									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.33	4.27	0.078		0.08	0.08	1.3		0.25	I
I	C-AB	0.20	3.48	0.057		0.06	0.06	0.9		0.30	I
I	A-B	0.00									I
I	A-C	14.00									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.33	4.27	0.078		0.08	0.08	1.3		0.25	I
I	C-AB	0.20	3.48	0.057		0.06	0.06	0.9		0.30	I
I	A-B	0.00									I
I	A-C	14.00									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.33	4.27	0.078		0.08	0.08	1.3		0.25	I
I	C-AB	0.20	3.48	0.057		0.06	0.06	0.9		0.30	I
I	A-B	0.00									I
I	A-C	14.00									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	20.0	I	20.0	I	5.0	I	0.25	I	5.0	I	0.25	I
I	C-AB	I	12.0	I	12.0	I	3.7	I	0.31	I	3.7	I	0.31	I
I	A-B	I	0.0	I	0.0	I		I		I		I		I
I	A-C	I	840.0	I	840.0	I		I		I		I		I
I	ALL	I	976.0	I	976.0	I	8.7	I	0.01	I	8.7	I	0.01	I

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* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Slope For Stream A-B	I
I	630.86	0.23	0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Slope For Stream A-B	Slope For Stream C-A	Slope For Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Slope For Stream A-B	I
I	608.71	0.22	0.22	I

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2009 AM Peak Traffic Flows (100 loads) (100% West) (Ex HH dev)

TIME PERIOD BEGINS 07.45 AND ENDS 08.45

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 07.45 - 08.45 I I I I I I
I I ARM A I 0.000 I 0.010 I 0.990 I
I I 0.0 I 4.0 I 380.0 I
I I (0.0)I (0.0)I (10.0)I
I I I I I I
I I ARM B I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 15.0 I
I I (0.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.972 I 0.028 I 0.000 I
I I 688.0 I 20.0 I 0.0 I
I I (3.0)I (75.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2009 AM Peak Traffic Flows (100 loads) (100% West) (Ex HH dev)
AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-AC	0.27	4.45	0.060		0.00	0.06	0.9		0.24	I
I	C-AB	0.33	4.90	0.068		0.00	0.08	1.2		0.22	I
I	A-B	0.07									I
I	A-C	6.33									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-AC	0.27	4.45	0.060		0.06	0.06	0.9		0.24	I
I	C-AB	0.33	4.90	0.068		0.08	0.08	1.3		0.22	I
I	A-B	0.07									I
I	A-C	6.33									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-AC	0.27	4.45	0.060		0.06	0.06	0.9		0.24	I
I	C-AB	0.33	4.90	0.068		0.08	0.08	1.3		0.22	I
I	A-B	0.07									I
I	A-C	6.33									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-AC	0.27	4.45	0.060		0.06	0.06	1.0		0.24	I
I	C-AB	0.33	4.90	0.068		0.08	0.08	1.3		0.22	I
I	A-B	0.07									I
I	A-C	6.33									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	16.0	I	16.0	I	3.7	I	0.23	I	3.7	I	0.23	I
I	C-AB	I	20.0	I	20.0	I	5.0	I	0.25	I	5.0	I	0.25	I
I	A-B	I	4.0	I	4.0	I		I		I		I		I
I	A-C	I	380.0	I	380.0	I		I		I		I		I
I	ALL	I	1108.0	I	1108.0	I	8.8	I	0.01	I	8.8	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2009 AM Peak Traffic Flows (100 loads) (80/20 West/East) (Ex HH dev)

TIME PERIOD BEGINS 07.45 AND ENDS 08.45

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 07.45 - 08.45 I I I I I
I I ARM A I 0.000 I 0.016 I 0.984 I
I I 0.0 I 6.0 I 380.0 I
I I (0.0)I (33.0)I (10.0)I
I I I I I I
I I ARM B I 0.133 I 0.000 I 0.867 I
I I 2.0 I 0.0 I 13.0 I
I I (100.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.975 I 0.025 I 0.000 I
I I 688.0 I 18.0 I 0.0 I
I I (3.0)I (72.0)I (0.0)I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2009 AM Peak Traffic Flows (100 loads) (80/20 West/East) (Ex HH dev)
AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-AC	0.27	4.01	0.067		0.00	0.07	1.0		0.27	I
I	C-AB	0.30	4.98	0.060		0.00	0.07	1.1		0.21	I
I	A-B	0.10									I
I	A-C	6.37									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-AC	0.27	4.01	0.067		0.07	0.07	1.1		0.27	I
I	C-AB	0.30	4.98	0.060		0.07	0.07	1.1		0.21	I
I	A-B	0.10									I
I	A-C	6.37									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-AC	0.27	4.01	0.067		0.07	0.07	1.1		0.27	I
I	C-AB	0.30	4.98	0.060		0.07	0.07	1.1		0.21	I
I	A-B	0.10									I
I	A-C	6.37									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-AC	0.27	4.01	0.067		0.07	0.07	1.1		0.27	I
I	C-AB	0.30	4.98	0.060		0.07	0.07	1.1		0.21	I
I	A-B	0.10									I
I	A-C	6.37									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	16.0	I	16.0	I	4.2	I	0.26	I	4.2	I	0.26	I
I	C-AB	I	18.1	I	18.1	I	4.3	I	0.24	I	4.3	I	0.24	I
I	A-B	I	6.0	I	6.0	I		I		I		I		I
I	A-C	I	382.0	I	382.0	I		I		I		I		I
I	ALL	I	1112.0	I	1112.0	I	8.5	I	0.01	I	8.5	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2009 AM Peak Traffic Flows (100 loads) (100% West) (Incl HH dev)

TIME PERIOD BEGINS 07.45 AND ENDS 08.45

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 07.45 - 08.45 I I I I I
I I ARM A I 0.000 I 0.010 I 0.990 I
I I 0.0 I 4.0 I 400.0 I
I I (0.0)I (0.0)I (10.0)I
I I I I I I
I I ARM B I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 15.0 I
I I (0.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.977 I 0.023 I 0.000 I
I I 832.0 I 20.0 I 0.0 I
I I (4.0)I (75.0)I (0.0)I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2009 AM Peak Traffic Flows (100 loads) (100% West) (Incl HH dev)
AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-AC	0.27	4.41	0.060		0.00	0.06	0.9		0.24	I
I	C-AB	0.33	4.86	0.069		0.00	0.09	1.3		0.22	I
I	A-B	0.07									I
I	A-C	6.67									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-AC	0.27	4.41	0.060		0.06	0.06	1.0		0.24	I
I	C-AB	0.33	4.86	0.069		0.09	0.09	1.3		0.22	I
I	A-B	0.07									I
I	A-C	6.67									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-AC	0.27	4.41	0.060		0.06	0.06	1.0		0.24	I
I	C-AB	0.33	4.86	0.069		0.09	0.09	1.3		0.22	I
I	A-B	0.07									I
I	A-C	6.67									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-AC	0.27	4.41	0.060		0.06	0.06	1.0		0.24	I
I	C-AB	0.33	4.86	0.069		0.09	0.09	1.3		0.22	I
I	A-B	0.07									I
I	A-C	6.67									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	16.0	I	16.0	I	3.8	I	0.24	I	3.8	I	0.24	I
I	C-AB	I	20.0	I	20.0	I	5.2	I	0.26	I	5.2	I	0.26	I
I	A-B	I	4.0	I	4.0	I		I		I		I		I
I	A-C	I	400.0	I	400.0	I		I		I		I		I
I	ALL	I	1272.0	I	1272.0	I	9.0	I	0.01	I	9.0	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2009 PM Peak Traffic Flows Demand Set (100 loads) (100% West) (Ex HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 712.0 I
I I (0.0)I (0.0)I (2.0)I
I I I I I I
I I ARM B I 0.167 I 0.000 I 0.833 I
I I 4.0 I 0.0 I 20.0 I
I I (0.0)I (0.0)I (75.0)I
I I I I I I
I I ARM C I 0.848 I 0.152 I 0.000 I
I I 84.0 I 15.0 I 0.0 I
I I (7.0)I (100.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2009 PM Peak Traffic Flows Demand Set (100 loads) (100% West) (Ex HH dev)
AND FOR TIME PERIOD 2

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.40	4.53	0.088		0.00	0.10	1.4		0.24	I
I	C-AB	0.25	3.73	0.068		0.00	0.07	1.1		0.29	I
I	A-B	0.00									I
I	A-C	11.87									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.40	4.53	0.088		0.10	0.10	1.4		0.24	I
I	C-AB	0.25	3.73	0.068		0.07	0.07	1.1		0.29	I
I	A-B	0.00									I
I	A-C	11.87									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.40	4.53	0.088		0.10	0.10	1.4		0.24	I
I	C-AB	0.25	3.73	0.068		0.07	0.07	1.1		0.29	I
I	A-B	0.00									I
I	A-C	11.87									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.40	4.53	0.088		0.10	0.10	1.4		0.24	I
I	C-AB	0.25	3.73	0.068		0.07	0.07	1.1		0.29	I
I	A-B	0.00									I
I	A-C	11.87									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I		
I		I		I	* DELAY *	I	* DELAY *	I		
I		I		I		I		I		
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I
I		I		I		I		I		I
I	B-AC	I	24.0	I	24.0	I	5.7	I	0.24	I
I	C-AB	I	15.2	I	15.2	I	4.4	I	0.29	I
I	A-B	I	0.0	I	0.0	I		I		I
I	A-C	I	712.0	I	712.0	I		I		I
I	ALL	I	836.0	I	836.0	I	10.1	I	0.01	I
I		I		I		I		I		I
I		I		I		I		I		I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2009 PM Peak Traffic Flows Demand Set (100 loads) (80/20 West/East) (Ex HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.003 I 0.997 I
I I 0.0 I 2.0 I 712.0 I
I I (0.0)I (100.0)I (2.0)I
I I I I I I
I I ARM B I 0.250 I 0.000 I 0.750 I
I I 6.0 I 0.0 I 18.0 I
I I (33.0)I (0.0)I (72.0)I
I I I I I I
I I ARM C I 0.866 I 0.134 I 0.000 I
I I 84.0 I 13.0 I 0.0 I
I I (7.0)I (100.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2009 PM Peak Traffic Flows Demand Set (100 loads) (80/20 West/East) (Ex HH dev)
AND FOR TIME PERIOD 2

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.47	4.33	0.108		0.00	0.12	1.7		0.26	I
I	C-AB	0.21	3.71	0.058		0.00	0.06	0.9		0.29	I
I	A-B	0.03									I
I	A-C	11.90									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.47	4.33	0.108		0.12	0.12	1.8		0.26	I
I	C-AB	0.21	3.71	0.058		0.06	0.06	0.9		0.29	I
I	A-B	0.03									I
I	A-C	11.90									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.47	4.33	0.108		0.12	0.12	1.8		0.26	I
I	C-AB	0.21	3.71	0.058		0.06	0.06	0.9		0.29	I
I	A-B	0.03									I
I	A-C	11.90									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.47	4.33	0.108		0.12	0.12	1.8		0.26	I
I	C-AB	0.21	3.71	0.058		0.06	0.06	0.9		0.29	I
I	A-B	0.03									I
I	A-C	11.90									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(MIN)	I	(MIN/VEH)	I						
I	B-AC	I	28.0	I	28.0	I	7.1	I	0.25	I	7.1	I	0.25	I
I	C-AB	I	12.9	I	12.9	I	3.7	I	0.29	I	3.7	I	0.29	I
I	A-B	I	2.0	I	2.0	I		I		I		I		I
I	A-C	I	714.0	I	714.0	I		I		I		I		I
I	ALL	I	840.0	I	840.0	I	10.8	I	0.01	I	10.8	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Slope For Stream A-B	I
I	630.86	0.23	0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Slope For Stream A-B	Slope For Stream C-A	Slope For Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Slope For Stream A-B	I
I	608.71	0.22	0.22	I

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2009 PM Peak Traffic Flows Demand Set (100 loads) (100% West) (Incl HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 840.0 I
I I (0.0)I (0.0)I (2.0)I
I I I I I I
I I ARM B I 0.167 I 0.000 I 0.833 I
I I 4.0 I 0.0 I 20.0 I
I I (0.0)I (0.0)I (75.0)I
I I I I I I
I I ARM C I 0.874 I 0.126 I 0.000 I
I I 104.0 I 15.0 I 0.0 I
I I (7.0)I (100.0)I (0.0)I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2009 PM Peak Traffic Flows Demand Set (100 loads) (100% West) (Incl HH dev)
AND FOR TIME PERIOD 2

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.40	4.21	0.095		0.00	0.10	1.5		0.26	I
I	C-AB	0.25	3.48	0.072		0.00	0.08	1.2		0.31	I
I	A-B	0.00									I
I	A-C	14.00									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.40	4.21	0.095		0.10	0.10	1.6		0.26	I
I	C-AB	0.25	3.48	0.072		0.08	0.08	1.2		0.31	I
I	A-B	0.00									I
I	A-C	14.00									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.40	4.21	0.095		0.10	0.10	1.6		0.26	I
I	C-AB	0.25	3.48	0.072		0.08	0.08	1.2		0.31	I
I	A-B	0.00									I
I	A-C	14.00									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.40	4.21	0.095		0.10	0.10	1.6		0.26	I
I	C-AB	0.25	3.48	0.072		0.08	0.08	1.2		0.31	I
I	A-B	0.00									I
I	A-C	14.00									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	24.0	I	24.0	I	6.1	I	0.26	I	6.1	I	0.26	I
I	C-AB	I	15.1	I	15.1	I	4.8	I	0.32	I	4.8	I	0.32	I
I	A-B	I	0.0	I	0.0	I		I		I		I		I
I	A-C	I	840.0	I	840.0	I		I		I		I		I
I	ALL	I	984.0	I	984.0	I	10.9	I	0.01	I	10.9	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Stream B-C	Slope For Stream A-C	Slope For Stream A-B	I
I	630.86	0.23	0.09	I

I	Intercept For Stream B-A	Slope For Stream A-C	Slope For Stream A-B	Slope For Stream C-A	Slope For Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Stream C-B	Slope For Stream A-C	Slope For Stream A-B	I
I	608.71	0.22	0.22	I

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2009 AM Peak Traffic Flows (100 loads) (80/20 West/East) (Incl HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.015 I 0.985 I
I I 0.0 I 6.0 I 400.0 I
I I (0.0)I (33.0)I (10.0)I
I I I I I I
I I ARM B I 0.133 I 0.000 I 0.867 I
I I 2.0 I 0.0 I 13.0 I
I I (100.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.979 I 0.021 I 0.000 I
I I 832.0 I 18.0 I 0.0 I
I I (4.0)I (72.0)I (0.0)I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2009 AM Peak Traffic Flows (100 loads) (80/20 West/East) (Incl HH dev)
AND FOR TIME PERIOD 465

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.27	3.90	0.068		0.00	0.07	1.0		0.27	I
I	C-AB	0.30	4.93	0.061		0.00	0.07	1.1		0.22	I
I	A-B	0.10									I
I	A-C	6.70									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.27	3.90	0.068		0.07	0.07	1.1		0.28	I
I	C-AB	0.30	4.93	0.061		0.07	0.08	1.1		0.22	I
I	A-B	0.10									I
I	A-C	6.70									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.27	3.90	0.068		0.07	0.07	1.1		0.28	I
I	C-AB	0.30	4.93	0.061		0.08	0.08	1.1		0.22	I
I	A-B	0.10									I
I	A-C	6.70									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.27	3.90	0.068		0.07	0.07	1.1		0.28	I
I	C-AB	0.30	4.93	0.061		0.08	0.08	1.1		0.22	I
I	A-B	0.10									I
I	A-C	6.70									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	16.0	I	16.0	I	4.3	I	0.27	I	4.3	I	0.27	I
I	C-AB	I	18.0	I	18.0	I	4.5	I	0.25	I	4.5	I	0.25	I
I	A-B	I	6.0	I	6.0	I		I		I		I		I
I	A-C	I	402.0	I	402.0	I		I		I		I		I
I	ALL	I	1276.0	I	1276.0	I	8.8	I	0.01	I	8.8	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2009 PM Peak Traffic Flows Demand Set (100 loads) (80/20 West/East) (Incl HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.002 I 0.998 I
I I 0.0 I 2.0 I 840.0 I
I I (0.0)I (100.0)I (2.0)I
I I I I I I
I I ARM B I 0.250 I 0.000 I 0.750 I
I I 6.0 I 0.0 I 18.0 I
I I (33.0)I (0.0)I (72.0)I
I I I I I I
I I ARM C I 0.889 I 0.111 I 0.000 I
I I 104.0 I 13.0 I 0.0 I
I I (7.0)I (100.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2009 PM Peak Traffic Flows Demand Set (100 loads) (80/20 West/East) (Incl HH dev)
AND FOR TIME PERIOD 990

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.47	4.00	0.117		0.00	0.13	1.8		0.28	I
I	C-AB	0.21	3.47	0.062		0.00	0.07	1.0		0.31	I
I	A-B	0.03									I
I	A-C	14.03									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.47	4.00	0.117		0.13	0.13	1.9		0.28	I
I	C-AB	0.21	3.47	0.062		0.07	0.07	1.0		0.31	I
I	A-B	0.03									I
I	A-C	14.03									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.47	4.00	0.117		0.13	0.13	2.0		0.28	I
I	C-AB	0.21	3.47	0.062		0.07	0.07	1.0		0.31	I
I	A-B	0.03									I
I	A-C	14.03									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.47	4.00	0.117		0.13	0.13	2.0		0.28	I
I	C-AB	0.21	3.47	0.062		0.07	0.07	1.0		0.31	I
I	A-B	0.03									I
I	A-C	14.03									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	28.0	I	28.0	I	7.7	I	0.27	I	7.7	I	0.27	I
I	C-AB	I	12.9	I	12.9	I	4.0	I	0.31	I	4.0	I	0.31	I
I	A-B	I	2.0	I	2.0	I		I		I		I		I
I	A-C	I	842.0	I	842.0	I		I		I		I		I
I	ALL	I	988.0	I	988.0	I	11.7	I	0.01	I	11.7	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

Appendix D – PICADY Outputs For 2018

TRL LIMITED

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CAPACITIES, QUEUES, AND DELAYS AT 3 OR 4-ARM MAJOR/MINOR PRIORITY JUNCTIONS

PICADY 5.0 ANALYSIS PROGRAM
RELEASE 3.0 (JUNE 2006)

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EMAIL: SoftwareBureau@trl.co.uk

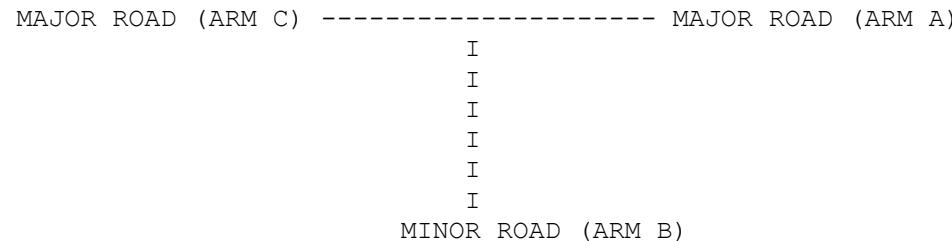
THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF HIS RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"\Pmcesrv1\project\2007\0710\PICADY\0710_PICADY_2018 Assessments.vpi"
(drive-on-the-left) at 11:06:05 on Friday, 29 February 2008

.RUN INFORMATION

RUN TITLE: Little Island Traffic Assessment
LOCATION: Little Island, Co. Cork
DATE: 28/02/08
CLIENT: Golder Associates Ireland
ENUMERATOR: DGF
JOB NUMBER: 0710
STATUS: Draft 2
DESCRIPTION: 2018 Assessments
MAJOR/MINOR JUNCTION CAPACITY AND DELAY

INPUT DATA



ARM A IS R623 East
ARM B IS Site Access
ARM C IS R623 West

STREAM LABELLING CONVENTION

STREAM A-B CONTAINS TRAFFIC GOING FROM ARM A TO ARM B

STREAM B-AC CONTAINS TRAFFIC GOING FROM ARM B TO ARM A AND TO ARM C

ETC.

GEOMETRIC DATA

I	DATA ITEM	I	MINOR ROAD B	I
I	TOTAL MAJOR ROAD CARRIAGeway WIDTH	I	(W) 7.30 M.	I
I	CENTRAL RESERVE WIDTH	I	(WCR) 0.00 M.	I
I		I		I
I	MAJOR ROAD RIGHT TURN - WIDTH	I	(WC-B) 2.20 M.	I
I	- VISIBILITY	I	(VC-B) 60.0 M.	I
I	- BLOCKS TRAFFIC	I	YES	I
I		I		I
I	MINOR ROAD - VISIBILITY TO LEFT	I	(VB-C) 10.0 M.	I
I	- VISIBILITY TO RIGHT	I	(VB-A) 11.0 M.	I
I	- LANE 1 WIDTH	I	(WB-C) 3.00 M.	I
I	- LANE 2 WIDTH	I	(WB-A) 0.00 M.	I

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2018 AM Peak Traffic Flows (50 loads) (100% West) (Ex HH dev)

TIME PERIOD BEGINS 07.45 AND ENDS 08.45

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 07.45 - 08.45 I I I I I I
I I ARM A I 0.000 I 0.009 I 0.991 I
I I 0.0 I 4.0 I 424.0 I
I I (0.0)I (0.0)I (10.0)I
I I I I I I
I I ARM B I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 12.0 I
I I (0.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.978 I 0.022 I 0.000 I
I I 768.0 I 17.0 I 0.0 I
I I (4.0)I (73.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2018 AM Peak Traffic Flows (50 loads) (100% West) (Ex HH dev)
AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-AC	0.20	4.36	0.046		0.00	0.05	0.7		0.24	I
I	C-AB	0.28	4.86	0.058		0.00	0.07	1.0		0.22	I
I	A-B	0.07									I
I	A-C	7.07									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-AC	0.20	4.36	0.046		0.05	0.05	0.7		0.24	I
I	C-AB	0.28	4.86	0.058		0.07	0.07	1.1		0.22	I
I	A-B	0.07									I
I	A-C	7.07									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-AC	0.20	4.36	0.046		0.05	0.05	0.7		0.24	I
I	C-AB	0.28	4.86	0.058		0.07	0.07	1.1		0.22	I
I	A-B	0.07									I
I	A-C	7.07									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-AC	0.20	4.36	0.046		0.05	0.05	0.7		0.24	I
I	C-AB	0.28	4.86	0.058		0.07	0.07	1.1		0.22	I
I	A-B	0.07									I
I	A-C	7.07									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	12.0	I	12.0	I	2.8	I	0.23	I	2.8	I	0.24	I
I	C-AB	I	17.0	I	17.0	I	4.2	I	0.25	I	4.2	I	0.25	I
I	A-B	I	4.0	I	4.0	I		I		I		I		I
I	A-C	I	424.0	I	424.0	I		I		I		I		I
I	ALL	I	1224.0	I	1224.0	I	7.0	I	0.01	I	7.0	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2018 PM Peak Traffic Flows Demand Set (50 loads) (100%West) (Ex HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 792.0 I
I I (0.0)I (0.0)I (2.0)I
I I I I I I
I I ARM B I 0.190 I 0.000 I 0.810 I
I I 4.0 I 0.0 I 17.0 I
I I (0.0)I (0.0)I (73.0)I
I I I I I I
I I ARM C I 0.889 I 0.111 I 0.000 I
I I 96.0 I 12.0 I 0.0 I
I I (7.0)I (100.0)I (0.0)I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2018 PM Peak Traffic Flows Demand Set (50 loads) (100%West) (Ex HH dev)
AND FOR TIME PERIOD 2

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.33	4.39	0.076		0.00	0.08	1.1		0.25	I
I	C-AB	0.20	3.57	0.056		0.00	0.06	0.9		0.30	I
I	A-B	0.00									I
I	A-C	13.20									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.33	4.39	0.076		0.08	0.08	1.2		0.25	I
I	C-AB	0.20	3.57	0.056		0.06	0.06	0.9		0.30	I
I	A-B	0.00									I
I	A-C	13.20									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.33	4.39	0.076		0.08	0.08	1.2		0.25	I
I	C-AB	0.20	3.57	0.056		0.06	0.06	0.9		0.30	I
I	A-B	0.00									I
I	A-C	13.20									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.33	4.39	0.076		0.08	0.08	1.2		0.25	I
I	C-AB	0.20	3.57	0.056		0.06	0.06	0.9		0.30	I
I	A-B	0.00									I
I	A-C	13.20									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	20.0	I	20.0	I	4.8	I	0.24	I	4.8	I	0.24	I
I	C-AB	I	12.0	I	12.0	I	3.6	I	0.30	I	3.6	I	0.30	I
I	A-B	I	0.0	I	0.0	I		I		I		I		I
I	A-C	I	792.0	I	792.0	I		I		I		I		I
I	ALL	I	920.0	I	920.0	I	8.4	I	0.01	I	8.4	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2018 AM Peak Traffic Flows (50 loads) (100% West) (Incl HH dev)

TIME PERIOD BEGINS 07.45 AND ENDS 08.45

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 07.45 - 08.45 I I I I I
I I ARM A I 0.000 I 0.009 I 0.991 I
I I 0.0 I 4.0 I 444.0 I
I I (0.0)I (0.0)I (10.0)I
I I I I I I
I I ARM B I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 12.0 I
I I (0.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.982 I 0.018 I 0.000 I
I I 912.0 I 17.0 I 0.0 I
I I (4.0)I (73.0)I (0.0)I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2018 AM Peak Traffic Flows (50 loads) (100% West) (Incl HH dev)
AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-AC	0.20	4.32	0.046		0.00	0.05	0.7		0.24	I
I	C-AB	0.28	4.81	0.059		0.00	0.07	1.1		0.22	I
I	A-B	0.07									I
I	A-C	7.40									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-AC	0.20	4.32	0.046		0.05	0.05	0.7		0.24	I
I	C-AB	0.28	4.81	0.059		0.07	0.07	1.1		0.22	I
I	A-B	0.07									I
I	A-C	7.40									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-AC	0.20	4.32	0.046		0.05	0.05	0.7		0.24	I
I	C-AB	0.28	4.81	0.059		0.07	0.07	1.1		0.22	I
I	A-B	0.07									I
I	A-C	7.40									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-AC	0.20	4.32	0.046		0.05	0.05	0.7		0.24	I
I	C-AB	0.28	4.81	0.059		0.07	0.07	1.1		0.22	I
I	A-B	0.07									I
I	A-C	7.40									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.0
08.15	0.0
08.30	0.0
08.45	0.0

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	12.0	I	12.0	I	2.8	I	0.24	I	2.8	I	0.24	I
I	C-AB	I	17.0	I	17.0	I	4.4	I	0.26	I	4.4	I	0.26	I
I	A-B	I	4.0	I	4.0	I		I		I		I		I
I	A-C	I	444.0	I	444.0	I		I		I		I		I
I	ALL	I	1388.0	I	1388.0	I	7.2	I	0.01	I	7.2	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2018 PM Peak Traffic Flows Demand Set (50 loads) (100% West) (Incl HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 920.0 I
I I (0.0)I (0.0)I (2.0)I
I I I I I I
I I ARM B I 0.190 I 0.000 I 0.810 I
I I 4.0 I 0.0 I 17.0 I
I I (0.0)I (0.0)I (73.0)I
I I I I I I
I I ARM C I 0.906 I 0.094 I 0.000 I
I I 116.0 I 12.0 I 0.0 I
I I (7.0)I (100.0)I (0.0)I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2018 PM Peak Traffic Flows Demand Set (50 loads) (100% West) (Incl HH dev)
AND FOR TIME PERIOD 2

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.33	4.07	0.082		0.00	0.09	1.2		0.27	I
I	C-AB	0.20	3.33	0.060		0.00	0.06	0.9		0.32	I
I	A-B	0.00									I
I	A-C	15.33									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.33	4.07	0.082		0.09	0.09	1.3		0.27	I
I	C-AB	0.20	3.33	0.060		0.06	0.07	1.0		0.32	I
I	A-B	0.00									I
I	A-C	15.33									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.33	4.07	0.082		0.09	0.09	1.3		0.27	I
I	C-AB	0.20	3.33	0.060		0.07	0.07	1.0		0.32	I
I	A-B	0.00									I
I	A-C	15.33									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.33	4.07	0.082		0.09	0.09	1.3		0.27	I
I	C-AB	0.20	3.33	0.060		0.07	0.07	1.0		0.32	I
I	A-B	0.00									I
I	A-C	15.33									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	20.0	I	20.0	I	5.2	I	0.26	I	5.2	I	0.26	I
I	C-AB	I	12.0	I	12.0	I	3.9	I	0.33	I	3.9	I	0.33	I
I	A-B	I	0.0	I	0.0	I		I		I		I		I
I	A-C	I	920.0	I	920.0	I		I		I		I		I
I	ALL	I	1068.0	I	1068.0	I	9.1	I	0.01	I	9.1	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2018 AM Peak Traffic Flows (100 loads) (100% West) (Ex HH dev)

TIME PERIOD BEGINS 07.45 AND ENDS 08.45

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 07.45 - 08.45 I I I I I I
I I ARM A I 0.000 I 0.009 I 0.991 I
I I 0.0 I 4.0 I 424.0 I
I I (0.0)I (0.0)I (10.0)I
I I I I I I
I I ARM B I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 15.0 I
I I (0.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.975 I 0.025 I 0.000 I
I I 768.0 I 20.0 I 0.0 I
I I (4.0)I (75.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2018 AM Peak Traffic Flows (100 loads) (100% West) (Ex HH dev)
AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-AC	0.27	4.36	0.061		0.00	0.06	0.9		0.24	I
I	C-AB	0.33	4.80	0.069		0.00	0.09	1.3		0.22	I
I	A-B	0.07									I
I	A-C	7.07									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-AC	0.27	4.36	0.061		0.06	0.06	1.0		0.24	I
I	C-AB	0.33	4.80	0.069		0.09	0.09	1.3		0.22	I
I	A-B	0.07									I
I	A-C	7.07									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-AC	0.27	4.36	0.061		0.06	0.06	1.0		0.24	I
I	C-AB	0.33	4.80	0.069		0.09	0.09	1.3		0.22	I
I	A-B	0.07									I
I	A-C	7.07									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-AC	0.27	4.36	0.061		0.06	0.06	1.0		0.24	I
I	C-AB	0.33	4.80	0.069		0.09	0.09	1.3		0.22	I
I	A-B	0.07									I
I	A-C	7.07									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	16.0	I	16.0	I	3.8	I	0.24	I	3.8	I	0.24	I
I	C-AB	I	20.0	I	20.0	I	5.2	I	0.26	I	5.2	I	0.26	I
I	A-B	I	4.0	I	4.0	I		I		I		I		I
I	A-C	I	424.0	I	424.0	I		I		I		I		I
I	ALL	I	1232.0	I	1232.0	I	9.1	I	0.01	I	9.1	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2018 AM Peak Traffic Flows (100 loads) (80/20 West/East) (Ex HH dev)

TIME PERIOD BEGINS 07.45 AND ENDS 08.45

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 07.45 - 08.45 I I I I I
I I ARM A I 0.000 I 0.014 I 0.986 I
I I 0.0 I 6.0 I 424.0 I
I I (0.0)I (33.0)I (10.0)I
I I I I I I
I I ARM B I 0.133 I 0.000 I 0.867 I
I I 2.0 I 0.0 I 13.0 I
I I (100.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.977 I 0.023 I 0.000 I
I I 768.0 I 18.0 I 0.0 I
I I (4.0)I (72.0)I (0.0)I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2018 AM Peak Traffic Flows (100 loads) (80/20 West/East) (Ex HH dev)
AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-AC	0.27	3.87	0.069		0.00	0.07	1.0		0.28	I
I	C-AB	0.30	4.87	0.062		0.00	0.07	1.1		0.22	I
I	A-B	0.10									I
I	A-C	7.10									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-AC	0.27	3.87	0.069		0.07	0.07	1.1		0.28	I
I	C-AB	0.30	4.87	0.062		0.07	0.08	1.1		0.22	I
I	A-B	0.10									I
I	A-C	7.10									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-AC	0.27	3.87	0.069		0.07	0.07	1.1		0.28	I
I	C-AB	0.30	4.87	0.062		0.08	0.08	1.1		0.22	I
I	A-B	0.10									I
I	A-C	7.10									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-AC	0.27	3.87	0.069		0.07	0.07	1.1		0.28	I
I	C-AB	0.30	4.87	0.062		0.08	0.08	1.1		0.22	I
I	A-B	0.10									I
I	A-C	7.10									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	16.0	I	16.0	I	4.3	I	0.27	I	4.3	I	0.27	I
I	C-AB	I	18.0	I	18.0	I	4.5	I	0.25	I	4.5	I	0.25	I
I	A-B	I	6.0	I	6.0	I		I		I		I		I
I	A-C	I	426.0	I	426.0	I		I		I		I		I
I	ALL	I	1236.0	I	1236.0	I	8.9	I	0.01	I	8.9	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2018 AM Peak Traffic Flows (100 loads) (100% West) (Incl HH dev)

TIME PERIOD BEGINS 07.45 AND ENDS 08.45

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 07.45 - 08.45 I I I I I
I I ARM A I 0.000 I 0.009 I 0.991 I
I I 0.0 I 4.0 I 444.0 I
I I (0.0)I (0.0)I (10.0)I
I I I I I I
I I ARM B I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 15.0 I
I I (0.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.979 I 0.021 I 0.000 I
I I 912.0 I 20.0 I 0.0 I
I I (4.0)I (75.0)I (0.0)I
I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2018 AM Peak Traffic Flows (100 loads) (100% West) (Incl HH dev)
AND FOR TIME PERIOD 1

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	07.45-08.00										I
I	B-AC	0.27	4.32	0.062		0.00	0.06	0.9		0.25	I
I	C-AB	0.33	4.75	0.070		0.00	0.09	1.3		0.23	I
I	A-B	0.07									I
I	A-C	7.40									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.00-08.15										I
I	B-AC	0.27	4.32	0.062		0.06	0.07	1.0		0.25	I
I	C-AB	0.33	4.75	0.070		0.09	0.09	1.4		0.23	I
I	A-B	0.07									I
I	A-C	7.40									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.15-08.30										I
I	B-AC	0.27	4.32	0.062		0.07	0.07	1.0		0.25	I
I	C-AB	0.33	4.75	0.070		0.09	0.09	1.4		0.23	I
I	A-B	0.07									I
I	A-C	7.40									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.30-08.45										I
I	B-AC	0.27	4.32	0.062		0.07	0.07	1.0		0.25	I
I	C-AB	0.33	4.75	0.070		0.09	0.09	1.4		0.23	I
I	A-B	0.07									I
I	A-C	7.40									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.1
08.45	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	16.0	I	16.0	I	3.9	I	0.24	I	3.9	I	0.24	I
I	C-AB	I	20.0	I	20.0	I	5.5	I	0.27	I	5.5	I	0.27	I
I	A-B	I	4.0	I	4.0	I		I		I		I		I
I	A-C	I	444.0	I	444.0	I		I		I		I		I
I	ALL	I	1396.0	I	1396.0	I	9.3	I	0.01	I	9.3	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2018 PM Peak Traffic Flows Demand Set (100 loads) (100% West) (Ex HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 792.0 I
I I (0.0)I (0.0)I (2.0)I
I I I I I I
I I ARM B I 0.167 I 0.000 I 0.833 I
I I 4.0 I 0.0 I 20.0 I
I I (0.0)I (0.0)I (75.0)I
I I I I I I
I I ARM C I 0.865 I 0.135 I 0.000 I
I I 96.0 I 15.0 I 0.0 I
I I (7.0)I (100.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2018 PM Peak Traffic Flows Demand Set (100 loads) (100% West) (Ex HH dev)
AND FOR TIME PERIOD 2

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.40	4.33	0.092		0.00	0.10	1.4		0.25	I
I	C-AB	0.25	3.57	0.071		0.00	0.08	1.1		0.30	I
I	A-B	0.00									I
I	A-C	13.20									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.40	4.33	0.092		0.10	0.10	1.5		0.25	I
I	C-AB	0.25	3.57	0.071		0.08	0.08	1.2		0.30	I
I	A-B	0.00									I
I	A-C	13.20									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.40	4.33	0.092		0.10	0.10	1.5		0.25	I
I	C-AB	0.25	3.57	0.071		0.08	0.08	1.2		0.30	I
I	A-B	0.00									I
I	A-C	13.20									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.40	4.33	0.092		0.10	0.10	1.5		0.25	I
I	C-AB	0.25	3.57	0.071		0.08	0.08	1.2		0.30	I
I	A-B	0.00									I
I	A-C	13.20									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	24.0	I	24.0	I	6.0	I	0.25	I	6.0	I	0.25	I
I	C-AB	I	15.1	I	15.1	I	4.6	I	0.31	I	4.6	I	0.31	I
I	A-B	I	0.0	I	0.0	I		I		I		I		I
I	A-C	I	792.0	I	792.0	I		I		I		I		I
I	ALL	I	928.0	I	928.0	I	10.6	I	0.01	I	10.6	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2018 PM Peak Traffic Flows Demand Set (100 loads) (80/20 West/East) (Ex HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.003 I 0.997 I
I I 0.0 I 2.0 I 792.0 I
I I (0.0)I (100.0)I (2.0)I
I I I I I I
I I ARM B I 0.250 I 0.000 I 0.750 I
I I 6.0 I 0.0 I 18.0 I
I I (33.0)I (0.0)I (72.0)I
I I I I I I
I I ARM C I 0.881 I 0.119 I 0.000 I
I I 96.0 I 13.0 I 0.0 I
I I (7.0)I (100.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2018 PM Peak Traffic Flows Demand Set (100 loads) (80/20 West/East) (Ex HH dev)
AND FOR TIME PERIOD 2

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.47	4.13	0.113		0.00	0.12	1.8		0.27	I
I	C-AB	0.21	3.56	0.060		0.00	0.06	0.9		0.30	I
I	A-B	0.03									I
I	A-C	13.23									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.47	4.13	0.113		0.12	0.13	1.9		0.27	I
I	C-AB	0.21	3.56	0.060		0.06	0.07	1.0		0.30	I
I	A-B	0.03									I
I	A-C	13.23									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.47	4.13	0.113		0.13	0.13	1.9		0.27	I
I	C-AB	0.21	3.56	0.060		0.07	0.07	1.0		0.30	I
I	A-B	0.03									I
I	A-C	13.23									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.47	4.13	0.113		0.13	0.13	1.9		0.27	I
I	C-AB	0.21	3.56	0.060		0.07	0.07	1.0		0.30	I
I	A-B	0.03									I
I	A-C	13.23									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	28.0	I	28.0	I	7.4	I	0.27	I	7.4	I	0.27	I
I	C-AB	I	12.9	I	12.9	I	3.9	I	0.30	I	3.9	I	0.30	I
I	A-B	I	2.0	I	2.0	I		I		I		I		I
I	A-C	I	794.0	I	794.0	I		I		I		I		I
I	ALL	I	932.0	I	932.0	I	11.3	I	0.01	I	11.3	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2018 PM Peak Traffic Flows Demand Set (100 loads) (100% West) (Incl HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.000 I 1.000 I
I I 0.0 I 0.0 I 920.0 I
I I (0.0)I (0.0)I (0.0)I
I I I I I I
I I ARM B I 0.167 I 0.000 I 0.833 I
I I 4.0 I 0.0 I 20.0 I
I I (2.0)I (0.0)I (0.0)I
I I I I I I
I I ARM C I 0.885 I 0.115 I 0.000 I
I I 116.0 I 15.0 I 0.0 I
I I (75.0)I (100.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2018 PM Peak Traffic Flows Demand Set (100 loads) (100% West) (Incl HH dev)
AND FOR TIME PERIOD 2

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.40	6.28	0.064		0.00	0.07	1.0		0.17	I
I	C-AB	0.25	3.37	0.075		0.00	0.08	1.2		0.32	I
I	A-B	0.00									I
I	A-C	15.33									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.40	6.28	0.064		0.07	0.07	1.0		0.17	I
I	C-AB	0.25	3.37	0.075		0.08	0.08	1.3		0.32	I
I	A-B	0.00									I
I	A-C	15.33									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.40	6.28	0.064		0.07	0.07	1.0		0.17	I
I	C-AB	0.25	3.37	0.075		0.08	0.08	1.3		0.32	I
I	A-B	0.00									I
I	A-C	15.33									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.40	6.28	0.064		0.07	0.07	1.0		0.17	I
I	C-AB	0.25	3.37	0.075		0.08	0.08	1.3		0.32	I
I	A-B	0.00									I
I	A-C	15.33									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	24.0	I	24.0	I	4.0	I	0.17	I	4.0	I	0.17	I
I	C-AB	I	15.1	I	15.1	I	5.0	I	0.33	I	5.0	I	0.33	I
I	A-B	I	0.0	I	0.0	I		I		I		I		I
I	A-C	I	920.0	I	920.0	I		I		I		I		I
I	ALL	I	1076.0	I	1076.0	I	9.0	I	0.01	I	9.0	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.

* THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2018 AM Peak Traffic Flows (100 loads) (80/20 West/East) (Incl HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.013 I 0.987 I
I I 0.0 I 6.0 I 444.0 I
I I (0.0)I (33.0)I (10.0)I
I I I I I I
I I ARM B I 0.133 I 0.000 I 0.867 I
I I 2.0 I 0.0 I 13.0 I
I I (100.0)I (0.0)I (100.0)I
I I I I I I
I I ARM C I 0.981 I 0.019 I 0.000 I
I I 912.0 I 18.0 I 0.0 I
I I (4.0)I (72.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2018 AM Peak Traffic Flows (100 loads) (80/20 West/East) (Incl HH dev)
AND FOR TIME PERIOD 465

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.27	3.76	0.071		0.00	0.07	1.1		0.29	I
I	C-AB	0.30	4.82	0.062		0.00	0.08	1.1		0.22	I
I	A-B	0.10									I
I	A-C	7.43									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.27	3.76	0.071		0.07	0.08	1.1		0.29	I
I	C-AB	0.30	4.82	0.062		0.08	0.08	1.2		0.22	I
I	A-B	0.10									I
I	A-C	7.43									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.27	3.76	0.071		0.08	0.08	1.1		0.29	I
I	C-AB	0.30	4.82	0.062		0.08	0.08	1.2		0.22	I
I	A-B	0.10									I
I	A-C	7.43									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.27	3.76	0.071		0.08	0.08	1.1		0.29	I
I	C-AB	0.30	4.82	0.062		0.08	0.08	1.2		0.22	I
I	A-B	0.10									I
I	A-C	7.43									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	16.0	I	16.0	I	4.5	I	0.28	I	4.5	I	0.28	I
I	C-AB	I	18.0	I	18.0	I	4.7	I	0.26	I	4.7	I	0.26	I
I	A-B	I	6.0	I	6.0	I		I		I		I		I
I	A-C	I	446.0	I	446.0	I		I		I		I		I
I	ALL	I	1400.0	I	1400.0	I	9.2	I	0.01	I	9.2	I	0.01	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD .

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END OF JOB

SLOPES AND INTERCEPT

(NB: Streams may be combined, in which case capacity
will be adjusted)

I	Intercept For Slope For Opposing Stream B-C	Slope For Opposing Stream A-C	I
I	630.86	0.23	0.09

I	Intercept For Slope For Opposing Stream B-A	Slope For Opposing Stream A-C	Slope For Opposing Stream A-B	Slope For Opposing Stream C-A	Slope For Opposing Stream C-B	I
I	486.34	0.21	0.08	0.13	0.30	I

I	Intercept For Slope For Opposing Stream C-B	Slope For Opposing Stream A-C	I
I	608.71	0.22	0.22

NB These values do not allow for any site specific corrections

TRAFFIC DEMAND DATA

I ARM I FLOW SCALE(%) I

I A I 100 I
I B I 100 I
I C I 100 I

Demand set: 2018 PM Peak Traffic Flows Demand Set (100 loads) (80/20 West/East) (Incl HH dev)

TIME PERIOD BEGINS 16.30 AND ENDS 17.30

LENGTH OF TIME PERIOD - 60 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE INPUT DIRECTLY.

I I TURNING PROPORTIONS I
I I TURNING COUNTS I
I I (PERCENTAGE OF H.V.S) I
I -----
I TIME I FROM/TO I ARM A I ARM B I ARM C I

I 16.30 - 17.30 I I I I I
I I ARM A I 0.000 I 0.002 I 0.998 I
I I 0.0 I 2.0 I 920.0 I
I I (0.0)I (100.0)I (2.0)I
I I I I I I
I I ARM B I 0.250 I 0.000 I 0.750 I
I I 6.0 I 0.0 I 18.0 I
I I (33.0)I (0.0)I (72.0)I
I I I I I I
I I ARM C I 0.899 I 0.101 I 0.000 I
I I 116.0 I 13.0 I 0.0 I
I I (7.0)I (100.0)I (0.0)I
I I I I I I

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

THE PERCENTAGE OF HEAVY VEHICLES VARIES OVER TURNING MOVEMENTS

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT

FOR DEMAND SET 2018 PM Peak Traffic Flows Demand Set (100 loads) (80/20 West/East) (Incl HH dev)
AND FOR TIME PERIOD 990

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.30-16.45										I
I	B-AC	0.47	3.80	0.123		0.00	0.14	1.9		0.30	I
I	C-AB	0.21	3.32	0.065		0.00	0.07	1.0		0.32	I
I	A-B	0.03									I
I	A-C	15.37									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	16.45-17.00										I
I	B-AC	0.47	3.80	0.123		0.14	0.14	2.1		0.30	I
I	C-AB	0.21	3.32	0.065		0.07	0.07	1.1		0.32	I
I	A-B	0.03									I
I	A-C	15.37									I
I											I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.00-17.15										I
I	B-AC	0.47	3.80	0.123		0.14	0.14	2.1		0.30	I
I	C-AB	0.21	3.32	0.065		0.07	0.07	1.1		0.32	I
I	A-B	0.03									I
I	A-C	15.37									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.15-17.30										I
I	B-AC	0.47	3.80	0.123		0.14	0.14	2.1		0.30	I
I	C-AB	0.21	3.32	0.065		0.07	0.07	1.1		0.32	I
I	A-B	0.03									I
I	A-C	15.37									I
I											I

QUEUE FOR STREAM B-AC

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT	NO. OF ENDING VEHICLES IN QUEUE
16.45	0.1
17.00	0.1
17.15	0.1
17.30	0.1

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I						
I		I		I	* DELAY *	I	* DELAY *	I						
I		I		I		I		I						
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	28.0	I	28.0	I	8.1	I	0.29	I	8.2	I	0.29	I
I	C-AB	I	12.9	I	12.9	I	4.3	I	0.33	I	4.3	I	0.33	I
I	A-B	I	2.0	I	2.0	I		I		I		I		I
I	A-C	I	922.0	I	922.0	I		I		I		I		I
I	ALL	I	1080.0	I	1080.0	I	12.4	I	0.01	I	12.4	I	0.01	I

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END OF JOB